



**UNIVERSIDAD NACIONAL AUTÓNOMA DE MÉXICO**

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**FACULTAD DE INGENIERÍA**

**DISEÑO ESTRUCTURAL DE UN RACK DE  
ACERO ESTRUCTURAL PARA  
PLANTAS INDUSTRIALES**

**TESIS**

QUE PARA OBTENER EL TÍTULO DE:

**INGENIERO CIVIL**

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***DEDICATORIA:***

*Esta tesis se la dedico con todo mi amor y cariño a mis hijos Ricardo Josué, Ulises Yael y Paulina Yareli por ser la fuente de motivación e inspiración para poder superarme día a día. Esperando que este trabajo también sea una fuente de inspiración para ellos para lograr sus metas y objetivos.*

*También se la dedico con mucho amor a mi madre y mis hermanos, por ser las personas que más me apoyaron y motivaron para este logro.*

**AGRADECIMIENTOS:**

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*Gracias a mi madre y mis hermanos por su apoyo incondicional, por su motivación para lograr esta meta.*

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*Gracias a todas aquellas personas que de alguna u otra manera ayudaron mediante su motivación para que continuara y terminara este trabajo.*

*Gracias a todos.*

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

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## **Introducción:**

Una de las estructuras que se encuentra presente en la mayoría de las plantas industriales son los soportes de tuberías o soportes de instalaciones eléctrica llamado comúnmente Rack de Tuberías o Rack de charolas eléctricas y de instrumentación, esta estructura es una sucesión de marcos de acero estructural restringidos longitudinalmente por vigas o trabes, y se caracterizan por tener una gran longitud y su sección transversal puede ser constante o variable dependiendo de las necesidades de cada proyecto, su elevación puede variar de acuerdo al número de “camas” necesarias para el proyecto. Su principal función es brindar soporte durante su trayectoria a tuberías y/o charolas eléctricas y/o charolas de instrumentación, en algunos casos pueden soportar equipos como aroenfriadores o tanques, en la siguiente imagen se muestra la forma clásica de un Rack de Tuberías, de acero estructural con sección transversal constante.



*Forma clásica de la estructura de un Rack de Tuberías de Acero.*

La falla de una estructura de este tipo puede causar grandes daños tanto económicos, como ambientales o de pérdidas humanas, por lo que estas estructuras se deben diseñar a conciencia considerando todas las fuerzas y combinaciones de estas, que incluyan todas las condiciones que afectan su estabilidad.

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La presente tesis ofrece una guía para el Ingeniero Civil de Diseño Estructural, al describir una metodología para el análisis, diseño y construcción de un Rack para transporte de Tuberías y/o charolas eléctricas y de instrumentación, utilizado en instalaciones industriales.

Con este fin en el capítulo 1 se comienza por dar una descripción general y particular del proyecto, así como de la estructura y su cimentación, aportando datos tales como:

- Ubicación geográfica
- Alcance del proyecto de esta tesis
- Geometría de la estructura

En el capítulo 2 se determinan los criterios de diseño como son: los materiales de construcción, se da una descripción de las cargas que actuarán en la estructura y se determinan los reglamentos y la normatividad para el análisis y diseño de la estructura,

En el capítulo 3 se determina toda la información necesaria para el modelo estructural mediante un software para análisis y diseño estructural (Staad Pro). Se calculan y se indican todas las fuerzas estáticas y dinámicas que actúan sobre el modelo estructural, se describen también las combinaciones necesarias de acuerdo a reglamento para la revisión de los estados límite de servicio, de los estados límite de falla y el diseño de los elementos tanto de la superestructura, como de su subestructura (cimentación).

En el capítulo 4 se realiza el análisis para la obtención de los periodos de la estructura y se revisa que se cumplan con los cortantes basales.

Habiéndose realizado el análisis en el capítulo 5 se hace la revisión de los estados límites de servicio, de los estados límite de falla, así como el diseño final de los miembros, se recurre a los resultados arrojados del modelo analítico de la estructura y parte de la cimentación (trabes de liga) mediante el software Staad Pro; en él se manejan las cargas, combinaciones de carga y resistencia de materiales, basados en los manuales y códigos mencionados en el capítulo 2 tanto nacionales como internacionales.

En el capítulo 6 se realiza el diseño de la subestructura (cimentación).

En el capítulo 7 se presenta el procedimiento constructivo en este tipo de estructuras donde con: detalles y croquis se logra mostrar los pasos y beneficios del método constructivo adoptado.

En el capítulo 8 se dan las conclusiones de este proyecto.

## 1.0 Antecedentes

### 1.1. Localización del proyecto.

Se pretende conformar un proyecto denominado “Central Ciclo Combinado “ 171 CC AGUA PRIETA II (con campo solar)” fase 1, se construirá en el sitio denominado El Ejido Agua Prieta que se localiza al oeste-suroeste de la ciudad de Agua Prieta, Sonora, aproximadamente a 7.4 km del centro de la ciudad de Agua Prieta y a 3.6 km al noroeste de la cabecera norte de la aeropista municipal la (Calera), que está ubicado en las coordenadas latitud norte 31° 15´ 59.09” y longitud oeste 109° 35´ 25.53”, y cuenta con una altura de 1198.50 msnm. En la Imagen 1.1 se indica la ubicación geográfica del proyecto.



Imagen 1.1 Ubicación geográfica del proyecto “Central Ciclo Combinado “ 171 CC AGUA PRIETA II (con campo solar)” fase 1, (Fuente: [www.Maps.com](http://www.Maps.com))

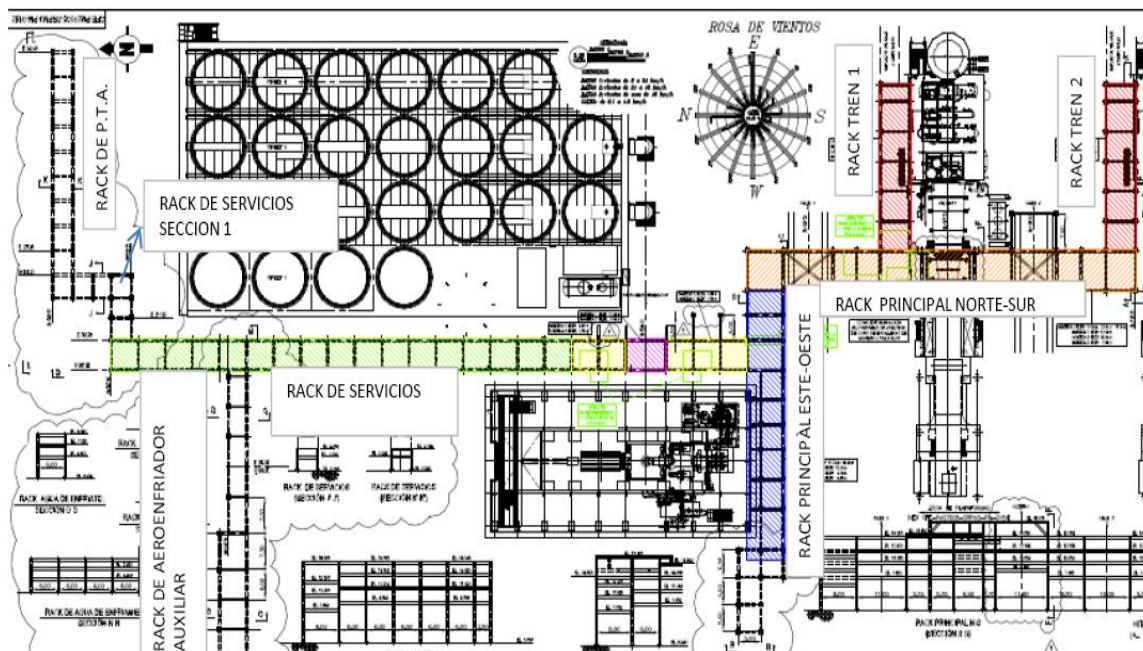


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El proyecto es el diseño y construcción (EPC) de una planta de ciclo combinado, la cual consta de varios equipos y estructuras, entre las cuales se encuentra como una de las principales por su magnitud el Rack de Tuberías y charolas eléctricas cuyo diseño motiva la presente tesis.

El Rack de Tuberías estará compuesto por cuatro módulos: Rack tren 1 y 2 (Norte y Sur), Rack Principal, Rack de Aeroenfriador y Rack de servicios. Para esta tesis solo se desarrollará el análisis y diseño del Rack Principal Norte-Sur. La estructura para este Rack será a base de marcos de acero, se instalará una plataforma para mantenimiento y control de válvulas de las tuberías. Estructuralmente, el rack se diseñará con marcos rígidos de acero en la dirección transversal y con marcos de acero contraventeados longitudinalmente. Para la cuestión de diseño y montaje se considerará lo siguiente: las conexiones o uniones hechas en taller serán soldadas, mientras que las que se realicen en campo serán atornilladas para los marcos longitudinales y conexiones soldadas en campo para los marcos transversales. La cimentación de este rack se resolverá con zapatas aisladas y de ser necesario de acuerdo con el diseño, también se utilizarían traveses de liga.

En la planta de arreglo general que se muestra en la Imagen 1.2 podemos apreciar la distribución de algunos de los diferentes equipos del proyecto además se indica la ubicación que tiene el Rack dentro de la planta.



*Imagen 1.2 Planta de arreglo general del proyecto, se indica la ubicación del Rack de Tuberías.*

*(Fuente: Plano de proyecto CFE-PM27005-SRPM3-PM-0152 Rev. 0)*

## **1.2. Alcances del proyecto.**

El diseño estructural del Rack de Tuberías se lleva a cabo contemplando las fuerzas: gravitacionales, por cambios de temperatura, sísmicas, de viento y de tuberías en operación, estas últimas son proporcionadas por un estudio previo de flexibilidad realizado por la disciplina de tuberías.

Como parte del análisis, se revisa los estados límite de falla y de servicio comparativamente con los valores permisibles establecidos para cada caso.

Para el análisis y diseño de la estructura, se realiza un modelo tridimensional utilizando el programa "STAAD-PRO", dicho modelo consta de barras o miembros conectados entre sí y que representan los diferentes elementos estructurales como vigas y columnas, en este modelo se especifican los materiales, sus apoyos, secciones, condiciones y combinaciones de carga.

Es importante mencionar que la definición de la geometría en cuanto a elevaciones y dimensiones que tendrá el Rack de Tuberías se hace mediante una conciliación con las diferentes disciplinas que participan en el proyecto.

## **1.3. Descripción de la estructura.**

El tipo de cimentación se determina de acuerdo a la capacidad del terreno, la cual es dada por el estudio de mecánica de suelos del proyecto (se indican los datos de esto último en el diseño de la cimentación).

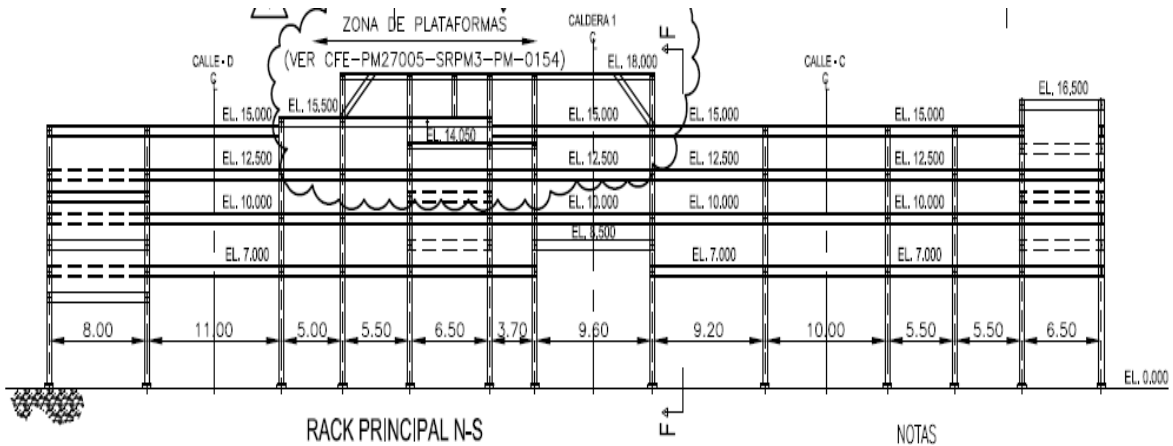
La cimentación para este rack de tuberías será superficial y estará formada por zapatas aisladas y trabes de liga de concreto reforzado tanto en el sentido transversal como en el longitudinal.

Por otro lado Estructuralmente, el rack se diseñará con marcos rígidos de acero en la dirección transversal y con marcos de acero contraventeados longitudinalmente. Las conexiones o uniones hechas en taller serán soldadas mientras que las que se realicen en campo serán atornilladas para los marcos longitudinales y conexiones soldadas en campo para los marcos transversales.

Un aspecto adicional de mucha importancia para el diseño de la estructura del rack es el hecho de que no podrán utilizarse secciones cerradas o tubulares ni tampoco perfiles de pared delgada como el caso de los laminados en frío, por lo que se utilizarán para los marcos secciones abiertas tipo "IR". Lo anterior es debido a los altos índices de corrosión, lo cual con secciones cerradas se agravan mayormente.

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En planta el Rack de Tuberías tiene un ancho de 8 m y una longitud total de 86 m que se divide como se muestran en la siguiente imagen 1.3.



*Imagen 1.3 Dimensiones en elevación del Rack de Tuberías.  
(Fuente: Plano de proyecto CFE-PM27005-SRPM3-PM-0152 Rev. 0)*

En la elevación de la Imagen 1.3 se observa que el Rack tiene una altura máxima de 18 m y está compuesto de hasta 5 niveles “camas” sobre las cuales se apoyan tuberías, válvulas, cableado eléctrico y de instrumentación.

## **2.0 Criterios de diseño y análisis estructural.**

### **2.1 Propiedades mecánicas de los materiales.**

A continuación, se describen los diferentes materiales que constituyen los elementos estructurales con sus respectivas resistencias.

- a) Concreto para elementos en contacto con el suelo con una resistencia mínima a la compresión (zapatas, dados y trabes de liga).

$$f'c = 250 \text{ Kg/cm}^2 = 24.5 \text{ Mpa}$$

- b) Acero de refuerzo (varillas corrugadas, Grado 60, Norma ASTM-A615 ) con límite de fluencia.

$$Fy = 4200 \text{ Kg/cm}^2 = 412 \text{ MPa}$$

- c) Acero Estructural (para placas, perfiles y redondos, Norma ASTM-A36) con un límite de fluencia.

$$Fy = 2530 \text{ Kg/cm}^2 = 248.193 \text{ MPa}$$

- d) Tornillos Estructurales para conexiones principales Norma ASTM-A325.

- e) Tornillos para conexiones secundarias Norma ASTM-307.

- f) Soldadura Electrodo tipo E70XX de acuerdo con la Norma AWS-A51.

- g) Rellenos para cimentaciones según Estudio de Mecánica de Suelos.

## **2.2 Análisis de cargas muertas y cargas especificadas.**

### **Carga muerta (D)**

Las cargas muertas incluyen el peso específico de los materiales de construcción que constituyen el Rack de Tuberías, además de plataformas, equipos y materiales que estén de forma permanente sobre la estructura.

En la carga muerta se considera el peso propio de la estructura, teniendo en cuenta que la densidad para el acero estructural es de 7.83 ton/m<sup>3</sup> y para el concreto reforzado es de 2.4 ton/m<sup>3</sup>.

El peso de la estructura es generado por el programa "STAAD PRO" a través del comando "selfweight".

Para las plataformas además del peso propio de los elementos, se considera rejilla Irving IS-05 4.8x25.4 con un peso de 35,2 kg/m = 0,345 KN/m

### **Cargas Vivas (L)**

<b>ELEMENTO</b>	<b>Lmax(KN/m<sup>2</sup>)</b>	<b>L acc(KN/m<sup>2</sup>)</b>
Plataformas y pasillos de operación	4.905	2.453

### **Carga por Tuberías**

Para el diseño de las estructuras que soportan rutas de tuberías de alta presión como el Rack de Tuberías que se está analizando en esta tesis, la carga estará de acuerdo a la concentración y peso de los tubos llenos de agua y a las válvulas que se tengan, siendo proporcionadas por la disciplina de tuberías. La carga de tuberías se define como la carga del tubo mismo, con sus accesorios y contenido, las cuales están divididas en tres categorías. Ver anexo A.

- 1) Tubería vacía (**Q1**)
- 2) Tubería en operación (**Q2**)
- 3) Tubería Llena de agua en prueba hidrostática (**Q3**)

**Q1.-** Deberá ser utilizada solo para la verificación de la estabilidad del equipo con carga de viento, para la revisión por sismo se usará Q2 para verificar la estabilidad.

**Q2 y Q3.** Deben combinarse con otras cargas como se especifica en el capítulo 3.5.

### **Carga por Charolas Eléctricas (CH)**

Se deben tomar en cuenta el peso de charolas con cableado, su soportería y combinarse como se indica en capítulo 3.5.

### **Carga de Sismo (S)**

La carga de sismo deberá estar de acuerdo a lo indicado en el “*Manual de Diseño de Obras Civiles de la CFE-2008*” (MDOC. CFE DS 2008). Se usará el método dinámico de análisis modal espectral con espectro de respuesta, los parámetros sísmicos se describen en el Capítulo 3.3.

Además, se considera el espectro transparente que es proporcionado en el estudio de mecánica de suelos del proyecto.

Para evaluar las masas de la estructura se considerarán las siguientes cargas:

Carga muerta total (peso propio y válvulas, peso de los largueros, etc.)

### **Carga de Viento (V)**

El diseño por viento será realizado de acuerdo al “*Manual de Diseño de Obras Civiles de la CFE-2008 Diseño por viento*” (MDOC. CFE DV 2008).

Considerando una velocidad regional  $V_r = 155$  km/hr (con un periodo de retorno  $T_r = 200$  años a 1200 metros de altura SNM)

Categoría del terreno es tipo 2 (plano u ondulado con pocas obstrucciones)

### **Carga de Temperatura (T)**

Las fuerzas térmicas actuantes sobre las estructuras y equipos debido a las restricciones de las tuberías calientes, deberán ser consideradas en el diseño tomando en cuenta su magnitud y dirección de acuerdo al reporte análisis hecho a las tuberías.

### **2.3 Códigos, normas y referencias aplicables.**

Los códigos y normas empleados para el diseño son los contenidos en los siguientes manuales y reglamentos.

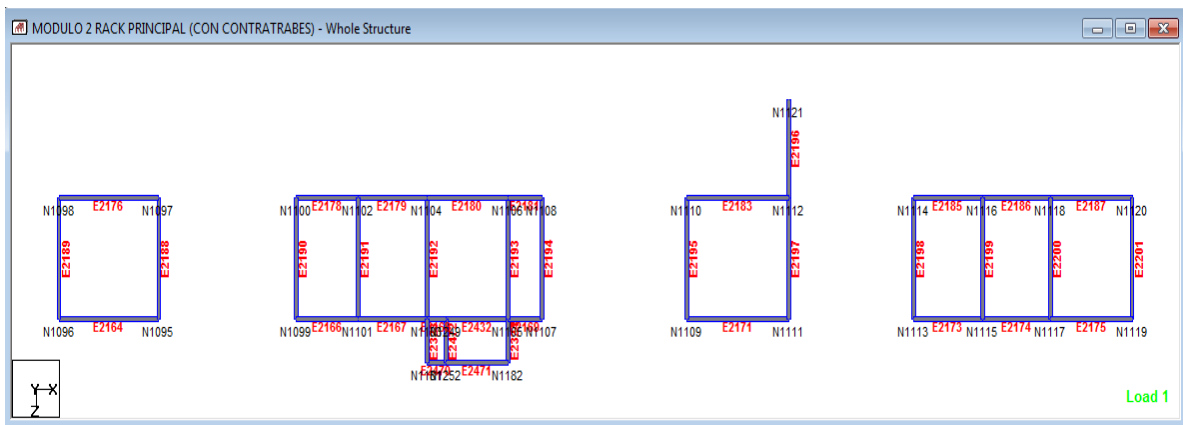
- a) Parámetros y criterios para Sismo, Manual de Diseño de Obras Civiles de la CFE-2008 (*MDOC. CFE DS 2008*).
- b) Parámetros y criterios para Viento. Manual de Diseño de Obras Civiles de la CFE-2008 (*MDOC. CFE DV 2008*).
- c) Características del Cemento Normas ASTM-C-150 Normas ASTM-C-150
- d) Características del Acero de Refuerzo Normas ASTM-A615
- e) Diseño de Estructuras de Concreto Manual del ACI-318-08
- f) Diseño de Estructura de Acero Manual AISC –LRFD, 3ª Ed.

### 3.0 Procesamiento de información para el análisis estructural.

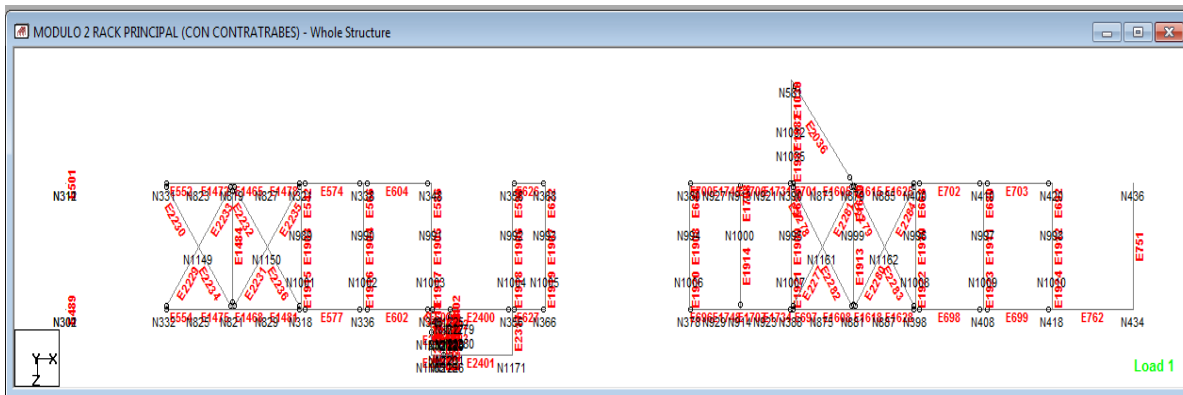
#### 3.1 Estructuración (Modelo estructural).

Para el análisis y diseño de la estructura, se realiza un modelo tridimensional utilizando el programa “STAAD-PRO”, dicho modelo consta de barras o miembros conectados entre sí que representan a las diferentes vigas, traveses de liga, contra vientos, largueros y columnas, en este modelo se especifican los materiales, sus apoyos, secciones y las diferentes condiciones y combinaciones de carga. Es importante hacer notar que las traveses de liga y los dados de apoyo forman parte de la cimentación.

De acuerdo a las dimensiones en planta y elevación de la estructura para el Rack de Tuberías se genera el modelo tridimensional, en las imágenes 3.1 a 3.7 se muestra la conformación del modelo analítico compuesto de nodos y barras.



*Imagen 3.1 Modelo analítico planta de cimentación*

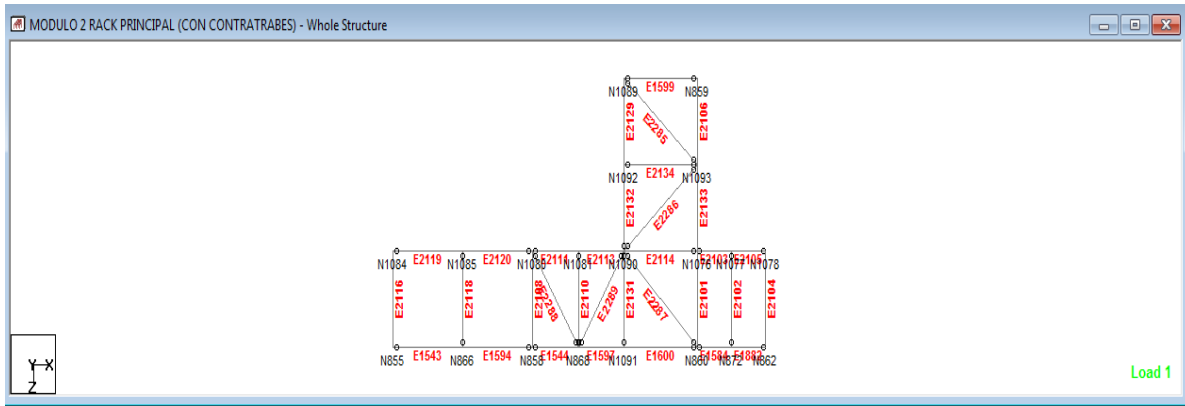


*Imagen 3.2 Modelo analítico planta estructural nivel 7.00*

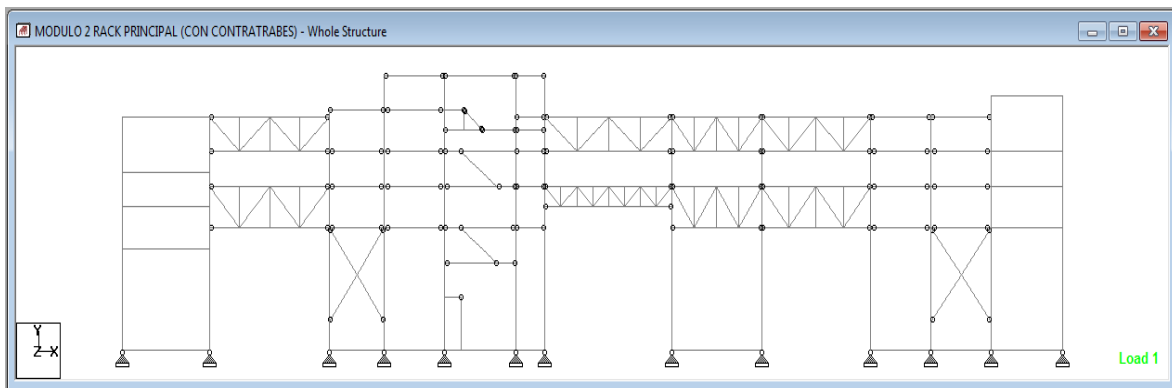




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*Imagen 3.6 Modelo analítico planta estructural nivel 18.00*



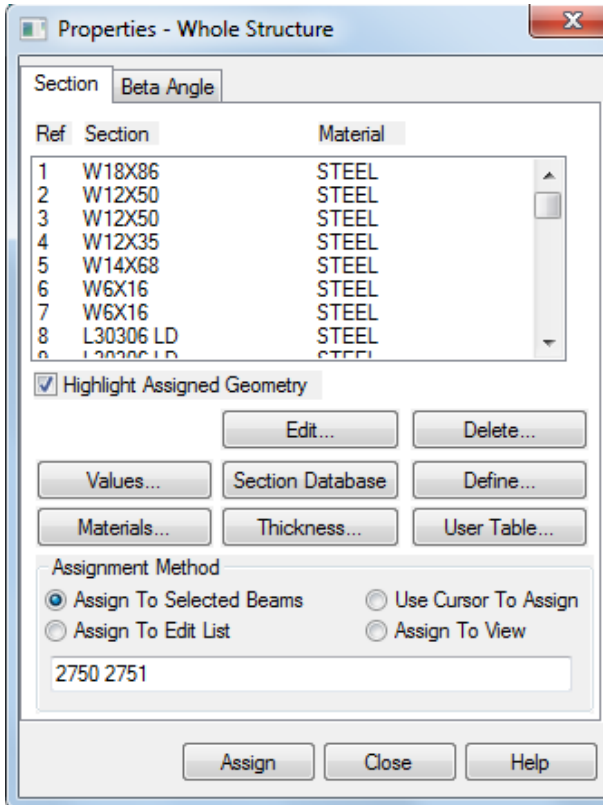
*Imagen 3.7 Modelo analítico elevación longitudinal*

Una vez generados los elementos barra se procede a generar cada una de las secciones para columnas, vigas y largueros.

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
Con lo anterior se definen las secciones en el programa como se puede ver en la Imagen 3.6.











*Imagen 3.6 Definición de secciones con sus respectivos materiales en el modelo analítico.*

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Para el diseño del Rack de Tuberías que motiva esta tesis las secciones quedan de la siguiente forma:

TABLA DE PERFILES						
MARCA	PERFIL	PESO k/m	d	bf	tf	tw
C-7	 IR.-457	128.10	467	281	19.56	12.19
C-7A	 IR.-457	144.30	472	283	22.09	13.59
C-7B	 IR.-305	59.81	303	203	13.1	7.5
C-8	 IR.-254	44.8	266	148	13.0	7.6
C-9	 IR.-305	74.40	310	205	16.3	9.4
C-9A	 IR.-305	52.2	318	167	13.2	7.6
C-10	 IR.-203	31.2	210	134	10.2	6.4
T-16	 IR.-305	74.40	310	205	16.3	9.4
T-30	 IR.-305	79.00	306	254	14.6	8.8
T-13	 IR.-356	101.3	357	255	18.3	10.5
T-32	 IR.-305	74.40	310	205	16.3	9.5
T-19	 IR.-305	52.2	318	167	13.2	7.6
T-20	 IR.-203	31.2	210	134	10.2	6.4
T-21	 IR.-254	44.8	266	148	13.0	7.6
T-22	 IR.-305	59.8	303	203	13.1	7.5
T-23	 IR.-305	59.8	303	203	13.1	7.5
T-24	 IR.-305	96.7	308	305	15.4	9.9
T-25	 IR.-203	35.9	201	165	10.2	6.2
CH-4	 IR.-203	31.2	210	134	10.2	6.4
CV-5	 IR.-254	44.8	266	148	13.0	7.6
CV-6	 IR.-203	31.2	210	134	10.2	6.4
CV-6A	 2U.-102x10	29.16	/	/	/	/
CH-6	 IR.-305	44.5	313	166	11.2	6.6
CH-7	 2U.-76x10	21.44	/	/	/	/

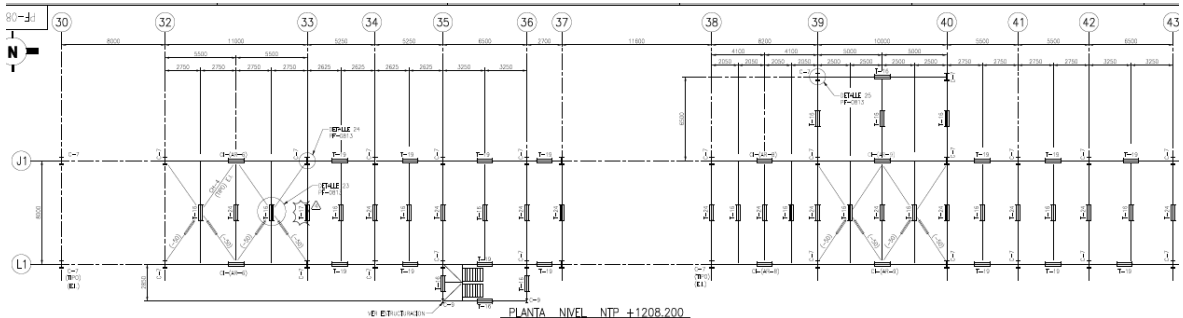
ARMADURAS AR-2, AL AR-6, AR-8, y AR-9						
MARCA	PERFIL	PESO k/m	d	bf	tf	tw
C.S.	 IR.-305	74.4	310	205	16.3	9.4
C.I.	 IR.-305	74.4	310	205	16.3	9.4
D	 2U.-76x10	21.44	/	/	/	/
M	 IR.-152	24	160	102	10.3	6.6

AR-7						
MARCA	PERFIL	PESO k/m	d	bf	tf	tw
C.S.	 IR.-305	52.2	318	167	13.2	7.6
C.I.	 IR.-305	52.2	318	167	13.2	7.6
D	 U.-64x10	11.16	/	/	/	/
M	 IR.-152	24	160	102	10.3	6.6

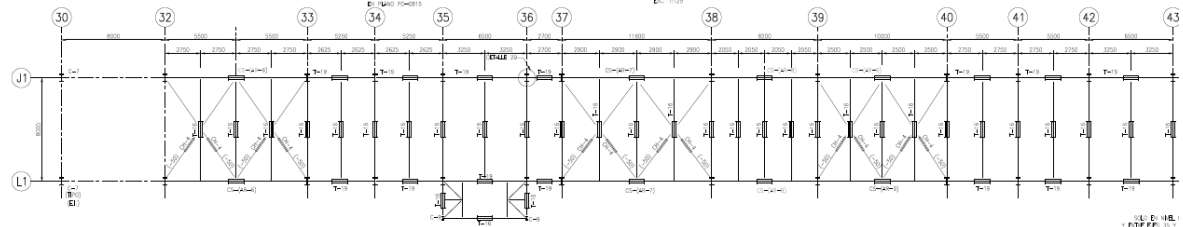
### NOMENCLATURA

C.S.	CUERDA SUPERIOR
C.I.	CUERDA INFERIOR
AR	ARMADURA
PB	PLACA BASE
D	DIAGONAL
M	MONTANTE
NPT	NIVEL PISO TERMINADO
NTP	NIVEL TOPE DE PERFIL
NDE	NIVEL DESPLANTE DE ESTRUCTURA
C	COLUMNA
T	TRABE
CH	CONTRAVENTO HORIZONTAL

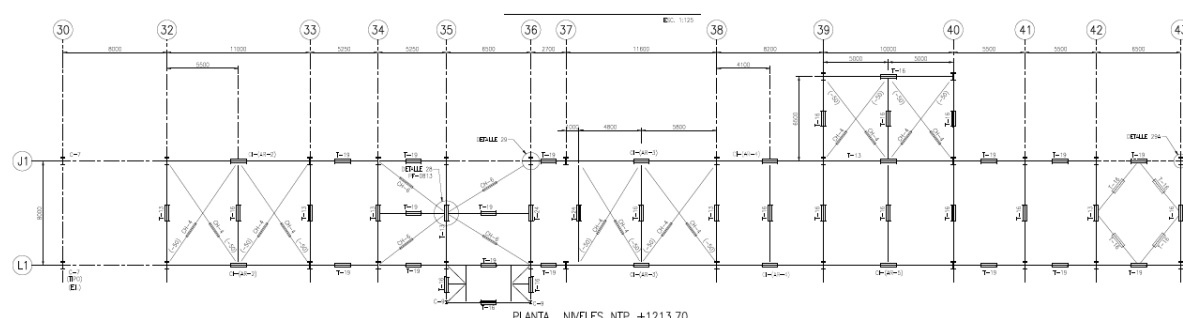
# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES



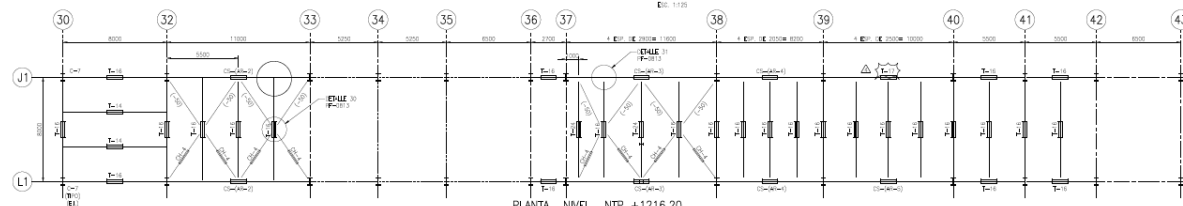
PLANTA NIVEL NTP +1208.200



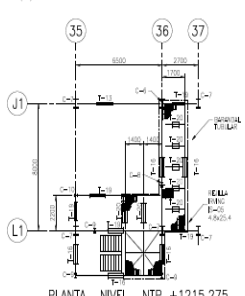
PLANTA NIVELES NTP +1211.20



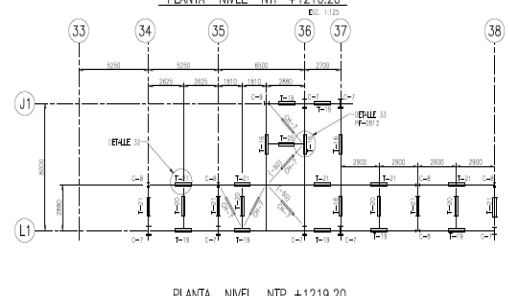
PLANTA NIVELES NTP +1213.70



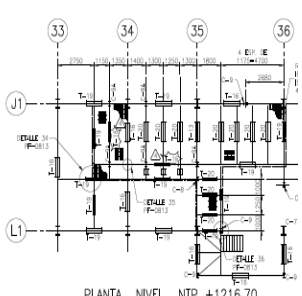
PLANTA NIVEL NTP +1216.20



PLANTA NIVEL NTP +1215.275



PLANTA NIVEL NTP +1219.20

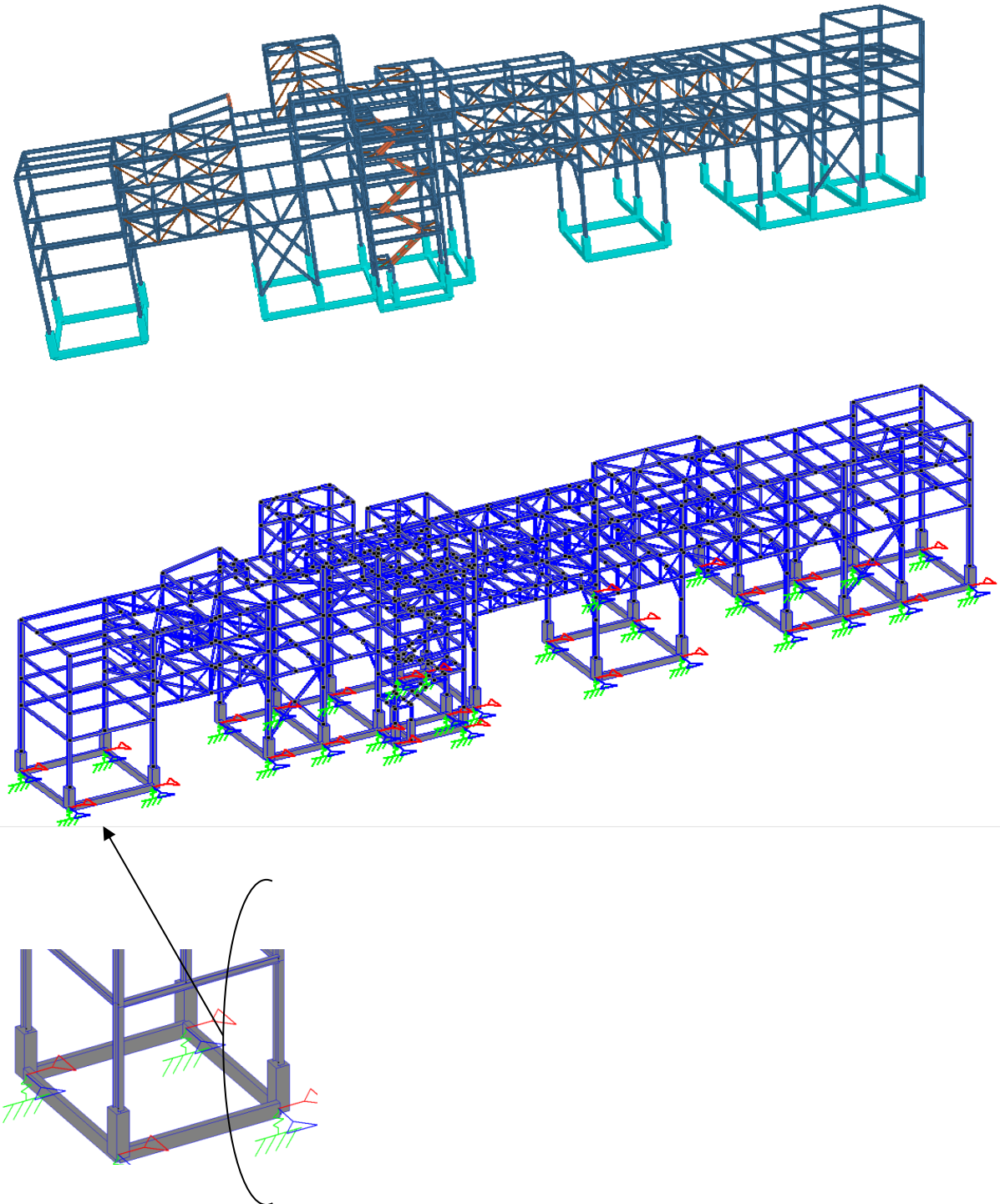


PLANTA NIVEL NTP +1216.70

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

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Se procede a asignar a cada elemento barra su sección correspondiente la Imagen 3.7 muestra el resultado de este proceso.



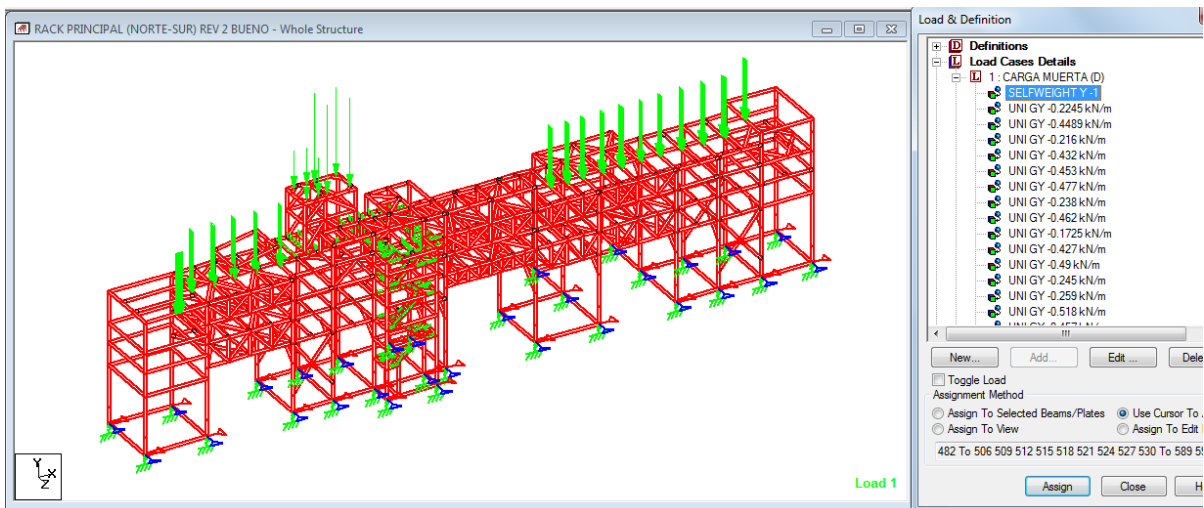
*Imagen 3.7 Perspectiva del modelo con las secciones y materiales asignados a cada elemento*

## 3.2. Obtención de fuerzas gravitacionales.

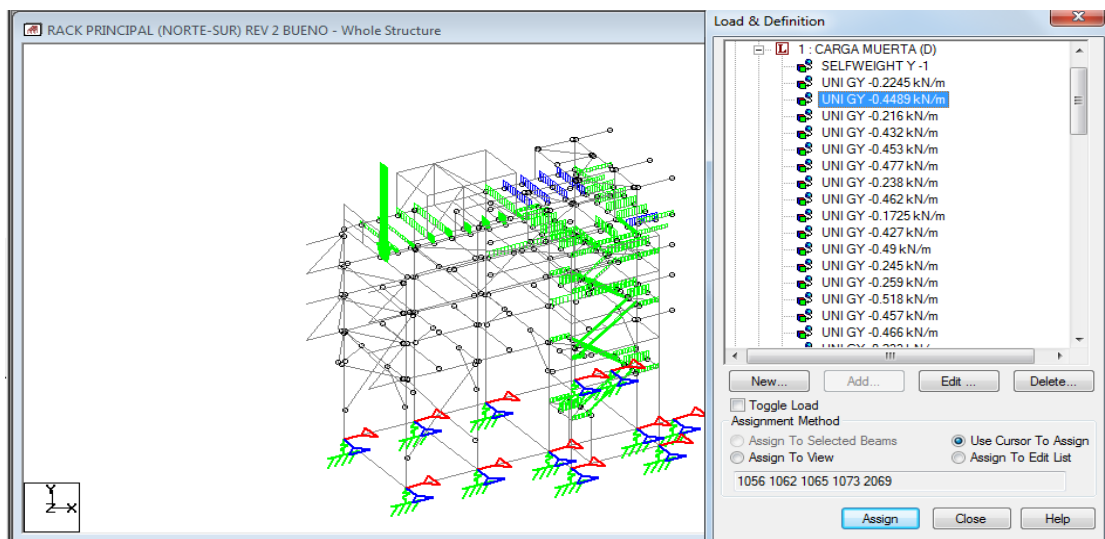
### • Carga muerta de la estructura

Se considera el peso propio de la estructura, el programa STAAD.Pro lo considera directamente, la densidad para el acero estructural es de  $7.85 \text{ ton/m}^3$  y para el concreto reforzado es de  $2.4 \text{ ton/m}^3$ .

En el modelo analítico se aplica el peso de la estructura con el comando "selfweight".



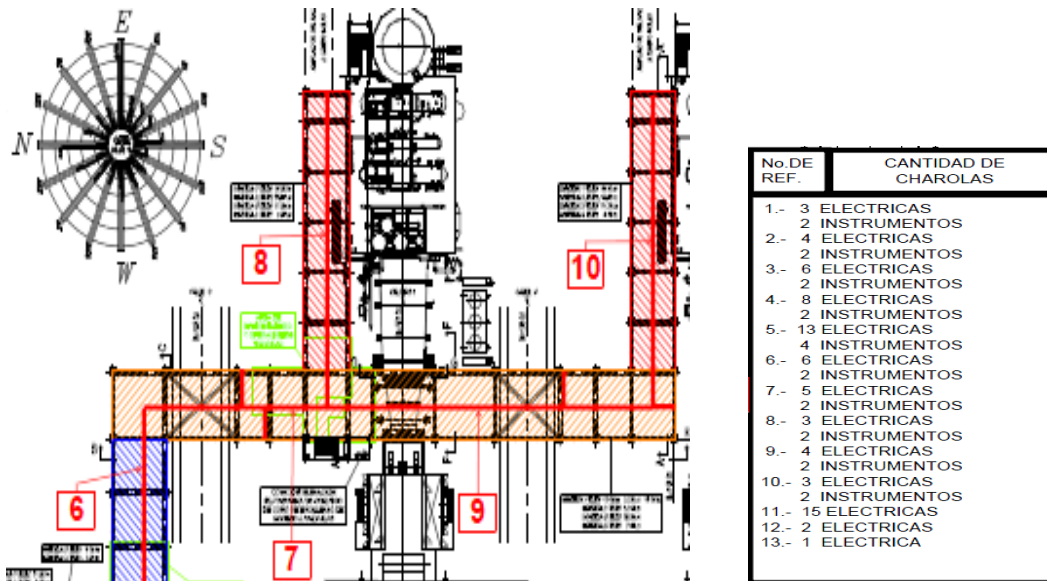
Para las plataformas además del peso propio de los elementos, se considera rejilla Irving IS-05 4.8x25.4 con un peso de  $35,2 \text{ kg/m} = 0,345 \text{ KN/m}$



## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

### • Carga por charolas de cableado de instrumentación y eléctrico

Se considera por información eléctrica e instrumentación que se colocan sobre el Rack de Tuberías Principal Norte-Sur 7 camas de charolas para cableado a lo largo de sus 86 m de longitud y se ubican en el nivel 11.20m, cinco charolas sobre el eje "A" y cinco charolas más sobre el eje "B" con el acomodo mostrado en la Imagen 3.9



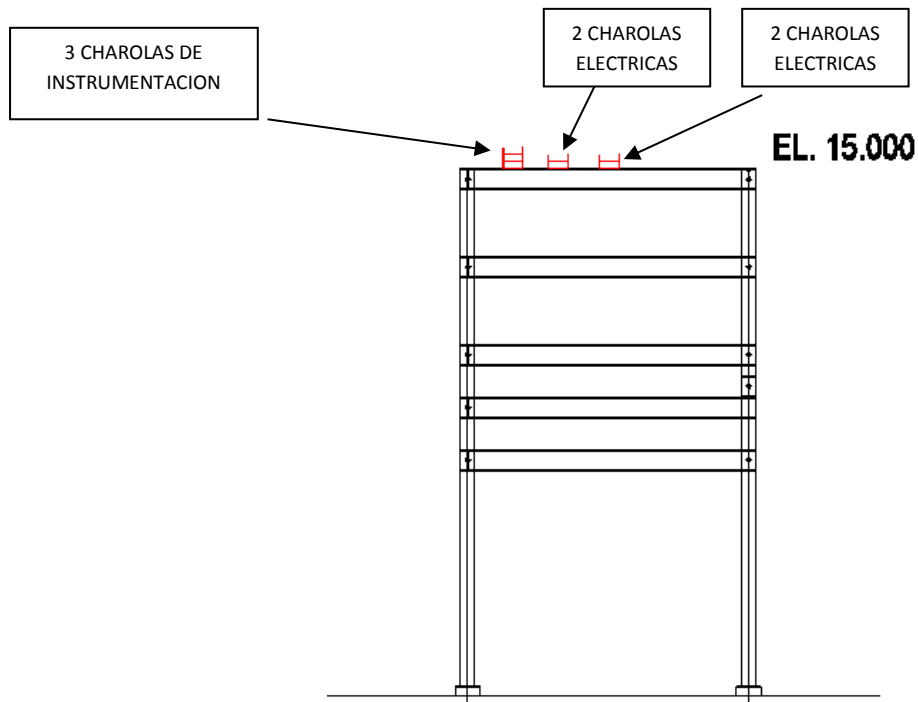
*Imagen 3.9 Acomodo de charolas sobre el Rack de Tuberías  
(Fuente: Plano de proyecto CFE-PM27005-SRPM3-PM-0152 Rev. 0)*

El peso de cada una de estas charolas con su correspondiente soporte es de 120 kg/m para charolas eléctricas y de 130 kg/m para charolas de instrumentación.



## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Calculando el siguiente arreglo:



Se colocan soportes intermedios entre los marcos:

Ejemplo:

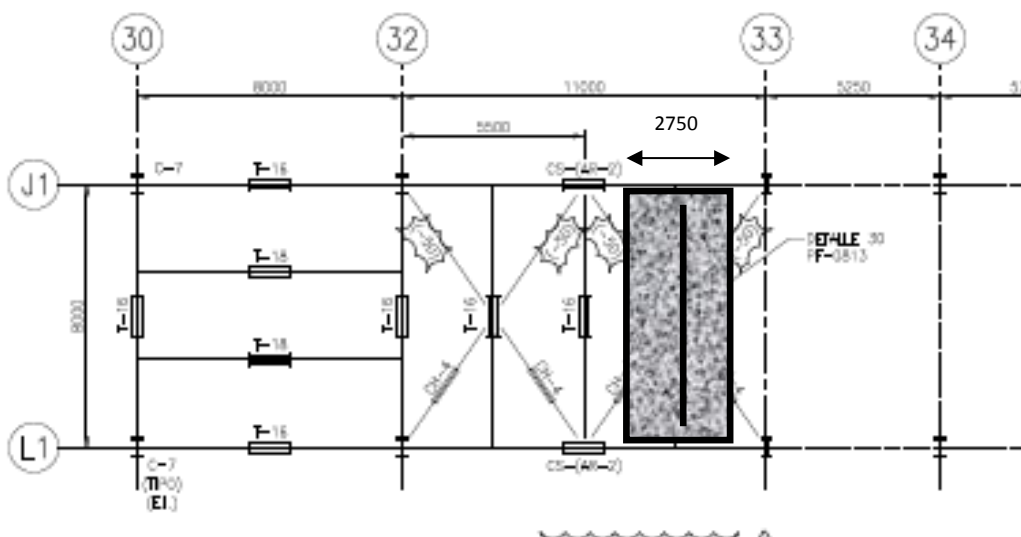


Imagen 3.10 Aplicación de cargas por charolas, las cargas son aplicadas en marcos principales y en ligueros intermedios del Nivel +11.20

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

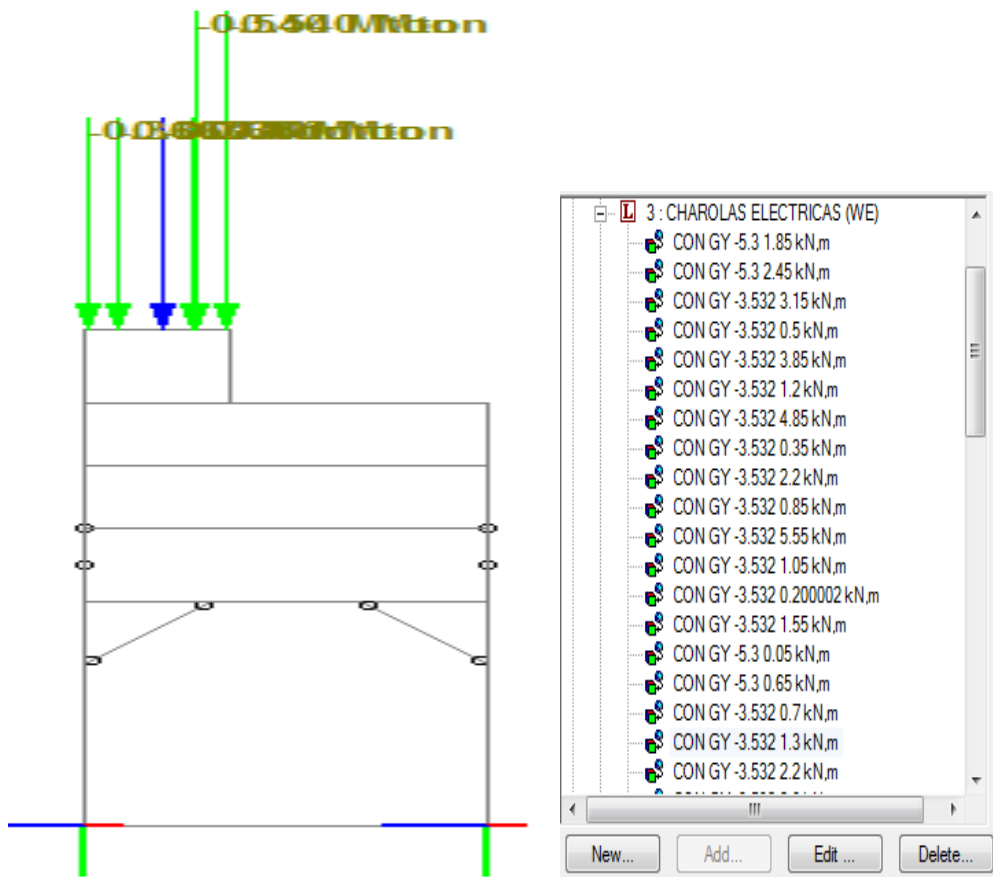
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Considerando el arreglo anterior se tiene lo siguiente:

$$W = (W_{CH} \times \text{No de charolas} \times \text{Long.}) / \text{No de apoyo del soporte}$$

$$W = (120 \times 2 \times 2.75) / 2 = 330 \text{ kg} = 3.237 \text{ KN por soporte.}$$

De esta forma se calculan todas las cargas puntuales.



### • Cargas por tuberías válvulas y equipos

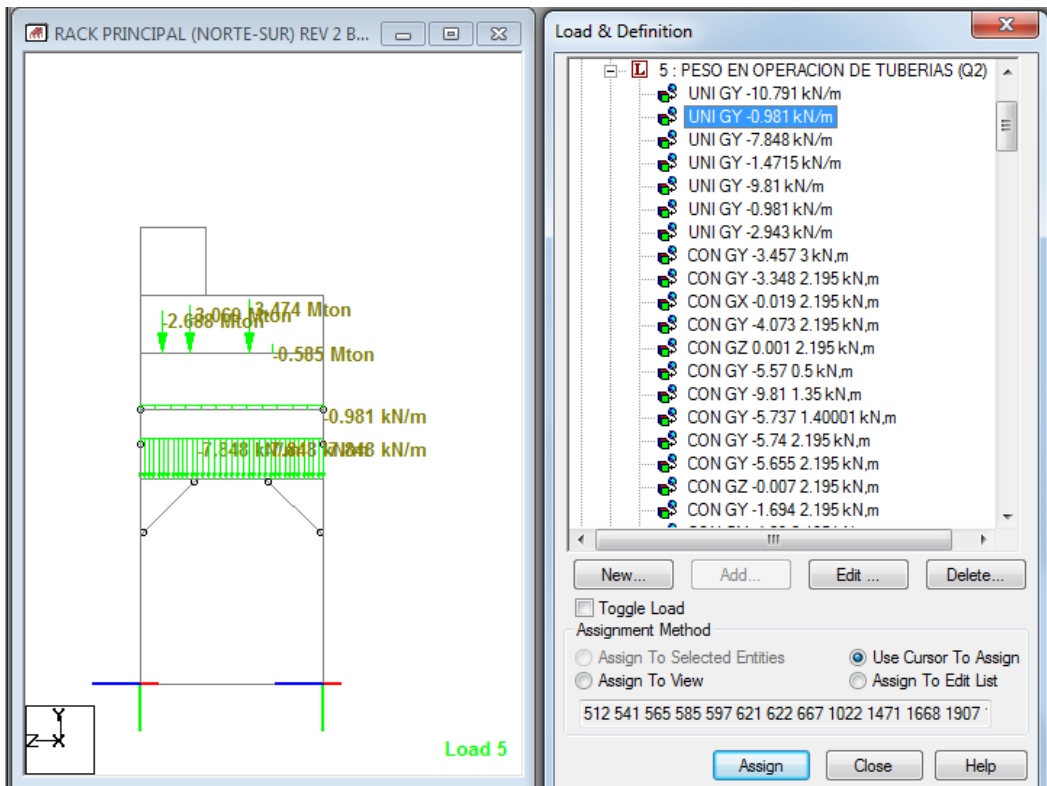
Como se ha mencionado la disciplina de tuberías proporciona las cargas que actúan sobre la estructura del Rack considerando el peso propio de las tuberías y equipos que se apoyen, estas cargas son reacciones en los puntos indicados en el anexo de cargas de tuberías.

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Ver cargas por flexible en documento No. CFE-PM27005-SRPM3-CC-101 "CARGAS SOBRE RACK DE TUBERIAS PARA OBRA CIVIL" Rev. A2 el cual se anexa a esta tesis.

Para tuberías frías se consideran las cargas que se muestran en el siguiente plano:

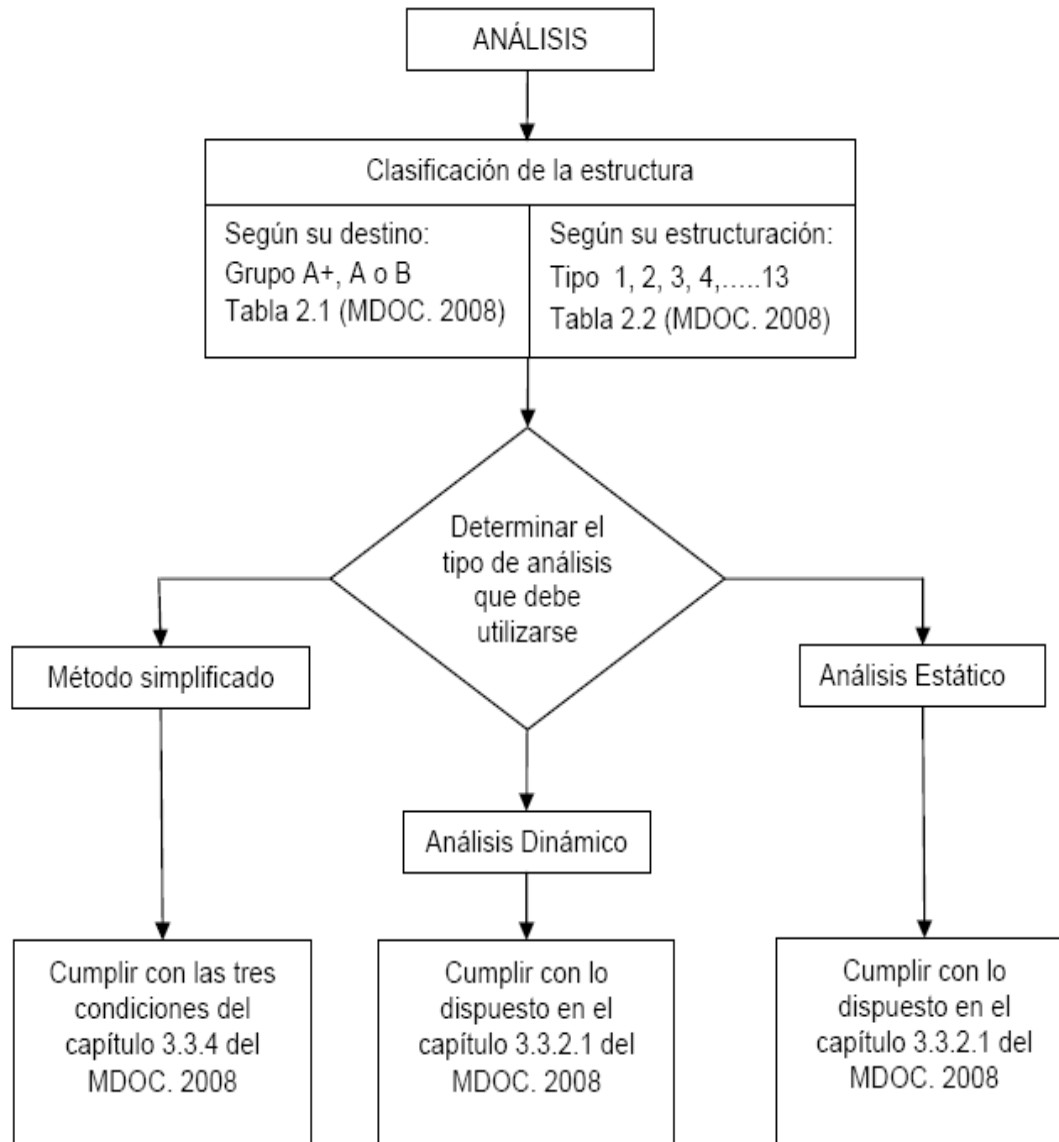
- CARGAS DE TUBERIAS SOBRE RACK PRINCIPAL Y RACK TREN 1 Y 2 Rev. 0 VER ANEXOS



*Imagen 3.12 Aplicación en el modelo estructural de fuerzas por tuberías*

### 3.3. Obtención de fuerzas sísmicas.

Para la obtención de fuerzas sísmicas se procede de acuerdo al siguiente diagrama de flujo que indica la ruta a seguir para determinar el tipo de análisis a utilizar, basado en el *MDOC CFE DS 2008*.



## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

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### • Clasificación de la estructura

Con base a la clasificación indicada en la Tabla 6 extraída del *MDOC CFE DS 2008* se puede determinar que el Rack de Tuberías que se está analizando, se encuentra dentro del grupo A, ya que el proyecto en general se trata de una planta de generación de energía.

GRUPO	DESCRIPCIÓN
<b>A+</b>	Las estructuras de "gran importancia", o del Grupo A+, son estructuras en que se requiere un grado de seguridad extrema. Su falla es inadmisibles porque, si se presenta, conduciría a la pérdida de miles de vidas humanas, a un grave daño ecológico, económico o social, o bien, impediría el desarrollo nacional o cambiaría el rumbo del país. Son estructuras de importancia extrema, como las grandes presas y las plantas nucleares.
<b>A</b>	Estructuras en que se requiere un grado de seguridad alto. Construcciones cuya falla estructural causaría la pérdida de un número elevado de vidas o pérdidas económicas o culturales de magnitud intensa o excepcionalmente alta, o que constituyan un peligro significativo por contener sustancias tóxicas o inflamables, así como construcciones cuyo funcionamiento sea esencial a raíz de un sismo. Tal es el caso de puentes principales, sistemas de abastecimiento de agua potable, subestaciones eléctricas, centrales telefónicas, estaciones de bomberos, archivos y registros públicos, monumentos, museos, hospitales, escuelas, estadios, templos, terminales de transporte, salas de espectáculos y hoteles que tengan áreas de reunión que pueden alojar un número elevado de personas, gasolineras, depósitos de sustancias inflamables o tóxicas y locales que alojen equipo especialmente costoso. Se incluyen también todas aquellas estructuras de plantas de generación de energía eléctrica cuya falla por movimiento sísmico pondría en peligro la operación de la planta, así como las estructuras para la transmisión y distribución de energía eléctrica.
<b>B</b>	Estructuras en que se requiere un grado de seguridad convencional. Construcciones cuya falla estructural ocasionaría pérdidas moderadas o pondría en peligro otras construcciones de este grupo o del grupo A, tales como naves industriales, locales comerciales, estructuras comunes destinadas a vivienda u oficinas, salas de espectáculos, hoteles, depósitos y estructuras urbanas o industriales no incluidas en el grupo A, así como muros de retención, bodegas ordinarias y bardas. También se incluyen todas aquellas estructuras de plantas de generación de energía eléctrica que en caso de fallar por temblor no paralizarían el funcionamiento de la planta.

*Tabla 6 Clasificación de las estructuras según su importancia de acuerdo al MDOC CFE DS 2008*

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

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Por otro lado, se clasifican las estructuras de acuerdo a su estructuración, en el capítulo 3.2.3 *CLASIFICACIÓN DE CONSTRUCCIONES SEGÚN SU ESTRUCTURACIÓN* del MDOC CFE DS 2008 se enumeran los 13 tipos de estructura que deben considerarse para el diseño sísmico. En este caso la estructura del Rack de Tuberías está dentro del Tipo 1, como se indica en la Imagen 3.13

<b>TIPO 1</b>	Estructuras de edificios: Estructuras comunes tales como edificios urbanos, naves industriales típicas, salas de espectáculos y estructuras semejantes, en que las fuerzas laterales se resisten en cada nivel por marcos continuos contraventeados o no, por diafragmas o muros o por la combinación de estos.
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*Imagen 3.13 Extracto de la Clasificación de las estructuras según su estructuración (estructuras tipo 1) MDOC CFE DS 2008*

### • Elección del tipo de análisis

Para la obtención de fuerzas sísmicas se debe determinar en primera instancia el tipo de análisis que se emplea, considerando que se puede recurrir a tres tipos:

- a) Método simplificado
- b) Análisis estático
- c) Análisis dinámico

De acuerdo al MDOC CFE DS 2008, para la utilización del método simplificado es necesario cumplir con tres condiciones que se describen en el capítulo 3.3.4 *MÉTODO SIMPLIFICADO*, para el caso de la estructura del Rack de Tuberías en cuestión no se cumple con la primera condición establecida es decir que:

*“1. En cada planta, al menos el 75% de las cargas verticales estarán soportadas por muros ligados entre sí mediante losas monolíticas u otros sistemas de piso suficientemente resistentes y rígidos al corte.”*

Por lo que este método queda descartado, ya que el Rack de Tuberías no cuenta con muros ni losas monolíticas que soporten las cargas verticales.

El método estático, descrito en la sección 3.3.5 *MDOC CFE DS 2008*, es aplicable a edificios regulares cuya altura sea menor o igual que 30 m y estructuras irregulares

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

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con altura no mayor de 20 m. Para que una estructura se considere regular, debe cumplir las condiciones descritas en la sección 3.3.2, en caso contrario, se considerará irregular al no cumplir con algunas de las condiciones indicadas en la sección 3.3.2 del MDOC CFE DS 2008 como se puede ver a continuación:

- 1) La distribución en planta de masas, muros y otros elementos resistentes, es sensiblemente simétrica con respecto a dos ejes ortogonales. Estos elementos son sensiblemente paralelos a los ejes ortogonales principales del edificio.

Ok cumple.

- 2) La relación entre la altura y la dimensión menor de la base no es mayor que 2.5.

Ok cumple ya que  $8 / 15.50 = 0.516$

- 3) La relación entre largo y ancho de la base no excede de 2.5.

No cumple ya que  $86 / 8 = 10.75$

- 4) En planta no se tienen entrantes ni salientes cuya dimensión exceda 20% de la dimensión de la planta medida paralelamente a la dirección en que se considera la entrante o saliente.

No cumple

- 5) En cada nivel se tiene un sistema de techo o piso rígido y resistente.

No cumple

- 6) No se tienen aberturas en los sistemas de techo o piso cuya dimensión exceda 20% de la dimensión de la planta medida paralelamente a la dirección en que se considera la abertura. Las áreas huecas no ocasionan asimetrías significativas ni difieren en posición de un piso a otro y el área total de aberturas no excede, en ningún nivel, 20% del área de la planta.

No cumple

La estructura del Rack de Tuberías es irregular y el método estático tampoco es aplicable, por lo que se emplea el método dinámico de análisis modal espectral con espectro de respuesta.

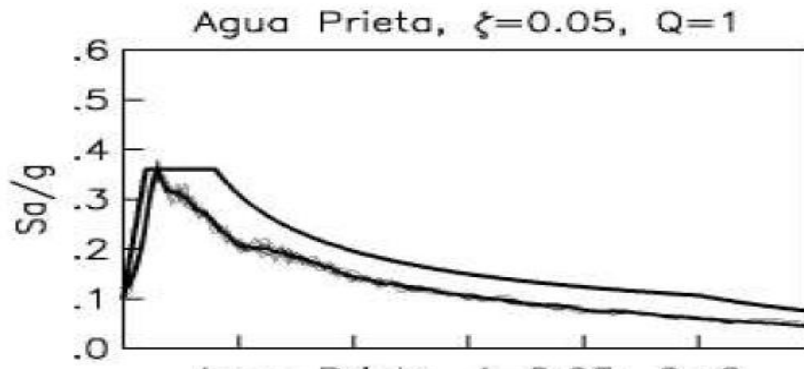
• **Obtención de espectro sísmico transparente**

Para el análisis sísmico se utilizará el Espectro de Diseño que proporcione el Estudio CFE-PM27005-SRJC-IN-0010 "ESPECTRO DE DISEÑO SISMICO", que corresponde a la zona donde se localizará la Central de Ciclo Combinado en Agua Prieta Sonora. Se aplicará en sus tres direcciones, considerando la masa de la estructura y las fuerzas gravitacionales como son las cargas muertas por rejilla y las cargas vivas máximas e instantáneas definidas anteriormente. El análisis se hará de acuerdo al MDOC CFE DS 2008.

**Parámetros para el espectro reducido de diseño "Estructuras tipo 1 (Rack)"**

0.1	g	
0.36	g	<i>Aceleración de la meseta</i>
0.1	s	<i>Límite inferior de la meseta</i>
0.4	s	<i>Límite superior de la meseta</i>
2.5	s	<i>Periodo de inicio de la rama descendente en que los desplazamientos tienden al desp. del terreno</i>
1		<i>Relación de amortiguamiento</i>
2		<i>Factor de comportamiento sísmico</i>
Q		<i>Factor reductor por ductilidad</i>
2		<i>Factor de sobrerresistencia</i>
0.667		<i>Factor que controla la caída de la tercera rama</i>
1		<i>Parametro que controla la caída espectral para <math>T_e \geq T_e</math></i>
0.7		<i>Factor correctivo por irregularidad (Capitulo 3.3.2.4)</i>
0.8		<i>Factor por redundancia (Capitulo 3.3.1.4) para dirección transversal del rack</i>
1		<i>Factor por redundancia (Capitulo 3.3.1.4) para dirección transversal del rack</i>





Los efectos en la respuesta tales como desplazamientos y elementos mecánicos, se combinarán empleando la raíz cuadrada de la suma de los cuadrados de los efectos en las tres direcciones ortogonales:

$$S = \sqrt{S_x^2 + S_y^2 + S_z^2} \quad (3.27)$$

El espectro presentado es un espectro transparente por reflejar la totalidad del peligro sísmico. Habrá que tomar en cuenta el tipo de estructura, la importancia estructural y para el estado límite de colapso, las reducciones por ductilidad y sobre resistencia para obtención del espectro de diseño en las direcciones X (longitudinal), Y (vertical) y Z (transversal), a continuación, se especifican los parámetros para la construcción del espectro de diseño para las dos direcciones de análisis X, Y y Z utilizados en el diseño del Rack de Tuberías.

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

**Espectro reducido de diseño con 5% de amortiguamiento para dirección transversal (Z)**

Te (s)	a	P	Q'	R	ρ	Te (s)	a'
0.00	0.100		0.70	2.00	0.8	0.00	0.089
0.05	0.230		0.79	2.00	0.8	0.05	0.183
0.10	0.360		0.88	2.00	0.8	0.10	0.257
0.40	0.360	1.000	1.40	2.00	0.8	0.40	0.161
0.50	0.310	1.000	1.40	2.00	0.8	0.50	0.138
0.60	0.275	1.000	1.40	2.00	0.8	0.60	0.123
0.70	0.248	1.000	1.40	2.00	0.8	0.70	0.111
0.80	0.227	1.000	1.40	2.00	0.8	0.80	0.101
0.90	0.210	1.000	1.40	2.00	0.8	0.90	0.094
1.00	0.195	1.000	1.40	2.00	0.8	1.00	0.087
1.50	0.149	1.000	1.40	2.00	0.8	1.50	0.067
2.00	0.123	1.000	1.40	2.00	0.8	2.00	0.055
2.50	0.106	1.000	1.40	2.00	0.8	2.50	0.047
3.00	0.045	1.000	1.40	2.00	0.8	3.00	0.020
3.50	0.022	1.000	1.40	2.00	0.8	3.50	0.010
4.00	0.012	1.000	1.40	2.00	0.8	4.00	0.005
4.50	0.007	1.000	1.40	2.00	0.8	4.50	0.003
5.00	0.004	1.000	1.40	2.00	0.8	5.00	0.002
5.50	0.003	1.000	1.40	2.00	0.8	5.50	0.001
6.00	0.002	1.000	1.40	2.00	0.8	6.00	0.001
6.50	0.001	1.000	1.40	2.00	0.8	6.50	0.001
7.00	0.001	1.000	1.40	2.00	0.8	7.00	0.000

$$Q' = 1 + (Q - 1) \sqrt{\frac{\beta T_e}{k T_b}}$$

si  $T_e < T_b$

$$Q' = 1 + (Q - 1) \sqrt{\frac{\beta p}{k}}$$

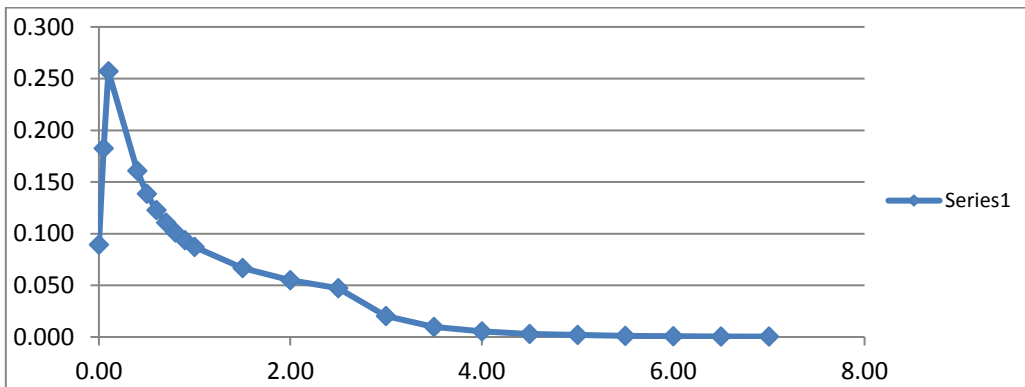
si  $T_e > T_b$

$$p = k + (1 - k) \left( \frac{T_b}{T_e} \right)^2$$

para  $T_e > T_b$

$$R = R_0 + 0.5 \left( 1 - \sqrt{\frac{T_e}{T_a}} \right)$$

si  $T_e \leq T_a$



ESPECTRO REDUCIDO DE DISEÑO PARA 5% DE AMORTIGUAMIENTO  
ESTRUCTURAS TIPO 1 (RACK) **Q=2, R=2**

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

**Espectro reducido de diseño con 5% de amortiguamiento para dirección transversal (X)**

Te (s)	a	p	Q'	R	ρ	Te (s)	a'
0.00	0.100		0.70	2.50	1.0	0.00	0.057
0.05	0.230		0.79	2.15	1.0	0.05	0.136
0.10	0.360		0.88	2.00	1.0	0.10	0.206
0.40	0.360	1.000	1.40	2.00	1.0	0.40	0.129
0.50	0.310	1.000	1.40	2.00	1.0	0.50	0.111
0.60	0.275	1.000	1.40	2.00	1.0	0.60	0.098
0.70	0.248	1.000	1.40	2.00	1.0	0.70	0.089
0.80	0.227	1.000	1.40	2.00	1.0	0.80	0.081
0.90	0.210	1.000	1.40	2.00	1.0	0.90	0.075
1.00	0.195	1.000	1.40	2.00	1.0	1.00	0.070
1.50	0.149	1.000	1.40	2.00	1.0	1.50	0.053
2.00	0.123	1.000	1.40	2.00	1.0	2.00	0.044
2.50	0.106	1.000	1.40	2.00	1.0	2.50	0.038
3.00	0.045	1.000	1.40	2.00	1.0	3.00	0.016
3.50	0.022	1.000	1.40	2.00	1.0	3.50	0.008
4.00	0.012	1.000	1.40	2.00	1.0	4.00	0.004
4.50	0.007	1.000	1.40	2.00	1.0	4.50	0.002
5.00	0.004	1.000	1.40	2.00	1.0	5.00	0.001
5.50	0.003	1.000	1.40	2.00	1.0	5.50	0.001
6.00	0.002	1.000	1.40	2.00	1.0	6.00	0.001
6.50	0.001	1.000	1.40	2.00	1.0	6.50	0.000
7.00	0.001	1.000	1.40	2.00	1.0	7.00	0.000

$$Q' = 1 + (Q - 1) \sqrt{\frac{\beta T_e}{k T_b}}$$

si  $T_e < T_b$

$$Q' = 1 + (Q - 1) \sqrt{\frac{\beta p}{k}}$$

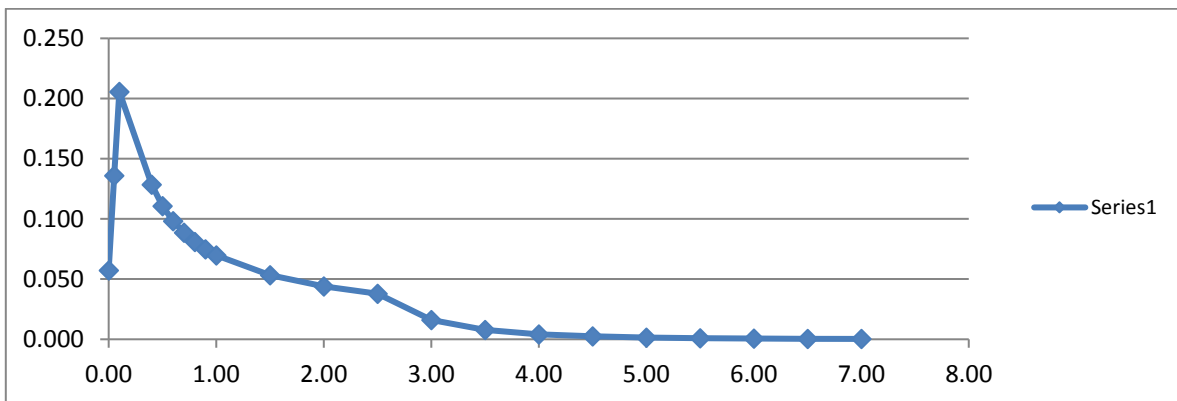
si  $T_e > T_b$

$$p = k + (1 - k) \left( \frac{T_b}{T_e} \right)^2$$

para  $T_e > T_b$

$$R = R_0 + 0.5 \left( 1 - \sqrt{\frac{T_e}{T_a}} \right)$$

si  $T_e \leq T_a$



ESPECTRO REDUCIDO DE DISEÑO PARA 5% DE AMORTIGUAMIENTO  
ESTRUCTURAS TIPO 1 (RACK) **Q=2, R=2**

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El componente vertical se tomará como  $1.4(0.05/T_v)^{2/3}$  de la componente horizontal mayor para  $T_v \geq 0.05s$  y 1.4 para  $T_v < 0.05s$ , donde  $T_v$  es el periodo fundamental estimado de la estructura en la dirección vertical. La acción de la componente vertical podrá despreciarse para estructuras desplantadas en suelos blandos y localizados a más de 80 km de una falla activa.

Para nuestro caso:

$T_v = 0.27821$  seg. (ver obtención del periodo en el capítulo 4)

Por lo tanto: Factor = 0.45

**Espectro reducido de diseño con 5% de amortiguamiento para dirección vertical (Y)**

Te (s)	a	p	Q'	R	$\rho$	Te (s)	a'	a'
0.00	0.100		0.70	2.50	0.8	0.00	0.071	0.0318
0.05	0.230		0.79	2.15	0.8	0.05	0.170	0.0758
0.10	0.360		0.88	2.00	0.8	0.10	0.257	0.1146
0.40	0.360	1.000	1.40	2.00	0.8	0.40	0.161	0.0717
0.50	0.310	1.000	1.40	2.00	0.8	0.50	0.138	0.0618
0.60	0.275	1.000	1.40	2.00	0.8	0.60	0.123	0.0547
0.70	0.248	1.000	1.40	2.00	0.8	0.70	0.111	0.0493
0.80	0.227	1.000	1.40	2.00	0.8	0.80	0.101	0.0451
0.90	0.210	1.000	1.40	2.00	0.8	0.90	0.094	0.0417
1.00	0.195	1.000	1.40	2.00	0.8	1.00	0.087	0.0389
1.50	0.149	1.000	1.40	2.00	0.8	1.50	0.067	0.0297
2.00	0.123	1.000	1.40	2.00	0.8	2.00	0.055	0.0245
2.50	0.106	1.000	1.40	2.00	0.8	2.50	0.047	0.0211
3.00	0.045	1.000	1.40	2.00	0.8	3.00	0.020	0.0090
3.50	0.022	1.000	1.40	2.00	0.8	3.50	0.010	0.0044
4.00	0.012	1.000	1.40	2.00	0.8	4.00	0.005	0.0024
4.50	0.007	1.000	1.40	2.00	0.8	4.50	0.003	0.0014
5.00	0.004	1.000	1.40	2.00	0.8	5.00	0.002	0.0008
5.50	0.003	1.000	1.40	2.00	0.8	5.50	0.001	0.0005
6.00	0.002	1.000	1.40	2.00	0.8	6.00	0.001	0.0004
6.50	0.001	1.000	1.40	2.00	0.8	6.50	0.001	0.0002
7.00	0.001	1.000	1.40	2.00	0.8	7.00	0.000	0.0002

$$Q' = 1 + (Q - 1) \sqrt{\frac{\beta T_e}{k T_b}}$$

si  $T_e < T_b$

$$Q' = 1 + (Q - 1) \sqrt{\frac{\beta p}{k}}$$

si  $T_e > T_b$

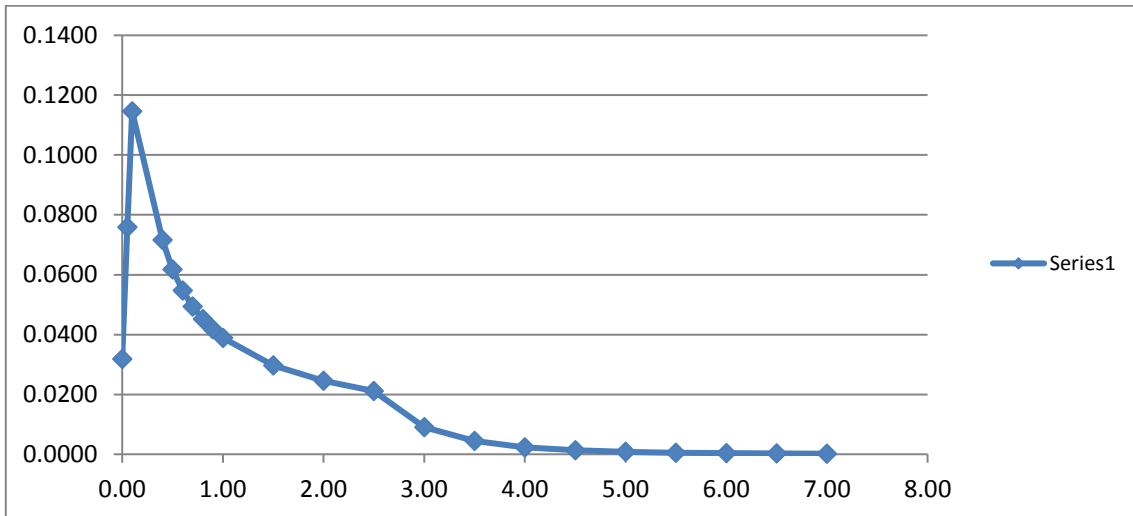
$$p = k + (1 - k) \left( \frac{T_b}{T_e} \right)^2$$

para  $T_e > T_b$

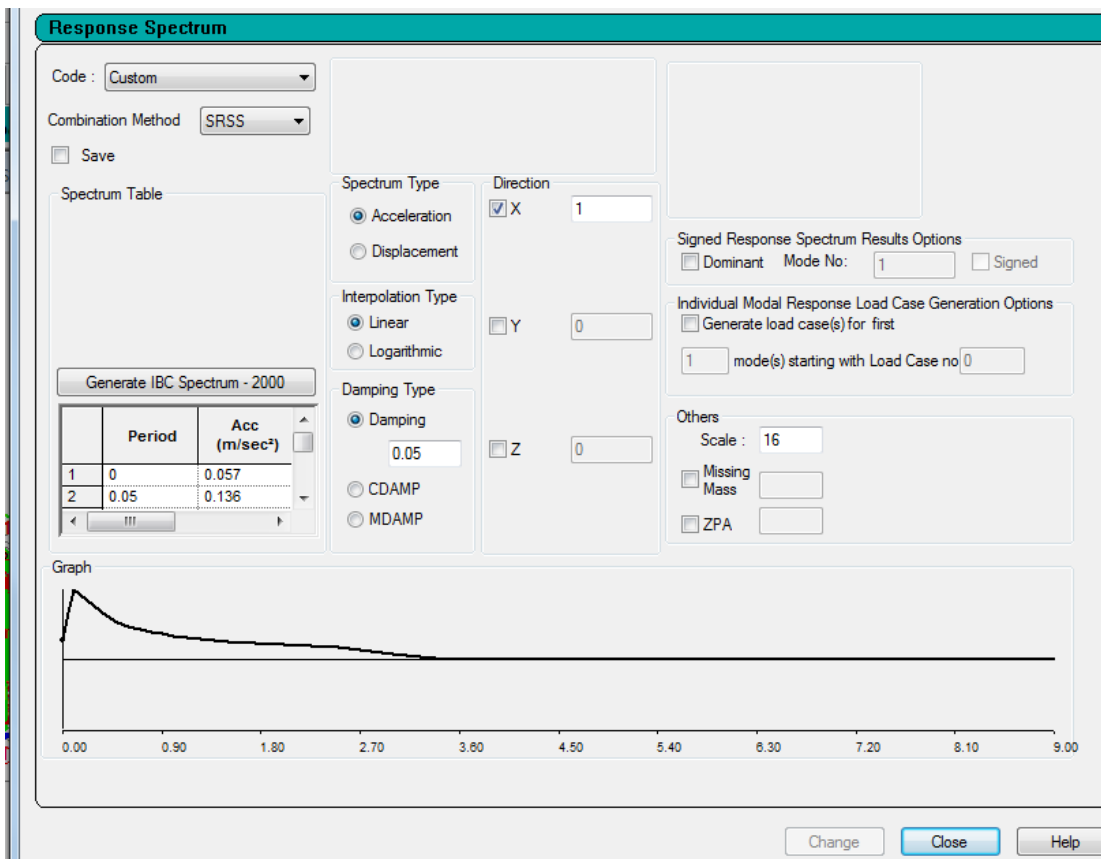
$$R = R_0 + 0.5 \left( 1 - \sqrt{\frac{T_e}{T_a}} \right)$$

si  $T_e \leq T_a$

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

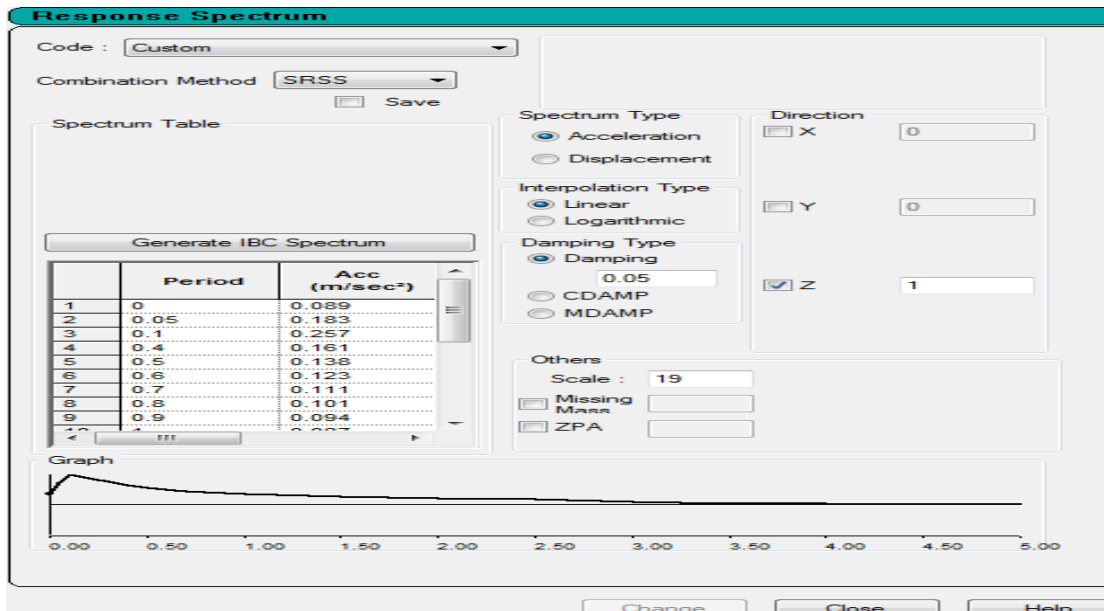


Los espectros son incorporados al modelo en sus respectivas direcciones las Imágenes 3.16 y 3.17 muestran el resultado de este proceso.

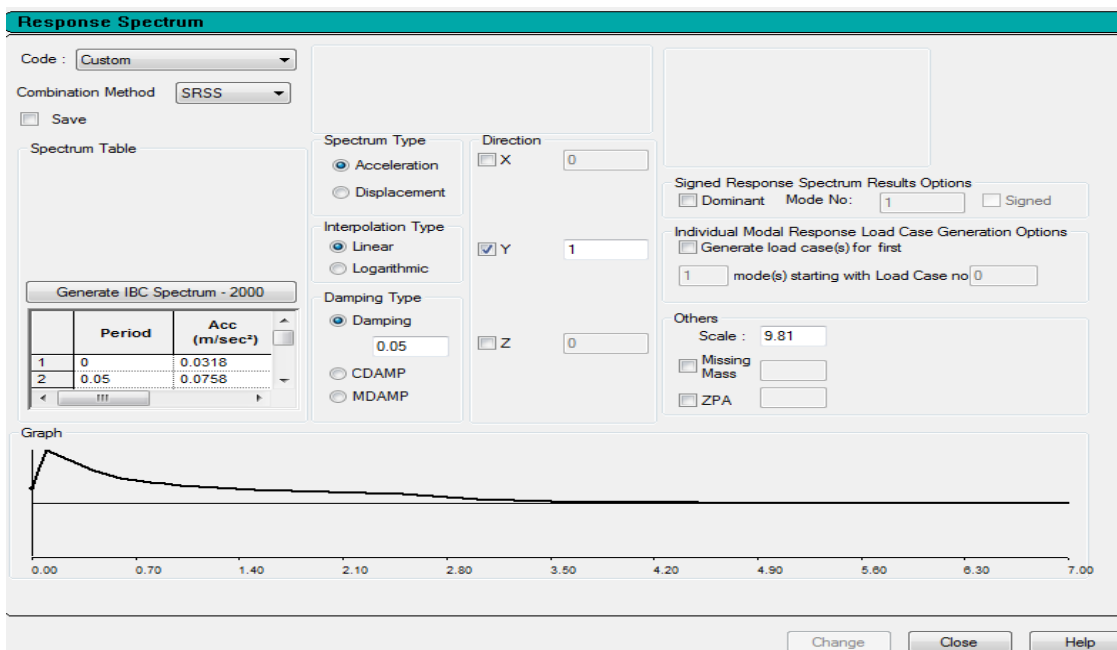


*Imagen 3.16 Aplicación al modelo analítico del espectro sísmico en dirección X (Sx)*

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES



*Imagen 3.17 Aplicación al modelo analítico del espectro sísmico en dirección Z (Sz)*

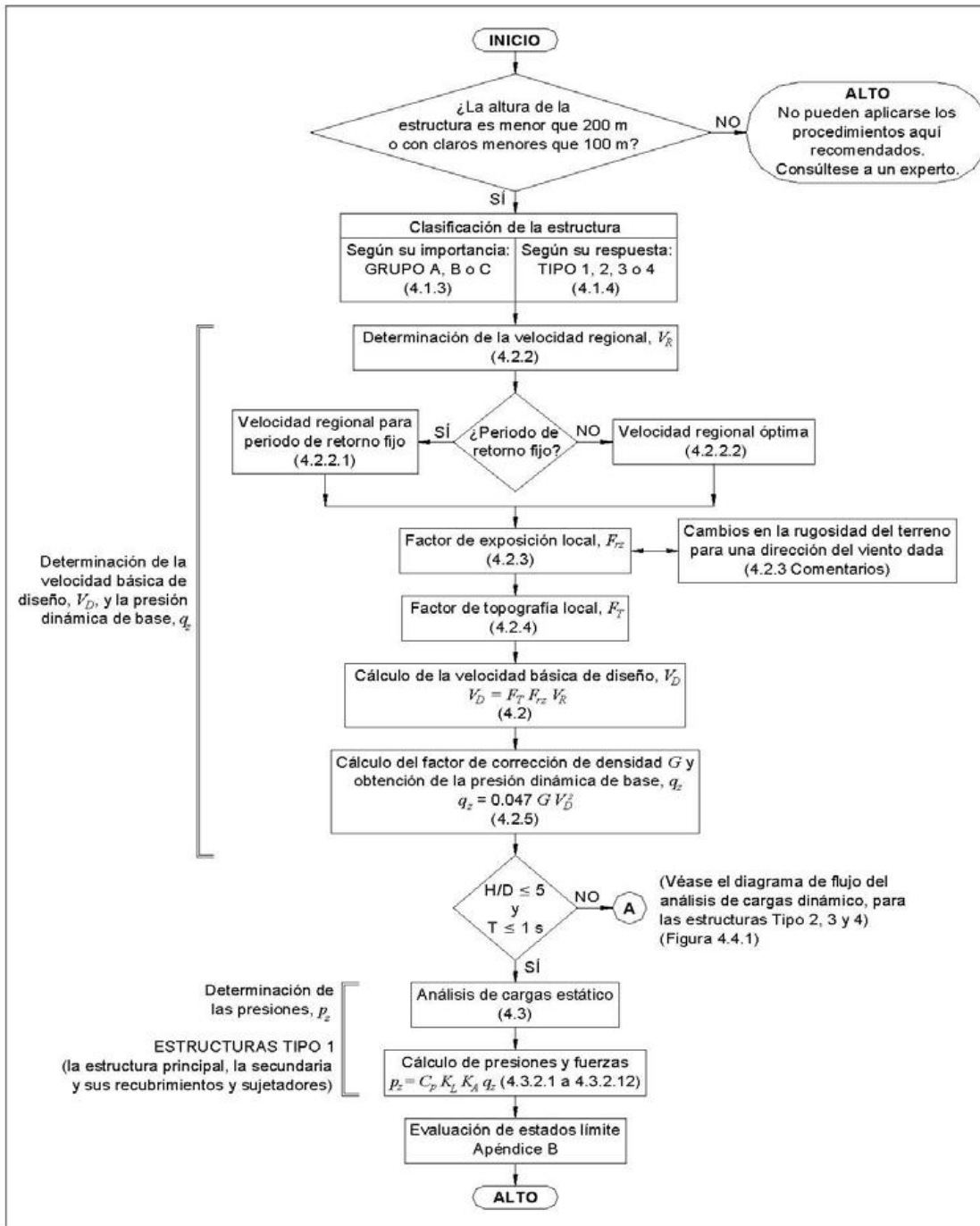


*Imagen 3.18 Aplicación al modelo analítico del espectro sísmico en dirección Y (Sy)*

De igual forma que para las fuerzas estáticas existen fuerzas sísmicas transmitidas por tuberías a la estructura del Rack los casos de carga se indican en el anexo correspondiente de la presente tesis.

### 3.4. Obtención de fuerzas por viento.

Para la obtención de fuerzas por viento se procede de acuerdo al diagrama de flujo mostrado en la Imagen 3.18 y contenido en el *MDOC CFE DV 2008*.



*Imagen 3.18 Diagrama de flujo para la obtención de fuerzas por viento.*  
 Los parámetros que se utilizan para el análisis por viento se definen a continuación:

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

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Altitud = **1200** m.s.n.m.

Estructura según su importancia Grupo: **A** Importancia (pág. 1.4.4 )

Estructura según su respuesta al Viento Tipo: **2** Respuesta (pág. 1.4.7 )

Estructura según su tamaño Clase: **A** Tamaño (Tabla I.2 ) (pág. 1.4.14)

Categoría del terreno según su rugosidad Categoría: Rugosidad (Tabla I.1 ) (pág. 1.4.13)

Temperatura media anual **T = 16.9 ° C** (Ver criterios y bases de diseño civil)

Velocidad Regional del Viento, para un periodo de retorno,  $T_r = 200$  años, para estructura del grupo A

VR = 155 km/h

Velocidad Diseño:	$VD = F_T F_a VR$		(Pág. 1.4.12)
Factor de Topografía:	$F_T =$	1.00	(Tabla I.5)
Factor de Exposición:	$F_a =$	1.00	$F_a = F_c F_r z$
	$F_a =$	1.07	
Factor de Tamaño:	$F_c =$	1.00	(Pág. 1.4.19)
	$F_{rz} = c$		si $z \leq 10$
Factor de Rug. y Altura:	$F_{rz} = c \left( \frac{z}{10} \right)^\alpha$		si $10 < z < \delta$
	$F_{rz} = c \left( \frac{\delta}{10} \right)^\alpha$		si $z \geq \delta$



**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

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$Z > 10.00$  m  
 $c = 1.0$   
 $\alpha = 0.128$   
 $\delta = 315.0$  Tabla 4,2,3 (Pág. 4,2,1,11)

Frz = 1.000 Para  $Z < 10$  m.  
Frz = 1.066 Para  $Z = 16.5$  m

$V_D = 155.00$  km/h Para  $Z < 10$  m.

$V_D = 165.26$  km/h Para  $Z = 16,50$  m.

( pag 4,2,1,15)

$G = 0.392 \Omega / ( 273 +T ) = 0.89$  (Factor de correccion por altura y temperatura)

$659.00$  mmHg Presión Barométrica

$q_z = 102.76$  kg/m<sup>2</sup> Para  $Z < 10$  m.

$q_z = 116.82$  kg/m<sup>2</sup> Para  $Z = 16,50$  m.

• **Tipo de análisis**

Se aplica el método de análisis estático por viento, donde la fuerza por viento se calcula como la suma de las fuerzas que actúan sobre cada uno de los miembros tomando en cuenta lo especificado en el capítulo 4.3.2.10 del *MDOC CFE DV 2008*. En este caso se considera como una estructura con columnas y vigas abiertas para formar un marco plano de celosía; cada una de estas partes podrá analizarse por separado, a continuación, lo establecido en el capítulo 4.3.2.10 del manual mencionado:

4.3.2.10 Fuerzas en miembros individuales. *La fuerza que el viento ejerce sobre elementos individuales expuestos directamente al flujo del viento, tales como perfiles estructurales, cuya relación de esbeltez ( $L_e/b$ ) sea mayor o igual que 8, se calcula con las ecuaciones en la dirección del flujo del viento:*

$$F_a = K_i K_{re} C_a b L_e q_z \quad (4.3.14)$$

En la dirección de los ejes del elemento:

$$F_x = K_i K_{re} C_{Fx} b_y q_z \quad (4.3.15.a)$$

$$F_y = K_i K_{re} C_{Fy} b_x q_z \quad (4.3.15.b)$$

En donde:

$L_e$  es la longitud del elemento, en m,

$b$  el ancho del elemento, normal al flujo del viento, en m,

$b_x$  el ancho del elemento, en la dirección x, en m,

$b_y$  el ancho del elemento, en la dirección y, en m,

$F_a$  la fuerza de arrastre sobre el elemento en la dirección del viento.

$F_x, F_y$  las fuerzas de arrastre, sobre el elemento en la dirección de los ejes x y y, respectivamente (véanse las Figuras A.1, A.2 y la Tabla A.3 del Apéndice A),

$K_i$  el factor que toma en cuenta el ángulo de inclinación del eje del miembro con respecto a la dirección del viento, adimensional:

$K_i = 1.0$  cuando el viento actúa perpendicularmente al miembro,

$K_i = \text{sen}^2\theta_m$  para miembros con formas cilíndricas,

$K_i = \text{sen}\theta_m$  para miembros prismáticos con aristas agudas, es decir, aquéllos con una relación  $b/r$  mayor que 16,

$\theta_m$  el ángulo entre la dirección del viento y el eje longitudinal del miembro, en grados,

$r$  el radio de las esquinas de la sección transversal de un elemento prismático, en m,

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

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- $K_{re}$  el factor de corrección por relación de esbeltez para miembros individuales (Tabla A.4 del Apéndice A), adimensional,
- $C_a$  el coeficiente de arrastre para un miembro en la dirección del flujo del viento, adimensional (véanse las Tablas A.1 y A.2 del Apéndice A),
- $C_{Fx}, C_{Fy}$  los coeficientes de arrastre para un miembro en la dirección de los ejes x y y, respectivamente, adimensionales (véanse las Figuras A.1, A.2 y la Tabla A.3 del Apéndice A), y  $q_z$  la presión dinámica de base del viento, de acuerdo con lo especificado en el inciso 4.2.5 y para una altura z igual a la altura en la que se encuentra el punto medio de la longitud del elemento.

Obtención de Fuerza de Arrastre  $F_a$

$$F_a = K_i K_{re} C_a b q_z$$

Dónde:

$K_i = 1$  viento perpendicular al miembro

$K_{re} = 0.8$  Con  $L_e/b > 14$

$C_{ax} = 1.60$  coef. Arrastre sobre almas vigas W

$C_{ax} = 1.9$  coef. Arrastre sobre patín vigas W

$A_z =$  área del elemento

$q_z = 116.82 \text{ kg/m}^2$

**VIENTO LONGITUDINAL (X)**

Fuerza de viento en sentido longitudinal del rack

elemento	A frontal (m <sup>2</sup> )	longitud (m)	A solida (m <sup>2</sup> )	Ca	Fa (kg/m)	Fa (KN/m)
W18X86	0.467	30.00	14.01	1.6	69.83	0.69
W12X50	0.313	32.00	10.02	1.6	46.80	0.46

$$\phi = \frac{A_{solidatotal}}{A_{encerrada}} = \quad 0.2 \quad 24.026$$

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

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Fuerza de viento en marcos subsecuentes del rack

separación / ancho= 0.8

elemento	A frontal (m <sup>2</sup> )	φ	Ke		Fa (kg/m)	Fa (KN/m)
W18X86	0.467	0.20	0.8		55.86	0.55
W12X50	0.313	0.20	0.8		37.44	0.37

**VIENTO TRANSVERSAL (Z)**

Fuerza de viento en sentido transversal del rack

elemento	A frontal (m <sup>2</sup> )	longitud (m)	A solida (m <sup>2</sup> )	Ca	Fa (kg/m)	Fa (KN/m)
W18X86	0.281	215.50	60.56	1.9	49.89	0.49
W12X50	0.313	312.20	97.72	1.6	55.58	0.55
W12X50	0.313	57.00	17.84	1.6	55.58	0.55

176.1151

$$\phi = \frac{A_{solidatotal}}{A_{encerrada}} = 0.13$$

Fuerza de viento en marcos subsecuentes del rack

separación / ancho= 0.9

elemento	A frontal (m <sup>2</sup> )	φ	Ke		Fa (kg/m)	Fa (KN/m)
W18x89	0.281	0.13	0.9		44.91	0.44
W12x50	0.313	0.13	0.9		50.02	0.49
W12x50	0.313	0.13	0.9		50.02	0.49

### 3.5. Combinación de fuerzas

De acuerdo al ACI-318-08 y AISC LRFD 3ª Ed. las combinaciones de carga que se usan para el diseño estructural son las especificadas por el ASCE 7-02 para elementos estructurales de Concreto y Acero.

También debe considerarse lo indicado en el *MDOC CFE DS 2008* donde se establece que:

*“Los efectos en la respuesta, tales como desplazamientos y elementos mecánicos, se combinarán empleando la raíz cuadrada de la suma de los cuadrados de los efectos en las tres direcciones ortogonales:*

$$S = \sqrt{S_x^2 + S_y^2 + S_z^2}$$

En seguida se definen los casos de carga primaria como sigue:

#### **CONDICIONES BASICAS ( Estas no actúan de manera independiente)**

1.-	_____	D	Carga muerta
2.-	_____	L	Carga viva
3.-	_____	CH	Carga de charolas electricas
4.-	_____	T	Carga Termica
5.-	_____	Q2	Carga por tuberias en Operación
6.-	_____	Q3	Carga hidraulica en tuberias
7.-	_____	Ex	Sismo dirección “X”
8.-	_____	Ez	Sismo dirección “Z”
9.-	_____	Ey	Sismo dirección “Y”
10.-	_____	Wx	Viento dirección “X” , Paralelo a las Generatrices
11.-	_____	Wz	Viento dirección “Z” , Normal a las Generatrices

**COMBINACIONES PARA CARGA DE SERVICIOS**

LOAD COMB 20 (D + CH + Q2) 1.0  
LOAD COMB 21 (D + CH + Q3) 1.0  
LOAD COMB 22 (D + CH + Q2 + L + T) 1.0  
LOAD COMB SRSS 23 (D + CH + Q2 + Lo +SISMO) 1.0  
LOAD COMB SRSS 24 (D + CH + Q2 + Lo -SISMO) 1.0  
LOAD COMB 25 (D + CH + Q2 + WX) 1.0  
LOAD COMB 26 (D + CH + Q2 - WX) 1.0  
LOAD COMB 27 (D + CH + Q2 + WZ) 1.0  
LOAD COMB 28 (D + CH + Q2 - WZ) 1.0

**COMBINACIONES PARA DISEÑO DE CONCRETO**

LOAD COMB 30 (D + CH + Q2) 1.4  
LOAD COMB 31 (D + CH + Q3) 1.4  
LOAD COMB 32 1.2 D + 1.2 CH + 1.2 Q2 + 1.6 L + 1.6 T  
LOAD COMB SRSS 33 1.2(D + CH + Q2) + Lo + 1.4 SISMO  
LOAD COMB SRSS 34 1.2(D + CH + Q2) + Lo - 1.4 SISMO  
LOAD COMB 35 1.2 (D + CH + Q2) + L + 1.3 WX  
LOAD COMB 36 1.2 (D + CH + Q2)+ L - 1.3 WX  
LOAD COMB 37 1.2 (D + CH + Q2) + L + 1.3 WZ  
LOAD COMB 38 1.2 (D + CH + Q2)+ L - 1.3 WZ

**COMBINACIONES PARA DISEÑO DE ACERO**

LOAD COMB 40 (D + CH + Q2) 1.4  
LOAD COMB 41 (D + CH + Q3) 1.4  
LOAD COMB 42 1.2 D + 1.2 CH + 1.2 Q2 + 1.6 L + 1.6 T  
LOAD COMB SRSS 43 1.2(D + CH + Q2) + Lo + 1.5 SISMO  
LOAD COMB SRSS 44 1.2(D + CH + Q2) + Lo - 1.5 SISMO  
LOAD COMB 45 1.2 (D + CH + Q2) + L + 1.3 WX  
LOAD COMB 46 1.2 (D + CH + Q2)+ L - 1.3 WX  
LOAD COMB 47 1.2 (D + CH + Q2) + L + 1.3 WZ  
LOAD COMB 48 1.2 (D + CH + Q2)+ L - 1.3 WZ

#### **4.0 Análisis estructural (resultados) mediante software comercial.**

Antes de proceder con la revisión de desplazamientos y el diseño de los miembros del Rack de Tuberías es indispensable hacer una revisión por cortante basal de la estructura, de acuerdo a lo indicado en el capítulo 3.3.6.3 *Revisión por cortante basal* del MDOC CFE DS 2008, a continuación, se incorpora un fragmento del capítulo mencionado.

##### **3.3.6.3 Revisión por cortante basal**

*Si con el método de análisis dinámico que se haya aplicado se encuentra que, en la dirección que se considera, la relación  $V/W$  es menor que  $0.8a(\beta)/Q'R_p$  ó  $0.8a(\beta)Acd/Q'R_p$  para estructuras con comportamiento histerético degradante ubicadas en suelos blandos, según sea el caso, se incrementarán todas las fuerzas de diseño y los desplazamientos laterales correspondientes en una proporción tal que  $V/W$  iguale a este valor.*

*Esta condición implica que la fuerza cortante basal de diseño no puede ser menor que 80% de la que suministraría un análisis estático tomando en cuenta el periodo fundamental de la estructura. Sin embargo, en ningún caso el cortante será menor que  $a_0W_{tot}$ , donde  $a_0$  es la aceleración máxima del terreno normalizada con la aceleración de la gravedad, (ecuación 1.13 de la sección 3.1) y  $W_{tot}$  es el peso total de la construcción.”*

Del modelo se obtuvo un peso propio total  $W_{tot}=759.738$  ton que se utiliza para la revisión por cortante basal, junto con los resultados obtenidos de periodo estructural y cortante dinámico de la corrida del modelo estructural.

Node	L/C	Force-X Mton	Force-Y Mton	Force-Z Mton	Moment-X MTon-m	Moment-Y MTon-m	Moment-Z m
1095	1	-0.060	16.831	-0.335	0.000	0.000	0.000
	2	0.007	2.148	0.019	0.000	0.000	0.000
	3	0.057	4.146	-0.017	0.000	0.000	0.000
	4	0.044	0.107	1.040	0.000	0.008	0.000
	5	0.349	19.860	0.164	0.000	0.000	0.000
1096	1	0.316	12.329	-0.345	0.000	-0.001	0.000
	2	0.007	0.655	0.008	0.000	0.000	0.000
	3	0.060	2.646	0.006	0.000	-0.001	0.000
	4	0.087	4.430	0.338	0.000	-0.001	0.000
	5	0.221	9.332	0.011	0.000	0.000	0.000
1097	1	-0.040	17.187	0.403	0.000	0.000	0.000
	2	0.019	1.912	0.019	0.000	0.000	0.000
	3	0.093	3.496	-0.017	0.000	0.000	0.000
	4	-0.175	-1.322	1.039	0.000	0.008	0.000
	5	0.298	17.986	0.099	0.000	0.000	0.000

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Node	L/C	Force-X Mton	Force-Y Mton	Force-Z Mton	Moment-X MTon-m	Moment-Y MTon-m	Moment-Z MTon-m
1098	1	0.338	12.120	0.373	0.000	-0.001	0.000
	2	0.021	0.135	0.008	0.000	0.000	0.000
	3	0.093	0.866	0.007	0.000	-0.001	0.000
	4	-0.180	-0.386	0.338	0.000	-0.001	0.000
	5	0.186	9.056	0.012	0.000	0.000	0.000
1099	1	0.076	16.233	-0.385	0.000	0.000	0.000
	2	-0.275	3.076	-0.030	0.000	0.000	0.000
	3	-0.003	3.973	0.032	0.000	0.000	0.000
	4	-0.170	2.478	-0.539	0.000	0.013	0.000
	5	-0.788	12.566	-0.580	0.000	0.000	0.000
1100	1	0.521	17.553	0.351	0.000	0.000	0.000
	2	0.300	3.714	-0.021	0.000	0.000	0.000
	3	-0.032	3.163	0.043	0.000	0.000	0.000
	4	0.023	1.258	-0.536	0.000	0.013	0.000
	5	-0.193	12.699	0.506	0.000	0.000	0.000
1101	1	-0.699	15.669	-0.173	0.000	0.000	0.000
	2	0.884	3.733	-0.007	0.000	0.000	0.000
	3	-0.191	3.433	0.143	0.000	0.000	0.000
	4	1.427	-2.262	1.321	0.000	0.013	0.000
	5	-2.151	11.497	0.043	0.000	0.001	0.000
1102	1	-0.075	17.331	0.550	0.000	0.000	0.000
	2	0.794	4.664	0.011	0.000	0.000	0.000
	3	-0.056	2.420	0.154	0.000	0.000	0.000
	4	-0.595	5.301	1.319	0.000	0.013	0.000
	5	0.118	12.742	1.200	0.000	0.001	0.000
1103	1	0.115	16.985	-0.524	0.000	-0.008	0.000
	2	0.475	11.390	0.016	0.000	0.308	0.000
	3	-0.011	3.247	-0.091	0.000	0.055	0.000
	4	0.219	1.114	1.164	0.000	-0.021	0.000
	5	-0.363	9.462	-0.899	0.000	0.141	0.000
1104	1	0.033	18.831	0.593	0.000	0.000	0.000
	2	0.077	9.179	0.380	0.000	0.001	0.000
	3	0.045	3.028	0.149	0.000	0.000	0.000
	4	-0.031	6.316	0.773	0.000	0.015	0.000
	5	0.028	18.446	0.590	0.000	0.000	0.000
1105	1	-0.131	14.940	-0.378	0.000	-0.003	0.000
	2	-0.419	9.454	-0.042	0.000	-0.061	0.000
	3	0.064	3.436	-0.127	0.000	-0.012	0.000
	4	0.547	1.463	0.328	0.000	0.017	0.000
	5	0.105	7.976	-0.668	0.000	-0.029	0.000



**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

Node	L/C	Force-X Mton	Force-Y Mton	Force-Z Mton	Moment-X MTon-m	Moment-Y MTon-m	Moment-Z MTon-m
1106	1	-0.043	16.911	0.614	0.000	0.000	0.000
	2	-0.150	9.045	0.209	0.000	-0.001	0.000
	3	-0.016	4.155	0.098	0.000	0.000	0.000
	4	-0.349	2.009	0.643	0.000	0.015	0.000
	5	-0.065	15.287	0.801	0.000	0.000	0.000
1107	1	0.009	15.703	-0.425	0.000	-0.002	0.000
	2	-0.226	3.880	-0.005	0.000	-0.003	0.000
	3	0.051	4.603	-0.032	0.000	0.000	0.000
	4	0.393	11.287	-4.122	0.000	0.020	0.000
	5	0.097	12.722	-0.028	0.000	-0.003	0.000
1108	1	-0.060	15.637	0.319	0.000	0.002	0.000
	2	-0.201	3.647	0.007	0.000	0.001	0.000
	3	-0.051	2.420	-0.012	0.000	0.000	0.000
	4	-0.500	-10.867	-4.099	0.000	0.023	0.000
	5	-0.173	13.182	0.103	0.000	0.008	0.000
1109	1	0.120	19.325	-0.314	0.000	0.000	0.000
	2	0.005	1.648	-0.002	0.000	0.000	0.000
	3	-0.019	5.818	0.015	0.000	-0.001	0.000
	4	-0.099	12.105	-5.944	0.000	0.013	0.000
	5	0.014	16.641	-0.512	0.000	0.001	0.000
1110	1	0.113	18.271	0.419	0.000	0.000	0.000
	2	-0.009	1.398	0.000	0.000	0.000	0.000
	3	0.006	3.755	0.028	0.000	-0.001	0.000
	4	0.066	-13.227	-5.946	0.000	0.013	0.000
	5	-0.008	17.640	0.355	0.000	0.001	0.000
1111	1	-0.113	19.217	-0.765	0.000	0.000	0.000
	2	-0.010	2.102	-0.035	0.000	0.000	0.000
	3	-0.010	5.064	-0.172	0.000	-0.001	0.000
	4	-0.059	0.428	3.515	0.000	0.014	0.000
	5	-0.011	15.047	-1.165	0.000	0.001	0.000
1112	1	-0.113	17.595	0.217	0.000	0.000	0.000
	2	-0.020	1.344	0.016	0.000	0.000	0.000
	3	0.013	3.839	0.010	0.000	0.000	0.000
	4	0.035	-4.387	4.430	0.000	0.013	0.000
	5	-0.022	16.537	0.604	0.000	0.000	0.000
1113	1	-0.036	17.082	-0.754	0.000	0.000	0.000
	2	-0.042	2.186	-0.052	0.000	0.000	0.000
	3	-0.034	3.874	-0.177	0.000	0.000	0.000
	4	0.231	-3.150	0.721	0.000	0.015	0.000
	5	-0.071	13.582	-0.801	0.000	0.002	0.000

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

Node	L/C	Force-X Mton	Force-Y Mton	Force-Z Mton	Moment-X MTon-m	Moment-Y MTon-m	Moment-Z MTon-m
1114	1	-0.011	15.464	0.037	0.000	0.000	0.000
	2	-0.036	1.359	-0.011	0.000	0.000	0.000
	3	0.010	3.257	-0.005	0.000	0.000	0.000
	4	-0.103	-0.114	1.366	0.000	0.013	0.000
	5	-0.057	10.079	0.369	0.000	0.001	0.000
1115	1	-0.111	14.156	-0.355	0.000	0.000	0.000
	2	-0.286	0.977	-0.001	0.000	0.000	0.000
	3	-0.141	2.035	-0.004	0.000	0.000	0.000
	4	0.618	3.720	-0.336	0.000	0.014	0.000
	5	0.738	9.377	-0.416	0.000	0.002	0.000
1116	1	0.087	14.098	0.355	0.000	0.000	0.000
	2	-0.258	0.692	0.001	0.000	0.000	0.000
	3	0.276	3.160	0.004	0.000	0.000	0.000
	4	-1.394	-1.682	-0.329	0.000	0.014	0.000
	5	1.529	9.662	0.416	0.000	0.002	0.000
1117	1	-0.085	14.019	-0.354	0.000	0.000	0.000
	2	-0.328	1.519	0.004	0.000	0.000	0.000
	3	-0.172	1.712	0.000	0.000	0.000	0.000
	4	-0.157	-2.350	0.008	0.000	0.013	0.000
	5	-0.022	9.534	-0.346	0.000	0.001	0.000
1118	1	-0.027	14.055	0.364	0.000	0.000	0.000
	2	-0.330	1.344	0.008	0.000	0.000	0.000
	3	-0.025	2.746	0.006	0.000	0.000	0.000
	4	-1.077	2.022	0.005	0.000	0.014	0.000
	5	0.618	11.380	0.503	0.000	0.002	0.000
1119	1	-0.128	11.039	-0.293	0.000	0.000	0.000
	2	-0.049	0.285	-0.006	0.000	0.000	0.000
	3	-0.028	0.197	-0.003	0.000	0.000	0.000
	4	0.200	0.232	-0.362	0.000	0.013	0.000
	5	-0.160	2.074	-0.065	0.000	0.000	0.000
1120	1	-0.076	11.485	0.284	0.000	0.000	0.000
	2	-0.033	0.278	-0.006	0.000	0.000	0.000
	3	-0.009	1.562	-0.003	0.000	0.000	0.000
	4	-0.074	2.641	-0.362	0.000	0.014	0.000
	5	-0.089	7.583	-0.092	0.000	0.001	0.000
1121	1	-0.020	10.259	0.743	0.000	-0.002	0.000
	2	0.000	0.225	0.050	0.000	-0.001	0.000
	3	0.001	0.823	0.175	0.000	0.000	0.000
	4	0.000	7.556	1.215	0.000	0.007	0.000
	5	-0.003	7.386	0.725	0.000	-0.001	0.000

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

Node	L/C	Force-X Mton	Force-Y Mton	Force-Z Mton	Moment-X MTon-m	Moment-Y MTon-m	Moment-Z MTon-m
1181	1	0.257	14.173	-0.488	0.000	-0.002	0.000
	2	0.248	10.169	-0.420	0.000	0.296	0.000
	3	0.019	2.324	-0.245	0.000	0.055	0.000
	4	0.685	-1.955	0.552	0.000	-0.036	0.000
	5	-0.127	5.331	-0.859	0.000	0.145	0.000
1182	1	-0.169	12.057	-0.256	0.000	-0.001	0.000
	2	-0.160	8.334	-0.149	0.000	-0.060	0.000
	3	0.011	2.324	-0.133	0.000	-0.012	0.000
	4	0.363	-0.639	-0.105	0.000	0.010	0.000
	5	-0.045	3.894	-0.420	0.000	-0.030	0.000
1298	1	0.011	9.392	0.523	0.000	-0.002	0.000
	2	-0.004	0.204	0.032	0.000	-0.001	0.000
	3	0.001	0.701	0.169	0.000	0.000	0.000
	4	-0.034	2.912	0.867	0.000	0.008	0.000
	5	0.001	2.746	0.257	0.000	-0.003	0.000

Sumando cada una de las combinaciones:

COMBINACIÓN	FACTOR	CARGA
1.- CARGA MUERTA	1.0	455.944 ton
2.- CARGA VIVA	0.5	50.347 ton
3.- CARGA DE CH ELECTR.	1.0	88.224 ton
4.- CARGA TERMICA	1.0	25.037 ton
5.- CARGA POR TUBERIAS	1.0	341.307 ton

TOTAL = 759.738 ton.

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

### • Revisión por cortante basal

De la tabla de participación de masa mostrada en la Imagen 4.1 se determina el modo fundamental de la estructura y el cortante basal dinámico en dirección X.

MODE	MASS PARTICIPATION FACTORS IN PERCENT						BASE SHEAR IN KN		
	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
1	0.11	0.01	56.79	0.106	0.011	56.795	0.90	0.00	0.00
2	0.02	0.00	8.41	0.129	0.014	65.210	0.20	0.00	0.00
3	1.57	0.03	0.27	1.698	0.046	65.481	14.82	0.00	0.00
4	78.39	0.00	0.49	80.083	0.050	65.966	759.07	0.00	0.00
5	7.79	0.00	11.31	87.873	0.052	77.274	76.62	0.00	0.00
6	0.00	0.03	4.41	87.874	0.080	81.682	0.00	0.00	0.00
7	0.35	0.00	0.22	88.224	0.080	81.906	3.80	0.00	0.00
8	0.56	0.00	2.05	88.786	0.080	83.952	6.50	0.00	0.00
9	0.01	0.00	0.04	88.792	0.085	83.988	0.07	0.00	0.00
10	0.00	0.00	0.66	88.795	0.088	84.644	0.04	0.00	0.00
190	0.01	0.00	0.00	93.497	99.106	95.233	0.23	0.00	0.00
191	0.00	0.00	0.00	93.500	99.111	95.233	0.08	0.00	0.00
192	0.03	0.02	0.00	93.530	99.134	95.234	0.83	0.00	0.00
193	0.01	0.00	0.00	93.537	99.134	95.234	0.21	0.00	0.00
194	0.00	0.00	0.00	93.538	99.134	95.234	0.01	0.00	0.00
195	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00
196	0.00	0.00	0.00	93.538	99.134	95.236	0.01	0.00	0.00
197	0.02	0.01	0.00	93.557	99.148	95.241	0.53	0.00	0.00
198	0.00	0.02	0.02	93.560	99.165	95.260	0.08	0.00	0.00
199	0.00	0.00	0.00	93.561	99.169	95.262	0.02	0.00	0.00
200	0.02	0.01	0.00	93.577	99.182	95.263	0.43	0.00	0.00
-----									
TOTAL SRSS SHEAR							763.35	0.00	0.00
TOTAL 10PCT SHEAR							851.30	0.00	0.00
TOTAL ABS SHEAR							958.31	0.00	0.00

*Imagen 4.1 Tabla de participación de masa y cortante dinámico en dirección X resultado de la corrida del modelo estructural*

De lo anterior se establece que en el cuarto modo se tiene un porcentaje de participación de masa del 78.39% por lo que este es el modo fundamental de la estructura en la dirección X, mientras que el cortante basal dinámico es de 763.35 KN como se indica en rojo en la Imagen 4.1.

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

De igual forma procedemos a obtener estos valores para la dirección Z

Obtención de modo fundamental y cortante basal dinámico en dirección Z.

MODE	MASS PARTICIPATION FACTORS IN PERCENT						BASE SHEAR IN KN		
	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
1	0.11	0.01	56.79	0.106	0.011	56.795	0.00	0.00	718.97
2	0.02	0.00	8.41	0.129	0.014	65.210	0.00	0.00	109.87
3	1.57	0.03	0.27	1.698	0.046	65.481	0.00	0.00	3.78
4	78.39	0.00	0.49	80.083	0.050	65.966	0.00	0.00	6.96
5	7.79	0.00	11.31	87.873	0.052	77.274	0.00	0.00	164.90
6	0.00	0.03	4.41	87.874	0.080	81.682	0.00	0.00	66.23
7	0.35	0.00	0.22	88.224	0.080	81.906	0.00	0.00	3.63
8	0.56	0.00	2.05	88.786	0.080	83.952	0.00	0.00	35.20
9	0.01	0.00	0.04	88.792	0.085	83.988	0.00	0.00	0.65
10	0.00	0.00	0.66	88.795	0.088	84.644	0.00	0.00	12.01
190	0.01	0.00	0.00	93.497	99.106	95.233	0.00	0.00	0.01
191	0.00	0.00	0.00	93.500	99.111	95.233	0.00	0.00	0.00
192	0.03	0.02	0.00	93.530	99.134	95.234	0.00	0.00	0.03
193	0.01	0.00	0.00	93.537	99.134	95.234	0.00	0.00	0.00
194	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00
195	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00
196	0.00	0.00	0.00	93.538	99.134	95.236	0.00	0.00	0.10
197	0.02	0.01	0.00	93.557	99.148	95.241	0.00	0.00	0.19
198	0.00	0.02	0.02	93.560	99.165	95.260	0.00	0.00	0.80
199	0.00	0.00	0.00	93.561	99.169	95.262	0.00	0.00	0.08
200	0.02	0.01	0.00	93.577	99.182	95.263	0.00	0.00	0.02
							-----		
TOTAL SRSS SHEAR							0.00	0.00	752.86
TOTAL 10PCT SHEAR							0.00	0.00	872.49
TOTAL ABS SHEAR							0.00	0.00	1438.02

*Imagen 4.2 Tabla de participación de masa y cortante dinámico en dirección Z resultado de la corrida del modelo estructural*

Para la dirección Z se tiene que en el modo 1 la participación de masa es la mayor con 56.79% y un cortante basal dinámico de 752.86 KN como se indica en la Imagen 4.2.

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Obtención de modo fundamental y cortante basal dinámico en dirección Y.

MODE	MASS PARTICIPATION FACTORS IN PERCENT						BASE SHEAR IN KN		
	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
1	0.11	0.01	56.79	0.106	0.011	56.795	0.00	0.04	0.00
2	0.02	0.00	8.41	0.129	0.014	65.210	0.00	0.01	0.00
3	1.57	0.03	0.27	1.698	0.046	65.481	0.00	0.12	0.00
4	78.39	0.00	0.49	80.083	0.050	65.966	0.00	0.02	0.00
5	7.79	0.00	11.31	87.873	0.052	77.274	0.00	0.01	0.00
6	0.00	0.03	4.41	87.874	0.080	81.682	0.00	0.11	0.00
7	0.35	0.00	0.22	88.224	0.080	81.906	0.00	0.00	0.00
8	0.56	0.00	2.05	88.786	0.080	83.952	0.00	0.00	0.00
9	0.01	0.00	0.04	88.792	0.085	83.988	0.00	0.02	0.00
10	0.00	0.00	0.66	88.795	0.088	84.644	0.00	0.01	0.00
11	0.03	0.01	0.24	88.822	0.093	84.882	0.00	0.03	0.00
12	0.00	0.00	0.00	88.822	0.093	84.882	0.00	0.00	0.00
13	0.00	0.00	0.00	88.822	0.093	84.882	0.00	0.00	0.00
14	0.57	0.00	0.00	89.397	0.093	84.882	0.00	0.00	0.00
15	0.00	0.03	1.31	89.397	0.118	86.190	0.00	0.14	0.00
16	0.01	0.09	2.07	89.405	0.207	88.258	0.00	0.54	0.00
17	0.01	0.00	0.02	89.412	0.208	88.279	0.00	0.00	0.00
18	0.58	0.00	0.04	89.990	0.211	88.320	0.00	0.02	0.00
19	0.69	0.00	0.03	90.678	0.212	88.354	0.00	0.01	0.00
20	0.03	0.01	0.00	90.711	0.221	88.355	0.00	0.06	0.00
21	0.01	0.00	0.02	90.719	0.221	88.373	0.00	0.00	0.00
22	0.24	0.00	0.01	90.959	0.223	88.381	0.00	0.01	0.00
23	0.27	0.00	0.00	91.225	0.223	88.381	0.00	0.00	0.00
24	0.00	0.08	0.47	91.228	0.304	88.850	0.00	0.57	0.00
25	0.01	0.00	0.00	91.239	0.305	88.853	0.00	0.01	0.00
26	0.06	0.07	0.00	91.294	0.372	88.853	0.00	0.50	0.00
27	0.04	0.03	0.01	91.332	0.398	88.858	0.00	0.19	0.00
28	0.07	0.00	0.00	91.404	0.398	88.860	0.00	0.00	0.00
29	0.00	0.19	0.39	91.408	0.586	89.246	0.00	1.49	0.00
30	0.00	0.14	0.00	91.408	0.731	89.249	0.00	1.16	0.00
31	0.01	0.05	0.00	91.415	0.777	89.250	0.00	0.37	0.00
32	0.00	0.18	0.00	91.417	0.957	89.251	0.00	1.46	0.00
33	0.00	0.05	0.00	91.418	1.005	89.251	0.00	0.39	0.00
34	0.00	0.06	0.03	91.418	1.064	89.281	0.00	0.48	0.00
35	0.02	16.03	0.02	91.441	17.094	89.299	0.00	131.77	0.00
36	0.00	0.02	0.00	91.443	17.116	89.299	0.00	0.17	0.00
37	0.00	0.03	0.00	91.443	17.148	89.299	0.00	0.27	0.00
38	0.00	6.63	0.05	91.446	23.776	89.352	0.00	55.04	0.00
39	0.00	0.00	0.16	91.446	23.776	89.509	0.00	0.00	0.00
40	0.00	0.00	0.00	91.446	23.776	89.509	0.00	0.00	0.00
41	0.00	0.24	0.06	91.447	24.019	89.573	0.00	2.03	0.00
42	0.00	0.67	0.27	91.450	24.686	89.844	0.00	5.60	0.00
43	0.00	0.04	0.02	91.453	24.725	89.863	0.00	0.33	0.00
44	0.00	0.00	0.00	91.456	24.726	89.864	0.00	0.01	0.00
45	0.00	0.01	0.02	91.456	24.739	89.884	0.00	0.11	0.00
46	0.00	0.00	0.00	91.456	24.739	89.884	0.00	0.01	0.00
47	0.00	0.00	0.00	91.456	24.739	89.884	0.00	0.00	0.00
48	0.03	0.56	0.00	91.483	25.295	89.887	0.00	4.75	0.00
49	0.00	0.21	0.01	91.483	25.505	89.900	0.00	1.81	0.00
50	0.00	0.01	0.24	91.484	25.519	90.136	0.00	0.12	0.00
51	0.04	19.57	0.00	91.519	45.088	90.137	0.00	170.50	0.00
190	0.01	0.00	0.00	93.497	99.106	95.233	0.00	0.00	0.00

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z	
191	0.00	0.00	0.00	93.500	99.111	95.233	0.00	0.05	0.00	
192	0.03	0.02	0.00	93.530	99.134	95.234	0.00	0.26	0.00	
193	0.01	0.00	0.00	93.537	99.134	95.234	0.00	0.00	0.00	
194	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00	
195	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00	
196	0.00	0.00	0.00	93.538	99.134	95.236	0.00	0.00	0.00	
197	0.02	0.01	0.00	93.557	99.148	95.241	0.00	0.15	0.00	
198	0.00	0.02	0.02	93.560	99.165	95.260	0.00	0.19	0.00	
199	0.00	0.00	0.00	93.561	99.169	95.262	0.00	0.04	0.00	
200	0.02	0.01	0.00	93.577	99.182	95.263	0.00	0.15	0.00	
-----										
							TOTAL SRSS SHEAR	0.00	281.34	0.00
							TOTAL 10PCT SHEAR	0.00	461.20	0.00
							TOTAL ABS SHEAR	0.00	892.82	0.00

*Imagen 4.3 Tabla de participación de masa y cortante dinámico en dirección Y resultado de la corrida del modelo estructural*

Para la dirección Y se tiene que en el modo 51 la participación de masa es la mayor con 19.57% y un cortante basal dinámico de 281.34 KN como se indica en la Imagen 4.3.

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

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Conocidos los modos fundamentales de la estructura podemos determinar el periodo fundamental de la estructura en las dos direcciones X y Z, en la tabla de la Imagen 4.4 generada en la corrida del modelo estructural tenemos que:

CALCULATED FREQUENCIES FOR LOAD CASE			7
)	MODE	FREQUENCY (CYCLES/SEC)	PERIOD (SEC)
	1	0.828	1.20774
	2	0.873	1.14592
	3	0.993	1.00755
	4	1.031	0.97037
	5	1.055	0.94785
	6	1.100	0.90884
	7	1.253	0.79838
	8	1.350	0.74088
	9	1.459	0.68561
	10	1.476	0.67768
	11	1.554	0.64362
	12	1.645	0.60778
	13	1.646	0.60769
	14	1.660	0.60245
	15	1.695	0.59009
	16	1.979	0.50540
	17	2.064	0.48445
	18	2.103	0.47545
	19	2.126	0.47029
	20	2.286	0.43754
	21	2.375	0.42097
	22	2.392	0.41799
	23	2.487	0.40206
	24	2.545	0.39300
	25	2.633	0.37974
	26	2.675	0.37383
	27	2.746	0.36421
	28	2.869	0.34855
	29	2.973	0.33635
	30	3.049	0.32800
	31	3.070	0.32574
	32	3.121	0.32038
	33	3.148	0.31767
	34	3.149	0.31756
	35	3.175	0.31497
	36	3.190	0.31344
	37	3.210	0.31148
	38	3.237	0.30894
	39	3.252	0.30754
	40	3.272	0.30566
	41	3.297	0.30333
	42	3.304	0.30270
	43	3.327	0.30059
	44	3.364	0.29725
	45	3.385	0.29541
	46	3.394	0.29462

---

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## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

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MODE	FREQUENCY (CYCLES/SEC)	PERIOD (SEC)
47	3.422	0.29221
48	3.440	0.29073
49	3.458	0.28917
50	3.548	0.28188
51	3.574	0.27981

*Imagen 4.4 Frecuencias, periodos y participación de masa para los modos fundamentales*

De acuerdo al MDOC CFE DS 2008 en su capítulo 3.3.6.3

*Revisión por cortante basal se debe cumplir que:*

$$0.8V_E \leq VD \qquad a_0W_{tot} \leq VD$$

Si no se cumple a la primera condición se deben incrementar las fuerzas sísmicas de forma que el cortante dinámico sea mayor, esto se logra multiplicando la aceleración de la gravedad por un factor que a continuación se obtiene.

$$\text{Factor} = \frac{80 \% V_d}{V_e}$$

Este factor se multiplica por la gravedad:

Factor x 9.81 Este valor se usa en el modelo para encontrar el constante dinámico que cumpla.

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Donde:

Coeficiente sísmico por dirección de acuerdo a su modo principal de vibración, ver sus espectros respectivos por dirección:

RACK	MODOS		PERIODOS $T_e$ (seg)		Coeficiente Sísmico		PESO (TON)
	X	Z	EN X	EN Z	EN X	EN Z	
PRINCIPAL NORTE-SUR	4	1	0.970	1.208	0.072	0.077	759.738

Datos obtenidos de cada una de las corridas

### REVISION DEL CORTANTE ESTATICO CONTRA EL DINAMICO

$V_e$  = Cortante estático = (Peso de la estructura + la carga viva + peso de tuberías en operación) por el coeficiente sísmico

$V_d$  = Cortante dinámico obtenido de la corrida del staad.

RACK	CORTANTE ESTATICO		80 %CORTANTE ESTATICO		CORTANTE DINAMICO	
	$V_{ex}$	$V_{ez}$	$0.8V_{ex}$	$0.8V_{ez}$	$V_{dx}$	$V_{dz}$
PRINCIPAL NORTE-SUR	54.701	58.500	43.761	46.800	<b>77.814</b>	<b>76.744</b>

Se comprueba que  $0.8 V_e < V_d$

$a_0 \times W = 0.1 \times 759.738 = 75.974 \text{ ton} < V_d$  en ambas direcciones

## **5.0 Diseño estructural.**

### **5.1. Revisión por desplazamientos.**

- Revisión de desplazamientos horizontales

De acuerdo a los resultados del modelo estructural las combinaciones de sismo son las que rigen la revisión por desplazamientos horizontales, la revisión se lleva a cabo conforme a lo indicado en el capítulo 3.3.7.1 *Desplazamientos horizontales por limitación de daños a elementos no estructurales (límite de servicio)* del MDOC CFE DS 2008 que indica:

*“Las diferencias entre los desplazamientos laterales de pisos consecutivos debidos a las fuerzas cortantes horizontales, calculadas con alguno de los métodos de análisis sísmico para las ordenadas espectrales reducidas ( $a(\beta)/Q'R\rho$ ) que se describen en esta sección y multiplicadas por el factor  $Q'R\rho/F_{ser}$ , no excederán a 0.002 veces las diferencias de elevaciones correspondientes.”*

Lo anterior se resume con la siguiente desigualdad.

$$\delta \left( \frac{Q'R\rho}{F_{ser}} \right) \leq 0.002H$$

Dónde:

$\delta$  Desplazamiento lateral de pisos consecutivos debidos a las fuerzas cortantes horizontales en dirección X ó Z según corresponda.

$Q'$ ,  $R$  y  $\rho$  Se calcularán para el periodo fundamental de la estructura.

$F_{ser}$  También se calculará para el periodo fundamental de la estructura siguiendo las especificaciones que se proporcionan en las secciones de recomendaciones 3.1.6.7 y de comentarios 3.1.7.2. del MDOC CFE DS 2008.

$H$  altura del entrepiso, en general 0.002H se considera el desplazamiento horizontal permisible.

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

El desplazamiento horizontal máximo obtenido del modelo analítico para el Rack de Tuberías en dirección X ocurre en el nodo 1329 con la combinación de carga 24  $\delta_x=2.731$  cm indicado en la Imagen 5.1.

Node	L/C	X-Trans cm	Y-Trans cm	Z-Trans cm	Absolute cm	X-Rotan rad	Y-Rotan rad	Z-Rotan rad
1329	24	-2.733	-2.242	-5.859	6.842	-0.003	-0.001	-0.002
1320	23	2.591	-1.785	9.142	9.668	0.000	0.002	0.001
1319	23	2.590	-0.984	5.367	6.040	0.000	0.003	-0.001
860	23	2.589	-1.149	4.105	4.988	0.001	0.001	0.003
1307	23	2.589	-1.061	6.548	7.121	0.002	0.001	0.003
1331	23	2.589	-1.437	6.767	7.387	0.001	0.003	-0.001
1323	23	2.589	-1.390	5.511	6.245	-0.000	0.001	0.003
1333	23	2.588	-1.578	6.336	7.024	0.001	0.001	0.000

*Imagen 5.1 Obtención del desplazamiento horizontal máximo en dirección X*

Desplazamiento crítico en dirección X rige carga de sismo, donde  $Q' = 1.4$ ,  $R=2$  y  $\rho=1.0$

Desplazamiento real =  $\Delta \times (Q'R \rho) / 5.5 = 1.39$  cm

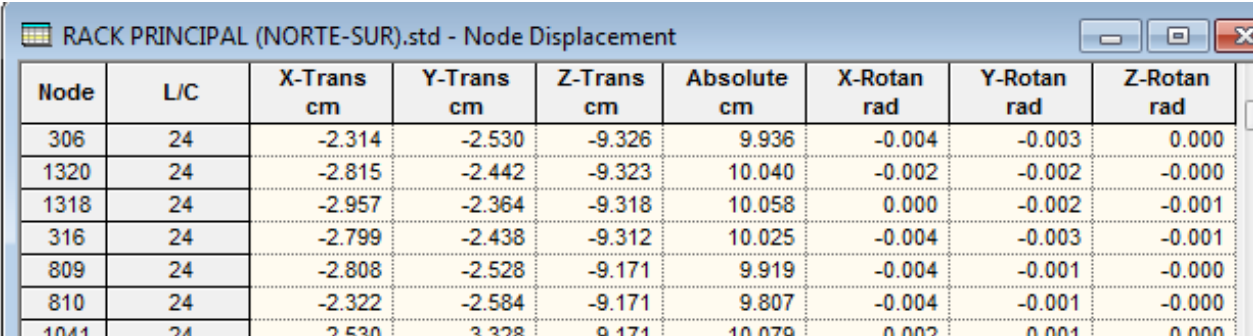
Desplazamiento máx =  $\Delta$  máx. = 1.39 cm

Desplazamiento permisible =  $0.012 H = 0.012 \times 1500 = 18$  cm

$\Delta$  perm. = 18.00 > 1.39 cm No hay problema por desplazamiento en X

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

El desplazamiento horizontal máximo en dirección Z ocurre en el nodo 306 con la combinación de carga 24  $\delta z=9.326$  cm tal como se indica en la Imagen 5.2



Node	L/C	X-Trans cm	Y-Trans cm	Z-Trans cm	Absolute cm	X-Rotan rad	Y-Rotan rad	Z-Rotan rad
306	24	-2.314	-2.530	-9.326	9.936	-0.004	-0.003	0.000
1320	24	-2.815	-2.442	-9.323	10.040	-0.002	-0.002	-0.000
1318	24	-2.957	-2.364	-9.318	10.058	0.000	-0.002	-0.001
316	24	-2.799	-2.438	-9.312	10.025	-0.004	-0.003	-0.001
809	24	-2.808	-2.528	-9.171	9.919	-0.004	-0.001	-0.000
810	24	-2.322	-2.584	-9.171	9.807	-0.004	-0.001	-0.000
1041	24	-2.530	-2.328	-9.171	10.079	-0.002	-0.001	-0.000

*Imagen 5.2 Obtención del desplazamiento horizontal máximo en dirección Z*

Desplazamiento crítico en dirección X rige carga de sismo, donde  $Q' = 1.4$ ,  $R=2$  y  $\rho=1.0$

Desplazamiento real =  $\Delta \times (Q'R \rho) / 5.5 = 4.75$  cm

Desplazamiento máx =  $\Delta$  máx. = 4.75 cm

Desplazamiento permisible =  $0.012 H = 0.012 \times 1500 = 18$  cm

$\Delta$  perm. = 18.00 > 4.75 cm No hay problema por desplazamiento en Z

Por lo que la estructura cumple con los desplazamientos horizontales permisibles.

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## 5.2. Diseño de la Estructura.

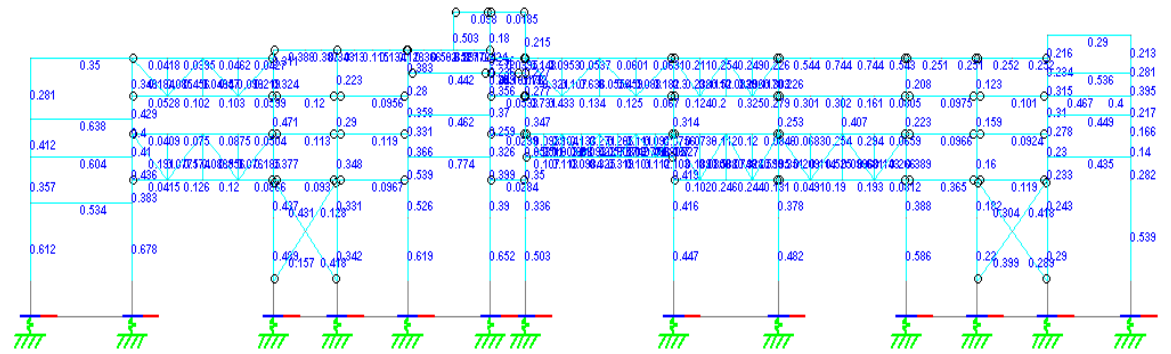
La revisión del diseño de los diferentes miembros de acero que conforman la estructura del Rack de Tuberías se hace mediante la revisión de la relación de esfuerzos que realiza el programa STAAD Pro en el modelo 3D.

None  
 Show Diagram (Based on Actual Ratio)  
 Show Diagram (Based on Normalized Ratio)  
 Basic Diagram  
 Detailed Diagram

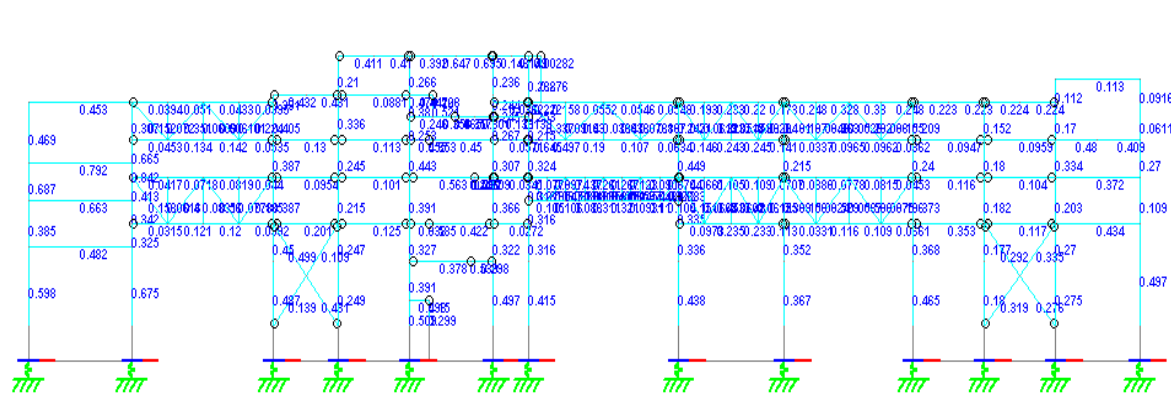
Actual Ratio		
	From	To
1	Not Designed	
2	0	1
3	1	1.5
	> 1.5	

A continuación, se muestran los elementos con su relación de esfuerzos.

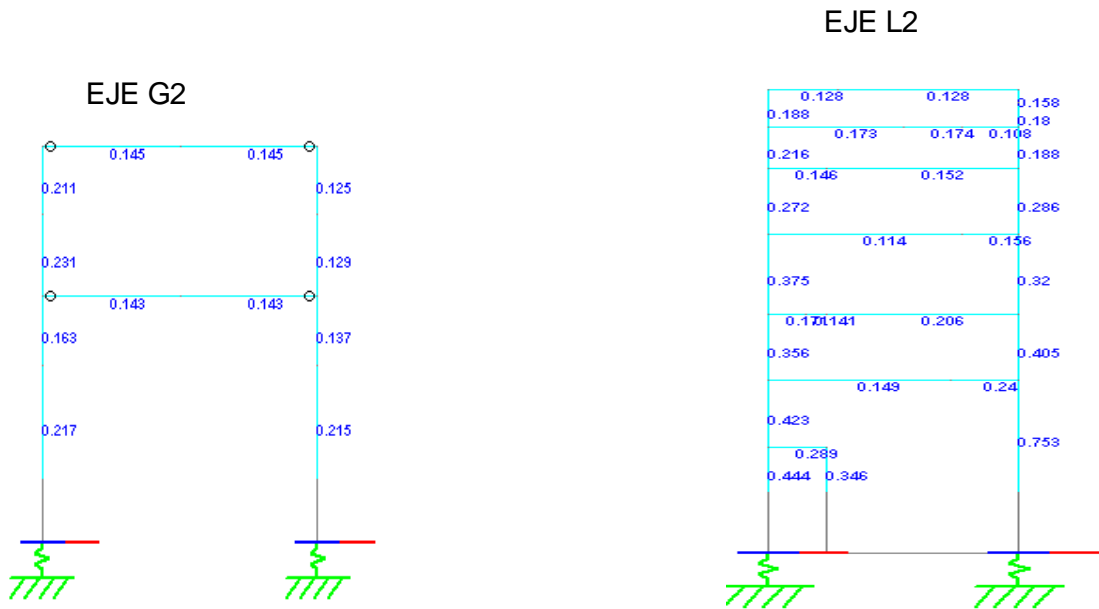
Eje J1



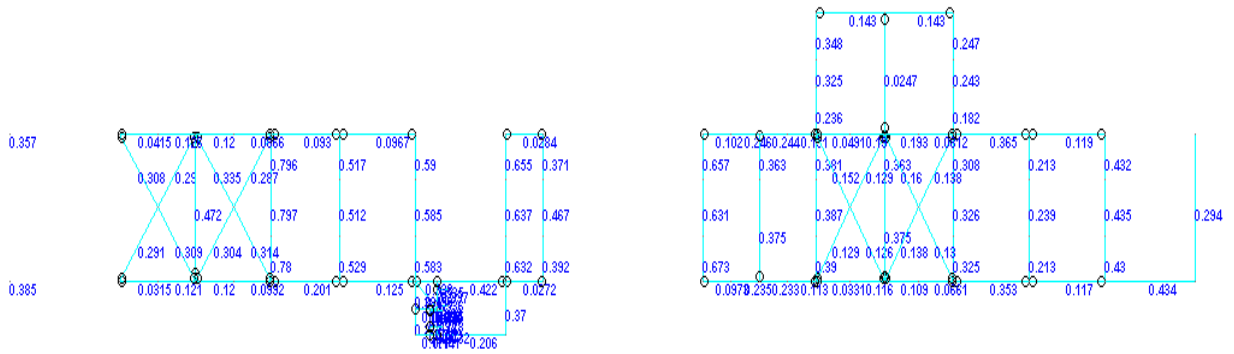
Eje L1



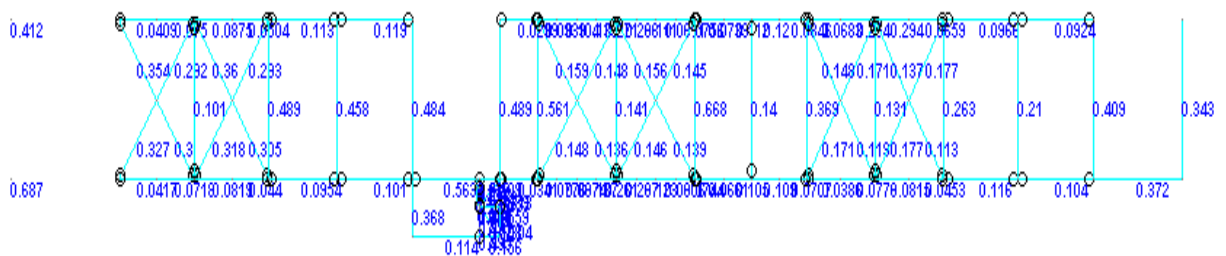
# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES



NIV +1208.20

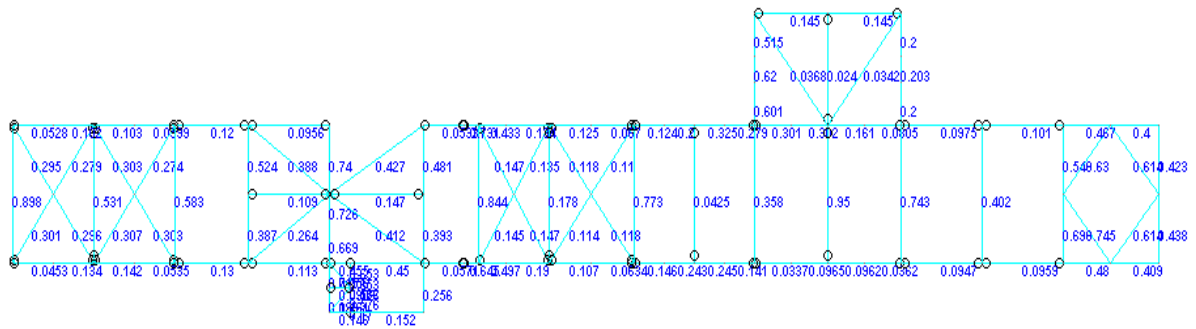


NIV +1211.20

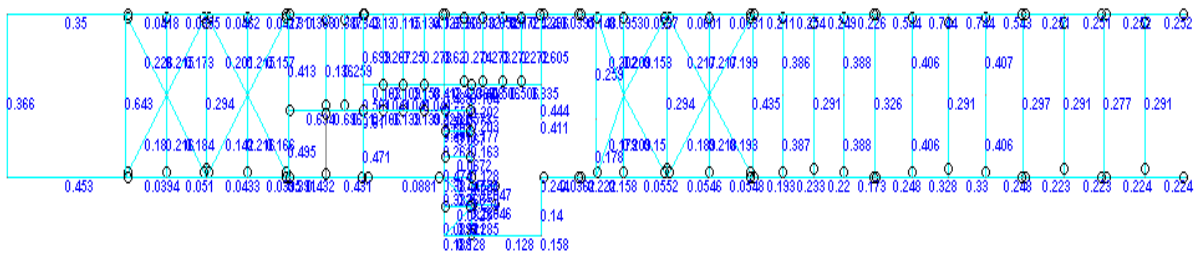


# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

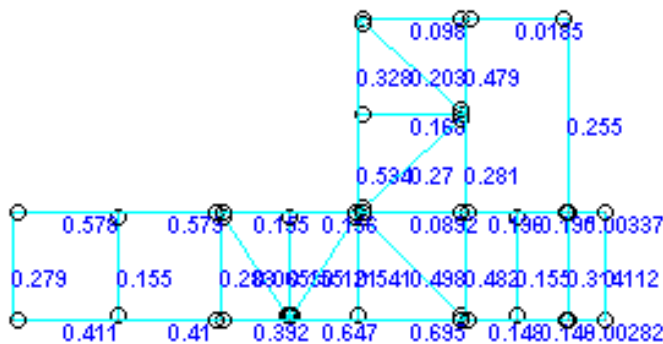
NIV +1213.70



NIV +1216.20



NIV +1218.20



Los resultados podrán revisarse también a detalle en la corrida de staad Correspondiente.



# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## 5.3. Diseño de conexiones principales.

### Conexiones Tipo a momento en estructura principal

#### Conexión a momento CW12X40 TW 10X26

Units system: Metric

File name: C:\MXPC9040\III SENER\III RACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\R1\conexiones\CONEXIONES EN RAM\conexiones CONEXION A MOMENTO C1 12X40 T1 10X26.cnx\

## Steel connections

### Detailed report







Connection name : EEP\_BCF\_1PL\_4B1  
Connection ID : 1  
Design code : AISC 360-05 LRFD

Family : Extended end plate (EEP)  
Type : Beam - Column flange (BCF)





### LOADS

Members M22 [Ton]	Load Axial [Ton]	Type [Ton*m]	V2 [Ton*m]	V3 [Ton]	M33
Right beam 5.50	1 - DL --	Design 1.68	3.23	--	















### GEOMETRIC CONSIDERATIONS

Dimensions References	Unit	Value	Min. value	Max. value	Sta.
<u>Extended end plate</u>					
Vertical edge distance Sec. J3.5	[cm]	4.00	2.86	15.24	
Horizontal edge distance Sec. J3.5	[cm]	5.50	2.86	15.24	
Horizontal center-to-center spacing (gage) DG4 Sec. 2.4	[cm]	9.00	7.62	14.66	
Distance from centerline of bolt to nearer surface o... DG4 Sec. 2.1	[cm]	3.00	2.86	--	
Bolt diameter DG4 Sec. 1.1	[cm]	1.59	--	3.81	
<u>Support</u>					
Column flange thickness (w/o stiffeners) DG4 Eq. 3.20	[cm]	1.31	0.94	--	




## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Horizontal edge distance Sec. J3.5	[cm]	5.67	2.86	15.24	
<b>Extended end plate behavior</b>					
Thick plate behavior for the end plate					
Thick plate behavior for the column flange					
<u>Transverse stiffeners</u>					
Width Sec. J10.8	[cm]	9.50	--	--	
Thickness Sec. J10.8	[cm]	1.00	0.63	--	
Weld size DG 13 Eq. 4.3-6	[1/16in]	5	3	--	

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Extended end plate</u>					
Shear yielding DG4 Eq. 3.12	[Ton]	43.19	11.41	1 - DL0.26	
Shear rupture DG4 Eq. 3.13	[Ton]	46.94	11.41	1 - DL0.24	
Flexural yielding DG4 Eq. 3.10, Sec. 2.2.3	[Ton*m]	12.31	5.50	1 - DL0.45	
No prying bolt moment strength DG4 Eq. 3.7	[Ton*m]	8.48	5.50	1 - DL0.65	
Bolts shear Eq. J3-1	[Ton]	20.05	3.23	1 - DL0.16	
Connector bolt bearing Eq. J3-6	[Ton]	73.08	3.23	1 - DL0.04	
<u>Beam</u>					
Beam web weld capacity Eq. J2-4	[Ton]	27.85	11.23	1 - DL0.40	
Shear yielding Eq. J4-3	[Ton]	26.24	3.23	1 - DL0.12	
Flange weld capacity Eq. J2-4	[Ton]	57.59	22.81	1 - DL0.40	
<u>Support</u>					
Flexural yielding DG4 Eq. 3.20, Sec. 2.2.3	[Ton*m]	12.33	5.50	1 - DL0.45	
Support bolt bearing Eq. J3-6	[Ton]	60.51	3.23	1 - DL0.05	
Panel web shear Eq. J10-9	[Ton]	30.95	22.81	1 - DL0.74	
Local web yielding Sec. J10	[Ton]	57.52	22.81	1 - DL0.40	
<u>Transverse stiffeners</u>					
Yielding strength due to axial load Eq. D2-1	[Ton]	38.72	4.02	1 - DL0.10	

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Compression strength Eq. J4-6	[Ton]	36.80	8.10	1 - DL0.22	
Flange weld capacity Eq. J2-4	[Ton]	57.47	8.10	1 - DL0.14	
Beam web weld capacity Eq. J2-4	[Ton]	59.72	8.10	1 - DL0.14	
<b>Critical strength ratio</b>		<b>0.74</b>			

### Conexión a momento CW18X86 TW12X50

Units system: Metric

File name: C:\MXPC9040\III SENER\III\RACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\racks\CONEXIONES RACK PRINCIPAL\CONEXION A MOMENTO C18X86 T12X50.cnx\

## Steel connections

### Detailed report



**Connection name** : EEP\_BCF\_1PL\_4B1  
**Connection ID** : 1  
**Design code** : AISC 360-05 LRFD

Family : Extended end plate (EEP)  
 Type : Beam - Column flange (BCF)









### LOADS

Members	Load	Type	V2	V3	M33
M22	Axial				
[Ton]	[Ton]	[Ton*m]	[Ton*m]	[Ton]	
Right beam	1 - DL	Design	4.23	--	
20.30	--	2.04			











### GEOMETRIC CONSIDERATIONS

Dimensions	Unit	Value	Min. value	Max. value	Sta.
<b>References</b>					
<u>Extended end plate</u>					
Vertical edge distance Sec. J3.5	[cm]	5.00	3.17	15.24	
Horizontal edge distance Sec. J3.5	[cm]	7.00	3.17	15.24	








## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Horizontal center-to-center spacing (gage) DG4 Sec. 2.4	[cm]	14.00	10.48	20.52	
Distance from centerline of bolt to nearer surface o... DG4 Sec. 2.1	[cm]	4.51	3.81	--	
Bolt diameter DG4 Sec. 1.1	[cm]	2.54	--	3.81	
<b>Support</b>					
Column flange thickness (w/o stiffeners) DG4 Eq. 3.20	[cm]	1.96	1.56	--	
Horizontal edge distance Sec. J3.5	[cm]	7.10	3.17	15.24	
<b>Extended end plate behavior</b>					
Thick plate behavior for the end plate					
Thick plate behavior for the column flange					
<b>Transverse stiffeners</b>					
Width Sec. J10.8	[cm]	12.50	--	--	
Thickness Sec. J10.8	[cm]	1.27	0.83	--	
Weld size DG 13 Eq. 4.3-6	[1/16in]	5	4	--	

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<b>Extended end plate</b>					
Shear yielding DG4 Eq. 3.12	[Ton]	84.96	35.07	1 - DL0.41	
Shear rupture DG4 Eq. 3.13	[Ton]	90.78	35.07	1 - DL0.39	
Flexural yielding DG4 Eq. 3.10, Sec. 2.2.3	[Ton*m]	26.19	20.30	1 - DL0.77	
No prying bolt moment strength DG4 Eq. 3.7	[Ton*m]	25.43	20.30	1 - DL0.80	
Bolts shear Eq. J3-1	[Ton]	51.27	4.23	1 - DL0.08	
Connector bolt bearing Eq. J3-6	[Ton]	142.26	4.23	1 - DL0.03	
<b>Beam</b>					
Beam web weld capacity Eq. J2-4	[Ton]	31.32	17.98	1 - DL0.57	
Shear yielding Eq. J4-3	[Ton]	44.23	4.23	1 - DL0.10	
Flange weld capacity Eq. J2-4	[Ton]	80.84	70.14	1 - DL0.87	
<b>Support</b>					
Flexural yielding DG4 Eq. 3.20, Sec. 2.2.3	[Ton*m]	30.05	20.30	1 - DL0.68	

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Support bolt bearing Eq. J3-6	[Ton]	145.85	4.23	1 - DL0.03	
Panel web shear Eq. J10-9	[Ton]	77.88	70.14	1 - DL0.90	
Local web yielding Sec. J10	[Ton]	131.37	70.14	1 - DL0.53	
<u>Transverse stiffeners</u>					
Yielding strength due to axial load Eq. D2-1	[Ton]	57.63	10.70	1 - DL0.19	
Compression strength Eq. J4-6	[Ton]	53.42	0.00	1 - DL0.00	
Flange weld capacity Eq. J2-4	[Ton]	68.36	10.70	1 - DL0.16	
Beam web weld capacity Eq. J2-4	[Ton]	89.89	10.70	1 - DL0.12	
<b>Critical strength ratio</b>		<b>0.90</b>			

### Conexión a momento CW18X86 TW14X68

Units system: Metric

File name: C:\MXPC9040\III SENER\IIIRACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\racks\CONEXIONES RACK PRINCIPAL\CONEXION A MOMENTO C18X86 T14X68.cnx\

## Steel connections

### Detailed report

**Connection name** : EEP\_BCF\_1PL\_4B1  
**Connection ID** : 1  
**Design code** : AISC 360-05 LRFD

Family : Extended end plate (EEP)  
 Type : Beam - Column flange (BCF)

### LOADS

Members M22 [Ton]	Load Axial [Ton]	Type [Ton*m]	V2 [Ton*m]	V3 [Ton]	M33
Right beam 25.46	1 - DL --	Design 3.71	20.41	--	

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## GEOMETRIC CONSIDERATIONS

Dimensions References	Unit	Value	Min. value	Max. value	Sta.
<u>Extended end plate</u>					
Vertical edge distance Sec. J3.5	[cm]	5.00	3.17	15.24	
Horizontal edge distance Sec. J3.5	[cm]	7.00	3.17	15.24	
Horizontal center-to-center spacing (gage) DG4 Sec. 2.4	[cm]	14.00	10.48	25.40	
Distance from centerline of bolt to nearer surface o... DG4 Sec. 2.1	[cm]	4.72	3.81	--	
Bolt diameter DG4 Sec. 1.1	[cm]	2.54	--	3.81	
<u>Support</u>					
Column flange thickness (w/o stiffeners) DG4 Eq. 3.20	[cm]	1.96	1.58	--	
Horizontal edge distance Sec. J3.5	[cm]	7.10	3.17	15.24	
<b>Extended end plate behavior</b>					
Thick plate behavior for the end plate					
Thick plate behavior for the column flange					
<u>Transverse stiffeners</u>					
Width Sec. J10.8	[cm]	13.00	--	--	
Thickness Sec. J10.8	[cm]	1.27	0.87	--	
Weld size DG 13 Eq. 4.3-6	[1/16in]	4	4	--	

## DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Extended end plate</u>					
Shear yielding DG4 Eq. 3.12	[Ton]	97.20	38.67	1 - DL0.40	
Shear rupture DG4 Eq. 3.13	[Ton]	103.87	38.67	1 - DL0.37	
Flexural yielding DG4 Eq. 3.10, Sec. 2.2.3	[Ton*m]	38.60	25.46	1 - DL0.66	
No prying bolt moment strength DG4 Eq. 3.7	[Ton*m]	29.21	25.46	1 - DL0.87	
Bolts shear Eq. J3-1	[Ton]	51.27	20.41	1 - DL0.40	
Connector bolt bearing Eq. J3-6	[Ton]	162.77	20.41	1 - DL0.13	
<u>Beam</u>					
Beam web weld capacity Eq. J2-4	[Ton]	43.12	23.14	1 - DL0.54	

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Shear yielding Eq. J4-3	[Ton]	56.92	20.41	1 - DL0.36	
Flange weld capacity Eq. J2-4	[Ton]	119.49	77.34	1 - DL0.65	
<b>Support</b>					
Flexural yielding DG4 Eq. 3.20, Sec. 2.2.3	[Ton*m]	33.75	25.46	1 - DL0.75	
Support bolt bearing Eq. J3-6	[Ton]	145.85	20.41	1 - DL0.14	
Panel web shear Eq. J10-9	[Ton]	77.88	77.34	1 - DL0.99	
Local web yielding Sec. J10	[Ton]	136.86	77.34	1 - DL0.57	
<b>Transverse stiffeners</b>					
Yielding strength due to axial load Eq. D2-1	[Ton]	60.52	17.46	1 - DL0.29	
Compression strength Eq. J4-6	[Ton]	56.10	4.70	1 - DL0.08	
Flange weld capacity Eq. J2-4	[Ton]	57.67	17.46	1 - DL0.30	
Beam web weld capacity Eq. J2-4	[Ton]	71.91	17.46	1 - DL0.24	
<b>Critical strength ratio</b>	<b>0.99</b>				

CONEXIONES A MOMENTO EN MARCOS PRINCIPALES																		
COLUMNA	TRABE	a	b	c	d	e	f	g	h	i	j	k	l	m	n	o	p	OBSERVACIONES
IR 305x59.80	IR 254x38.5	19	400	203	180	70	40	45			8	22	19	5	13	5		EJES DEL C AL K
IR 305x79	IR 254x49.2	22	400	254	140	90	40	70	25	5	8	25	22	5	16	5	213	EJES DEL 1 AL 6 Y EJE N
IR 356x110.4	IR 305x74.4	29	500	256	200	100	50	70			8	27	25	5	19	6		EJES DEL 5 AL 22
IR 457x144.30	IR 305x44.5	29	550	280	210	100	70	70			8	25	22	5	16	6		EJES DEL 23 AL 28
IR 457x144.30	IR 305x74.4	29	550	280	210	100	70	70			8	25	22	5	16	6		EJES DEL P1 AL S1 Y EJES DEL 23 AL 28
IR 457x128.10	IR 457x144.30	29	700	280	330	115	70	70			8	32	28	5	16	6		EJES J1 Y L1
IR 457x105.3	IR 305x74.4	25	500	205	195	100	52.5	70			8	21	19	5	16	6		EJES DEL M1 AL O
IR 457x128.10	IR 305x74.4	25	500	205	195	100	52.5	70			8	32	28	5	16	5	364	EJES DEL 32 AL 43
IR 457x144.30	IR 305x59.8	25	500	280	160	100	70	70			8	21	19	5	13	5		EJES DEL 32 AL 43
IR 457x128.10	IR 356x101.3	32	600	281	320	120	70	70	16	6	8	38	32	6	16	6		EJES DEL B1 AL J1
IR 457x144.30	IR 356x79.0	32	600	270	210	120	75	70			8	38	32	6	16	6		RACK DE ENFRIAMINETO
IR 457x144.30	IR 457x144.30	32	700	280	320	120	70	70			8	38	32	6	16	6		RACK DE ENFRIAMINETO
IR 610x285	IR 457x144.30	32	900	280	480	140	70	70			8	38	32	6	16	8		RACK DE ENFRIAMINETO
IR 457x128.10	IR 305x96.7	25	550	310	210	120	70	70	16	6	8	30	28	5	16	6		RACK DE ENFRIAMINETO N.-S.
IR 457x128.10	IR 305x52.2	25	550	280	210	100	70	70			8	21	19	6	13	5		RACK DE ENFRIAMINETO N.-S.

**CONEXION  
A MOMENTO**  
SIN/ESC.

**CORTE A-A**  
SIN/ESC.

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

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**Conexiones tipo a cortante en estructura principal**

**Conexión a cortante CW18X86 TW12X35**

Units system: Metric

File name: C:\MXPC9040\III SENER\III RACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\R1\conexiones\CONEXIONES EN RAM\CONEXIONES RACK PRINCIPAL\CONEXION A CORTANTE C18X86 T12X35.cnx\

**Steel connections**

**Detailed report**





Connection name : DA\_BCW\_L 3x3x1/4\_2B3/4  
 Connection ID : 1V  
 Design code : AISC 360-05 LRFD

Family : Angle(s) (CA)  
 Type : Beam - Column web (BCW)

**LOADS**

Members	Load	Type	V2	V3	M33
M22	Axial				
[Ton]	[Ton]	[Ton*m]	[Ton*m]	[Ton]	
Beam	1V - DL	Design	13.96	--	--
--	2.22				


**GEOMETRIC CONSIDERATIONS**

Dimensions	Unit	Value	Min. value	Max. value	Sta.
<b>References</b>					
<u>Angle</u>					
Length	[cm]	23.00	13.79	27.58	
p. 10-8					
Thickness	[cm]	0.63	--	1.59	
p. 10-9					
<u>Angle (Beam side)</u>					
Weld size	[1/16in]	2	2	3	
Sec. J2.2b					
<u>Angle (Support side)</u>					
Vertical edge distance	[cm]	4.50	2.22	--	
Tables J3.4,					
J3.5					
Horizontal edge distance	[cm]	3.81	2.22	--	
Tables J3.4,					
J3.5					
Vertical center-to-center spacing (pitch)	[cm]	7.00	4.23	15.24	
Sec. J3.5					




## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES













### Beam

Beam web thickness p. 9-5	[cm]	0.76	0.54	--	
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### Support

Horizontal edge distance Tables J3.4, J3.5	[cm]	3.81	2.22	--	
--	------	------	------	----	---

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Angle (Beam side)</u>					
Weld capacity p. 9-5	[Ton]	40.35	14.14	1V - DL0.35	
Shear yielding Eq. J4-3	[Ton]	44.36	13.96	1V - DL0.31	
<u>Angle (Support side)</u>					
Bolts shear Tables (7-1..14)	[Ton]	30.08	13.96	1V - DL0.46	
Bolt bearing under shear load Eq. J3-6	[Ton]	44.40	13.96	1V - DL0.31	
Shear yielding Eq. J4-3	[Ton]	44.36	13.96	1V - DL0.31	
Shear rupture Eq. J4-4	[Ton]	40.28	13.96	1V - DL0.35	
Block shear Eq. J4-5	[Ton]	37.86	13.96	1V - DL0.37	
Resulting tension capacity due prying action p. 9-10	[Ton]	5.76	2.22	1V - DL0.39	
<u>Beam</u>					
Shear yielding Eq. J4-3	[Ton]	36.74	13.96	1V - DL0.38	
Shear rupture Eq. J4-4	[Ton]	44.40	13.96	1V - DL0.31	
Tear out under axial load Eq. J4-5	[Ton]	71.24	2.22	1V - DL0.03	
<u>Support</u>					
Bolt bearing under shear load Eq. J3-6	[Ton]	85.24	13.96	1V - DL0.16	
<b>Critical strength ratio</b>	<b>0.46</b>				

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## Conexión a cortante CW18X86 TW12X50

Units system: Metric

File name: C:\MXPC9040\III SENER\III RACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\R1\conexiones\CONEXIONES EN RAM\CONEXIONES RACK PRINCIPAL\CONEXION A CORTANTE C18X86 T12X50.cnx\

## Steel connections

### Detailed report




**Connection name** : DA\_BCW\_L 3x3x1/4\_2B3/4  
**Connection ID** : 1V  
**Design code** : AISC 360-05 LRFD

Family : Angle(s) (CA)  
 Type : Beam - Column web (BCW)

### LOADS

Members M22 [Ton]	Load Axial [Ton]	Type [Ton*m]	V2 [Ton*m]	V3 [Ton]	M33
Beam --	1V - DL 2.55	Design	13.18	--	--

### GEOMETRIC CONSIDERATIONS

Dimensions References	Unit	Value	Min. value	Max. value	Sta.
<u>Angle</u>					
Length p. 10-8	[cm]	23.00	12.60	25.20	
Thickness p. 10-9	[cm]	0.95	--	1.59	
<u>Angle (Beam side)</u>					
Weld size Sec. J2.2b	[1/16in]	3	3	5	
<u>Angle (Support side)</u>					
Vertical edge distance Tables J3.4, J3.5	[cm]	4.50	2.22	--	
Horizontal edge distance Tables J3.4, J3.5	[cm]	3.81	2.22	--	
Vertical center-to-center spacing (pitch) Sec. J3.5	[cm]	7.00	4.23	22.86	
<u>Beam</u>					
Beam web thickness p. 9-5	[cm]	0.94	0.81	--	

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

### Support

Horizontal edge distance Tables J3.4, J3.5	[cm]	3.81	2.22	--	
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### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Angle (Beam side)</u>					
Weld capacity p. 9-5	[Ton]	56.82	13.42	1V - DL0.24	
Shear yielding Eq. J4-3	[Ton]	66.54	13.18	1V - DL0.20	
<u>Angle (Support side)</u>					
Bolts shear Tables (7-1..14)	[Ton]	30.08	13.18	1V - DL0.44	
Bolt bearing under shear load Eq. J3-6	[Ton]	66.59	13.18	1V - DL0.20	
Shear yielding Eq. J4-3	[Ton]	66.54	13.18	1V - DL0.20	
Shear rupture Eq. J4-4	[Ton]	60.42	13.18	1V - DL0.22	
Block shear Eq. J4-5	[Ton]	56.79	13.18	1V - DL0.23	
Resulting tension capacity due prying action p. 9-10	[Ton]	26.74	2.55	1V - DL0.10	
<u>Beam</u>					
Shear yielding Eq. J4-3	[Ton]	44.23	13.18	1V - DL0.30	
Shear rupture Eq. J4-4	[Ton]	53.44	13.18	1V - DL0.25	
Tear out under axial load Eq. J4-5	[Ton]	82.42	2.55	1V - DL0.03	
<u>Support</u>					
Bolt bearing under shear load Eq. J3-6	[Ton]	85.24	13.18	1V - DL0.15	
<b>Critical strength ratio</b>	<b>0.44</b>				

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

CONEXIONES A CORTANTE EN MARCOS PRINCIPALES												
COLUMNA	TRABE	a	b	c	d	e	f	g	h	j	k	OBSERVACIONES
IR 305x59.8	IR 254x38.5	13	200	50	50	3	18	16	6	50	50	EJES 1 Y 2 (NIV. 1204.700)
IR 305x79	IR 254x49.2	13	190	35	60	3	24	22	6	50	50	EJES L Y M
IR 356x110.40	IR 305x52.20	13	230	45	70	3	18	16	6	50	50	EJES Ñ, O, DEL 5 AL 22 (NIV. 1205.700)
IR 457x144.30	IR 305x44.5	13	230	45	70	3	24	22	6	50	50	EJES Ñ Y O DEL 5 AL 22 (AR-1)
IR 457x144.30	IR 305x74.4	13	230	45	70	3	21	19	6	50	50	EJES 35, 36, 42, Y 43, EJES Ñ Y O DEL 22 AL 30
IR 457x144.30	IR 305x59.8	13	240	40	75	3	27	25	6	50	50	EJES 35, 36, 42 Y 43 TREN 1 Y TREN 2)
IR 457x128.10	IR 305x74.4	13	230	45	70	3	24	22	6	50	50	EJES RACK PRINCIPAL ESTE-OESTE
IR 457x144.30	IR 457x144.30	13	250	50	75	4	27	25	6	50	50	RACK AEROENFRIADOR
IR 457x112.9	IR 305x74.4	13	230	45	70	3	24	22	6	50	50	RACK PRINCIPAL ESTE-OESTE DE O S1
* IR 457x144.30	IR 356x79.0	13	250	45	75	6	27	25	6	50	75	AEROENFRIADOR
IR 305x59.80	IR 305x52.20	13	230	45	70	3	18	16	6	50	50	EJES 1 Y 2 (NIV. 1206.200)
IR 356x110.40	IR 356x63.80	13	250	50	75	3	21	19	6	50	50	EJES Ñ Y O DE 5 AL 22 (NIV.1207.700)
IR 457x112.90	IR 356x110.40	13	250	50	75	3	21	19	6	50	50	EJES 31 Y 32 ENTRE O Y P1
IR 457x128.10	IR 457x96.70	13	370	65	80	4	27	25	6	50	75	RACK PRINCIPAL ESTE-OESTE N1 A O (30 Y 32)
IR 305x86.10	IR 356x101.30	13	280	60	80	3	27	25	6	50	75	RACK ELECTRICO, EJES 29, 31 Y 32
IR 457x128.10	IR 305x52.20	13	230	45	70	3	24	22	6	50	50	RACK PRINCIPAL N-S EJES J1 Y L1
IR 457x128.10	IR 305x74.40	13	230	40	75	3	27	25	6	50	75	RACK PRINCIPAL N-S
IR 254x44.80	IR 254x44.8	13	200	50	50	3	18	16	6	50	50	RACK PRINCIPAL N-S
IR 457x144.30	IR 305x79.0	13	240	40	75	3	27	25	6	50	50	RACK TREN 1 Y 2

**CONEXION A CORTANTE**  
SIN/ESC.

**CORTE B-B**  
SIN/ESC.

**\* CONEXION A CORTANTE**  
SIN/ESC.

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## Conexiones criticas

### Detalle 23

Units system: Metric

File name: C:\MXPC9040\III SENER\III\RACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\R1\conexiones\CONEXIONES EN RAM\CONEXIONES RACK PRINCIPAL\CONWXION DE DIAGONALES HORIZONTALES 8X18 Y 12X30.cnx\

## Steel connections

### Detailed report

Connection name : SP\_BG\_1/4PL\_2B3/4  
Connection ID : 2V  
Design code : AISC 360-05 LRFD

Family : Single plate (SP)  
Type : Beam - Girder (BG)



### LOADS

Members	Load	Type	V2	V3	M33
M22	Axial				
[Ton]	[Ton]	[Ton*m]	[Ton*m]	[Ton]	
Beam	2V - DL	Design	--	--	--
--	6.83	--	--	--	--




















### GEOMETRIC CONSIDERATIONS

Dimensions	Unit	Value	Min. value	Max. value	Sta.
<u>References</u>					
<u>Shear plate</u>					
Length p. 10-49	[cm]	14.00	8.74	16.08	
Vertical edge distance Tables J3.4, J3.5	[cm]	4.00	3.17	--	
Horizontal edge distance p. 10-102	[cm]	5.00	3.81	--	
Vertical center-to-center spacing (pitch) Sec. J3.5	[cm]	6.00	5.08	14.02	
<u>Beam</u>					
Thickness p. 10-102	[cm]	0.58	--	1.11	
Vertical edge distance Tables J3.4, J3.5	[cm]	6.08	3.17	--	



## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

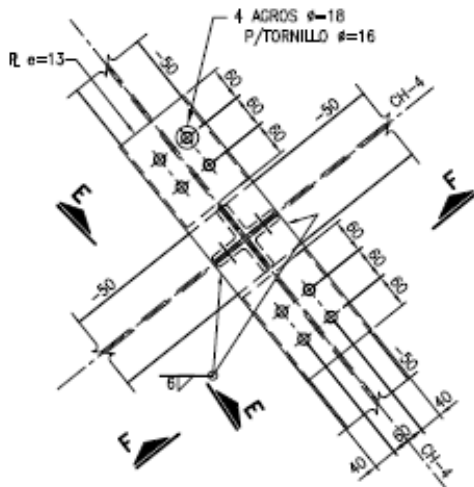
Horizontal edge distance p. 10-102	[cm]	6.64	3.81	--	
<u>Support</u>					
Weld size Sec. J2.2b	[1/16in]	6	6	7	

### DESIGN CHECK

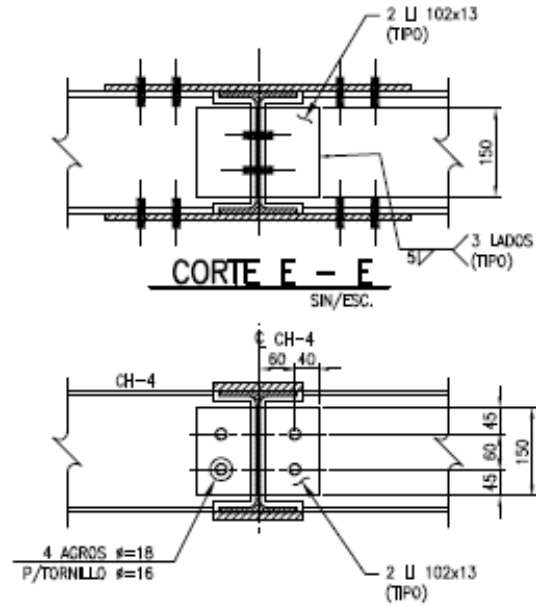
Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Shear plate</u>					
Bolts shear Tables (7-1..14)	[Ton]	14.44	6.83	2V - DL0.47	
Bolt bearing under shear load HSS Manual Eq. 4-4	[Ton]	31.59	0.00	2V - DL0.00	
Shear yielding Eq. J4-3	[Ton]	27.00	0.00	2V - DL0.00	
Shear rupture Eq. J4-4	[Ton]	22.27	0.00	2V - DL0.00	
Block shear Eq. J4-5	[Ton]	29.57	0.00	2V - DL0.00	
Bolt bearing under axial load HSS Manual Eq. 4-4	[Ton]	35.52	6.83	2V - DL0.19	
Yielding strength due to axial load Eq. J4-1	[Ton]	40.50	6.83	2V - DL0.17	
Rupture due to axial load Eq. J4-2	[Ton]	37.11	6.83	2V - DL0.18	
Tear out under axial load Eq. J4-5	[Ton]	29.14	6.83	2V - DL0.23	
<u>Plate (support side)</u>					
Weld capacity p. 9-5	[Ton]	62.65	6.83	2V - DL0.11	
<u>Beam</u>					
Bolt bearing under shear load HSS Manual Eq. 4-4	[Ton]	16.34	0.00	2V - DL0.00	
Shear yielding Eq. J4-3	[Ton]	15.68	0.00	2V - DL0.00	
Shear rupture Eq. J4-4	[Ton]	14.18	0.00	2V - DL0.00	
Flexural yielding	[Ton]	10.10	0.00	2V - DL	0.00 
Local web buckling p. 9-7	[Ton]	10.10	0.00	2V - DL0.00	
Block shear Eq. J4-5	[Ton]	17.91	0.00	2V - DL0.00	
Flexural rupture p. 9-6	[Ton]	13.56	0.00	2V - DL0.00	
Bolt bearing under axial load Eq. J3-6	[Ton]	16.34	6.83	2V - DL0.42	
Yielding strength due to axial load Eq. J4-1	[Ton]	23.52	6.83	2V - DL0.29	

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Rupture due to axial load Eq. J4-2	[Ton]	23.64	6.83	2V - DL0.29	
Tear out under axial load Eq. J4-5	[Ton]	15.58	6.83	2V - DL0.44	
<u>Support</u>					
<b>Critical strength ratio</b>		<b>0.47</b>			



**DETALLE 23**  
PF-0806 SIN/ESC.



**CORTE F - F**  
SIN/ESC.

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## Detalle 24

Units system: Metric

File name: C:\MXPC9040\III SENER\III RACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\R1\conexiones\CONEXIONES EN RAM\CONEXIONES RACK PRINCIPAL\CONEXION DE DIAGONALES HORIZONTALES 8X18 Y 12X30.cnx\

## Steel connections

### Detailed report








**Connection name** : SP\_BG\_1/4PL\_2B3/4  
**Connection ID** : 2V  
**Design code** : AISC 360-05 LRFD

Family : Single plate (SP)  
 Type : Beam - Girder (BG)

### LOADS


Members	Load	Type	V2	V3	M33
M22	Axial				
[Ton]	[Ton]	[Ton*m]	[Ton*m]	[Ton]	
Beam	2V - DL	Design	--	--	--
--	6.83				

### GEOMETRIC CONSIDERATIONS





















Dimensions	Unit	Value	Min. value	Max. value	Sta.
<b>References</b>					
<u>Shear plate</u>					
Length	[cm]	14.00	8.74	16.08	
p. 10-49					
Vertical edge distance	[cm]	4.00	3.17	--	
Tables J3.4,					
J3.5					
Horizontal edge distance	[cm]	5.00	3.81	--	
p. 10-102					
Vertical center-to-center spacing (pitch)	[cm]	6.00	5.08	14.02	
Sec. J3.5					
<u>Beam</u>					
Thickness	[cm]	0.58	--	1.11	
p. 10-102					
Vertical edge distance	[cm]	6.08	3.17	--	
Tables J3.4,					
J3.5					
Horizontal edge distance	[cm]	6.64	3.81	--	
p. 10-102					
<u>Support</u>					



## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Weld size Sec. J2.2b	[1/16in]	6	6	7	
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### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Shear plate</u>					
Bolts shear Tables (7-1..14)	[Ton]	14.44	6.83	2V - DL0.47	
Bolt bearing under shear load HSS Manual Eq. 4-4	[Ton]	31.59	0.00	2V - DL0.00	
Shear yielding Eq. J4-3	[Ton]	27.00	0.00	2V - DL0.00	
Shear rupture Eq. J4-4	[Ton]	22.27	0.00	2V - DL0.00	
Block shear Eq. J4-5	[Ton]	29.57	0.00	2V - DL0.00	
Bolt bearing under axial load HSS Manual Eq. 4-4	[Ton]	35.52	6.83	2V - DL0.19	
Yielding strength due to axial load Eq. J4-1	[Ton]	40.50	6.83	2V - DL0.17	
Rupture due to axial load Eq. J4-2	[Ton]	37.11	6.83	2V - DL0.18	
Tear out under axial load Eq. J4-5	[Ton]	29.14	6.83	2V - DL0.23	
<u>Plate (support side)</u>					
Weld capacity p. 9-5	[Ton]	62.65	6.83	2V - DL0.11	
<u>Beam</u>					
Bolt bearing under shear load HSS Manual Eq. 4-4	[Ton]	16.34	0.00	2V - DL0.00	
Shear yielding Eq. J4-3	[Ton]	15.68	0.00	2V - DL0.00	
Shear rupture Eq. J4-4	[Ton]	14.18	0.00	2V - DL0.00	
Flexural yielding	[Ton]	10.10	0.00	2V - DL	0.00 
Local web buckling p. 9-7	[Ton]	10.10	0.00	2V - DL0.00	
Block shear Eq. J4-5	[Ton]	17.91	0.00	2V - DL0.00	
Flexural rupture p. 9-6	[Ton]	13.56	0.00	2V - DL0.00	
Bolt bearing under axial load Eq. J3-6	[Ton]	16.34	6.83	2V - DL0.42	
Yielding strength due to axial load Eq. J4-1	[Ton]	23.52	6.83	2V - DL0.29	
Rupture due to axial load Eq. J4-2	[Ton]	23.64	6.83	2V - DL0.29	

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Tear out under axial load  
Eq. J4-5

[Ton]

15.58

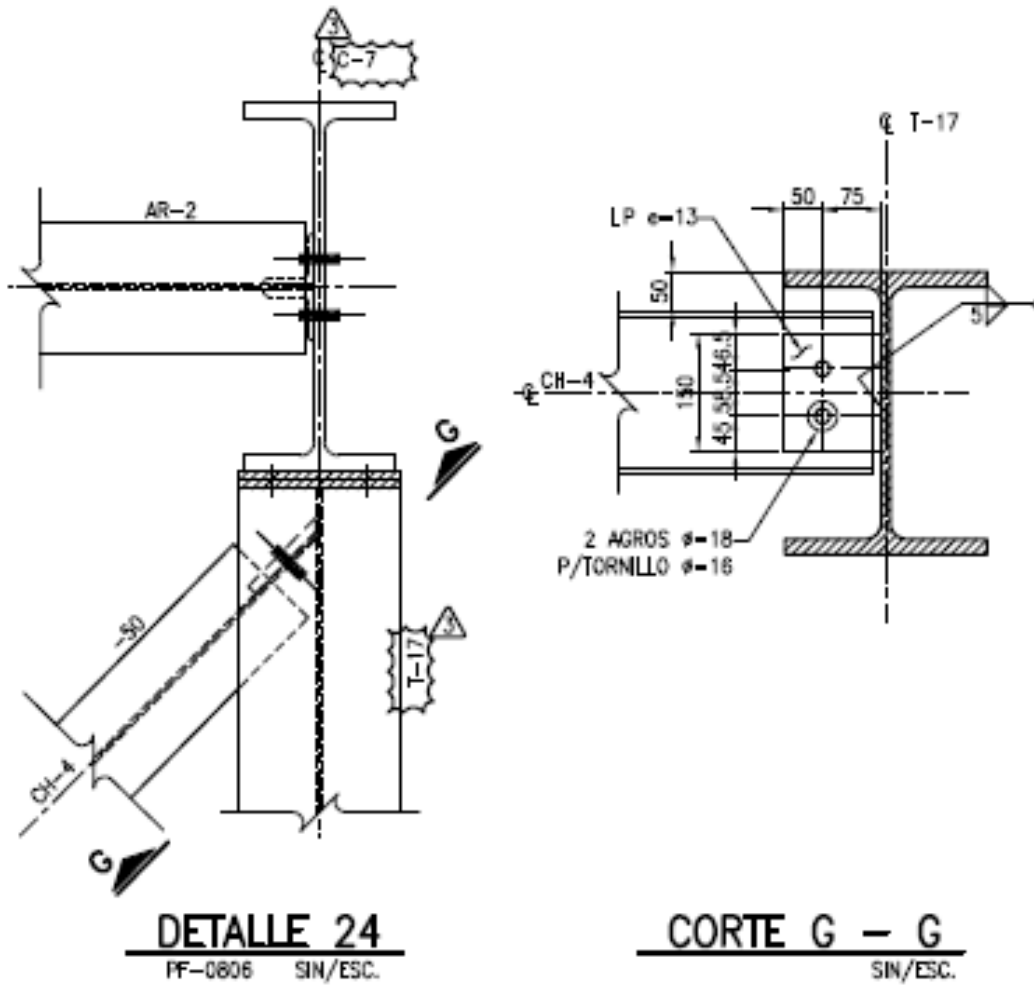
6.83 2V - DL0.44



Support

Critical strength ratio

0.47



# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## Detalle 43

**Units system:** Metric

**File name:** C:\MXPC9040\III SENER\III\RACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\R1\conexiones\CONEXIONES EN RAM\CONEXIONES RACK PRINCIPAL\CONEXION DE DIAGONALES X 10X30.cnx\

## **Steel connections**

### Detailed report

**Connection name** : (VXB)  
**Connection ID** : 1  
**Design code** : AISC 360-05 LRFD

Family : Gusset (GP)  
 Type : Vertical X braces (VXB)




### LOADS

Members M22 [Ton]	Load Axial [Ton]	Type [Ton*m]	V2 [Ton*m]	V3 [Ton]	M33
Braces1 --	1 - eq 31.20	Design	--	--	--
Braces2 --	1 - eq 31.20	Design	--	--	--
Braces3 --	1 - eq -31.20	Design	--	--	--
Braces4 --	1 - eq -31.20	Design	--	--	--

### Interface between Gusset - Upper right brace

#### Connection: Splice plate

#### GEOMETRIC CONSIDERATIONS

Dimensions References	Unit	Value	Min. value	Max. value	Sta.
<u>Splice plate</u>					
Transverse edge distance Tables J3.4, J3.5	[cm]	5.00	2.86	--	
Longitudinal edge distance Tables J3.4, J3.5	[cm]	5.00	2.86	--	
Longitudinal center-to-center spacing (pitch) Sec. J3.5	[cm]	7.50	5.93	30.48	

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Transverse center-to-center spacing (gage) Sec. J3.5	[cm]	7.50	5.93	30.48	
<u>Gusset</u>					

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Splice plate</u>					
Bolts shear Tables (7-1..14)	[Ton]	58.88	31.20	1 - eq0.53	
Yielding strength due to axial load Eq. J4-1	[Ton]	50.63	31.20	1 - eq0.62	
Rupture due to axial load Eq. J4-2	[Ton]	48.24	31.20	1 - eq0.65	
Bolt bearing under axial load Eq. J3-6	[Ton]	118.38	31.20	1 - eq0.26	
Outside block shear Eq. J4-5	[Ton]	86.83	31.20	1 - eq0.36	
Inside block shear Eq. J4-5	[Ton]	77.12	31.20	1 - eq0.40	
<u>Brace</u>					
Block shear rupture at brace flange Eq. J4-5	[Ton]	46.27	31.20	1 - eq0.67	
Bolt bearing under axial load Eq. J3-6	[Ton]	71.03	31.20	1 - eq0.44	
<u>Gusset</u>					
Block shear on gusset Eq. J4-5	[Ton]	77.12	31.20	1 - eq0.40	
Bolt bearing under axial load Eq. J3-6	[Ton]	118.38	31.20	1 - eq0.26	
<b>Critical strength ratio</b>	<b>0.67</b>				

### Checks for gusset and brace

#### REQUIRED RESISTANCE OF BRACED CONNECTIONS

Requirement	Unit	Value
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#### GEOMETRIC CONSIDERATIONS

Dimensions References	Unit	Value	Min. value	Max. value	Sta.
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Reinforcement plate

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
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## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

### Brace

Yielding strength due to axial load Eq. J4-1	[Ton]	129.92	31.20	1 - eq0.24	
Rupture due to axial load Eq. J4-2	[Ton]	135.11	31.20	1 - eq0.23	
<u>Gusset</u>					
Tension yielding on the Whitmore section Eq. J4-1	[Ton]	71.81	31.20	1 - eq0.43	
<b>Critical strength ratio</b>		<b>0.43</b>			

### Interface between Gusset - Upper left brace

#### Connection: Splice plate





#### GEOMETRIC CONSIDERATIONS

Dimensions References	Unit	Value	Min. value	Max. value	Sta.
<u>Splice plate</u>					
Transverse edge distance Tables J3.4, J3.5	[cm]	5.00	2.86	--	
Longitudinal edge distance Tables J3.4, J3.5	[cm]	5.00	2.86	--	
Longitudinal center-to-center spacing (pitch) Sec. J3.5	[cm]	7.50	5.93	30.48	
Transverse center-to-center spacing (gage) Sec. J3.5	[cm]	7.50	5.93	30.48	
<u>Gusset</u>					

#### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Splice plate</u>					
Bolts shear Tables (7-1..14)	[Ton]	58.88	31.20	1 - eq0.53	
Yielding strength due to axial load Eq. J4-1	[Ton]	50.63	31.20	1 - eq0.62	
Rupture due to axial load Eq. J4-2	[Ton]	48.24	31.20	1 - eq0.65	
Bolt bearing under axial load Eq. J3-6	[Ton]	118.38	31.20	1 - eq0.26	
Outside block shear Eq. J4-5	[Ton]	86.83	31.20	1 - eq0.36	
Inside block shear Eq. J4-5	[Ton]	77.12	31.20	1 - eq0.40	
<u>Brace</u>					

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Bock shear rupture at brace flange Eq. J4-5	[Ton]	46.27	31.20	1 - eq0.67	
Bolt bearing under axial load Eq. J3-6	[Ton]	71.03	31.20	1 - eq0.44	
<u>Gusset</u>					
Block shear on gusset Eq. J4-5	[Ton]	77.12	31.20	1 - eq0.40	
Bolt bearing under axial load Eq. J3-6	[Ton]	118.38	31.20	1 - eq0.26	
<b>Critical strength ratio</b>		<b>0.67</b>			

### Checks for gusset and brace

#### REQUIRED RESISTANCE OF BRACED CONNECTIONS

<b>Requirement</b>	<b>Unit</b>	<b>Value</b>			
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#### GEOMETRIC CONSIDERATIONS


<b>Dimensions</b>	<b>Unit</b>	<b>Value</b>	<b>Min. value</b>	<b>Max. value</b>	<b>Sta.</b>
<b>References</b>					


#### Reinforcement plate

#### DESIGN CHECK


<b>Verification</b>	<b>Unit</b>	<b>Capacity</b>	<b>Demand</b>	<b>Ctrl EQ</b>	<b>Ratio</b>
<b>References</b>					

#### Brace

Yielding strength due to axial load Eq. J4-1	[Ton]	129.92	31.20	1 - eq0.24	
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Rupture due to axial load Eq. J4-2	[Ton]	135.11	31.20	1 - eq0.23	
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#### Gusset

Tension yielding on the Whitmore section Eq. J4-1	[Ton]	71.81	31.20	1 - eq0.43	
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<b>Critical strength ratio</b>		<b>0.43</b>			
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### Interface between Gusset - Lower left brace

#### Connection: Splice plate




#### GEOMETRIC CONSIDERATIONS

<b>Dimensions</b>	<b>Unit</b>	<b>Value</b>	<b>Min. value</b>	<b>Max. value</b>	<b>Sta.</b>
<b>References</b>					

#### Splice plate





Transverse edge distance Tables J3.4, J3.5	[cm]	5.00	2.86	--	
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## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Longitudinal edge distance Tables J3.4, J3.5	[cm]	5.00	2.86	--	
Longitudinal center-to-center spacing (pitch) Sec. J3.5	[cm]	7.50	5.93	30.48	
Transverse center-to-center spacing (gage) Sec. J3.5	[cm]	7.50	5.93	30.48	

Gusset

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Splice plate</u>					
Bolts shear Tables (7-1..14)	[Ton]	58.88	31.20	1 - eq0.53	
Bolt bearing under axial load Eq. J3-6	[Ton]	118.38	31.20	1 - eq0.26	
<u>Brace</u>					
Bolt bearing under axial load Eq. J3-6	[Ton]	71.03	31.20	1 - eq0.44	
<u>Gusset</u>					
Bolt bearing under axial load Eq. J3-6	[Ton]	118.38	31.20	1 - eq0.26	
<b>Critical strength ratio</b>	<b>0.53</b>				

### Checks for gusset and brace

#### REQUIRED RESISTANCE OF BRACED CONNECTIONS


Requirement	Unit	Value
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#### GEOMETRIC CONSIDERATIONS

Dimensions References	Unit	Value	Min. value	Max. value	Sta.
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Reinforcement plate

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Gusset</u>					
Buckling on the Whitmore section Eq. E3-1	[Ton]	54.19	31.20	1 - eq0.58	
<b>Critical strength ratio</b>	<b>0.58</b>				

### Interface between Gusset - Lower right brace

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## Connection: Splice plate

### GEOMETRIC CONSIDERATIONS

Dimensions References	Unit	Value	Min. value	Max. value	Sta.
<u>Splice plate</u>					
Transverse edge distance Tables J3.4, J3.5	[cm]	5.00	2.86	--	
Longitudinal edge distance Tables J3.4, J3.5	[cm]	5.00	2.86	--	
Longitudinal center-to-center spacing (pitch) Sec. J3.5	[cm]	7.50	5.93	30.48	
Transverse center-to-center spacing (gage) Sec. J3.5	[cm]	7.50	5.93	30.48	
<u>Gusset</u>					

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Splice plate</u>					
Bolts shear Tables (7-1..14)	[Ton]	58.88	31.20	1 - eq0.53	
Bolt bearing under axial load Eq. J3-6	[Ton]	118.38	31.20	1 - eq0.26	
<u>Brace</u>					
Bolt bearing under axial load Eq. J3-6	[Ton]	40.25	31.20	1 - eq0.78	
<u>Gusset</u>					
Bolt bearing under axial load Eq. J3-6	[Ton]	118.38	31.20	1 - eq0.26	
<b>Critical strength ratio</b>		<b>0.78</b>			

## Checks for gusset and brace

### REQUIRED RESISTANCE OF BRACED CONNECTIONS

Requirement	Unit	Value
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### GEOMETRIC CONSIDERATIONS

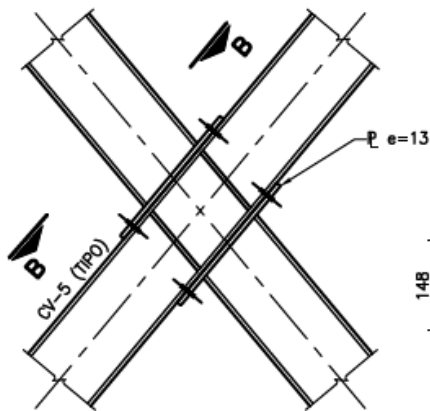
Dimensions References	Unit	Value	Min. value	Max. value	Sta.
<u>Reinforcement plate</u>					



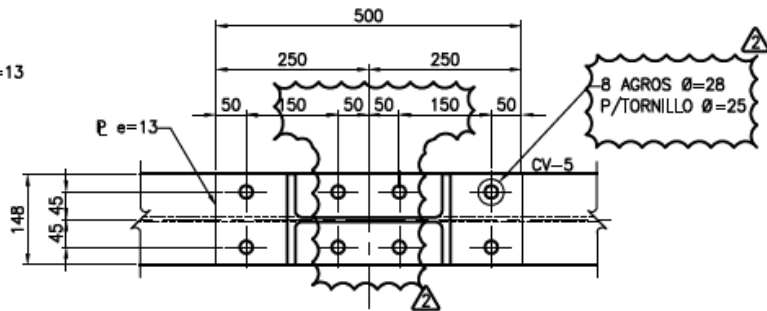
# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Gusset</u>					
Buckling on the Whitmore section Eq. E3-1	[Ton]	54.19	31.20	1 - eq0.58	
<b>Critical strength ratio</b>	<b>0.58</b>				
<b>Global critical strength ratio</b>	<b>0.78</b>				



**DETALLE 43**  
PF-0807 SIN/ESC.



**CORTE B-B**  
SIN/ESC.

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## Conexión a momento columna en alma y viga

Units system: Metric

File name: C:\MXPC9040\III SENER\III RACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\R1\conexiones\CONEXIONES EN RAM\CONEXIONES RACK PRINCIPAL\CONEXION A MOMENTO CORTANTE C18X86 T14X68.cnx\

## Steel connections

### Detailed report







**Connection name** : CA\_BCW\_LU 3X2-1\_2X1\_4\_r=2\_r=2  
**Connection ID** : 1V  
**Design code** : AISC 360-05 LRFD

Family : Angle(s) (CA)  
 Type : Beam - Column web (BCW)

### LOADS

Members	Load	Type	V2	V3	M33
M22	Axial				
[Ton]	[Ton]	[Ton*m]	[Ton*m]	[Ton]	
Beam	1V - DL	Design	7.04	--	--
--	1.30				















### GEOMETRIC CONSIDERATIONS

Dimensions	Unit	Value	Min. value	Max. value	Sta.
<b>References</b>					
<u>Angle</u>					
Length	[cm]	25.00	14.45	28.91	
p. 10-8					
Thickness	[cm]	0.95	--	1.59	
p. 10-9					
<u>Angle (Beam side)</u>					
Vertical edge distance	[cm]	5.00	2.54	--	
Tables J3.4,					
J3.5					
Horizontal edge distance	[cm]	3.12	2.54	--	
Tables J3.4,					
J3.5					
Vertical center-to-center spacing (pitch)	[cm]	7.50	5.08	22.86	
Sec. J3.5					
<u>Angle (Support side)</u>					
Weld size	[1/16in]	5	3	5	
Sec. J2.2b					
<u>Beam</u>					
Horizontal edge distance	[cm]	3.23	2.54	--	
Tables J3.4,					

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

J3.5  
Support

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Angle (Beam side)</u>					
Bolts shear Tables (7-1..14)	[Ton]	43.31	7.16	1V - DL0.17	
Bolt bearing under shear load Eq. J3-6	[Ton]	79.91	7.04	1V - DL0.09	
Shear yielding Eq. J4-3	[Ton]	72.32	7.04	1V - DL0.10	
Shear rupture Eq. J4-4	[Ton]	64.08	7.04	1V - DL0.11	
Block shear Eq. J4-5	[Ton]	55.10	7.04	1V - DL0.13	
Bolt bearing under axial load Eq. J3-6	[Ton]	43.80	1.30	1V - DL0.03	
Tear out under axial load Eq. J4-5	[Ton]	75.03	1.30	1V - DL0.02	
<u>Angle (Support side)</u>					
Weld capacity p. 10-11	[Ton]	41.86	7.04	1V - DL0.17	
Shear yielding Eq. J4-3	[Ton]	72.32	7.04	1V - DL0.10	
<u>Beam</u>					
Bolt bearing under shear load Eq. J3-6	[Ton]	44.22	7.04	1V - DL0.16	
Shear yielding Eq. J4-3	[Ton]	56.92	7.04	1V - DL0.12	
Shear rupture Eq. J4-4	[Ton]	55.89	7.04	1V - DL0.13	
Bolt bearing under axial load Eq. J3-6	[Ton]	25.51	1.30	1V - DL0.05	
Tear out under axial load Eq. J4-5	[Ton]	41.78	1.30	1V - DL0.03	
<u>Support</u>					
<b>Critical strength ratio</b>	<b>0.17</b>				

Units system: Metric

File name: C:\MXPC9040\III SENER\III\RACK DE TUBERIAS CIMENTACION PLANTA 1 DE 2\R1\conexiones\CONEXIONES EN RAM\CONEXIONES RACK PRINCIPAL\CONEXION A MOMENTO CORTANTE C18X86 T14X68.cnx\

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## Steel connections

### Detailed report









**Connection name** : FP\_BCW\_1PL\_7B3/4  
**Connection ID** : 1M  
**Design code** : AISC 360-05 LRFD

Family : Flange-plated (FP)  
 Type : Beam - Column web (BCW)

### LOADS

Members	Load	Type	V2	V3	M33
M22	Axial				
[Ton]	[Ton]	[Ton*m]	[Ton*m]	[Ton]	
Beam	1M - DL	Design	--	--	
26.32	--	1.30			

### GEOMETRIC CONSIDERATIONS

Dimensions	Unit	Value	Min. value	Max. value	Sta.
<b>References</b>					
<u>Plate (beam side)</u>					
Vertical center-to-center spacing (pitch) Sec. J3.5	[cm]	7.62	7.62	30.48	
Horizontal center-to-center spacing (gage) Sec. J3.5	[cm]	14.00	7.62	30.48	
<u>Top flange plate data</u>					
Vertical edge distance Tables J3.4, J3.5	[cm]	4.45	3.81	--	
Horizontal edge distance Tables J3.4, J3.5	[cm]	5.00	3.81	--	
<u>Bottom flange plate data</u>					
Vertical edge distance Tables J3.4, J3.5	[cm]	5.00	3.81	--	
Horizontal edge distance Tables J3.4, J3.5	[cm]	5.00	3.81	--	
<u>Plate (support side)</u>					
Top plate weld size - column flange Sec. J2.2b	[1/16in]	8	3	6	
Bottom plate weld size - column flange Sec. J2.2b	[1/16in]	8	3	6	
<u>Beam</u>					

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Vertical edge distance Tables J3.4, J3.5	[cm]	5.00	3.81	--	
Horizontal edge distance Tables J3.4, J3.5	[cm]	5.70	3.81	--	
<u>Support</u>					
Flange thickness p. 9-5	[cm]	1.96	1.08	--	
Web thickness p. 9-5	[cm]	1.22	1.08	--	

### DESIGN CHECK

Verification References	Unit	Capacity	Demand	Ctrl EQ	Ratio
<u>Moment - Flange-Plated</u>					
Compression strength (Top plate) Eq. J4-6	[Ton]	201.83	69.01	1M - DL0.34	
Bottom plate tension yielding Eq. J4-1	[Ton]	202.08	70.31	1M - DL0.35	
Bolts shear (Top plate) Tables (7-1..14)	[Ton]	129.85	77.37	1M - DL0.60	
Bolt bearing (Top plate) Eq. J3-6	[Ton]	272.91	69.01	1M - DL0.25	
Bolts shear (Bottom plate) Tables (7-1..14)	[Ton]	129.85	78.67	1M - DL0.61	
Bottom plate tension rupture Eq. J4-2	[Ton]	228.20	70.31	1M - DL0.31	
Bottom plate block shear Eq. J4-5	[Ton]	209.95	70.31	1M - DL0.33	
Bolt bearing (Bottom plate) Eq. J3-6	[Ton]	281.95	70.31	1M - DL0.25	
<u>Plate (support side)</u>					
Top plate to column weld Eq. J2-4	[Ton]	90.68	77.37	1M - DL0.85	
Bottom plate to column weld Eq. J2-4	[Ton]	90.68	78.67	1M - DL0.87	
Top plate shear capacity at column flange weld [3]	[Ton]	76.85	69.01	1M - DL0.90	
Bottom plate shear capacity at column flange weld [3]	[Ton]	76.85	70.31	1M - DL0.91	
<u>Beam</u>					
Flexural strength of the beam Sec. F13.1	[Ton*m]	42.93	26.32	1M - DL0.61	
Bolt bearing on flange Eq. J3-6	[Ton]	232.27	78.67	1M - DL0.34	
Beam block shear Eq. J4-5	[Ton]	158.41	78.67	1M - DL0.50	
<u>Support</u>					
<b>Critical strength ratio</b>	<b>0.91</b>				

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

**6.0 Diseño de la Cimentación.**

Se diseñarán las cimentaciones mediante zapatas aisladas con contratrabes.

Se consideran las siguientes capacidades de terreno de acuerdo a Estudio de Mecánica de suelos.

El NAF no se encontró hasta la máxima profundidad de perforación de 45.40 metros.

La capacidad de carga del terreno es muy alta, estando el diseño gobernado por los asentamientos elásticos inmediatos que se producen por la arcilla dura.

La capacidad de carga superficial para la zona sur de la planta es de 130 t/m<sup>2</sup> para zapatas de 1 metro por 1 metro y de 300 t/m<sup>2</sup> para la zona Norte de la planta.

**CAPACIDAD DE CARGA DE SERVICIO EN ZAPATAS DE LA ZONA NORTE DE LA PLANTA**

ASENTAMIENTO EN ZAPATAS CUADRADAS DF=2.00 M			
ANCHO M	LARGO M	ASENTAMIENTO CM	CONTACTO T/M2
0.50	0.50	2.50	55.00
1.00	1.00	2.50	28.00
1.50	1.50	2.50	18.60
2.00	2.00	2.50	8.50
2.50	2.50	2.50	6.80
3.00	3.00	2.50	5.70
3.50	3.50	2.50	5.00
4.00	4.00	2.50	3.55
4.50	4.50	2.50	3.20

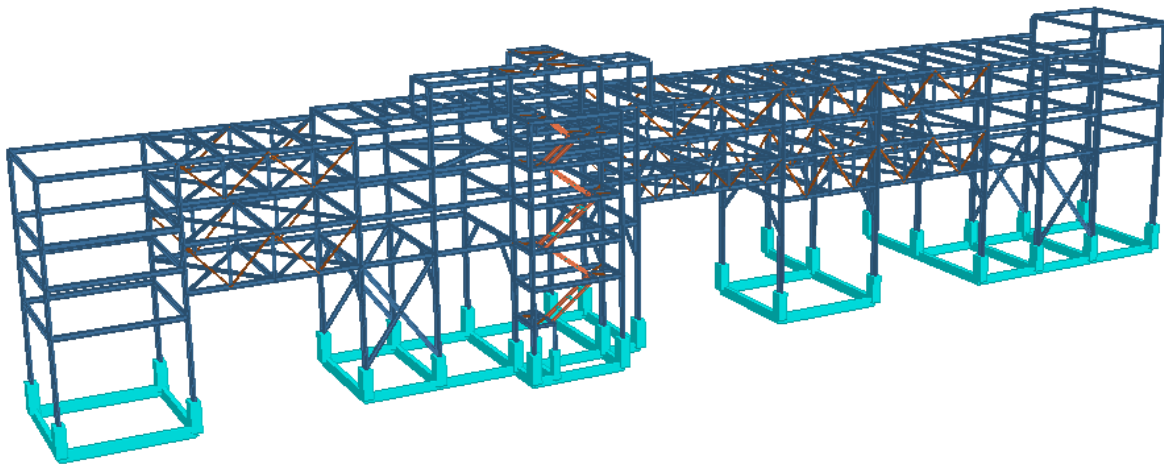
ASENTAMIENTO EN ZAPATAS CUADRADAS DF=2.00M			
ANCHO M	LARGO M	ASENTAMIENTO CM	PRESIÓN CONTACTO T/M2
0.50	0.50	5.00	110.00
1.00	1.00	5.00	56.00
1.50	1.50	5.00	37.20
2.00	2.00	5.00	17.00
2.50	2.50	5.00	13.60
3.00	3.00	5.00	11.40
3.50	3.50	5.00	10.00
4.00	4.00	5.00	7.10
4.50	4.50	5.00	6.40

**CAPACIDAD DE CARGA DE SERVICIO EN ZAPATAS DE LA ZONA SUR DE LA PLANTA**

ASENTAMIENTO EN ZAPATAS CUADRADAS DF=2.00 M			
ANCHO M	LARGO M	ASENTAMIENTO CM	PRESIÓN CONTACTO T/M2
0.50	0.50	2.50	70.00
1.00	1.00	2.50	36.00
1.50	1.50	2.50	23.00
2.00	2.00	2.50	16.00
2.50	2.50	2.50	13.00
3.00	3.00	2.50	9.90
3.50	3.50	2.50	4.20
4.00	4.00	2.50	4.00
4.50	4.50	2.50	3.50

ASENTAMIENTO EN ZAPATAS CUADRADAS DF=2.00 M			
ANCHO M	LARGO M	ASENTAMIENTO CM	PRESIÓN CONTACTO T/M2
0.50	0.50	5.00	140.00
1.00	1.00	5.00	72.00
1.50	1.50	5.00	46.00
2.00	2.00	5.00	32.00
2.50	2.50	5.00	26.00
3.00	3.00	5.00	19.80
3.50	3.50	5.00	8.40
4.00	4.00	5.00	8.00
4.50	4.50	5.00	7.00

**RACK PRINCIPAL NORTE-SUR**



*Imagen 5.7 Cimentación a base de dados y traves de liga*

Considerando que las contratrabes se llevan los momentos se tiene lo siguiente:

<b>Node</b>	<b>L/C</b>	<b>Force-X Mton</b>	<b>Force-Y Mton</b>	<b>Force-Z Mton</b>	<b>Moment-X MTon-m</b>	<b>Moment-Y MTon-m</b>	<b>Moment-Z MTon-m</b>
1095	21	0.443	47.472	-0.242	0.000	-0.000	0.000
1097	21	0.525	44.345	0.428	0.000	-0.000	0.000
1104	21	0.059	41.179	1.424	0.000	0.000	0.000

<b>Node</b>	<b>L/C</b>	<b>Force-X Mton</b>	<b>Force-Y Mton</b>	<b>Force-Z Mton</b>	<b>Moment-X MTon-m</b>	<b>Moment-Y MTon-m</b>	<b>Moment-Z MTon-m</b>
1095	21	0.443	47.472	-0.242	0.000	-0.000	0.000
1097	21	0.525	44.345	0.428	0.000	-0.000	0.000
1104	21	0.059	41.179	1.424	0.000	0.000	0.000

<b>Node</b>	<b>L/C</b>	<b>Force-X Mton</b>	<b>Force-Y Mton</b>	<b>Force-Z Mton</b>	<b>Moment-X MTon-m</b>	<b>Moment-Y MTon-m</b>	<b>Moment-Z MTon-m</b>
1095	31	0.620	66.460	-0.339	0.000	-0.000	0.000
1097	31	0.735	62.083	0.599	0.000	-0.000	0.000
1104	31	0.083	57.650	1.993	0.000	0.000	0.000
1110	31	0.131	55.837	0.993	0.000	0.000	0.000

<b>Node</b>	<b>L/C</b>	<b>Force-X Mton</b>	<b>Force-Y Mton</b>	<b>Force-Z Mton</b>	<b>Moment-X MTon-m</b>	<b>Moment-Y MTon-m</b>	<b>Moment-Z MTon-m</b>
1104	33	1.624	71.578	7.251	0.000	0.004	0.000
1095	33	1.897	66.341	5.208	0.000	0.002	0.000
1106	33	0.750	64.647	7.754	0.000	0.006	0.000
1102	33	13.878	62.953	6.590	0.000	0.004	0.000

Para la más crítica

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

## Geometría de la zapata

$L1 = 0.80 \text{ m}$   
 $L2 = 0.80 \text{ m}$   
 $L = 1.60 \text{ m}$

$B1 = 0.80 \text{ m}$   
 $B2 = 0.80 \text{ m}$   
 $B = 1.60 \text{ m}$

$C1 = 0.400 \text{ m}$   
 $C = 0.80 \text{ m}$

$D1 = 0.325 \text{ m}$   
 $D = 0.65 \text{ m}$

Area = 2.56 m<sup>2</sup>

Area dado = 0.32 m<sup>2</sup>

$I_{xx} = 0.55 \text{ m}^4$

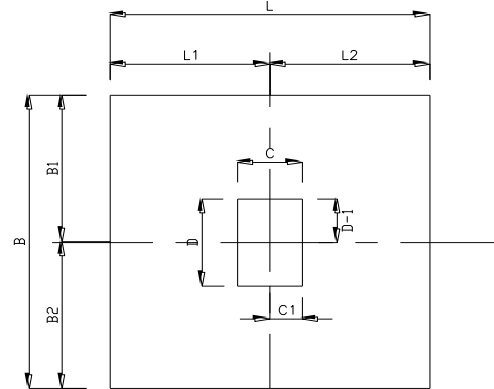
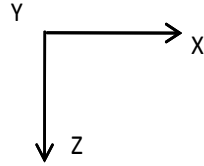
$I_{zz} = 0.55 \text{ m}^4$

$h1 = 0.45 \text{ m}$

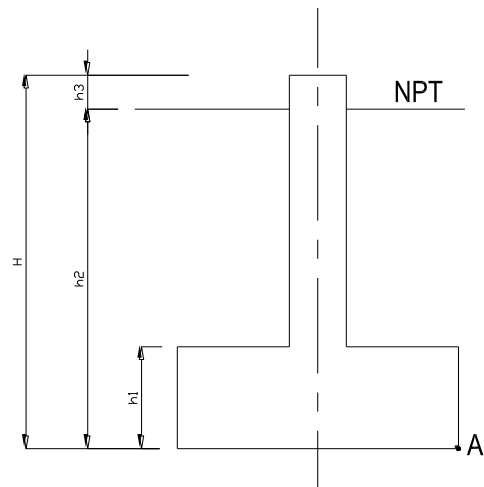
$h2 = 2.00 \text{ m}$

$h3 = 0.20 \text{ m}$

$H = 2.20 \text{ m}$



PLANTA



ELEVACION

## MATERIALES

concreto  $f'c = 250 \text{ kg/cm}^2$

acero de refuerzo  $f_y = 4200 \text{ kg/cm}^2$

Peso volumetrico del concreto  $\gamma_c = 2.4 \text{ ton/m}^3$

Peso volumetrico del relleno  $\gamma_r = 1.8 \text{ ton/m}^3$

Capacidad del terreno  $q_{adm} = 44 \text{ ton/m}^2$



## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

---

Cargas gravitacionales

### CARGAS DE SERVICIO EN LA ZAPATA

$$\begin{aligned} P &= 47.47 \text{ ton.} \\ M_x &= 0.00 \text{ t-m} \\ M_z &= 0.00 \text{ t-m} \\ V_x &= 0.44 \text{ ton.} \\ V_z &= -0.24 \text{ ton.} \end{aligned}$$

### CARGAS ULTIMAS EN LA ZAPATA

$$\begin{aligned} P &= 66.46 \text{ ton.} \\ M_x &= 0.00 \text{ t-m} \\ M_z &= 0.00 \text{ t-m} \\ V_x &= 0.62 \text{ ton.} \\ V_z &= -0.34 \text{ ton.} \end{aligned}$$

Cargas accidentales

### CARGAS DE SERVICIO EN LA ZAPATA

$$\begin{aligned} P &= 57.48 \text{ ton.} \\ M_x &= 0.00 \text{ t-m} \\ M_z &= 0.00 \text{ t-m} \\ V_x &= 1.18 \text{ ton.} \\ V_z &= 5.45 \text{ ton.} \end{aligned}$$

### CARGAS ULTIMAS EN LA ZAPATA

$$\begin{aligned} P &= 71.58 \text{ ton.} \\ M_x &= 0.00 \text{ t-m} \\ M_z &= 0.00 \text{ t-m} \\ V_x &= 1.62 \text{ ton.} \\ V_z &= 7.25 \text{ ton.} \end{aligned}$$

$$\begin{aligned} \text{Peso de la zapata } W_z &= 2.765 \text{ ton.} \\ \text{Peso del dado } W_d &= 1.934 \text{ ton.} \\ \text{Peso del relleno } W_r &= 5.692 \text{ ton.} \\ \hline & \mathbf{10.391 \text{ ton.}} \end{aligned}$$

## REVISION POR VOLTEO

Calculo del momento resistente en direccion X

$$M_r = W_z (B/2) + W_d B/2 + W_r (B/2) + P B/2 = 54.298 \text{ t-m}$$

$$\frac{M_r}{M_{act}} = 5429824 > 2 \text{ NO HAY VOLTEO}$$

Calculo del momento resistente en direccion Z

$$M_r = W_z (L/2) + W_d L/2 + W_r (L/2) + P L/2 = 54.298 \text{ t-m}$$

$$\frac{M_r}{M_{act}} = 5429824 > 2 \text{ NO HAY VOLTEO}$$

# DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

---

## REVISION POR CAPACIDADES DE TERRENO

Calculo de excentricidades

$$e = \frac{Mr - Mact}{R}$$

$$R = 67.873 \text{ ton.}$$

$$ex = 0.80 \text{ m} \quad \text{En direccion X}$$

$$ez = 0.80 \text{ m} \quad \text{En direccion z}$$

$$L/2 - ex = 0 \text{ m} < L/6 = 0.27 \text{ m}$$

$$B/2 - ez = 0 \text{ m} < B/6 = 0.27 \text{ m}$$

LA RESULTANTE CAE EN EL TERCIO MEDIO

LA RESULTANTE CAE EN EL TERCIO MEDIO

Calculo de reacciones en el terreno:

$$q = \frac{P}{A} \quad \text{Para cargas gravitacionales}$$

$$q = \frac{P}{A} + \frac{Mx}{Ixx} (B/2) + \frac{Mz}{Izz} (L/2) \quad \text{Para cargas accidentales}$$

Para cargas de servicio gravitacionales:

$$q = 22.603 \text{ t/m}^2$$

Para cargas ultimas gravitacionales:

$$q = 30.020 \text{ t/m}^2$$

Para cargas de servicio accidentales

$$q1 = 26.513 \text{ t/m}^2$$

$$q2 = 26.513 \text{ t/m}^2$$

$$q3 = 26.513 \text{ t/m}^2$$

$$q4 = 26.513 \text{ t/m}^2$$

Para cargas ultimas accidentales

$$q1 = 32.019 \text{ t/m}^2$$

$$q2 = 32.019 \text{ t/m}^2$$

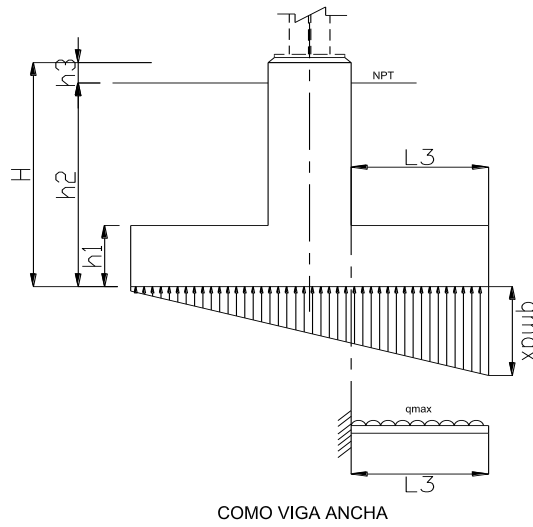
$$q3 = 32.019 \text{ t/m}^2$$

$$q4 = 32.019 \text{ t/m}^2$$

Todas las cargas actuantes sobre del terreno son menores a la capacidad admisible de carga del terreno.

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

Revisión de peralte de zapata:



$$\begin{aligned}
 h1 &= 0.45 \text{ m} \\
 bw &= 100 \text{ cm} \\
 d &= 38 \text{ cm} \\
 L3 &= 0.475 \text{ m} \\
 q_{max} &= 32.02 \text{ t/m}^2 \\
 f'c &= 250 \text{ kg/cm}^2 \\
 fy &= 4200 \text{ kg/cm}^2 \\
 M_{act} &= q L3^2 / 2 = 3.612 \text{ t-m} = 361215 \text{ kg-cm} \\
 V_{act} &= q * L = 15.209 \text{ ton.} = 15209 \text{ kg}
 \end{aligned}$$

REVISIÓN POR CORTANTE

$$V_c = \phi (0,53 \lambda \sqrt{f'c} bw d) = 23883 \text{ kg} > V_{act} \quad \text{SE ACEPTA EL PERALTE}$$

$$\begin{aligned}
 \lambda &= 1 \\
 \phi &= 0.75
 \end{aligned}$$

REVISION POR FLEXION

$$\phi M_n = \phi A_s f_y d \left( 1 - 0.59 \frac{A_s f_y}{b d f_c'} \right) \geq M_u,$$

$$M_u - \phi A_s f_y d \left( 1 - 0.59 \frac{A_s f_y}{b d f_c'} \right) \leq 0.$$

$$\phi = 0.85$$

$$\rho_{min} = \frac{0.8 \sqrt{f_c'}}{f_y} \geq \frac{14}{f_y}.$$

$$\rho_{min} = \frac{0.8 \sqrt{f_c'}}{f_y} = 0.0030$$

$$\frac{14}{f_y} = 0.0033 \quad \text{Este es el porcentaje minimo a flexion}$$

Se propone porcentaje minimo como primera opción:

$$A_{smin} = \rho b w d = 12.67 \text{ cm}^2 / \text{m}$$

$$\phi M_n = 1661585.4 \text{ kg-cm} > M_{act} \quad \text{SE ACEPTA EL ARMADO}$$

Se propone Vs # 6 @ 20 Para cara inferior

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

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### REVISION POR PENETRACION

Esfuerzo cortante permisible de acuerdo al ACI-318-08 cap. 11.12.2

$$V_c = 0.17 \left( 1 + \frac{2}{\beta} \right) \sqrt{f'_c}$$

b= lado largo / lado corto= 80/65 = 1.23  
 $V_c = 2.21 \text{ Mpa} = 22.52 \text{ kg/cm}^2$  ecuación 11-

$$V_c = 0.083 \left( \frac{\alpha_s d}{b_0} + 2 \right) \sqrt{f'_c}$$

Para el dado:

b= 0.65 m L= 0.80 m  
 as= 40 columnas interiores  
 d= 0.43 m  
 b0= 4.62 m Perimetro  
 Ao= 1.99 m<sup>2</sup>  
 $V_c = 2.35 \text{ Mpa} = 24.0 \text{ kg/cm}^2$

$$V_c = 0.33 \sqrt{f'_c}$$

$V_c = 1.63 \text{ Mpa} = 16.7 \text{ kg/cm}^2$  ecuación 11-33

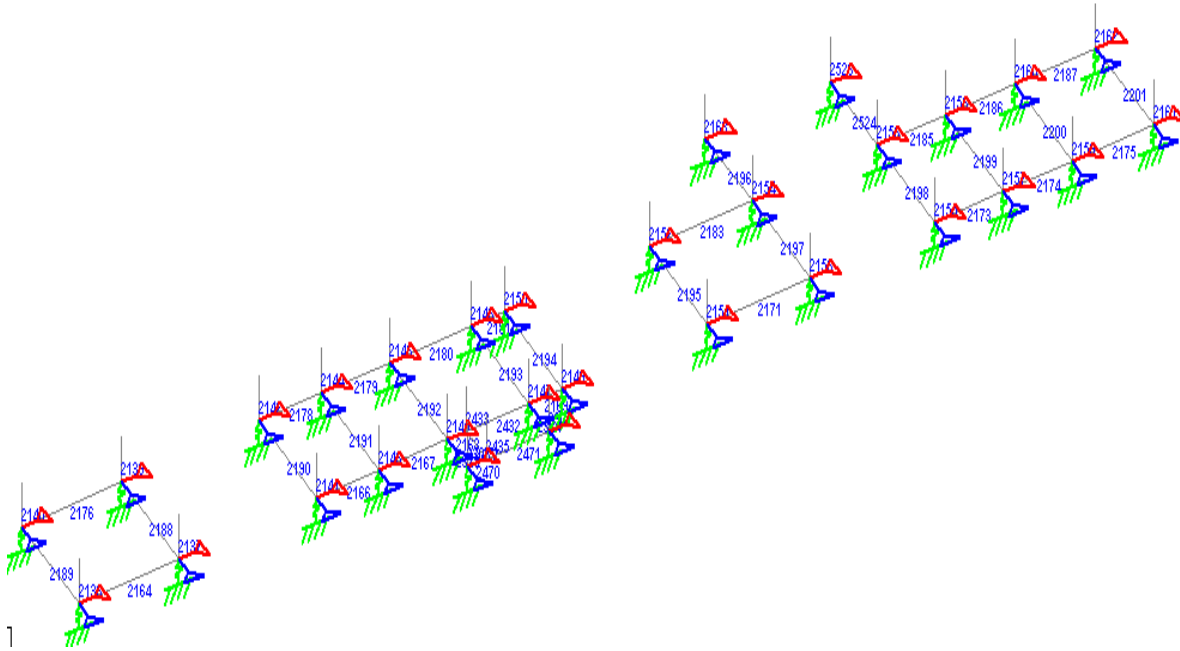
$V_c \text{ min} = 16.65 \text{ kg/cm}^2$

$\phi = 0.75$  Cortante y torsión

$\phi V_c = 12.49 \text{ kg/cm}^2$

$V_u = P_u / A_o = 3.60 \text{ kg/cm}^2 < V_c \text{ min.}$

- **Diseño traves de Contratraves**



Las Contratraves se diseñan considerando que estas absorben todos los momentos de volteo que trasmite la superestructura a la cimentación. Las Traves de liga se dividen en los siguientes grupos:

- Contratraves transversales “CT-1”

De igual forma que para la superestructura se determina aquella viga que presenta las sollicitaciones más desfavorables en este caso el miembro número 2190

Beam	L/C	INode	Axial Force Mton	Shear-Y Mton	Shear-Z Mton	Torsion MTon-m	Moment-Y MTon-m	Moment-Z MTon-m
2190	33	1100	0.000	11.506	0.000	0.639	0.000	40.125
2190	33	1099	0.000	11.570	0.000	0.481	0.000	32.549
2188	33	1097	0.000	9.953	0.000	0.498	0.000	32.525
2192	33	1104	0.000	9.392	0.000	0.288	0.000	30.988
2193	33	1106	0.000	9.246	0.000	1.441	0.000	30.611
2190	23	1100	0.000	8.502	0.000	0.467	0.000	29.093
2191	33	1102	0.000	8.492	0.000	0.376	0.000	28.081
2188	33	1095	0.000	9.636	0.000	0.454	0.000	27.720
2192	33	1103	0.000	6.886	0.000	0.927	0.000	24.777

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

BEAM NO. 2190 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	63.	4 - 25MM	0.	8000.	YES YES
-----					
CRITICAL POS MOMENT= 368.00 KN-MET AT 4000.MM, LOAD 34					
REQD STEEL= 1703.MM2, RHO=0.0089, RHOMX=0.0190 RHOMN=0.0033					
MAX/MIN/ACTUAL BAR SPACING= 254./ 50./ 58. MMS					
REQD. DEVELOPMENT LENGTH = 1632. MMS					
-----					

Cracked Moment of Inertia Iz at above location = 397222.9 cm<sup>4</sup>

2	637.	4 - 25MM	0.	8000.	YES YES
STAAD SPACE					-- PAGE NO. 152
-----					
CRITICAL NEG MOMENT= 395.89 KN-MET AT 8000.MM, LOAD 34					
REQD STEEL= 1847.MM2, RHO=0.0097, RHOMX=0.0190 RHOMN=0.0033					
MAX/MIN/ACTUAL BAR SPACING= 254./ 50./ 58. MMS					
REQD. DEVELOPMENT LENGTH = 1632. MMS					
-----					

Cracked Moment of Inertia Iz at above location = 397222.9 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	1475./ 1841.	322./ 393.	34/ 33
667.	1546./ 1767.	336./ 379.	34/ 33
1333.	1605./ 1706.	348./ 368.	34/ 33
2000.	1651./ 1660.	357./ 358.	34/ 33
2667.	1684./ 1627.	363./ 352.	34/ 33
3333.	1703./ 1608.	367./ 348.	34/ 33
4000.	1709./ 1603.	368./ 347.	34/ 33
4667.	1701./ 1610.	366./ 349.	34/ 33
5333.	1680./ 1631.	362./ 353.	34/ 33
6000.	1645./ 1666.	355./ 360.	34/ 33
6667.	1597./ 1715.	346./ 369.	34/ 33
7333.	1535./ 1777.	334./ 381.	34/ 33
8000.	1462./ 1854.	319./ 396.	33/ 34

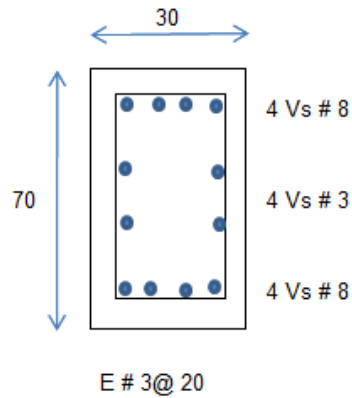
B E A M N O. 2190 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 109.06 KNS Vc= 154.77 KNS Vs= 0.00 KNS  
 Tu= 5.40 KN-MET Tc= 6.8 KN-MET Ts= 0.0 KN-MET LOAD 33  
 NO STIRRUPS ARE REQUIRED FOR TORSION.

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

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El Croquis 7 muestra el armado para las Contratraves “CT-1”.



*Croquis 7 Armado de Contratrabe “CT-1”*

- Contratraves Longitudinales “CT-2”

Estas contratraves presentan solicitaciones menores que las “CT-1” por lo que su acero de refuerzo es menor como se puede ver a continuación.

Beam	L/C	INode	Axial Force Mton	Shear-Y Mton	Shear-Z Mton	Torsion MTon-m	Moment-Y MTon-m	Moment-Z MTon-m
2179	33	1102	0.000	10.598	0.000	0.406	0.000	35.124
2167	33	1101	0.000	9.889	0.000	0.836	0.000	28.843
2185	33	1116	0.000	8.961	0.000	0.580	0.000	26.514
2179	23	1102	0.000	7.997	0.000	0.296	0.000	26.069
2167	23	1101	0.000	7.379	0.000	0.649	0.000	21.119

Checando la más crítica:



## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

BEAM NO. 2179 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	61.	4 - 20MM	0.	5500.	YES YES
-----					
CRITICAL POS MOMENT= 211.57 KN-MET AT 458.MM, LOAD 34					
REQD STEEL= 1133.MM2, RHO=0.0070, RHOMX=0.0190 RHOMN=0.0033					
MAX/MIN/ACTUAL BAR SPACING= 254./ 45./ 59. MMS					
REQD. DEVELOPMENT LENGTH = 1015. MMS					
-----					

Cracked Moment of Inertia Iz at above location = 195002.9 cm<sup>4</sup>

2	533.	3 - 32MM	0.	5500.	YES YES
-----					
CRITICAL NEG MOMENT= 344.45 KN-MET AT 0.MM, LOAD 33					
REQD STEEL= 1978.MM2, RHO=0.0124, RHOMX=0.0190 RHOMN=0.0033					
MAX/MIN/ACTUAL BAR SPACING= 254./ 64./ 83. MMS					
REQD. DEVELOPMENT LENGTH = 1858. MMS					
-----					

Cracked Moment of Inertia Iz at above location = 307656.4 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	1138./ 1969.	211./ 344.	34/ 33
458.	1143./ 1796.	212./ 318.	34/ 33
917.	1142./ 1634.	211./ 293.	34/ 33
1375.	1134./ 1483.	210./ 268.	34/ 33
1833.	1121./ 1341.	208./ 245.	34/ 33
2292.	1101./ 1209.	204./ 223.	34/ 33
2750.	1075./ 1085.	200./ 202.	34/ 33
3208.	1042./ 969.	194./ 182.	34/ 33
3667.	1004./ 862.	188./ 163.	34/ 33
4125.	960./ 762.	180./ 145.	34/ 33
4583.	911./ 669.	171./ 128.	34/ 33
5042.	855./ 583.	161./ 112.	34/ 33
5500.	794./ 504.	150./ 97.	33/ 34

B E A M N O. 2179 D E S I G N R E S U L T S - S H E A R

STAAD SPACE -- PAGE NO. 138  
 AT START SUPPORT - Vu= 101.20 KNS Vc= 129.95 KNS Vs= 4.99 KNS  
 Tu= 3.38 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 33  
 NO STIRRUPS ARE REQUIRED FOR TORSION

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

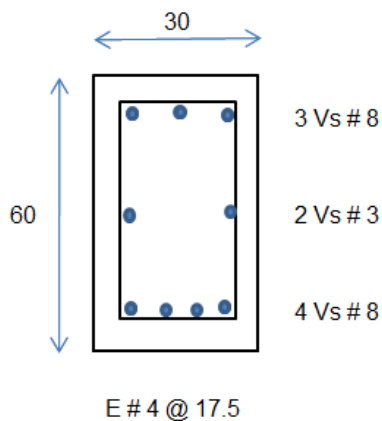
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El acero de refuerzo longitudinal solicitado en el lecho inferior es de  $11.43 \text{ cm}^2$  y que se alcanza con 3 var #8 =  $15.21 \text{ cm}^2$ .

El acero de refuerzo longitudinal solicitado en el lecho superior es de  $19.69 \text{ cm}^2$  y que se alcanza con 4 var #8 =  $20.28 \text{ cm}^2$ .

Para el refuerzo por cortante se requieren estribos del #3 de dos ramas con una separación de  $s = 27 \text{ cm}$ .

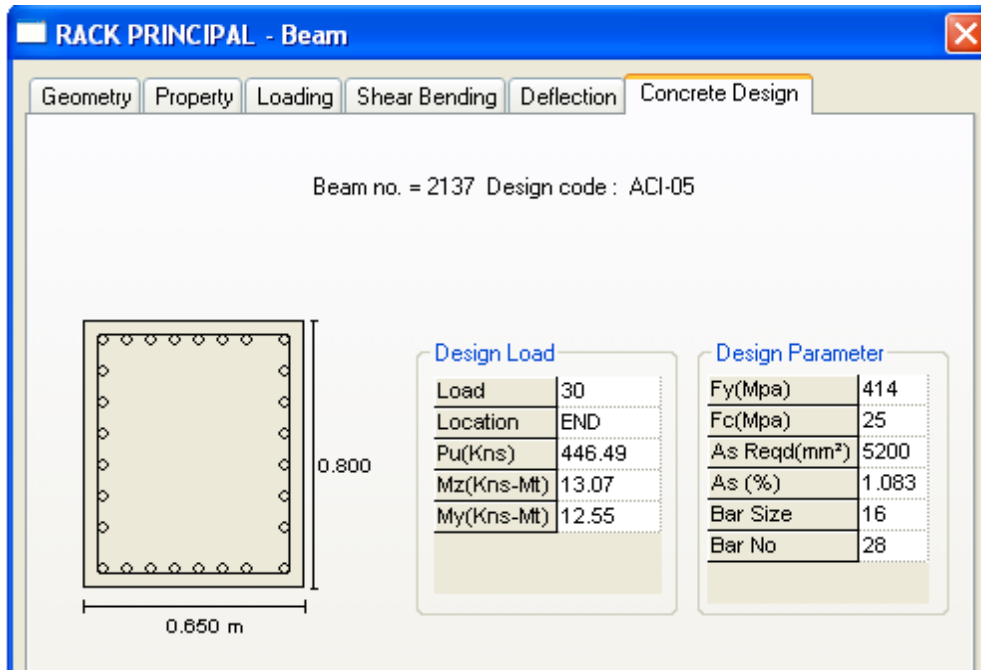
El Croquis 8 muestra el armado para las Contrtrabes "CT-2".



*Croquis 8 Armado de Contratrabe "CT-2"*

- Diseño traveses de Dados:

De igual forma que para las contratrabes se determina aquel elemento que presenta las solicitaciones más desfavorables en este caso el miembro número 2137



Acero de refuerzo en dados:

$$A_s = 14 V_s \# 8 = 70.94 \text{ cm}^2$$

Se colocarán estribos del # 3 @ 15

## 7.0 Procedimiento Constructivo.

La primera etapa para la construcción de un Rack es la cimentación, donde los dados y las contratraves se construyen con un habilitando del acero de refuerzo en forma continua, se deben dejar ahogadas las anclas necesarias para recibir la estructura metálica ver Imagen 7.1 y el colado de los elementos de concreto se hace de forma monolítico ver Imagen 7.2.



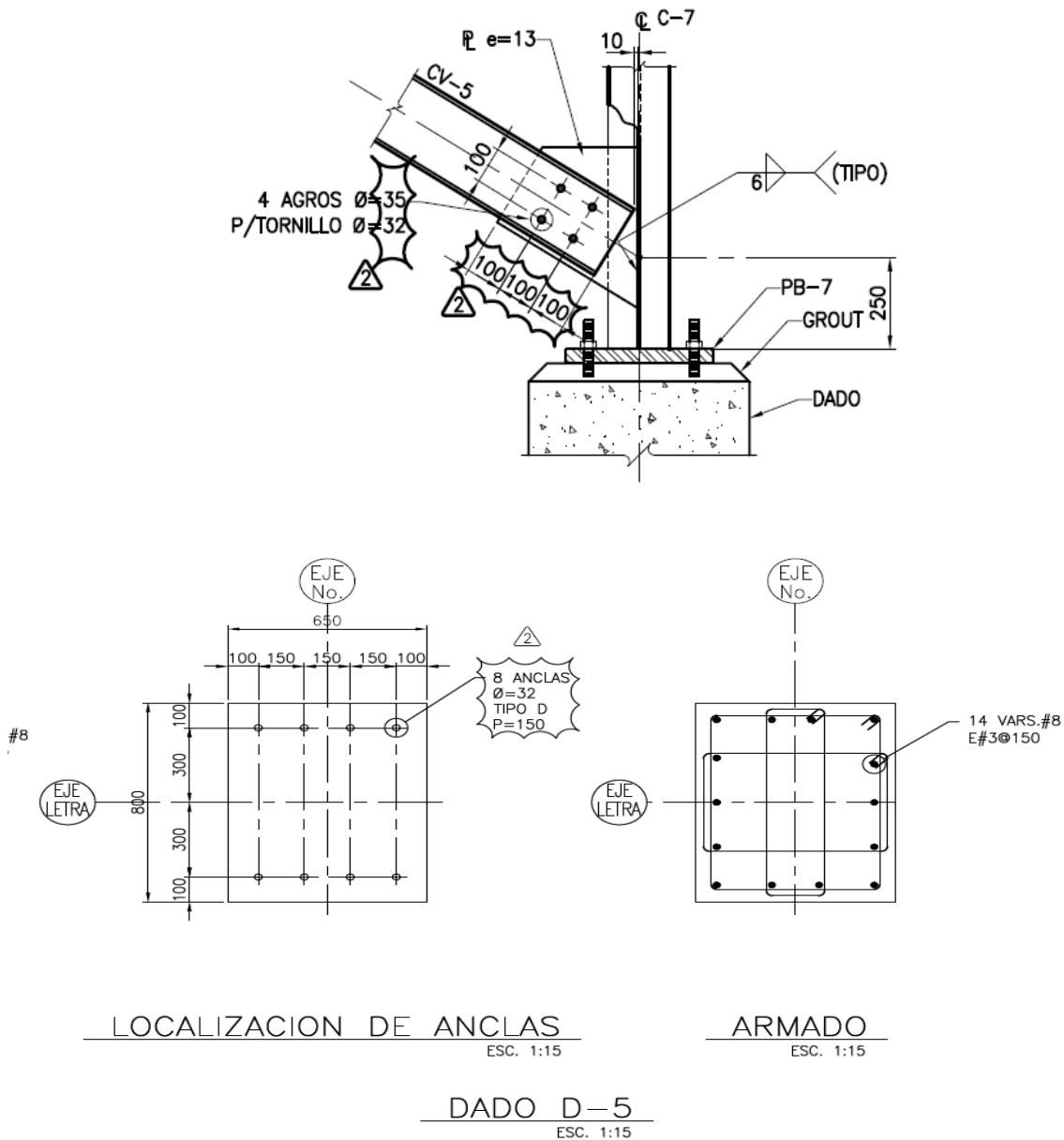
*Imagen 7.1 Habilitado de refuerzo en traves de liga y dados para la cimentación de un Rack*



*Imagen 7.2 Colado monolítico de la cimentación para un Rack de Tuberías*

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

En la segunda etapa se construye la superestructura, la cual se conforma de marcos transversales y vigas longitudinales de acero estructural prefabricado en taller. El taller es el encargado de cortar y preparar cada uno de los elementos para su instalación, es responsabilidad del taller hacer llegar al cliente los planos de taller para su aprobación antes de trasladar los elementos estructurales al sitio. Una vez que estos elementos se encuentran cortados y listos se integraran a la cimentación mediante un procedimiento de izaje, empotrando las columnas con placa base en los dados con sus anclajes, el Croquis 9 muestra a detalle el resultado de este.

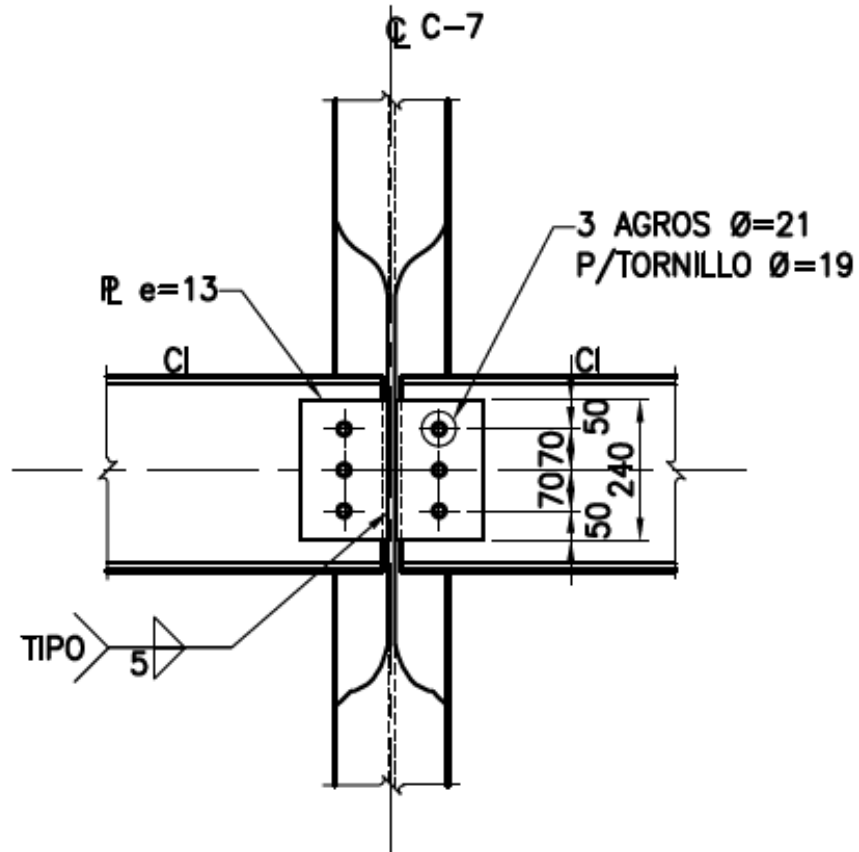


Croquis 9 Detalle de empotramiento de columna C-1 en dados

**DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES**

---

Para ligar los marcos transversales se utilizan las vigas longitudinales “Puntales”, por lo que, para lograr una continuidad de la estructura se debe contemplar una conexión a cortante mostrada en Croquis 11.



*Croquis 11 Conexión a cortantes puntales longitudinales a columna*

## DISEÑO ESTRUCTURAL DE UN RACK DE ACERO ESTRUCTURAL PARA PLANTAS INDUSTRIALES

---

Podemos ver estos procedimientos para el montaje de la superestructura del Rack en las siguientes imágenes.



*Imagen 7.3 Montaje de marcos transversales de Rack de Tuberías*



*Imagen 7.4 Almacenaje de columnas de Rack de Tuberías*

## **8.0 Conclusiones.**

En conclusión, el análisis, diseño y construcción de un Rack de Tuberías con lleva el estudio de aspectos que no son propios de una estructura común. Con esta tesis se está proporcionando una herramienta que auxilia al ingeniero civil en el detallado para su análisis y diseño que influye de forma importante en los costos y tiempos de ejecución de obra.



## Bibliografía

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## **Anexos**

- 1.0 Cargas de tuberías
- 2.0 Cargas de charolas eléctricas
- 3.0 Corrida de modelo matemático (STAD Pro)
- 4.0 Planos estructurales

Todos los anexos se encuentran el disco DVD anexo.

# ANEXOS

## 1.0 CARGAS DE TUBERIAS

### CARGAS SOBRE RACK DE TUBERIAS PARA OBRA CIVIL

Doc. N°:

Revisión: A4 Fecha: 14/10/2011

Realizado	Comprobado	Aprobado	Aprobado (PEAP)
		-	-

PARA REVISIÓN INTERNA (IDC).....   
PARA DISEÑO / COMENTARIOS CFE (IFD).....   
PARA PETICIÓN DE OFERTA (IFB).....   
PARA ORDEN DE COMPRA (IPO) .....

PARA CONSTRUCCIÓN .....   
PARA INFORMACIÓN (INF) .....   
SEGÚN CONSTRUIDO (ASB) .....   
DOCUMENTO CANCELADO (NULL).....

	<b>CARGAS SOBRE RACK DE TUBERIAS PARA OBRA CIVIL</b>	<b>Doc. N°:</b> <b>Rev. A4 Fecha: 14/10/2011</b> <b>Pág. 2 de 7</b>
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**REGISTRO DE CAMBIOS**

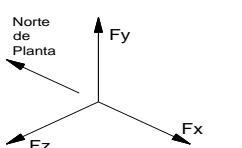
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A2	2011-04-15	TODAS	Añadidas cargas Vapor Rec.Caliente y vapor de baja presión.
A3	2011-07-21	TODAS	Modificadas coordenadas Cargas actualizadas
A4	2011-10-21	TODAS	Cargas actualizadas

**RACK DE TUBERIAS**  
**HOJAS DE CARGAS DE TUBERIAS**

Doc.  
**Cargas de tuberías**  
 Rev. A4 Fecha: 14-10-2011

Página 3 de 7

<b>CARGAS SOBRE RACK</b>																												PLANOS DE REFERENCIA				
LÍNEAS	SOPORTE	115500grvc2/N.165500grv			E.-337.300 / TOS 1215.200			E.-331.300 / TOS 1215.200			E.-325.300 / TOS 1215.200			N.-122.000/ TOS 1213.700			N.-127.500/ TOS 1213.700			N.-133.000/ TOS 1213.700			E.-325.300 / Apoyo lira			N.-138.000 / Apoyo lira			Doc. Nº: xxxxx-xxxx-xxx, Racks			
		FUERZAS (N)	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy				
VAPOR PRINCIPAL DE CALDERA GVR C2 A TURBINA	HIDRAULICA																															
	TÉRMICO			-10607																												
	PESO																															
	SISMO																															
	VIENTO																															
VAPOR REC. FRIO DE TURBINA A CALDERA GVR C2	HIDRAULICA			-48368																												
	TÉRMICO																															
	PESO																															
	SISMO																															
	VIENTO																															
VAPOR REC. CALIENTE DE CALDERA GVR C2 A TURBINA	HIDRAULICA			-17139																												
	TÉRMICO																															
	PESO																															
	SISMO																															
	VIENTO																															
VAPOR REC. CALIENTE DE CALDERA GVR C1 A TURBINA	HIDRAULICA			-26080																												
	TÉRMICO																															
	PESO																															
	SISMO																															
	VIENTO																															



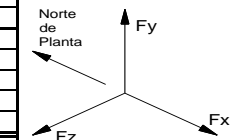
NOTAS

- Localización de los apoyos de acuerdo con los planos de referencia de tuberías
- Cargas para tuberías  $\Phi \geq 12"$
- Cargas para bandejas de tuberías  $\Phi < 12"$  se considerará 150 kg/m<sup>2</sup>





CARGAS SOBRE RACK																												PLANOS DE REFERENCIA					
LÍNEAS	SOPORTE	N.165.500/TOS 1215.200			E.355.300/TOS 121.440			E.349.300 /TOS 1212.440			E.343.440/TOS 1212.440			N.-337.300 /TOS 1212.440			Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz					
		FUERZAS (N)	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy	Fz	Fx	Fy													Fz				
ASPIRACION BOMBAS ALTA PRESION GVR C1	HIDRAULICA		-12000			-10000		-6000			-13000		-3000																				
	TÉRMICO	5000	-4000	1000	-9000	-4000	-7000	7000	5000	5000	5000	-28000	3000	-3000																			
	PESO		-10000			-11000		-5000			-10000		-8000																				
	SISMO	±1000	±1500	±1000																													
	VIENTO	±6000	±400	±8000	±3000			±4000	±600	±9000																							
ASPIRACION BOMBAS ALTA PRESION GVR C1	HIDRAULICA																																
	TÉRMICO																																
	PESO																																
	SISMO																																
	VIENTO																																
ASPIRACION BOMBAS ALTA PRESION GVR C2	HIDRAULICA		-12000			-10000		-6000			-13000		-3000																				
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	VIENTO	±6000	±400	±8000	±3000			±4000	±600	±9000																							
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	TÉRMICO																																
	PESO																																
	SISMO																																
	VIENTO																																



Doc. Nº: xxxxx-xxxx-xxx, Racks

SISTEMA DE COORDENADAS

NOTAS

- Localización de los apoyos de acuerdo con los planos de referencia de tuberías
- Cargas para tuberías  $\Phi \geq 12"$
- Cargas para bandejas de tuberías  $\Phi < 12"$  se considerará 150 kg/m2



RACK DE TUBERIAS

HOJAS DE CARGAS DE TUBERIAS

CARGAS SOBRE PLATAFORMA PARA VALVULAS

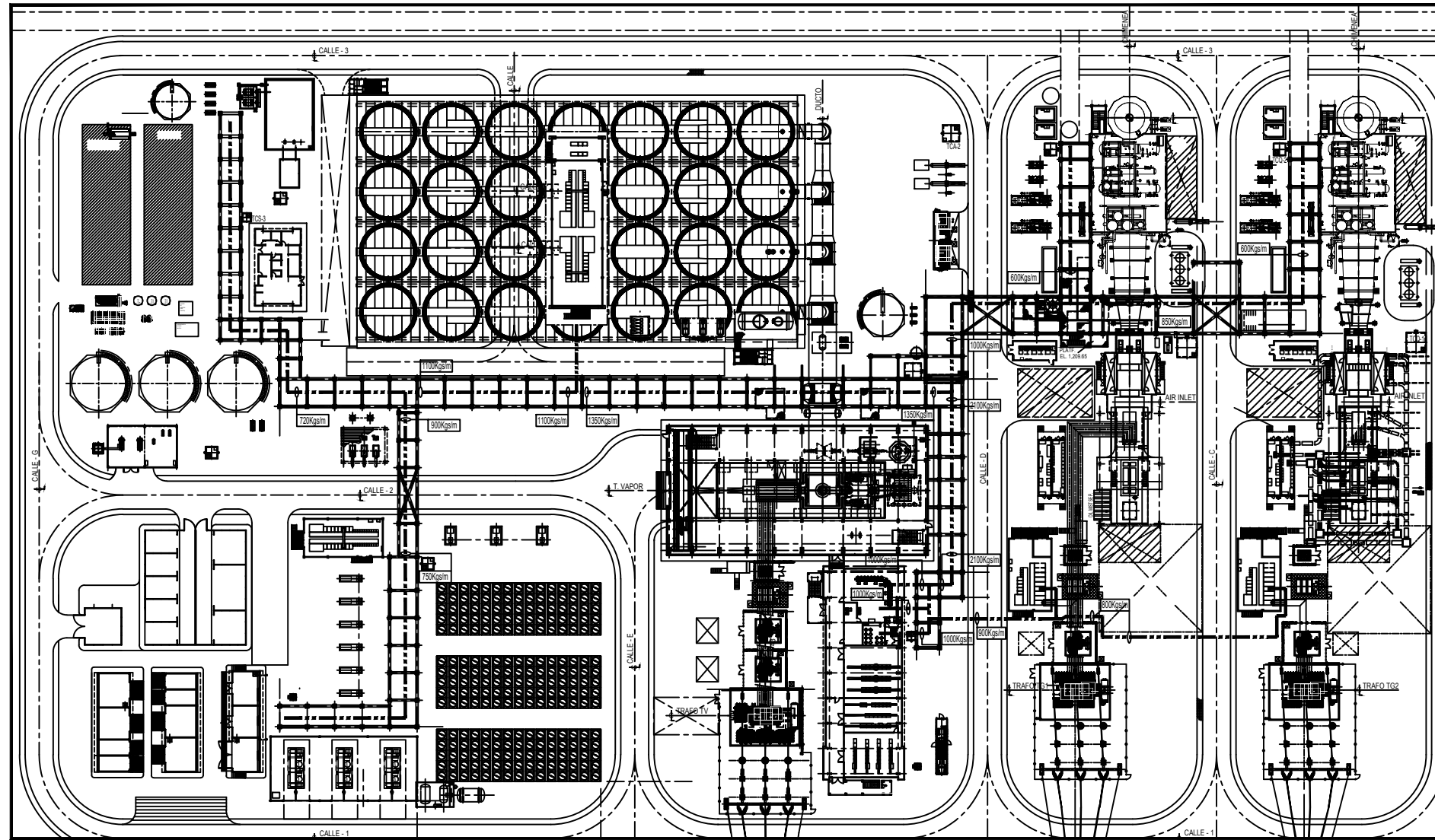
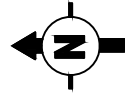
Doc.  
Cargas de tuberías  
Rev. A4 Fecha: 14-10-2011

Página 7 de 7

CARGAS SOBRE RACK																										PLANOS DE REFERENCIA					
LÍNEAS	SOPORTE FUERZAS (N)	Soporte nº 3 Amortig.			Soporte nº 4 muelle			Soporte nº 5 colgante			Soporte nº 6 Amortig.			Soporte nº 7 Amortig.			Soporte nº 8 Amortig.			Soporte nº 9 Amortig.			Soporte nº 10 colgante			Soporte nº 11 muelle			Doc. Nº: xxxxx-xxxx-xxx, Racks		
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POR PRINCIPAL	HIDRAULICA																														
	TERMICO																														
	PESO																														
	SISMO																														
	VIENTO																														
VAPOR REC. FRIO DE TURBINA CALDERA	HIDRAULICA																														
	TERMICO																														
	PESO																														
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	VIENTO																														
REC. CALIENTE DE CALDERA	HIDRAULICA																														
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VAPOR REC. CALIENTE DE CALDERA	HIDRAULICA																														
	TERMICO																														
	PESO																														
	SISMO																														
	VIENTO																														
LÍNEAS	SOPORTE FUERZAS (N)	Soporte nº 12 Amortig.			Soporte nº 16 Amortig.			Soporte nº 16 muelle			Soporte nº 17 Amortig.			Soporte nº 21 muelle			Soporte nº 25 muelle			Soporte nº 25 amortig.											
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POR PRINCIPAL	HIDRAULICA																														
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REC. CALIENTE DE CALDERA	HIDRAULICA																														
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VAPOR REC. BAJA PRESION DE CALDERA	HIDRAULICA																														
	TERMICO																														
	PESO																														
	SISMO																														
	VIENTO																														
LÍNEAS	SOPORTE FUERZAS (N)	Soporte nº 26 restr.mec.			Soporte nº 27 amortig			Soporte nº 27 restr.mec.																							
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POR PRINCIPAL	HIDRAULICA																														
	TERMICO																														
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	SISMO																														
	VIENTO																														
VAPOR REC. BAJA PRESION DE CALDERA	HIDRAULICA																														
	TERMICO																														
	PESO																														
	SISMO																														
	VIENTO																														

NOTAS

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- Cargas para tuberías  $\Phi \geq 12"$
- Cargas para bandejas de tuberías  $\Phi < 12"$  se considerará 150 kg/m2



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  - 5  TAL COMO SE CONSTRUYO

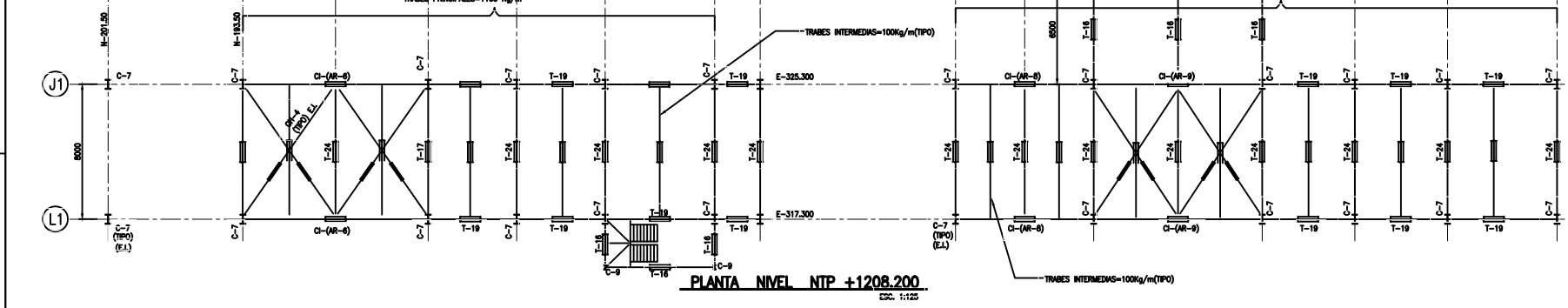
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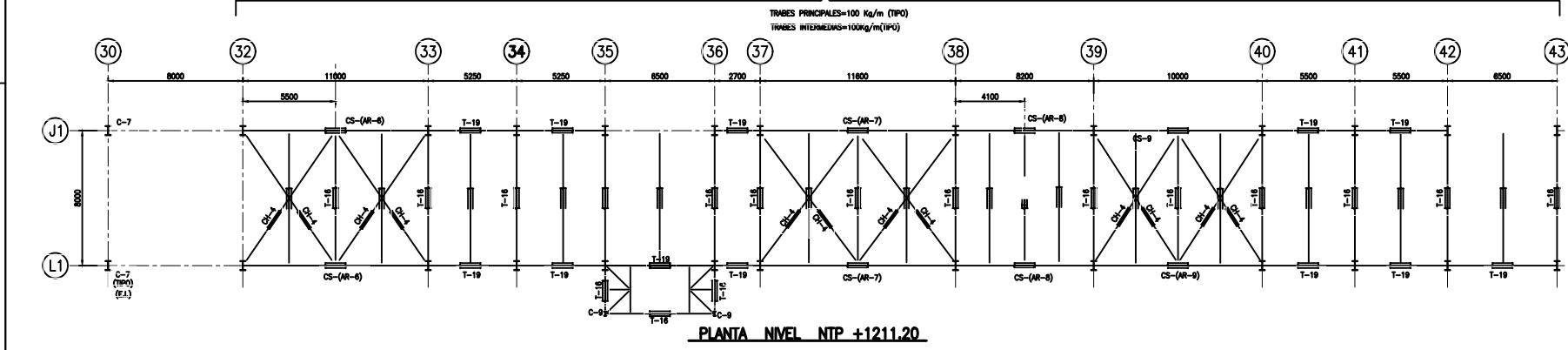
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REVISIONES			
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TITULO: ARREGLO GENERAL DE LA CENTRAL PLANTA			
APROBADO:		TITULO: CARGAS DE CHAROLAS POR LOS RACK'S	
CONPROBADO:		ESCALA: 1:400	
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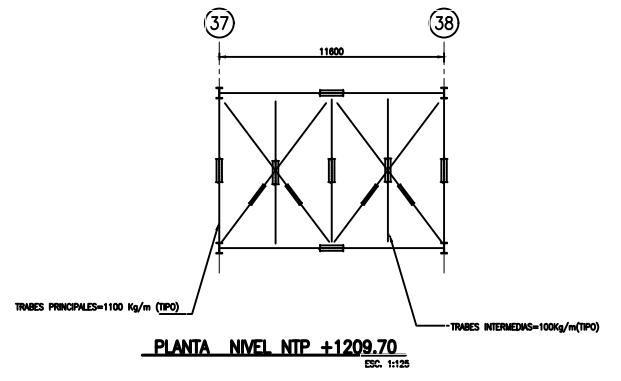
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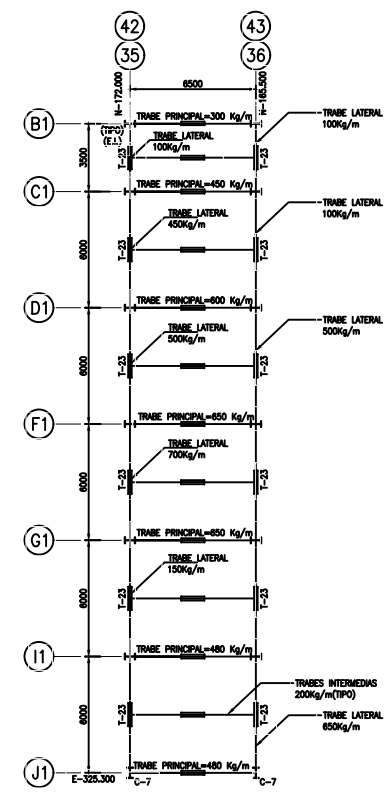
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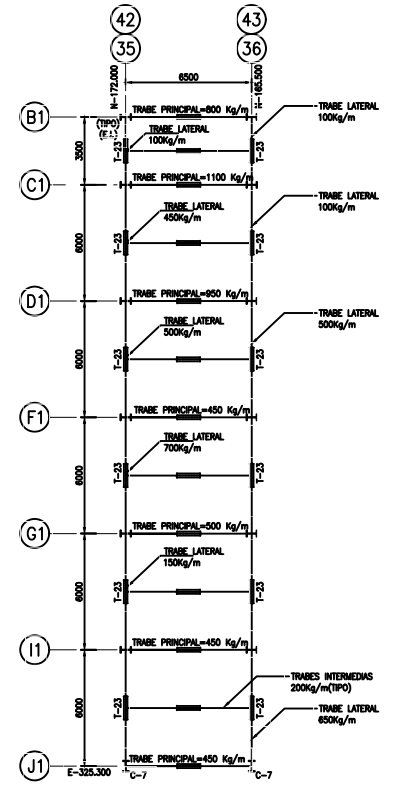
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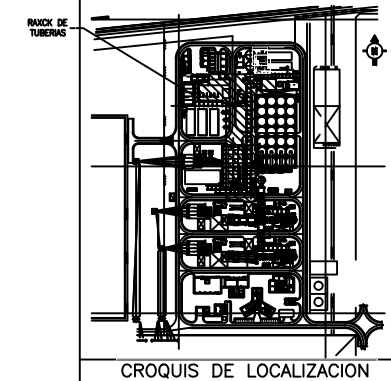
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ESC. 1:125



PLANTAS NIVELES NTP +1209.70  
ESC. 1:125



PLANTAS NIVELES NTP +1212.45  
ESC. 1:125



CROQUIS DE LOCALIZACION

- NOTAS:**
- 1.- ESTE PLANO ES SOLO PARA REVISAR CARGAS DE TUBERIAS DE SERVICIOS SOBRE LOS MARCOS DEL RACK.
  - 2.- DIMENSIONES EN MILIMETROS.
  - 3.- NIVELES Y COORDENADAS EN METROS.

**PLANOS DE REFERENCIA**  
CFE-PM27005-SRJT-PM-0141 DISPOSICION GENERAL DEL RACK DE TUBERIAS.

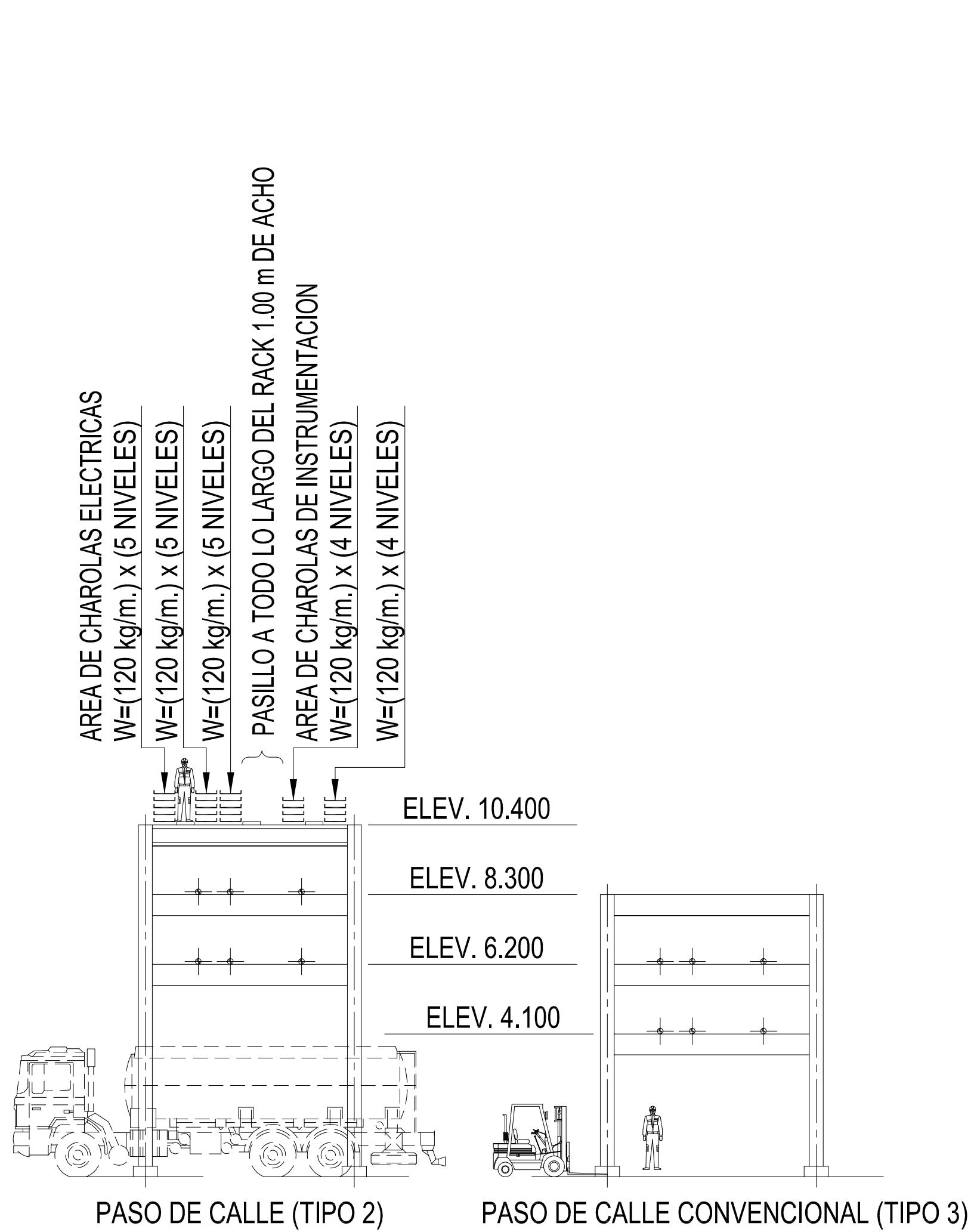
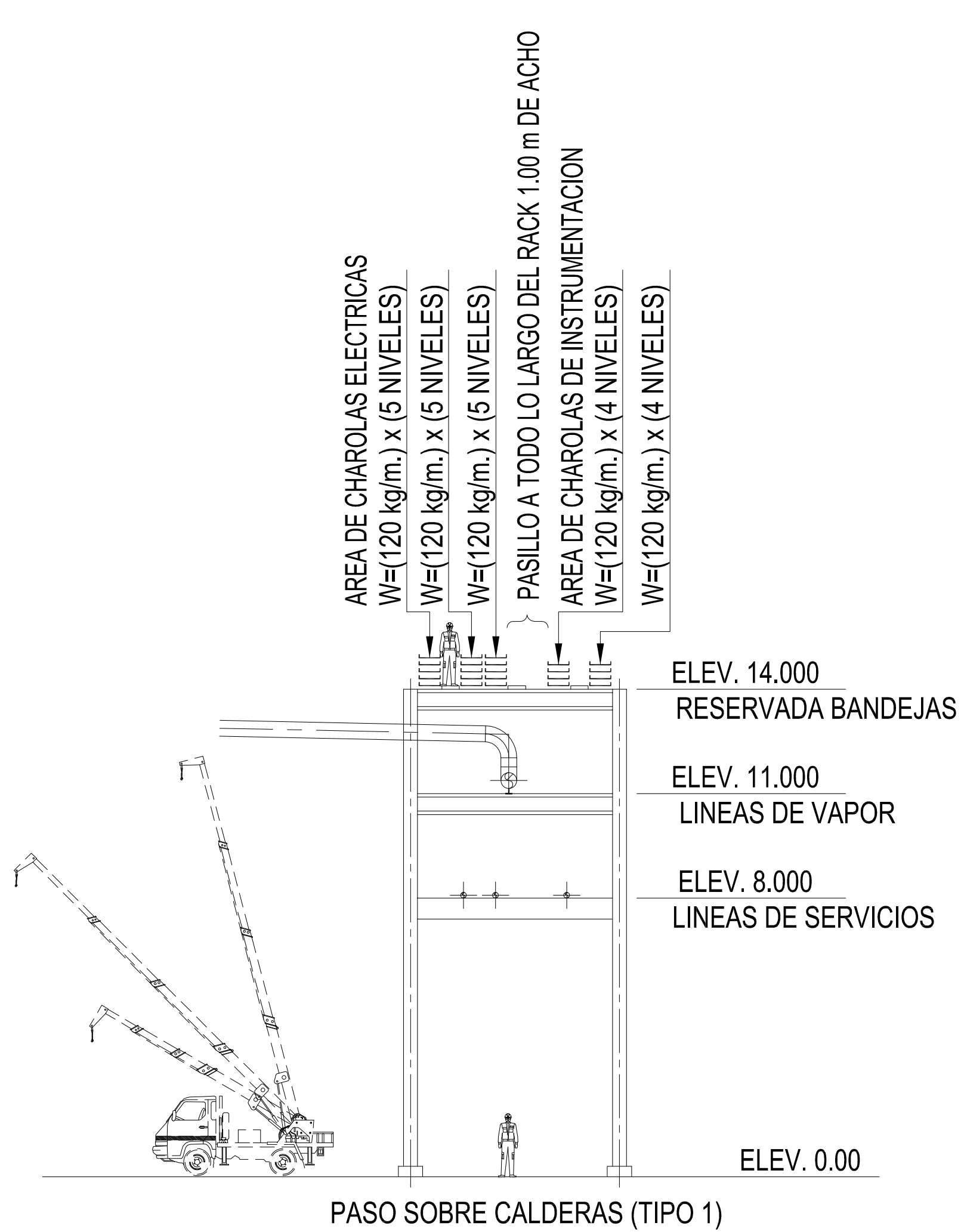
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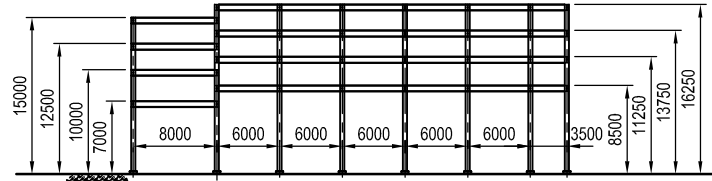
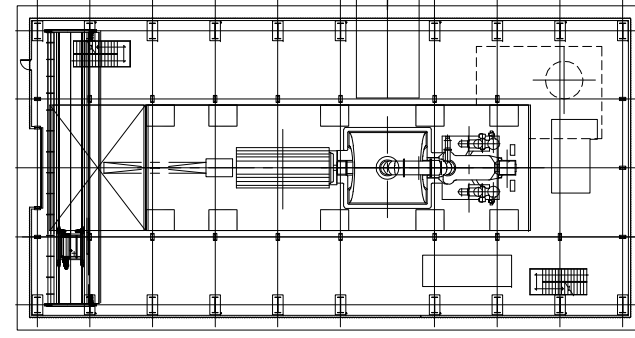
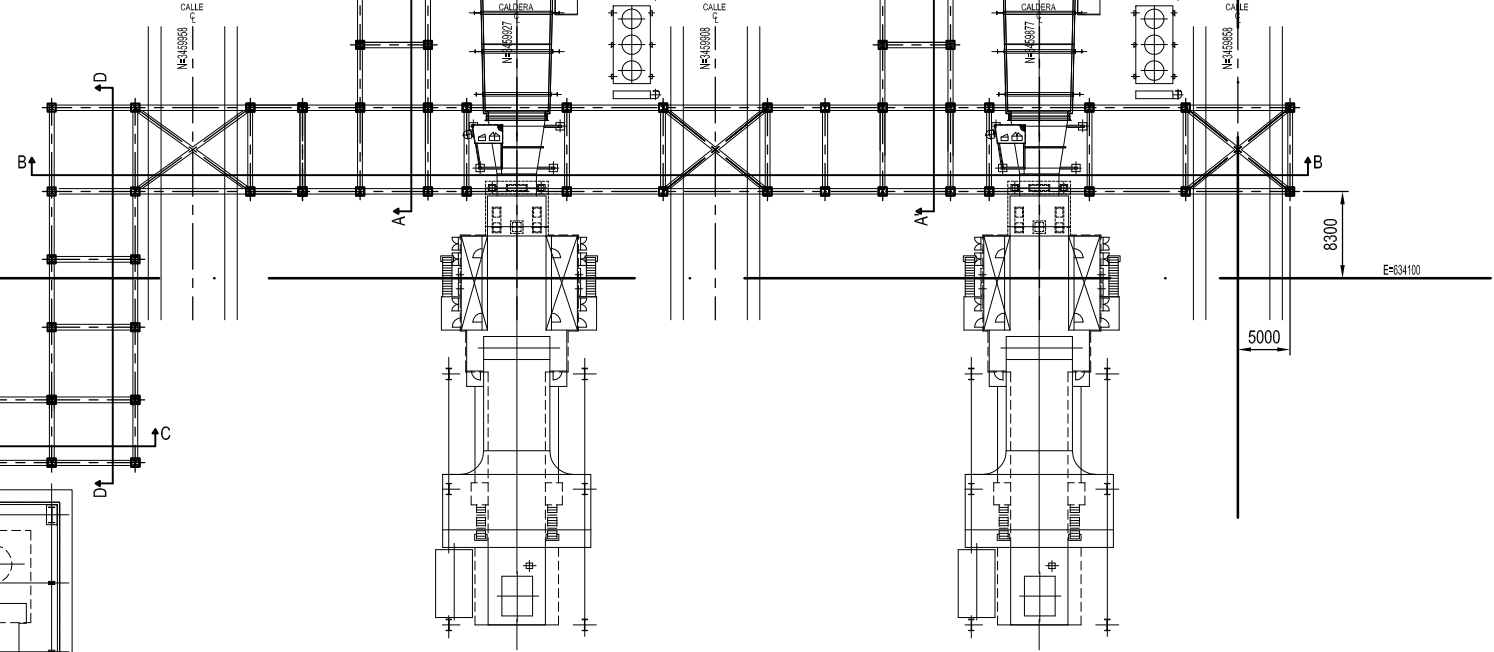
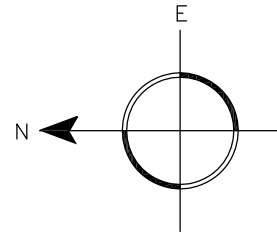
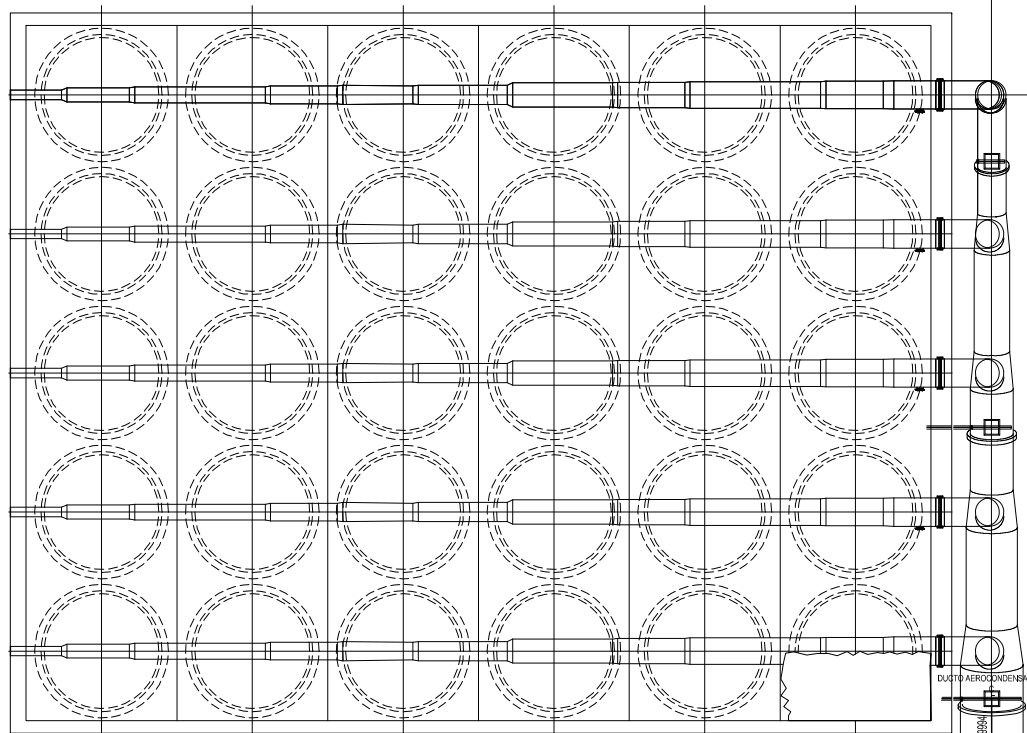
  

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IMPORANTE:		ORDEN DE COMPRA:	REVISOR:
NO. DE CONTROL:		DOCUMENTO DE PROYECTO:	PM27005-SRJT-PM-0141
		DOCUMENTO SUBASTADOR:	

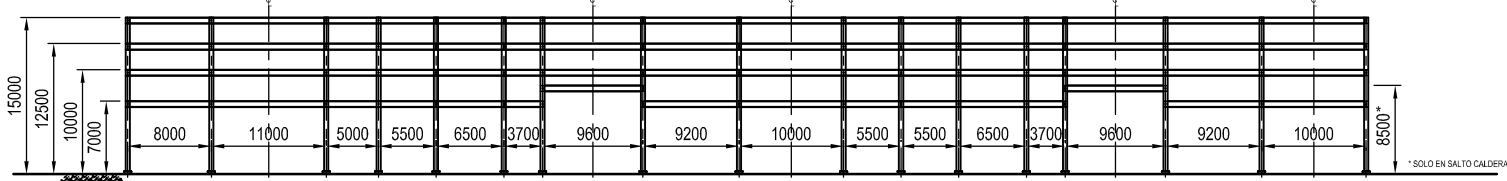
  

PROYECTO:	PLANTA DE CICLO COMBINADO
TITULO:	CARGAS DE TUBERIAS DE SERVICIOS SOBRE RACK PRINCIPAL Y RACK TREN 1 Y 2
PROBADO:	
COMPROBADO:	
REALIZADO:	
ESCALA:	1:125
REVISOR:	
REVISOR:	

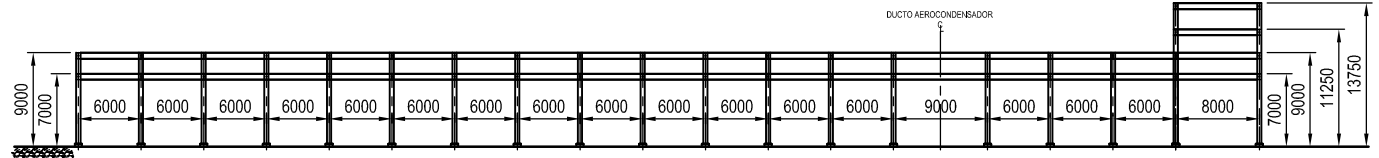




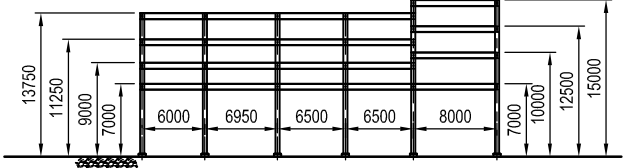
RACK CALDERAS  
(SECCIÓN A-A)  
(SECCIÓN A'-A')



RACK PRINCIPAL N-S  
(SECCIÓN B-B)



RACK DUCTO  
(SECCIÓN C-C)



RACK PRINCIPAL E-W  
(SECCIÓN D-D)

NOTAS  
1' - COTAS EN MILIMETROS  
2' - COORDENADAS EN METROS

PARA INFORMACION Y/O CONSTRUCCION

REV.	FECHA	DESCRIPCION	FECHO	APROBO
REVISIONES				

COMISION FEDERAL DE ELECTRICIDAD  
SUBDIRECCION DE CONSTRUCCION  
COORDINACION DE PROYECTOS TERMOELECTRICOS  
ESTADO DEL DOCUMENTO

ESTADO DE DOCUMENTO COMO SE ANOTA  
 1  SOLO PARA INFORMACION  
 2  CONFORME SIN COMENTARIOS  
 3  CONFORME CON COMENTARIOS  
 4  NO CONFORME  
 5  TAL COMO SE CONSTRUYO

FECHA DE RECEPCION: Revisado por Ing. Supervisor  
 Fecha:

IMPORTANTE:  
 La revisión de CFE no releva al PROVEEDOR de su responsabilidad de acuerdo a lo establecido en el contrato.

NO. DE CONTROL DE CFE:

COMISION FEDERAL DE ELECTRICIDAD  
SUBDIRECCION DE CONSTRUCCION  
COORDINACION DE PROYECTOS TERMOELECTRICOS

PROYECTO: 171 CC AGUA PRIETA II ( con campo solar )

TITULO: CROQUIS N° 1 PLANTA Y ALZADOS

PROYECTOS ELECTRICOS AGUA PRIETA S.A.P.I. DE C.V. (PEAP)

APROBADO: \_\_\_\_\_  
 COMPROBADO: \_\_\_\_\_  
 REALIZADO: \_\_\_\_\_  
 ESCALA: \_\_\_\_\_  
 ORDEN DE COMPRA: \_\_\_\_\_  
 FECH: \_\_\_\_\_

DOCUMENTO DE PROYECTO N°: \_\_\_\_\_  
 DOCUMENTO SUBPROYECTOR N°: \_\_\_\_\_

REV. N°: \_\_\_\_\_  
 REV. N°: \_\_\_\_\_

elecnor SENER

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### 3.0 CORRIDA DE MODELO MATEMATICO

PAGE NO. 1

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*****
*
*          STAAD.Pro          *
*          Version 2007   Build 04      *
*          Proprietary Program of      *
*          Research Engineers, Intl.    *
*          Date=   SEP 5, 2018          *
*          Time=   19:53:13            *
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*          USER ID:                *
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2. START JOB INFORMATION
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  4. JOB NAME RACK PRINCIPAL (NORTE-SUR)
  5. JOB CLIENT CFE
  6. JOB REV 0
  7. ENGINEER NAME GVZ
  8. CHECKER NAME GPVR
  9. APPROVED NAME RHC
  10. CHECKER DATE 19-AGO-11
  11. APPROVED DATE 19-AGO-11
  12. END JOB INFORMATION
  13. INPUT WIDTH 79
  14. UNIT METER KN
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447. 2620 1343 1337; 2621 1342 1343; 2622 1344 1334; 2623 1345 1340; 2624 1344 1345  
448. 2625 1346 1339; 2626 1344 1346; 2627 1347 1341; 2628 1346 1347; 2629 1345 1342  
449. 2630 1347 1343; 2631 1345 1347; 2632 1338 617; 2633 1348 632; 2634 1348 1338  
450. 2635 1349 631; 2636 1349 1350; 2637 1351 632; 2638 1350 1351; 2639 1352 1354  
451. 2640 1353 1355; 2641 1354 1335; 2642 1355 1337; 2643 1356 1358; 2644 1357 1365  
452. 2645 1358 1337; 2646 1359 1341; 2647 1357 1352; 2648 1359 1353; 2649 1354 1356  
453. 2650 1355 1358; 2651 1360 1362; 2652 1361 1363; 2653 1362 1340; 2654 1363 1341  
454. 2655 1364 1368; 2656 1365 1369; 2657 1364 1360; 2658 1362 1365; 2659 1368 1339  
455. 2660 1369 1359; 2661 1368 1361; 2662 1363 1369; 2663 348 1374; 2664 1370 338  
456. 2665 1371 336; 2666 1372 346; 2667 1373 348; 2668 1370 1371; 2669 1373 1372  
457. 2670 1374 358; 2671 1375 356; 2672 1374 1375; 2673 927 929; 2674 921 923

458. 2675 1376 408; 2676 1377 418; 2677 1378 410; 2678 1379 420; 2679 1378 1376  
459. 2680 1379 1377; 2681 823 1149; 2682 827 1150; 2683 873 1161; 2684 885 1162  
460. 2685 1380 411; 2686 1381 413; 2687 1380 1381; 2688 1382 421; 2689 1383 423  
461. 2690 1382 1383; 2691 828 1048; 2692 824 1047; 2693 1384 339; 2694 1385 341  
462. 2695 1384 1385; 2696 1386 349; 2697 1387 351; 2698 1386 1387; 2699 948 1151  
463. 2700 952 1152; 2701 928 930; 2702 922 924; 2703 874 1159; 2704 886 1160  
464. 2705 1389 1221; 2706 1388 1389; 2707 349 1388; 2708 1388 359; 2709 421 1390  
465. 2710 1390 437; 2711 1391 439; 2712 1390 1391; 2713 1047 826; 2714 1048 830  
466. 2715 1151 946; 2716 1152 950; 2717 1159 876; 2718 1160 888; 2719 1150 829  
467. 2720 1149 825; 2721 1162 887; 2722 1161 875; 2723 951 1156; 2724 947 1155  
468. 2725 1156 949; 2726 1155 945; 2727 1338 1347; 2728 1336 1347; 2729 1346 1341  
469. 2730 1343 1341; 2731 628 1345; 2732 353 1345; 2733 1344 1340; 2734 1342 1340  
470. DEFINE MATERIAL START  
471. ISOTROPIC STEEL  
472. E 1.99947E+008  
473. POISSON 0.3  
474. DENSITY 76.8191  
475. ALPHA 6.5E-006  
476. DAMP 0.03  
477. G 0  
478. ISOTROPIC CONCRETE  
479. E 2.17185E+007  
480. POISSON 0.17  
481. DENSITY 23.5616  
482. ALPHA 1E-005  
483. DAMP 0.05  
484. END DEFINE MATERIAL  
485. MEMBER PROPERTY AMERICAN  
486. 482 TO 484 488 TO 492 494 TO 496 500 TO 504 530 531 534 TO 539 543 545 547 -  
487. 549 551 553 555 556 558 TO 563 567 568 578 TO 583 587 588 591 TO 595 599 -  
488. 600 614 TO 619 623 624 634 636 TO 641 645 646 648 TO 653 657 658 660 TO 665 -  
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489. 669 670 672 TO 677 681 682 684 TO 689 693 694 734 735 737 738 740 741 743 -  
490. 744 TO 748 752 753 755 756 877 879 881 883 885 887 1009 1011 1013 1015 1017 -  
491. 1019 1028 1029 1253 1528 1531 1533 1883 TO 1902 1983 1984 2059 2061 2062 -  
492. 2064 2090 2094 2121 2411 2510 2511 2515 2516 2550 2551 2590 2605 -  
493. 2619 TABLE ST W18X86

494. 485 486 493 497 TO 499 505 512 515 518 521 524 527 541 557 565 569 585 597 -  
495. 621 643 647 654 655 659 666 667 671 678 679 683 691 742 749 750 757 1027 -  
496. 1030 1033 1035 1063 1083 1086 1105 1114 1174 1183 1440 1447 1452 1471 1495 -  
497. 1501 1504 1507 1510 1513 1516 1519 1522 1525 1667 1668 1696 1699 1702 1705 -  
498. 1711 1805 1808 TO 1811 1981 1982 1987 1988 1995 1996 2005 2006 2019 2020 -  
499. 2029 2030 2066 2067 2070 2073 2076 2079 2099 2101 2106 2115 2126 2127 2129 -  
500. 2131 TO 2133 2310 2311 2313 TO 2315 2317 TO 2319 2321 2322 2331 2333 2360 -  
501. 2361 TO 2362 2408 2413 2449 2451 2452 2509 2512 TO 2514 2519 2520 2530 2531 -  
502. 2554 2555 2559 2562 2564 2566 2568 TO 2575 2588 2687 2690 TO 2692 2695 2698 -  
503. 2699 TO 2704 2706 2712 TO 2718 2723 TO 2726 TABLE ST W12X50  
504. 532 533 544 546 548 550 552 554 696 697 700 701 705 706 710 715 716 720 721 -  
505. 725 727 TO 733 889 1031 1032 1438 1439 1441 1443 1445 1446 1450 1451 1453 -  
506. 1455 1465 1466 1468 1469 1472 1473 1475 1476 1478 1479 1481 1482 1493 1494 -  
507. 1496 1497 1499 1500 1503 1506 1509 1511 1512 1514 1515 1517 1518 1520 1521 -  
508. 1523 1524 1605 1606 1608 1609 1613 1615 1616 1618 1619 1623 1625 1626 1628 -  
509. 1629 1633 1694 1697 1698 1703 1704 1706 1707 1712 1718 1731 1732 1734 1735 -  
510. 1737 1739 1745 1746 1748 1749 1751 1753 1879 1880 2326 TO 2330 2332 2334 -  
511. 2336 2337 2350 2370 2383 2401 2415 2539 2540 2542 2543 TABLE ST W12X50  
512. 570 TO 577 602 TO 611 613 626 TO 631 633 635 698 699 702 703 707 708 -  
513. 712 TO 714 717 718 722 723 736 758 TO 762 890 891 1022 1023 1090 1095 1100 -  
514. 1107 1111 1543 1544 1584 1594 1597 1599 1600 1700 1701 1709 1710 1803 1806 -  
515. 1807 1812 1813 1815 1816 1820 1821 1823 1824 1826 1827 1829 1830 1832 1833 -  
516. 1835 1836 1838 1839 1841 1842 1844 1845 1847 1848 1869 1870 2063 2065 2086 -  
517. 2087 2095 2097 2124 2349 2369 2392 2399 2400 2414 2547 2549 2556 2557 2561 -  
518. 2582 2595 2597 2599 2602 2635 2663 TO 2667 2670 2671 2675 TO 2678 2685 2686 -  
519. 2688 2689 2693 2694 2696 2697 2705 2707 TO 2711 TABLE ST W12X35  
520. 487 540 564 584 589 642 690 1021 1034 1051 1074 1077 1080 1685 1686 1804 2053 -  
521. 2092 2093 2107 2546 2606 TABLE ST W14X68  
522. 1442 1444 1448 1449 1454 1456 1467 1470 1474 1477 1480 1483 1607 1610 1617 -  
523. 1620 1627 1630 1713 1715 TO 1717 1719 1721 1733 1736 1738 1740 TO 1744 1747 -  
524. 1750 1752 1754 2532 TO 2534 TABLE ST W6X16  
525. 1814 1817 TO 1819 1822 1825 1828 1831 1834 1837 1840 1843 1846 -  
526. 1849 TABLE ST W6X16  
527. 1643 TO 1646 1651 TO 1658 1722 TO 1727 1729 1730 1755 TO 1770 2535 TO 2537 -  
528. 2538 TABLE LD L30306  
529. 1851 TO 1853 1855 TO 1865 1872 1873 TABLE LD L30306  
530. 2007 TO 2018 2021 TO 2024 2037 TO 2052 2229 TO 2260 2269 TO 2284 2544 2545 -  
531. 2576 TO 2581 TABLE ST W8X21

532. 1690 TO 1693 TABLE ST W12X30  
533. 1771 1772 1777 1778 1781 TO 1784 1787 1788 1793 1794 1797 TO 1799 -  
534. 1800 TABLE ST W10X30  
535. 1926 TO 1945 1985 1986 1989 1990 2517 2518 2521 2522 TABLE ST W10X30  
536. 601 612 1052 1053 1055 1061 1064 1067 1070 1075 1078 1081 1085 1087 1089 1091 -  
537. 1093 1096 1098 1101 1103 1106 1109 1110 1112 1113 1165 2054 2084 2085 2089 -  
538. 2096 2098 2125 2610 2632 TO 2634 2636 TO 2638 TABLE ST W12X35  
539. 1056 1062 1065 1073 1076 1079 1092 1094 1097 1099 1102 1104 2055 2069 2072 -  
540. 2075 2078 2081 TO 2083 TABLE ST W8X21  
541. 2091 2100 2103 2108 2109 2111 2113 2114 2116 2117 2119 2120 2583 2585 2589 -  
542. 2593 2594 2596 2598 2601 TABLE ST W10X30  
543. 2110 2118 2134 2600 2603 TABLE ST W8X21  
544. MEMBER PROPERTY AMERICAN

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545. 2137 TO 2163 2523 PRIS YD 0.65 ZD 0.8  
546. 2164 2171 2176 2183 2188 TO 2201 2323 2324 2524 PRIS YD 0.7 ZD 0.3  
547. 2166 TO 2169 2173 TO 2175 2178 TO 2181 2185 TO 2187 2432 2470 TO 2471 -  
548. 2472 PRIS YD 0.6 ZD 0.3  
549. 2285 TO 2289 TABLE LD L30306  
550. 2430 2431 2445 2446 TABLE ST W8X21  
551. 2335 2338 TO 2340 2342 TO 2346 2351 TO 2359 2363 TO 2366 2371 TO 2374 2376 -  
552. 2381 2385 2387 2388 2393 2394 2402 TO 2407 2409 2410 2416 TO 2421 -  
553. 2424 TO 2429 2450 2453 2454 TABLE ST W8X18  
554. 2455 TO 2469 TABLE ST L30306  
555. 2347 2348 2367 2368 2377 2378 2390 2391 2397 2398 2422 2423 2447 2448 2473 -  
556. 2474 TO 2480 2485 TO 2500 TABLE ST C10X15  
557. MEMBER PROPERTY AMERICAN  
558. 2433 2435 PRIS YD 0.5 ZD 0.5  
559. 2290 TO 2293 2295 TO 2298 2301 TO 2305 2412 TABLE ST W12X50  
560. MEMBER PROPERTY AMERICAN  
561. 2481 TO 2484 2501 TO 2508 PRIS YD 0.03 ZD 0.25  
562. 2526 TO 2529 TABLE ST W12X50  
563. 542 711 726 1502 1505 1508 1611 1621 1631 1903 1915 TABLE ST W14X68  
564. 1639 TO 1642 1659 TO 1666 TABLE LD L35358  
565. 506 509 566 586 596 598 620 622 625 644 656 668 680 692 719 751 1484 1498 -  
566. 1669 1689 1695 1708 1714 1720 1904 TO 1914 1916 TO 1924 2135 2541 -  
567. 2584 TABLE ST W12X65

568. MEMBER PROPERTY AMERICAN  
 569. 2306 2307 PRIS YD 0.5 ZD 0.65  
 570. 2604 2607 2611 2615 2617 2620 2622 2623 2625 2627 TABLE ST W12X35  
 571. 2608 2609 2612 TO 2614 2616 2618 2621 2624 2626 2628 TO 2631 2639 TO 2646 -  
 572. 2651 TO 2656 2659 2660 TABLE ST W12X30  
 573. 2647 TO 2650 2657 2658 2661 2662 TABLE ST W12X40  
 574. 2668 2669 2672 TO 2674 2679 TO 2684 2719 TO 2722 TABLE ST W12X50  
 575. 2727 TO 2734 TABLE LD L40406  
 576. CONSTANTS  
 577. BETA 90 MEMB 482 TO 484 488 TO 492 494 TO 496 500 TO 504 530 531 534 TO 539 -  
 578. 543 545 547 549 551 553 555 556 558 TO 563 567 568 578 TO 583 587 588 591 -  
 579. 592 TO 595 599 600 614 TO 619 623 624 634 636 TO 641 645 646 648 TO 653 657 -  
 580. 658 660 TO 665 669 670 672 TO 677 681 682 684 TO 689 693 694 734 735 737 -  
 581. 738 740 741 743 TO 748 752 753 755 756 877 879 881 883 885 887 1009 1011 -  
 582. 1013 1015 1017 1019 1028 1029 1253 1528 1531 1533 1883 TO 1902 1983 1984 -  
 583. 2059 2061 2062 2064 2090 TO 2092 2094 2098 2100 2109 2117 2121 2290 TO 2293 -  
 584. 2295 TO 2298 2301 TO 2305 2411 2412 2510 2511 2515 2516 2550 2551 2582 2585 -  
 585. 2589 2590 2604 2605 2607 2611 2615 2617 2619 2620 2622 2623 2625 2627  
 586. BETA 45 MEMB 2455 TO 2469  
 587. MATERIAL STEEL MEMB 482 TO 506 509 512 515 518 521 524 527 530 TO 589 591 -  
 588. 592 TO 631 633 TO 694 696 TO 703 705 TO 708 710 TO 723 725 TO 738 740 TO 753 -  
 589. 755 TO 762 877 879 881 883 885 887 889 TO 891 1009 1011 1013 1015 1017 1019 -  
 590. 1021 TO 1023 1027 TO 1035 1051 TO 1053 1055 1056 1061 TO 1065 1067 1070 1073 -  
 591. 1074 TO 1081 1083 1085 TO 1087 1089 TO 1107 1109 TO 1114 1165 1174 1183 1253 -  
 592. 1438 TO 1456 1465 TO 1484 1493 TO 1525 1528 1531 1533 1543 1544 1584 1594 -  
 593. 1597 1599 1600 1605 TO 1611 1613 1615 TO 1621 1623 1625 TO 1631 1633 1639 -  
 594. 1640 TO 1646 1651 TO 1669 1685 1686 1689 TO 1727 1729 TO 1772 1777 1778 1781 -  
 595. 1782 TO 1784 1787 1788 1793 1794 1797 TO 1800 1803 TO 1849 1851 TO 1853 1855 -  
 596. 1856 TO 1865 1869 1870 1872 1873 1879 1880 1883 TO 1924 1926 TO 1945 1981 -  
 597. 1982 TO 1990 1995 1996 2005 TO 2024 2029 2030 2037 TO 2055 2059 2061 TO 2067 -  
 598. 2069 2070 2072 2073 2075 2076 2078 2079 2081 TO 2087 2089 TO 2101 2103 2106 -  
 599. 2107 TO 2111 2113 TO 2121 2124 TO 2127 2129 2131 TO 2135 2229 TO 2260 2269 -  
 600. 2270 TO 2293 2295 TO 2298 2301 TO 2305 2310 2311 2313 TO 2315 2317 TO 2319 -  
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 601. 2321 2322 2326 TO 2340 2342 TO 2374 2376 TO 2378 2381 2383 2385 2387 2388 -  
 602. 2390 TO 2394 2397 TO 2431 2445 TO 2469 2473 TO 2522 2526 TO 2547 -  
 603. 2549 TO 2551 2554 TO 2557 2559 2561 2562 2564 2566 2568 TO 2585 2588 TO 2590 -

604. 2593 TO 2734  
605. MATERIAL CONCRETE MEMB 2137 TO 2164 2166 TO 2169 2171 2173 TO 2176 -  
606. 2178 TO 2181 2183 2185 TO 2201 2306 2307 2323 2324 2432 2433 2435 -  
607. 2470 TO 2472 2523 2524  
608. MEMBER RELEASE  
609. 532 533 544 546 548 550 552 554 570 TO 577 602 604 TO 607 609 611 613 626 -  
610. 627 TO 631 633 635 696 TO 703 705 TO 708 710 TO 723 725 TO 733 736 761 1021 -  
611. 1035 1599 1869 1870 1879 1880 2063 2065 2526 2527 2547 2549 2572 2575 2663 -  
612. 2668 2669 2672 TO 2674 2679 TO 2684 2709 START MY MZ  
613. 572 575 606 609 611 626 TO 631 633 635 712 713 722 723 1021 1063 1100 1107 -  
614. 1450 1451 1453 1455 1478 1479 1481 1482 1493 1494 1502 1503 1511 1512 1520 -  
615. 1521 1523 1524 1599 1625 1626 1628 1629 1631 1633 1718 1720 1731 1732 1734 -  
616. 1735 1737 1739 1832 1833 1835 1836 2087 2124 2528 2529 2547 2549 2557 2561 -  
617. 2572 2575 2664 TO 2670 2672 TO 2680 2685 2686 2688 2689 2693 2694 2696 2697 -  
618. 2710 2711 2719 TO 2722 END MY MZ  
619. 1033 1070 1106 1471 1484 1507 1667 TO 1669 2687 2690 TO 2692 2695 2698 2699 -  
620. 2701 2703 2704 2706 2712 START MY MZ  
621. 1033 1075 1109 1471 1484 1507 1667 1668 1913 2687 2690 2695 2698 2701 2706 -  
622. 2712 TO 2715 2717 2718 END MY MZ  
623. 1056 1062 1065 1073 1076 1079 1094 1099 1104 1110 1113 2055 2069 2072 2075 -  
624. 2078 2081 2082 2089 2095 2125 2559 2636 START MY MZ  
625. 1056 1062 1065 1073 1076 1079 1094 1099 1104 2055 2069 2072 2075 2078 2081 -  
626. 2082 2085 2125 2562 2610 2633 2638 END MX MY MZ  
627. 603 608 610 1543 1544 1584 2103 2110 2111 2114 2118 2119 2134 2399 2556 2594 -  
628. 2595 TO 2597 2600 2603 2707 START MY MZ  
629. 1584 1594 1600 2103 2110 2113 2114 2118 2120 2131 2134 2369 2414 2556 2598 -  
630. 2599 TO 2603 2671 2708 END MY MZ  
631. 1440 1447 1452 1495 1498 1501 1504 1510 1513 1516 1519 1522 1525 1696 1699 -  
632. 1702 1705 1708 1711 2700 2702 2723 2724 START MY MZ  
633. 1447 1516 1522 1525 1696 1699 1702 1705 1711 1914 1995 1996 2005 2006 2019 -  
634. 2020 2029 2030 2584 2702 2716 2725 2726 END MY MZ  
635. 1771 1772 1777 1778 1787 1788 1793 1794 2007 2009 2011 2013 2015 TO 2018 2037 -  
636. 2038 TO 2040 2045 TO 2048 2229 TO 2237 2239 2241 2243 2245 2247 2249 2251 -  
637. 2253 2255 2257 2259 2269 TO 2272 2277 TO 2289 START MX MY MZ  
638. 1781 TO 1784 1797 TO 1800 2008 2010 2012 2014 2021 TO 2024 2041 TO 2044 2049 -  
639. 2050 TO 2052 2229 TO 2236 2238 2240 2242 2244 2246 2248 2250 2252 2254 2256 -  
640. 2258 2260 2273 TO 2289 END MX MY MZ  
641. 1926 TO 1933 1936 TO 1945 1985 1986 1989 1990 2517 2518 2521 2522 END MX MY MZ

642. 1926 TO 1933 1936 TO 1945 1985 1986 1989 1990 2517 2518 2521 -  
643. 2522 START MX MY MZ  
644. 2335 2338 2339 2351 2363 2364 2366 2371 2402 2409 2416 2424 2450 2453 -  
645. 2454 START MX MY MZ  
646. 2340 2342 2356 2359 2363 2365 2366 2376 2388 2406 2409 2420 2428 2450 2453 -  
647. 2454 END MX MY MZ  
648. 2347 2348 2367 2368 2377 2378 2390 2391 2422 2423 2447 2448 2530 -  
649. 2531 START MX MY  
650. 2347 2348 2367 2368 2397 2398 2487 2488 2497 TO 2500 2530 2531 END MX MY  
651. 2647 TO 2650 2657 2658 2661 2662 START MY MZ  
652. 2647 TO 2650 2657 2658 2661 2662 END MY MZ  
653. MEMBER TRUSS  
654. 1690 TO 1693 2535 TO 2538 2544 2545 2576 2577  
655. MEMBER TRUSS  
656. 1639 TO 1646 1651 TO 1666 1722 TO 1727 1729 1730 1755 TO 1770 1851 TO 1853 -  
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657. 1855 TO 1865 1872 1873  
658. MEMBER TRUSS  
659. 2455 TO 2469 2544 2545 2576 TO 2581  
660. MEMBER TRUSS  
661. 2727 TO 2734  
662. SUPPORTS  
663. 1095 TO 1121 1181 1182 1298 FIXED BUT MX MZ KFY 25113.6  
664. CUT OFF MODE SHAPE 200  
665. LOAD 1 LOADTYPE DEAD TITLE CARGA MUERTA (D)  
666. SELFWEIGHT Y -1  
667. MEMBER LOAD  
668. 601 633 2063 2561 UNI GY -0.2245  
669. 1056 1062 1065 1073 2069 UNI GY -0.4489  
670. 1113 2633 2636 2638 UNI GY -0.216  
671. 1110 2610 UNI GY -0.432  
672. 569 1086 UNI GY -0.453  
673. 1094 1099 1104 UNI GY -0.477  
674. 1051 1080 2606 UNI GY -0.238  
675. 589 UNI GY -0.462  
676. 1053 1055 UNI GY -0.1725  
677. 1079 UNI GY -0.427



678. 1076 2055 UNI GY -0.49  
679. 612 UNI GY -0.245  
680. 2067 2070 2083 UNI GY -0.259  
681. 2082 UNI GY -0.518  
682. 2072 UNI GY -0.457  
683. 2075 2078 2081 UNI GY -0.466  
684. 635 2065 2557 UNI GY -0.233  
685. \*\*\*\*\*  
686. 608 612 2369 2392 2705 2707 2708 UNI GY -0.383  
687. \*\*\*\*\*  
688. 603 610 2086 2087 2327 2329 2330 2336 2337 2369 2370 2414 2415 2430 -  
689. 2431 UNI GY -0.25  
690. 2363 TO 2366 2450 2453 2454 UNI GY -0.5  
691. 2347 2348 2367 2368 2377 2378 2390 2391 2422 2423 2447 2448 2473 TO 2480 2485 -  
692. 2486 TO 2500 UNI GY -0.157  
693. 2647 TO 2650 CON GY -3.25 0.7  
694. 2657 2658 2661 2662 CON GY -1.425 0.835  
695. 493 659 671 683 742 1447 1507 1516 1522 1525 UNI GY -5.563 3.9 4.8  
696. 2571 UNI GY -5.563 1.25 2.15  
697. 1105 1995 1996 2019 2020 2029 UNI GY -5.563 0.001 0.8  
698. LOAD 2 LOADTYPE LIVE TITLE CARGA VIVA (L)  
699. MEMBER LOAD  
700. 601 633 2063 2561 UNI GY -3.188  
701. 1056 1062 1065 1073 2069 UNI GY -6.377  
702. 1113 2633 2636 2638 UNI GY -3.066  
703. 1110 2610 UNI GY -6.132  
704. 569 1086 UNI GY -6.438  
705. 1094 1099 1104 UNI GY -6.744  
706. 1051 1080 2606 UNI GY -3.372  
707. 589 UNI GY -6.56  
708. 1053 1055 UNI GY -2.453  
709. 1079 UNI GY -5.95  
710. 1076 2055 UNI GY -6.99  
711. 612 UNI GY -3.495  
712. 2067 2070 2083 2096 UNI GY -3.679

713. 2082 UNI GY -7.358

714. 2072 UNI GY -6.5  
715. 2075 2078 2081 UNI GY -6.622  
716. 635 2065 2557 UNI GY -3.311  
717. \*\*\*\*\*  
718. 608 612 2369 2392 2705 2707 2708 UNI GY -5.445  
719. 603 610 2086 2087 2327 2329 2330 2336 2337 2369 2370 2414 2415 2430 -  
720. 2431 UNI GY -3.483  
721. 2363 TO 2366 2450 2453 2454 UNI GY -6.966  
722. 2347 2348 2367 2368 2377 2378 2390 2391 2422 2423 2447 2448 2473 TO 2480 2485 -  
723. 2486 TO 2500 UNI GY -2.23  
724. 493 659 671 683 742 1447 1507 1516 1522 1525 UNI GY -14.715 3.9 4.8  
725. 2571 UNI GY -14.715 1.25 2.15  
726. 1105 1995 1996 2019 2020 2029 UNI GY -14.715 0.001 0.8  
727. LOAD 3 LOADTYPE GRAVITY TITLE CHAROLAS ELECTRICAS (WE)  
728. MEMBER LOAD  
729. 493 524 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
730. 1525 2575 CON GY -5.3 1.85  
731. 493 524 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
732. 1525 2575 CON GY -5.3 2.45  
733. 493 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
734. 1525 2575 CON GY -3.532 3.15  
735. 2571 CON GY -3.532 0.5  
736. 493 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
737. 1525 2575 CON GY -3.532 3.85  
738. 2571 CON GY -3.532 1.2  
739. 493 659 671 683 742 758 1447 1507 1516 1522 1525 2575 CON GY -3.532 4.85  
740. 2571 CON GY -3.532 2.2  
741. 1995 1996 2019 2020 2029 2030 CON GY -3.532 0.85  
742. 493 659 671 683 742 758 1447 1507 1516 1522 1525 2575 CON GY -3.532 5.55  
743. 2574 CON GY -3.532 0.200002  
744. 1995 1996 2019 2020 2029 2030 CON GY -3.532 1.55  
745. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
746. 2603 CON GY -5.3 0.05  
747. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
748. 2603 CON GY -5.3 0.65  
749. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
750. 2603 CON GY -3.532 0.7  
751. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -

752. 2603 CON GY -3.532 1.3  
753. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
754. 2603 CON GY -3.532 2.2  
755. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
756. 2603 CON GY -3.532 2.8  
757. LOAD 4 LOADTYPE TEMPERATURE TITLE CARGA TERMICA (T)  
758. \*TUBERIA VAPOR BAJA PRESION GVRC2 A TURBINA\*\*\*\*\*  
759. MEMBER LOAD  
760. 889 1021 CON GY 1.448 3  
761. 690 CON GY -1.903 2.195  
762. 690 CON GX 0.389 2.195  
763. 678 CON GY 0.585 2.195  
764. 678 CON GZ 5.033 2.195  
765. 1621 CON GY -0.101 0.5  
766. 1027 2509 CON GY 0.125 1.35  
767. 1751 CON GY -0.045 1.4  
768. 642 CON GY 0.016 2.195

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769. 642 CON GZ -0.648 2.195  
770. 620 CON GY -0.018 2.195  
771. 620 CON GZ 0.654 2.195  
772. 596 CON GY 0.008 2.195  
773. 584 CON GY 0.003 2.195  
774. 584 CON GX -0.674 2.195  
775. 564 CON GY -0.019 2.195  
776. 564 CON GZ 1.657 2.195  
777. 540 CON GY 0.067 2.195  
778. 540 CON GZ -2.577 2.195  
779. 1033 CON GY -0.248 2.195  
780. 1034 CON GY 0.577 2.195  
781. 487 CON GY 1.086 2.195  
782. \*TUBERIA VAPOR BAJA PRESION GVRC1 A TURBINA\*\*\*\*\*  
783. 584 CON GY -3.958 1.47  
784. 584 CON GX 4.065 1.47  
785. 564 CON GY -1.294 1.47  
786. 564 CON GZ 5.655 1.47  
787. 540 CON GY -0.287 1.47

788. 540 CON GZ -4.73 1.47  
789. 1033 CON GY -0.414 1.47  
790. 1034 CON GY 0.574 1.47  
791. 487 CON GY -1.348 1.47  
792. \*TUBERIA VAPOR REC FRIO TURBINA A GVRC2\*\*\*\*\*  
793. 889 1021 CON GY 9.123 4.05  
794. 690 CON GY -27.719 3.195  
795. 690 CON GX 52.319 3.195  
796. 690 CON GZ 1.002 3.195  
797. 678 CON GY -5.869 3.195  
798. 678 CON GX -0.431 3.195  
799. 678 CON GZ 0.773 3.195  
800. 666 CON GY -1.81 3.195  
801. 666 CON GZ 4.448 3.195  
802. 1611 CON GY 2.974 2  
803. 1611 CON GX -0.124 2  
804. 1611 CON GZ -0.466 2  
805. 1667 CON GY 0.869 3.195  
806. 1667 CON GX 0.944 3.195  
807. 1667 CON GZ -1.057 3.195  
808. 1027 CON GY 8.92 2.35  
809. 1027 CON GZ -60.912 2.35  
810. 1697 CON GY -8.282 1.05  
811. 1697 CON GX 1.853 1.05  
812. 1697 CON GZ -0.382 1.05  
813. 642 CON GY -10.429 3.195  
814. 642 CON GZ 70.315 3.195  
815. 620 CON GY -3.06 3.195  
816. 596 CON GY 6.425 3.195  
817. 584 CON GY 6.13 3.195  
818. 584 CON GX -40.151 3.195  
819. 564 CON GY 6.782 3.195  
820. 564 CON GZ 0.694 3.195  
821. 540 CON GY -16.897 3.195  
822. 1034 CON GY 30.882 3.195  
823. 487 CON GY -32.908 3.195  
824. \*TUBERIA VAPOR REC. CALIENTE DE GVRC2 A TURBINA\*\*\*\*\*

825. 889 CON GY -47.752 5.325  
826. 889 CON GZ 1.734 5.325  
827. 1804 CON GY 62.867 1.8  
828. 1804 CON GX 42.924 1.8  
829. 1804 CON GZ 2.027 1.8  
830. 678 CON GY -38.413 5.8  
831. 678 CON GX -0.33 5.8  
832. 678 CON GZ 0.716 5.8  
833. 666 CON GY -8.103 5.8  
834. 666 CON GX -2.562 5.8  
835. 666 CON GZ 7.76 5.8  
836. 1611 CON GY 1.203 0.7  
837. 1611 CON GX -0.476 0.7  
838. 1611 CON GZ -1.112 0.7  
839. 1667 CON GY -0.706 5.8  
840. 1667 CON GX 1.139 5.8  
841. 1667 CON GZ -1.427 5.8  
842. 1981 CON GY -0.486 1.15  
843. 1981 CON GX 6.05 1.15  
844. 1981 CON GZ -43.22 1.15  
845. 1697 CON GY 1.502 1.4  
846. 1697 CON GX 1.937 1.4  
847. 1697 CON GZ -1.06 1.4  
848. 642 CON GY -0.386 5.8  
849. 642 CON GX 5.002 5.8  
850. 642 CON GZ 47.225 5.8  
851. 620 CON GY -0.37 5.8  
852. 620 CON GX 1.226 5.8  
853. 620 CON GZ 0.224 5.8  
854. 1689 CON GY 0.776 1.8  
855. 1689 CON GX 0.782 1.8  
856. 1689 CON GZ 0.177 1.8  
857. 2546 CON GY -1.862 0.4  
858. 2546 CON GX -24.641 0.4  
859. 2546 CON GZ -28.668 0.4  
860. 1685 CON GY 7.422 1.8  
861. 1685 CON GX -0.631 1.8

862. 1685 CON GZ -0.244 1.8  
863. 540 CON GY -27.731 5.8  
864. 540 CON GX 1.263 5.8  
865. 540 CON GZ -0.531 5.8  
866. 1033 CON GY 39.715 5.8  
867. 1034 CON GY 18.11 5.8  
868. 499 CON GY -30 5.8  
869. 487 CON GY -68.842 5.8  
870. 487 CON GX -2.804 5.8  
871. 487 CON GZ -0.86 5.8  
872. \*TUBERIA VAPOR RECALENTADO CALIENTE DE GVRC1 A TURB.\*\*\*\*\*

873. 1021 CON GY -19.894 5.325  
874. 1686 CON GY -23.846 0.47  
875. 1686 CON GZ 3.646 0.47  
876. 1685 CON GY -31.11 0.47  
877. 1685 CON GX -0.632 0.47  
878. 1685 CON GZ 1.197 0.47  
879. 540 CON GY -18.037 4.47  
880. 540 CON GX -1.263 4.47

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881. 540 CON GZ 0.516 4.47  
882. 1033 CON GY 7.198 4.47  
883. 1034 CON GY 19.996 4.47  
884. 499 CON GY 39.386 4.47  
885. 487 CON GY -42.648 4.47  
886. \*TUBERIA VAPOR PRINCIPAL\*\*\*\*\*

887. MEMBER LOAD  
888. 889 CON GY 7.589 1  
889. 1804 CON GY 9.8 3  
890. 1804 CON GX 53.097 3  
891. 1804 CON GZ 2.215 3  
892. 678 CON GY -19.129 7  
893. 666 CON GY -3.517 7  
894. 666 CON GX -2.001 7  
895. 666 CON GZ 1.618 7  
896. 711 CON GY -1.748 2  
897. 711 CON GX 0.563 2

898. 711 CON GZ -1.767 2  
899. 1737 CON GY 0.275 0.3  
900. 1737 CON GX 1.469 0.3  
901. 1737 CON GZ -1.712 0.3  
902. 1988 CON GY -1.097 0.65  
903. 1988 CON GX 6.971 0.65  
904. 1988 CON GZ -20.414 0.65  
905. 642 CON GY 0.131 7  
906. 642 CON GX 2.38 7  
907. 642 CON GZ 0.407 7  
908. 620 CON GY -18.748 7  
909. 620 CON GX 2.452 7  
910. 620 CON GZ 79.959 7  
911. 2546 CON GY -15.426 1.6  
912. 2546 CON GX -95.579 1.6  
913. 2546 CON GZ 0.482 1.6  
914. 1685 CON GY -4.658 3  
915. 1685 CON GX -1.263 3  
916. 1685 CON GZ -49.61 3  
917. 540 CON GY 2.309 7  
918. 540 CON GX -1.263 7  
919. 540 CON GZ -0.603 7  
920. 1033 CON GY -4.234 7  
921. 1033 CON GX -1.925 7  
922. 1033 CON GZ -1.377 7  
923. 1034 CON GY 23.692 7  
924. 1034 CON GX -0.164 7  
925. 1034 CON GZ -0.13 7  
926. 1021 CON GY -3.961 1  
927. 1021 CON GX -9.331 1  
928. JOINT LOAD  
929. 905 FY 9.757  
930. MEMBER LOAD  
931. 1696 CON GY 11.995 7  
932. 1023 CON GY -65.572 5.325  
933. 2671 CON GY 18.26 2.075  
934. LOAD 5 LOADTYPE FLUIDS TITLE PESO EN OPERACION DE TUBERIAS (Q2)  
935. MEMBER LOAD

936. 486 509 542 566 586 598 1484 1903 TO 1906 1915 TO 1918 UNI GY -10.791

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937. 512 541 565 585 597 621 622 667 1022 1471 1668 1907 1919 2687 2691 2692 2695 -

938. 2698 2699 2704 2706 2713 TO 2715 2718 UNI GY -0.981

939. 499 644 656 668 680 692 1669 1708 1711 1908 TO 1914 1920 TO 1924 2723 TO 2725 -

940. 2726 UNI GY -7.848

941. 750 891 1023 UNI GY -1.4715

942. 498 UNI GY -9.81

943. 643 655 679 691 1702 1705 2690 2700 TO 2703 2712 2716 2717 UNI GY -0.981

944. 890 UNI GY -2.943

945. \*TUBERIA VAPOR BAJA PRESION GVRC2 A TURBINA\*\*\*\*\*

946. MEMBER LOAD

947. 889 1021 CON GY -3.457 3

948. 690 CON GY -3.348 2.195

949. 690 CON GX -0.019 2.195

950. 678 CON GY -4.073 2.195

951. 678 CON GZ 0.001 2.195

952. 1621 CON GY -5.57 0.5

953. 1027 2509 CON GY -9.81 1.35

954. 1751 CON GY -5.737 1.40001

955. 642 CON GY -5.74 2.195

956. 620 CON GY -5.655 2.195

957. 620 CON GZ -0.007 2.195

958. 596 CON GY -1.694 2.195

959. 584 CON GY -4.09 2.195

960. 584 CON GX -0.042 2.195

961. 564 CON GY -2.816 2.195

962. 564 CON GZ 0.083 2.195

963. 540 CON GY -4.163 2.195

964. 540 CON GZ -0.099 2.195

965. 1033 CON GY -0.691 2.195

966. 1034 CON GY -7.396 2.195

967. 487 CON GY -7.868 2.195

968. \*TUBERIA VAPOR BAJA PRESION GVRC1 A TURBINA\*\*\*\*\*

969. 584 CON GY -3.096 1.47

970. 564 CON GY -3.062 1.47

971. 564 CON GZ 0.18 1.47



972. 540 CON GY -4.264 1.47  
973. 540 CON GZ -0.155 1.47  
974. 1033 CON GY -0.226 1.47  
975. 1034 CON GY -8.325 1.47  
976. 487 CON GY -9.041 1.47  
977. \*TUBERIA VAPOR REC FRIO TURBINA A GVRC2\*\*\*\*\*  
978. 889 1021 CON GY -9.123 4.05  
979. 690 CON GY -39.035 3.195  
980. 678 CON GY -13.855 3.195  
981. 666 CON GY -10.372 3.195  
982. 666 CON GZ -0.036 3.195  
983. 1611 CON GY -19.276 2  
984. 1667 CON GY -31.149 3.195  
985. 1027 CON GY -13.845 2.35  
986. 1697 CON GY -20.394 1.05  
987. 642 CON GY -34.071 3.195  
988. 620 CON GY -38.406 3.195  
989. 596 CON GY -29.687 3.195  
990. 584 CON GY -39.169 3.195  
991. 584 CON GX 1.615 3.195  
992. 564 CON GY -17.18 3.195

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993. 540 CON GY -24.314 3.195  
994. 1034 CON GY -34.417 3.195  
995. \*TUBERIA VAPOR REC. CALIENTE DE GVRC2 A TURBINA\*\*\*\*\*  
996. 889 CON GY -46.21 5.325  
997. 1804 CON GY -33.62 1.8  
998. 678 CON GY -6.09 5.8  
999. 666 CON GY -25.345 5.8  
1000. 1611 CON GY -21.893 0.7  
1001. 1667 CON GY -17.91 5.8  
1002. 1981 CON GY -26.145 1.15  
1003. 1697 CON GY -22.333 1.4  
1004. 642 CON GY -30.094 5.8  
1005. 620 CON GY -27.47 5.8  
1006. 1689 CON GY -9.932 1.8  
1007. 2546 CON GY -20.597 0.399998

1008. 2546 CON GX -0.425 0.399998  
1009. 1685 CON GY -16.122 1.8  
1010. 540 CON GY -18.169 5.8  
1011. 1033 CON GY 16.453 5.8  
1012. 1034 CON GY -18.11 5.8  
1013. 499 CON GY 42.736 5.8  
1014. 487 CON GY -23.508 5.8  
1015. \*TUBERIA VAPOR RECALENTADO CALIENTE DE GVRC1 A TURB.\*\*\*\*\*  
1016. 1021 CON GY -19.894 5.325  
1017. 1686 CON GY -23.888 0.47  
1018. 1685 CON GY -11.919 0.47  
1019. 540 CON GY -19.817 4.47  
1020. 1033 CON GY -18.725 4.47  
1021. 1034 CON GY -19.996 4.47  
1022. 499 CON GY -37.075 4.47  
1023. 487 CON GY -36.248 4.47  
1024. \*TUBERIA VAPOR PRINCIPAL\*\*\*\*\*  
1025. MEMBER LOAD  
1026. 889 1021 CON GY -45.94 1  
1027. 1804 CON GY -47.642 3  
1028. 1804 CON GX -0.246 3  
1029. 678 CON GY -19.129 7  
1030. 666 CON GY -32.028 7  
1031. 711 CON GY -24.732 2  
1032. 1737 CON GY -26.808 0.3  
1033. 1988 CON GY -34.982 0.65  
1034. 1988 CON GZ 0.136 0.65  
1035. 642 CON GY -26.357 7  
1036. 620 CON GY -44.777 7  
1037. 620 CON GZ 0.817 7  
1038. 2546 CON GY -41.716 1.6  
1039. 2546 CON GX -0.432 1.6  
1040. 1685 CON GY -18.883 3  
1041. 540 CON GY -21.953 7  
1042. 1033 CON GY -21.227 7  
1043. 1034 CON GY -25.761 7  
1044. 1021 CON GY -43.855 1  
1045. 2549 CON GY -6.065 1

1046. JOINT LOAD

1047. 905 FY -12.529

1048. MEMBER LOAD

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1049. 1696 CON GY -15.584 7

1050. 1023 CON GY -29.269 5.325

1051. 2671 CON GY -22.831 2.075

1052. 2668 2669 2672 TO 2674 2679 TO 2684 2719 TO 2722 UNI GY -0.98

1053. LOAD 6 LOADTYPE FLUIDS TITLE PESO PRUEBA HIDRAULICA TUBERIAS (Q3)

1054. MEMBER LOAD

1055. 486 509 542 566 586 598 1484 1903 TO 1906 1915 TO 1918 UNI GY -10.791

1056. 512 541 565 585 597 621 622 667 1022 1471 1668 1907 1919 2687 2691 2692 2695 -

1057. 2698 2699 2704 2706 2713 TO 2715 2718 UNI GY -0.981

1058. 499 644 656 668 680 692 1669 1708 1711 1908 TO 1914 1920 TO 1924 2723 TO 2725 -

1059. 2726 UNI GY -7.848

1060. 750 891 1023 UNI GY -1.4715

1061. 498 UNI GY -9.81

1062. 643 655 679 691 1702 1705 2690 2700 TO 2703 2712 2716 2717 UNI GY -0.981

1063. 890 UNI GY -2.943

1064. \*TUBERIA VAPOR BAJA PRESION GVRC2 A TURBINA\*\*\*\*\*

1065. MEMBER LOAD

1066. 889 1021 CON GY -5.31 3

1067. 690 CON GY -5.157 2.195

1068. 690 CON GX -0.038 2.195

1069. 678 CON GY -5.208 2.195

1070. 678 CON GZ 0.004 2.195

1071. 1621 CON GY -7.972 0.5

1072. 1027 2509 CON GY -14.041 1.35

1073. 1751 CON GY -8.203 1.40001

1074. 642 CON GY -9.656 2.195

1075. 620 CON GY -8.704 2.195

1076. 620 CON GZ -0.011 2.195

1077. 596 CON GY -2.605 2.195

1078. 584 CON GY -6.308 2.195

1079. 584 CON GX -0.058 2.195

1080. 564 CON GY -4.274 2.195

1081. 564 CON GZ 0.129 2.195

1082. 540 CON GY -6.25 2.195  
1083. 540 CON GZ -0.157 2.195  
1084. 1033 CON GY -1.1 2.195  
1085. 1034 CON GY -11.385 2.195  
1086. 487 CON GY -11.246 2.195  
1087. \*TUBERIA VAPOR BAJA PRESION GVRC1 A TURBINA\*\*\*\*\*  
1088. 584 CON GY -4.817 1.47  
1089. 584 CON GX -0.124 1.47  
1090. 564 CON GY -4.649 1.47  
1091. 564 CON GZ 0.279 1.47  
1092. 540 CON GY -6.406 1.47  
1093. 540 CON GZ -0.241 1.47  
1094. 1033 CON GY -0.092 1.47  
1095. 1034 CON GY -12.809 1.47  
1096. 487 CON GY -12.333 1.47  
1097. \*TUBERIA VAPOR REC FRIO TURBINA A GVRC2\*\*\*\*\*  
1098. 889 1021 CON GY -38.654 4.05  
1099. 690 CON GY -70.838 3.195  
1100. 678 CON GY -8.545 3.195  
1101. 666 CON GY -17.578 3.195  
1102. 666 CON GZ -0.059 3.195  
1103. 1611 CON GY -31.799 2  
1104. 1667 CON GY -52.62 3.195

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1105. 1027 CON GY -23.579 2.35  
1106. 1697 CON GY -33.53 1.05  
1107. 642 CON GY -57.887 3.195  
1108. 620 CON GY -56.551 3.195  
1109. 596 CON GY -40.369 3.195  
1110. 584 CON GY -60.362 3.195  
1111. 584 CON GX 1.423 3.195  
1112. 564 CON GY -28.343 3.195  
1113. 540 CON GY -45.777 3.195  
1114. 1034 CON GY -62.872 3.195  
1115. 487 CON GY -52.238 3.195  
1116. \*TUBERIA VAPOR REC. CALIENTE DE GVRC2 A TURBINA\*\*\*\*\*  
1117. 889 CON GY -64.68 5.325

1118. 1804 CON GY -52.161 1.8  
1119. 678 CON GY -7.914 5.8  
1120. 666 CON GY -42.193 5.8  
1121. 1611 CON GY -33.955 0.7  
1122. 1667 CON GY -28.743 5.8  
1123. 1981 CON GY -41.245 1.15  
1124. 1697 CON GY -35.386 1.4  
1125. 642 CON GY -47.841 5.8  
1126. 620 CON GY -43.61 5.8  
1127. 1689 CON GY -15.852 1.8  
1128. 2546 CON GY -32.374 0.4  
1129. 2546 CON GX -0.199 0.4  
1130. 1685 CON GY -24.637 1.8  
1131. 540 CON GY -26.685 5.8  
1132. 1034 CON GY -26.06 5.8  
1133. 1034 CON GY -29.294 5.8  
1134. 499 CON GY -45.36 5.8  
1135. 487 CON GY -36.087 5.8  
1136. \*TUBERIA VAPOR RECALENTADO CALIENTE DE GVRC1 A TURB.\*\*\*\*\*  
1137. 1021 CON GY -19.894 5.325  
1138. 1686 CON GY -47.058 0.47  
1139. 1685 CON GY -14.783 0.47  
1140. 540 CON GY -26.181 4.47  
1141. 1034 CON GY -44.608 4.47  
1142. 1034 CON GY -32.309 4.47  
1143. 487 CON GY -52.377 4.47  
1144. \*TUBERIA VAPOR PRINCIPAL\*\*\*\*\*  
1145. MEMBER LOAD  
1146. 889 1021 CON GY -48.416 1  
1147. 1804 CON GY -49.452 3  
1148. 1804 CON GX 0.492 3  
1149. 678 CON GY -19.129 7  
1150. 666 CON GY -34.542 7  
1151. 711 CON GY -26.391 2  
1152. 1737 CON GY -28.718 0.3  
1153. 1988 CON GY -37.433 0.65  
1154. 1988 CON GZ -0.132 0.65  
1155. 642 CON GY -28.228 7

1156. 620 CON GY -47.456 7  
1157. 620 CON GZ 1.506 7  
1158. 2546 CON GY -44.472 1.6  
1159. 2546 CON GX -0.938 1.6  
1160. 1685 CON GY -21.355 3

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1161. 540 CON GY -23.591 7  
1162. 1033 CON GY -22.735 7  
1163. 1034 CON GY -28.084 7  
1164. 1021 CON GY -48.055 1  
1165. 2549 CON GY -4.504 1  
1166. JOINT LOAD  
1167. 905 FY -20.903  
1168. MEMBER LOAD  
1169. 1696 CON GY -24.323 7  
1170. 1023 CON GY -46.341 5.325  
1171. LOAD 7 LOADTYPE SEISMIC TITLE SISMO EN DIRECCION HORIZONTAL X (EX)  
1172. SPECTRUM SRSS X 1 ACC SCALE 16 DAMP 0.05 LIN  
1173. 0 0.057; 0.05 0.136; 0.1 0.206; 0.4 0.129; 0.5 0.111; 0.6 0.098; 0.7 0.089  
1174. 0.8 0.08; 0.9 0.075; 1 0.07; 1.5 0.053; 2 0.044; 2.5 0.038; 3 0.016  
1175. 3.5 0.0008; 4 0.004; 4.5 0.002; 5 0.001; 6 0.001; 7 0.0004; 8 0.0002  
1176. 9 0.0001  
1177. SELFWEIGHT Y 1  
1178. SELFWEIGHT X 1  
1179. SELFWEIGHT Z 1  
1180. \*\*\*\*\*POR CARGA MUERTA\*\*\*\*\*  
1181. MEMBER LOAD  
1182. 601 633 2063 2561 UNI GY 0.2245  
1183. 1056 1062 1065 1073 2069 UNI GY 0.4489  
1184. 1113 2633 2636 2638 UNI GY 0.216  
1185. 1110 2610 UNI GY 0.432  
1186. 569 1086 UNI GY 0.453  
1187. 1094 1099 1104 UNI GY 0.477  
1188. 1051 1080 2606 UNI GY 0.238  
1189. 589 UNI GY 0.462  
1190. 1053 1055 UNI GY 0.1725  
1191. 1079 UNI GY 0.427

1192. 1076 2055 UNI GY 0.49  
1193. 612 UNI GY 0.245  
1194. 2067 2070 2083 2096 UNI GY 0.259  
1195. 2082 UNI GY 0.518  
1196. 2072 UNI GY 0.457  
1197. 2075 2078 2081 UNI GY 0.466  
1198. 635 2065 2557 UNI GY 0.233  
1199. 608 612 2369 2392 2705 2707 2708 UNI GY 0.383  
1200. 603 610 2086 2087 2327 2329 2330 2336 2337 2369 2370 2414 2415 2430 -  
1201. 2431 UNI GY 0.25  
1202. 2363 TO 2366 2450 2453 2454 UNI GY 0.5  
1203. 2347 2348 2367 2368 2377 2378 2390 2391 2422 2423 2447 2448 2473 TO 2480 2485 -  
1204. 2486 TO 2500 UNI GY 0.157  
1205. 2647 TO 2650 CON GY 3.25 0.7  
1206. 2657 2658 2661 2662 CON GY 1.425 0.835  
1207. 493 659 671 683 742 1447 1507 1516 1522 1525 UNI GY 5.563 3.9 4.8  
1208. 2571 UNI GY 5.563 1.25 2.15  
1209. 1105 1995 1996 2019 2020 2029 UNI GY 5.563 0.001 0.8  
1210. \*\*\*\*\*  
1211. 601 633 2063 2561 UNI GX 0.2245  
1212. 1056 1062 1065 1073 2069 UNI GX 0.4489  
1213. 1113 2633 2636 2638 UNI GX 0.216  
1214. 1110 2610 UNI GX 0.432  
1215. 569 1086 UNI GX 0.453  
1216. 1094 1099 1104 UNI GX 0.477

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1217. 1051 1080 2606 UNI GX 0.238  
1218. 589 UNI GX 0.462  
1219. 1053 1055 UNI GX 0.1725  
1220. 1079 UNI GX 0.427  
1221. 1076 2055 UNI GX 0.49  
1222. 612 UNI GX 0.245  
1223. 2067 2070 2083 2096 UNI GX 0.259  
1224. 2082 UNI GX 0.518  
1225. 2072 UNI GX 0.457  
1226. 2075 2078 2081 UNI GX 0.466  
1227. 635 2065 2557 UNI GX 0.233

1228. 608 612 2369 2392 2705 2707 2708 UNI GX 0.383  
1229. 603 610 2086 2087 2327 2329 2330 2336 2337 2369 2370 2414 2415 2430 -  
1230. 2431 UNI GX 0.25  
1231. 2363 TO 2366 2450 2453 2454 UNI GX 0.5  
1232. 2347 2348 2367 2368 2377 2378 2390 2391 2422 2423 2447 2448 2473 TO 2480 2485 -  
1233. 2486 TO 2500 UNI GX 0.157  
1234. 2647 TO 2650 CON GX 3.25 0.7  
1235. 2657 2658 2661 2662 CON GX 1.425 0.835  
1236. 493 659 671 683 742 1447 1507 1516 1522 1525 UNI GX 5.563 3.9 4.8  
1237. 2571 UNI GX 5.563 1.25 2.15  
1238. 1105 1995 1996 2019 2020 2029 UNI GX 5.563 0.001 0.8  
1239. \*\*\*\*\*  
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1244. 569 1086 UNI GZ 0.453  
1245. 1094 1099 1104 UNI GZ 0.477  
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1253. 2082 UNI GZ 0.518  
1254. 2072 UNI GZ 0.457  
1255. 2075 2078 2081 UNI GZ 0.466  
1256. 635 2065 2557 UNI GZ 0.233  
1257. 608 612 2369 2392 2705 2707 2708 UNI GZ 0.383  
1258. 603 610 2086 2087 2327 2329 2330 2336 2337 2369 2370 2414 2415 2430 -  
1259. 2431 UNI GZ 0.25  
1260. 2363 TO 2366 2450 2453 2454 UNI GZ 0.5  
1261. 2347 2348 2367 2368 2377 2378 2390 2391 2422 2423 2447 2448 2473 TO 2480 2485 -  
1262. 2486 TO 2500 UNI GZ 0.157  
1263. 2647 TO 2650 CON GZ 3.25 0.7  
1264. 2657 2658 2661 2662 CON GZ 1.425 0.835  
1265. 493 659 671 683 742 1447 1507 1516 1522 1525 UNI GZ 5.563 3.9 4.8



1266. 2571 UNI GZ 5.563 1.25 2.15  
1267. 1105 1995 1996 2019 2020 2029 UNI GZ 5.563 0.001 0.8  
1268. \*\*\*\*\*POR CHAROLAS ELECTRICAS\*\*\*\*\*  
1269. MEMBER LOAD  
1270. 493 524 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
1271. 1525 2575 CON GY 5.3 1.85  
1272. 493 524 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
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1273. 1525 2575 CON GY 5.3 2.45  
1274. 493 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
1275. 1525 2575 CON GY 3.532 3.15  
1276. 2571 CON GY 3.532 0.5  
1277. 493 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
1278. 1525 2575 CON GY 3.532 3.85  
1279. 2571 CON GY 3.532 1.2  
1280. 493 659 671 683 742 758 1447 1507 1516 1522 1525 2575 CON GY 3.532 4.85  
1281. 2571 CON GY 3.532 2.2  
1282. 1995 1996 2019 2020 2029 2030 CON GY 3.532 0.85  
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1284. 2574 CON GY 3.532 0.200002  
1285. 1995 1996 2019 2020 2029 2030 CON GY 3.532 1.55  
1286. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
1287. 2603 CON GY 5.3 0.05  
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1289. 2603 CON GY 5.3 0.65  
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1291. 2603 CON GY 3.532 0.7  
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1293. 2603 CON GY 3.532 1.3  
1294. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
1295. 2603 CON GY 3.532 2.2  
1296. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
1297. 2603 CON GY 3.532 2.8  
1298. \*\*\*\*\*  
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1301. 1525 2575 CON GX 5.3 1.85

1302. 493 524 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
1303. 1525 2575 CON GX 5.3 2.45  
1304. 493 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
1305. 1525 2575 CON GX 3.532 3.15  
1306. 2571 CON GX 3.532 0.5  
1307. 493 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
1308. 1525 2575 CON GX 3.532 3.85  
1309. 2571 CON GX 3.532 1.2  
1310. 493 659 671 683 742 758 1447 1507 1516 1522 1525 2575 CON GX 3.532 4.85  
1311. 2571 CON GX 3.532 2.2  
1312. 1995 1996 2019 2020 2029 2030 CON GX 3.532 0.85  
1313. 493 659 671 683 742 758 1447 1507 1516 1522 1525 2575 CON GX 3.532 5.55  
1314. 2574 CON GX 3.532 0.200002  
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1319. 2603 CON GX 5.3 0.65  
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1323. 2603 CON GX 3.532 1.3  
1324. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
1325. 2603 CON GX 3.532 2.2  
1326. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
1327. 2603 CON GX 3.532 2.8  
1328. \*\*\*\*\*

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1329. MEMBER LOAD  
1330. 493 524 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -  
1331. 1525 2575 CON GZ 5.3 1.85  
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1333. 1525 2575 CON GZ 5.3 2.45  
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1335. 1525 2575 CON GZ 3.532 3.15  
1336. 2571 CON GZ 3.532 0.5  
1337. 493 659 671 683 742 758 1440 1447 1452 1504 1507 1510 1513 1516 1519 1522 -

1338. 1525 2575 CON GZ 3.532 3.85  
1339. 2571 CON GZ 3.532 1.2  
1340. 493 659 671 683 742 758 1447 1507 1516 1522 1525 2575 CON GZ 3.532 4.85  
1341. 2571 CON GZ 3.532 2.2  
1342. 1995 1996 2019 2020 2029 2030 CON GZ 3.532 0.85  
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1344. 2574 CON GZ 3.532 0.200002  
1345. 1995 1996 2019 2020 2029 2030 CON GZ 3.532 1.55  
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1347. 2603 CON GZ 5.3 0.05  
1348. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
1349. 2603 CON GZ 5.3 0.65  
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1353. 2603 CON GZ 3.532 1.3  
1354. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
1355. 2603 CON GZ 3.532 2.2  
1356. 1105 1599 2101 2108 2110 2114 2116 2118 2125 2131 2134 2555 2583 2593 2600 -  
1357. 2603 CON GZ 3.532 2.8  
1358. \*\*\*\*\*POR CARGA VIVA\*\*\*\*\*  
1359. 601 633 2063 2561 UNI GY 3.188  
1360. 1056 1062 1065 1073 2069 UNI GY 6.377  
1361. 1113 2633 2636 2638 UNI GY 3.066  
1362. 1110 2610 UNI GY 6.132  
1363. 569 1086 UNI GY 6.438  
1364. 1094 1099 1104 UNI GY 6.744  
1365. 1051 1080 2606 UNI GY 3.372  
1366. 589 UNI GY 6.56  
1367. 1053 1055 UNI GY 2.453  
1368. 1079 UNI GY 5.95  
1369. 1076 2055 UNI GY 6.99  
1370. 612 UNI GY 3.495  
1371. 2067 2070 2083 2096 UNI GY 3.679  
1372. 2082 UNI GY 7.358  
1373. 2072 UNI GY 6.5  
1374. 2075 2078 2081 UNI GY 6.622  
1375. 635 2065 2557 UNI GY 3.311

1376. 608 612 2369 2392 2705 2707 2708 UNI GY 5.445  
1377. 603 610 2086 2087 2327 2329 2330 2336 2337 2369 2370 2414 2415 2430 -  
1378. 2431 UNI GY 3.483  
1379. 2363 TO 2366 2450 2453 2454 UNI GY 6.966  
1380. 2347 2348 2367 2368 2377 2378 2390 2391 2422 2423 2447 2448 2473 TO 2480 2485 -  
1381. 2486 TO 2500 UNI GY 2.23  
1382. 493 659 671 683 742 1447 1507 1516 1522 1525 UNI GY 14.715 3.9 4.8  
1383. 2571 UNI GY 14.715 1.25 2.15  
1384. 1105 1995 1996 2019 2020 2029 UNI GY 14.715 0.001 0.8

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1385. \*\*\*\*\*  
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1399. 2082 UNI GX 7.358  
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1401. 2075 2078 2081 UNI GX 6.622  
1402. 635 2065 2557 UNI GX 3.311  
1403. 608 612 2369 2392 2705 2707 2708 UNI GX 5.445  
1404. 603 610 2086 2087 2327 2329 2330 2336 2337 2369 2370 2414 2415 2430 -  
1405. 2431 UNI GX 3.483  
1406. 2363 TO 2366 2450 2453 2454 UNI GX 6.966  
1407. 2347 2348 2367 2368 2377 2378 2390 2391 2422 2423 2447 2448 2473 TO 2480 2485 -  
1408. 2486 TO 2500 UNI GX 2.23  
1409. 493 659 671 683 742 1447 1507 1516 1522 1525 UNI GX 14.715 3.9 4.8  
1410. 2571 UNI GX 14.715 1.25 2.15  
1411. 1105 1995 1996 2019 2020 2029 UNI GX 14.715 0.001 0.8

1412. \*\*\*\*\*  
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 1426. 2082 UNI GZ 7.358  
 1427. 2072 UNI GZ 6.5  
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 1434. 2347 2348 2367 2368 2377 2378 2390 2391 2422 2423 2447 2448 2473 TO 2480 2485 -  
 1435. 2486 TO 2500 UNI GZ 2.23  
 1436. 493 659 671 683 742 1447 1507 1516 1522 1525 UNI GZ 14.715 3.9 4.8  
 1437. 2571 UNI GZ 14.715 1.25 2.15  
 1438. 1105 1995 1996 2019 2020 2029 UNI GZ 14.715 0.001 0.8  
 1439. \*\*\*\*\*POR PESOS DE TUBERIAS\*\*\*\*\*  
 1440. MEMBER LOAD  
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 1441. 486 509 542 566 586 598 1484 1903 TO 1906 1915 TO 1918 UNI GY 10.791  
 1442. 512 541 565 585 597 621 622 667 1022 1471 1668 1907 1919 2687 2691 2692 2695 -  
 1443. 2698 2699 2704 2706 2713 TO 2715 2718 UNI GY 0.981  
 1444. 499 644 656 668 680 692 1669 1708 1711 1908 TO 1914 1920 TO 1924 2723 TO 2725 -  
 1445. 2726 UNI GY 7.848  
 1446. 750 891 1023 UNI GY 1.4715  
 1447. 498 UNI GY 9.81

1448. 643 655 679 691 1702 1705 2690 2700 TO 2703 2712 2716 2717 UNI GY 0.981  
1449. 890 UNI GY 2.943  
1450. \*TUBERIA VAPOR BAJA PRESION GVRC2 A TURBINA\*\*\*\*\*  
1451. MEMBER LOAD  
1452. 889 1021 CON GY 3.457 3  
1453. 690 CON GY 3.348 2.195  
1454. 678 CON GY 4.073 2.195  
1455. 1621 CON GY 5.57 0.5  
1456. 1027 2509 CON GY 9.81 1.35  
1457. 1751 CON GY 5.737 1.40001  
1458. 642 CON GY 5.74 2.195  
1459. 620 CON GY 5.655 2.195  
1460. 596 CON GY 1.694 2.195  
1461. 584 CON GY 4.09 2.195  
1462. 564 CON GY 2.816 2.195  
1463. 540 CON GY 4.163 2.195  
1464. 1033 CON GY 0.691 2.195  
1465. 1034 CON GY 7.396 2.195  
1466. 487 CON GY 7.868 2.195  
1467. \*TUBERIA VAPOR BAJA PRESION GVRC1 A TURBINA\*\*\*\*\*  
1468. 584 CON GY 3.096 1.47  
1469. 564 CON GY 3.062 1.47  
1470. 540 CON GY 4.264 1.47  
1471. 1033 CON GY 0.226 1.47  
1472. 1034 CON GY 8.325 1.47  
1473. 487 CON GY 9.041 1.47  
1474. \*TUBERIA VAPOR REC FRIO TURBINA A GVRC2\*\*\*\*\*  
1475. 889 1021 CON GY 9.123 4.05  
1476. 690 CON GY 39.035 3.195  
1477. 678 CON GY 13.855 3.195  
1478. 666 CON GY 10.372 3.195  
1479. 1611 CON GY 19.276 2  
1480. 1667 CON GY 31.149 3.195  
1481. 1027 CON GY 13.845 2.35  
1482. 1697 CON GY 20.394 1.05  
1483. 642 CON GY 34.071 3.195  
1484. 620 CON GY 38.406 3.195  
1485. 596 CON GY 29.687 3.195

1486. 584 CON GY 39.169 3.195  
1487. 564 CON GY 17.18 3.195  
1488. 540 CON GY 24.314 3.195  
1489. 1034 CON GY 34.417 3.195  
1490. \*TUBERIA VAPOR REC. CALIENTE DE GVRC2 A TURBINA\*\*\*\*\*  
1491. 889 CON GY 46.21 5.325  
1492. 1804 CON GY 33.62 1.8  
1493. 678 CON GY 6.09 5.8  
1494. 666 CON GY 25.345 5.8  
1495. 1611 CON GY 21.893 0.7  
1496. 1667 CON GY 17.91 5.8

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1497. 1981 CON GY 26.145 1.15  
1498. 1697 CON GY 22.333 1.4  
1499. 642 CON GY 30.094 5.8  
1500. 620 CON GY 27.47 5.8  
1501. 1689 CON GY 9.932 1.8  
1502. 2546 CON GY 20.597 0.4  
1503. 1685 CON GY 16.122 1.8  
1504. 540 CON GY 18.169 5.8  
1505. 1033 CON GY 16.453 5.8  
1506. 1034 CON GY 18.11 5.8  
1507. 499 CON GY 42.736 5.8  
1508. 487 CON GY 23.508 5.8  
1509. \*TUBERIA VAPOR RECALENTADO CALIENTE DE GVRC1 A TURB.\*\*\*\*\*  
1510. 1021 CON GY 19.894 5.325  
1511. 1686 CON GY 23.888 0.47  
1512. 1685 CON GY 11.919 0.47  
1513. 540 CON GY 19.817 4.47  
1514. 1033 CON GY 18.725 4.47  
1515. 1034 CON GY 19.996 4.47  
1516. 499 CON GY 37.075 4.47  
1517. 487 CON GY 36.248 4.47  
1518. \*TUBERIA VAPOR PRINCIPAL\*\*\*\*\*  
1519. MEMBER LOAD  
1520. 889 1021 CON GY 45.94 1  
1521. 1804 CON GY 47.642 3

1522. 678 CON GY 19.129 7  
1523. 666 CON GY 32.028 7  
1524. 711 CON GY 24.732 2  
1525. 1737 CON GY 26.808 0.3  
1526. 1988 CON GY 34.982 0.65  
1527. 642 CON GY 26.357 7  
1528. 620 CON GY 44.777 7  
1529. 2546 CON GY 41.716 1.6  
1530. 1685 CON GY 18.883 3  
1531. 540 CON GY 21.953 7  
1532. 1033 CON GY 21.227 7  
1533. 1034 CON GY 25.761 7  
1534. 1021 CON GY 43.855 1  
1535. 2549 CON GY 6.065 1  
1536. JOINT LOAD  
1537. 905 FY 12.529  
1538. MEMBER LOAD  
1539. 1696 CON GY 15.584 7  
1540. 1023 CON GY 29.269 5.325  
1541. 2671 CON GY 22.831 2.075  
1542. 2668 2669 2672 TO 2674 2679 TO 2684 2719 TO 2722 UNI GY 0.98  
1543. \*\*\*\*\*EN X\*\*\*\*\*  
1544. MEMBER LOAD  
1545. 486 509 542 566 586 598 1484 1903 TO 1906 1915 TO 1918 UNI GX 10.791  
1546. 512 541 565 585 597 621 622 667 1022 1471 1668 1907 1919 2687 2691 2692 2695 -  
1547. 2698 2699 2704 2706 2713 TO 2715 2718 UNI GX 0.981  
1548. 499 644 656 668 680 692 1669 1708 1711 1908 TO 1914 1920 TO 1924 2723 TO 2725 -  
1549. 2726 UNI GX 7.848  
1550. 750 891 1023 UNI GX 1.4715  
1551. 498 UNI GX 9.81  
1552. 643 655 679 691 1702 1705 2690 2700 TO 2703 2712 2716 2717 UNI GX 0.981  
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1553. 890 UNI GX 2.943  
1554. \*TUBERIA VAPOR BAJA PRESION GVRC2 A TURBINA\*\*\*\*\*  
1555. MEMBER LOAD  
1556. 889 1021 CON GX 3.457 3  
1557. 690 CON GX 3.348 2.195



1558. 678 CON GX 4.073 2.195  
1559. 1621 CON GX 5.57 0.5  
1560. 1027 2509 CON GX 9.81 1.35  
1561. 1751 CON GX 5.737 1.40001  
1562. 642 CON GX 5.74 2.195  
1563. 620 CON GX 5.655 2.195  
1564. 596 CON GX 1.694 2.195  
1565. 584 CON GX 4.09 2.195  
1566. 564 CON GX 2.816 2.195  
1567. 540 CON GX 4.163 2.195  
1568. 1033 CON GX 0.691 2.195  
1569. 1034 CON GX 7.396 2.195  
1570. 487 CON GX 7.868 2.195  
1571. \*TUBERIA VAPOR BAJA PRESION GVRC1 A TURBINA\*\*\*\*\*  
1572. 584 CON GX 3.096 1.47  
1573. 564 CON GX 3.062 1.47  
1574. 540 CON GX 4.264 1.47  
1575. 1033 CON GX 0.226 1.47  
1576. 1034 CON GX 8.325 1.47  
1577. 487 CON GX 9.041 1.47  
1578. \*TUBERIA VAPOR REC FRIO TURBINA A GVRC2\*\*\*\*\*  
1579. 889 1021 CON GX 9.123 4.05  
1580. 690 CON GX 39.035 3.195  
1581. 678 CON GX 13.855 3.195  
1582. 666 CON GX 10.372 3.195  
1583. 1611 CON GX 19.276 2  
1584. 1667 CON GX 31.149 3.195  
1585. 1027 CON GX 13.845 2.35  
1586. 1697 CON GX 20.394 1.05  
1587. 642 CON GX 34.071 3.195  
1588. 620 CON GX 38.406 3.195  
1589. 596 CON GX 29.687 3.195  
1590. 584 CON GX 39.169 3.195  
1591. 564 CON GX 17.18 3.195  
1592. 540 CON GX 24.314 3.195  
1593. 1034 CON GX 34.417 3.195  
1594. \*TUBERIA VAPOR REC. CALIENTE DE GVRC2 A TURBINA\*\*\*\*\*  
1595. 889 CON GX 46.21 5.325

1596. 1804 CON GX 33.62 1.8  
1597. 678 CON GX 6.09 5.8  
1598. 666 CON GX 25.345 5.8  
1599. 1611 CON GX 21.893 0.7  
1600. 1667 CON GX 17.91 5.8  
1601. 1981 CON GX 26.145 1.15  
1602. 1697 CON GX 22.333 1.4  
1603. 642 CON GX 30.094 5.8  
1604. 620 CON GX 27.47 5.8  
1605. 1689 CON GX 9.932 1.8  
1606. 2546 CON GX 20.597 0.4  
1607. 1685 CON GX 16.122 1.8  
1608. 540 CON GX 18.169 5.8

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1609. 1033 CON GX 16.453 5.8  
1610. 1034 CON GX 18.11 5.8  
1611. 499 CON GX 42.736 5.8  
1612. 487 CON GX 23.508 5.8  
1613. \*TUBERIA VAPOR RECALENTADO CALIENTE DE GVRC1 A TURB.\*\*\*\*\*  
1614. 1021 CON GX 19.894 5.325  
1615. 1686 CON GX 23.888 0.47  
1616. 1685 CON GX 11.919 0.47  
1617. 540 CON GX 19.817 4.47  
1618. 1033 CON GX 18.725 4.47  
1619. 1034 CON GX 19.996 4.47  
1620. 499 CON GX 37.075 4.47  
1621. 487 CON GX 36.248 4.47  
1622. \*TUBERIA VAPOR PRINCIPAL\*\*\*\*\*  
1623. MEMBER LOAD  
1624. 889 1021 CON GX 45.94 1  
1625. 1804 CON GX 47.642 3  
1626. 678 CON GX 19.129 7  
1627. 666 CON GX 32.028 7  
1628. 711 CON GX 24.732 2  
1629. 1737 CON GX 26.808 0.3  
1630. 1988 CON GX 34.982 0.65  
1631. 642 CON GX 26.357 7

1632. 620 CON GX 44.777 7  
1633. 2546 CON GX 41.716 1.6  
1634. 1685 CON GX 18.883 3  
1635. 540 CON GX 21.953 7  
1636. 1033 CON GX 21.227 7  
1637. 1034 CON GX 25.761 7  
1638. 1021 CON GX 43.855 1  
1639. 2549 CON GX 6.065 1  
1640. JOINT LOAD  
1641. 905 FX 12.529  
1642. MEMBER LOAD  
1643. 1696 CON GX 15.584 7  
1644. 1023 CON GX 29.269 5.325  
1645. 2671 CON GX 22.831 2.075  
1646. 2668 2669 2672 TO 2674 2679 TO 2684 2719 TO 2722 UNI GX 0.98  
1647. \*\*\*\*\*EN Z\*\*\*\*\*  
1648. MEMBER LOAD  
1649. 486 509 542 566 586 598 1484 1903 TO 1906 1915 TO 1918 UNI GZ 10.791  
1650. 512 541 565 585 597 621 622 667 1022 1471 1668 1907 1919 2687 2691 2692 2695 -  
1651. 2698 2699 2704 2706 2713 TO 2715 2718 UNI GZ 0.981  
1652. 499 644 656 668 680 692 1669 1708 1711 1908 TO 1914 1920 TO 1924 2723 TO 2725 -  
1653. 2726 UNI GZ 7.848  
1654. 750 891 1023 UNI GZ 1.4715  
1655. 498 UNI GZ 9.81  
1656. 643 655 679 691 1702 1705 2690 2700 TO 2703 2712 2716 2717 UNI GZ 0.981  
1657. 890 UNI GZ 2.943  
1658. \*TUBERIA VAPOR BAJA PRESION GVRC2 A TURBINA\*\*\*\*\*  
1659. MEMBER LOAD  
1660. 889 1021 CON GZ 3.457 3  
1661. 690 CON GZ 3.348 2.195  
1662. 678 CON GZ 4.073 2.195  
1663. 1621 CON GZ 5.57 0.5  
1664. 1027 2509 CON GZ 9.81 1.35

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1665. 1751 CON GZ 5.737 1.40001  
1666. 642 CON GZ 5.74 2.195  
1667. 620 CON GZ 5.655 2.195

1668. 596 CON GZ 1.694 2.195  
1669. 584 CON GZ 4.09 2.195  
1670. 564 CON GZ 2.816 2.195  
1671. 540 CON GZ 4.163 2.195  
1672. 1033 CON GZ 0.691 2.195  
1673. 1034 CON GZ 7.396 2.195  
1674. 487 CON GZ 7.868 2.195  
1675. \*TUBERIA VAPOR BAJA PRESION GVRC1 A TURBINA\*\*\*\*\*  
1676. 584 CON GZ 3.096 1.47  
1677. 564 CON GZ 3.062 1.47  
1678. 540 CON GZ 4.264 1.47  
1679. 1033 CON GZ 0.226 1.47  
1680. 1034 CON GZ 8.325 1.47  
1681. 487 CON GZ 9.041 1.47  
1682. \*TUBERIA VAPOR REC FRIO TURBINA A GVRC2\*\*\*\*\*  
1683. 889 1021 CON GZ 9.123 4.05  
1684. 690 CON GZ 39.035 3.195  
1685. 678 CON GZ 13.855 3.195  
1686. 666 CON GZ 10.372 3.195  
1687. 1611 CON GZ 19.276 2  
1688. 1667 CON GZ 31.149 3.195  
1689. 1027 CON GZ 13.845 2.35  
1690. 1697 CON GZ 20.394 1.05  
1691. 642 CON GZ 34.071 3.195  
1692. 620 CON GZ 38.406 3.195  
1693. 596 CON GZ 29.687 3.195  
1694. 584 CON GZ 39.169 3.195  
1695. 564 CON GZ 17.18 3.195  
1696. 540 CON GZ 24.314 3.195  
1697. 1034 CON GZ 34.417 3.195  
1698. \*TUBERIA VAPOR REC. CALIENTE DE GVRC2 A TURBINA\*\*\*\*\*  
1699. 889 CON GZ 46.21 5.325  
1700. 1804 CON GZ 33.62 1.8  
1701. 678 CON GZ 6.09 5.8  
1702. 666 CON GZ 25.345 5.8  
1703. 1611 CON GZ 21.893 0.7  
1704. 1667 CON GZ 17.91 5.8  
1705. 1981 CON GZ 26.145 1.15

1706. 1697 CON GZ 22.333 1.4  
1707. 642 CON GZ 30.094 5.8  
1708. 620 CON GZ 27.47 5.8  
1709. 1689 CON GZ 9.932 1.8  
1710. 2546 CON GZ 20.597 0.4  
1711. 1685 CON GZ 16.122 1.8  
1712. 540 CON GZ 18.169 5.8  
1713. 1033 CON GZ 16.453 5.8  
1714. 1034 CON GZ 18.11 5.8  
1715. 499 CON GZ 42.736 5.8  
1716. 487 CON GZ 23.508 5.8  
1717. \*TUBERIA VAPOR RECALENTADO CALIENTE DE GVRC1 A TURB.\*\*\*\*\*  
1718. 1021 CON GZ 19.894 5.325  
1719. 1686 CON GZ 23.888 0.47  
1720. 1685 CON GZ 11.919 0.47

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1721. 540 CON GZ 19.817 4.47  
1722. 1033 CON GZ 18.725 4.47  
1723. 1034 CON GZ 19.996 4.47  
1724. 499 CON GZ 37.075 4.47  
1725. 487 CON GZ 36.248 4.47  
1726. \*TUBERIA VAPOR PRINCIPAL\*\*\*\*\*  
1727. MEMBER LOAD  
1728. 889 1021 CON GZ 45.94 1  
1729. 1804 CON GZ 47.642 3  
1730. 678 CON GZ 19.129 7  
1731. 666 CON GZ 32.028 7  
1732. 711 CON GZ 24.732 2  
1733. 1737 CON GZ 26.808 0.3  
1734. 1988 CON GZ 34.982 0.65  
1735. 642 CON GZ 26.357 7  
1736. 620 CON GZ 44.777 7  
1737. 2546 CON GZ 41.716 1.6  
1738. 1685 CON GZ 18.883 3  
1739. 540 CON GZ 21.953 7  
1740. 1033 CON GZ 21.227 7  
1741. 1034 CON GZ 25.761 7

1742. 1021 CON GZ 43.855 1  
1743. 2549 CON GZ 6.065 1  
1744. JOINT LOAD  
1745. 905 FZ 12.529  
1746. MEMBER LOAD  
1747. 1696 CON GZ 15.584 7  
1748. 1023 CON GZ 29.269 5.325  
1749. 2671 CON GZ 22.831 2.075  
1750. 2668 2669 2672 TO 2674 2679 TO 2684 2719 TO 2722 UNI GZ 0.98  
1751. LOAD 8 LOADTYPE SEISMIC TITLE SISMO EN DIRECCION HORIZONTAL Z (EX)  
1752. SPECTRUM SRSS Z 1 ACC SCALE 19 DAMP 0.05 LIN  
1753. 0 0.089; 0.05 0.183; 0.1 0.257; 0.4 0.161; 0.5 0.138; 0.6 0.123; 0.7 0.111  
1754. 0.8 0.101; 0.9 0.094; 1 0.087; 1.5 0.067; 2 0.055; 2.5 0.047; 3 0.02  
1755. 3.5 0.01; 4 0.005; 4.5 0.003; 5 0.002  
1756. LOAD 9 LOADTYPE SEISMIC TITLE SISMO EN DIRECCION VERTICAL Y (EY)  
1757. SPECTRUM SRSS Y 1 ACC SCALE 9.81 DAMP 0.05 LIN  
1758. \*0 0.089; 0.05 0.183; 0.1 0.257; 0.4 0.161; 0.5 0.138; 0.6 0.123; 0.7 0.111  
1759. \*0.8 0.101; 0.9 0.094; 1 0.087; 1.5 0.067; 2 0.055; 2.5 0.047; 3 0.020  
1760. \*3.5 0.010; 4 0.005; 4.5 0.003; 5 0.002  
1761. 0 0.0318; 0.05 0.0758; 0.1 0.1146; 0.4 0.0717; 0.5 0.0618; 0.6 0.0547  
1762. 0.7 0.0493; 0.8 0.0451; 0.9 0.0417; 1 0.0389; 1.5 0.0297; 2 0.0245; 2.5 0.0211  
1763. 3 0.009; 3.5 0.0044; 4 0.0024; 4.5 0.0014; 5 0.0008; 6 0.0004; 7 0.0002  
1764. LOAD 10 LOADTYPE WIND TITLE VIENTO EN DIRECCION X (WX)  
1765. \*TUBERIA VAPOR BAJA PRESION GVRC2 A TURBINA\*\*\*\*\*  
1766. MEMBER LOAD  
1767. 889 1021 CON GY -1.081 3  
1768. 690 CON GY -0.387 2.195  
1769. 690 CON GX 7.584 2.195  
1770. 678 CON GY -0.588 2.195  
1771. 678 CON GZ 1.621 2.195  
1772. 666 CON GY -0.501 2.195  
1773. 1621 CON GY -0.242 0.5  
1774. 1027 2509 CON GY -0.04 1.35  
1775. 1751 CON GY -0.248 1.4  
1776. 642 CON GY -0.291 2.195  
  
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1777. 620 CON GY -0.172 2.195

1778. 596 CON GY -0.092 2.195  
1779. 584 CON GY -0.005 2.195  
1780. 584 CON GX 8.379 2.195  
1781. 564 CON GY -0.101 2.195  
1782. 540 CON GY -0.347 2.195  
1783. 1033 CON GY -1.27 2.195  
1784. 1034 CON GY -1.832 2.195  
1785. 487 CON GY -0.615 2.195  
1786. \*TUBERIA VAPOR BAJA PRESION GVRC1 A TURBINA\*\*\*\*\*  
1787. 584 CON GY -0.794 1.47  
1788. 584 CON GX 6.295 1.47  
1789. 564 CON GY -0.476 1.47  
1790. 540 CON GY -0.425 1.47  
1791. 1033 CON GY -1.185 1.47  
1792. 1034 CON GY -1.594 1.47  
1793. 487 CON GY -0.632 1.47  
1794. \*TUBERIA VAPOR REC FRIO TURBINA A GVRC2\*\*\*\*\*  
1795. 690 CON GY -4.062 3.195  
1796. 690 CON GX 19.041 3.195  
1797. 678 CON GY -2.737 3.195  
1798. 666 CON GY -1.995 3.195  
1799. 1611 CON GY -1.799 2  
1800. 1667 CON GY -0.291 3.195  
1801. 1027 CON GY -1.183 2.35  
1802. 1697 CON GY -1.256 1.05  
1803. 642 CON GY -1.343 3.195  
1804. 620 CON GY -2.862 3.195  
1805. 596 CON GY -2.648 3.195  
1806. 584 CON GY -1.135 3.195  
1807. 584 CON GX 41.828 3.195  
1808. 564 CON GY -1.599 3.195  
1809. 540 CON GY -2.955 3.195  
1810. 1034 CON GY -4.83 3.195  
1811. 487 CON GY -3.807 3.195  
1812. \*TUBERIA VAPOR REC. CALIENTE DE GVRC2 A TURBINA\*\*\*\*\*  
1813. 889 CON GY -2.237 5.325  
1814. 1804 CON GX 19.858 1.8  
1815. 678 CON GY -0.857 5.8

1816. 666 CON GY -0.876 5.8  
1817. 1611 CON GY -1.111 0.7  
1818. 1611 CON GX 0.121 0.7  
1819. 1667 CON GY -0.281 5.8  
1820. 1667 CON GX 0.147 5.8  
1821. 1981 CON GY -0.183 1.15  
1822. 1981 CON GX 0.018 1.15  
1823. 1697 CON GY -0.491 1.4  
1824. 1697 CON GX 0.102 1.4  
1825. 642 CON GY -0.149 5.8  
1826. 620 CON GY -0.056 5.8  
1827. 1689 CON GY -0.26 1.8  
1828. 2546 CON GY -1.85 0.399998  
1829. 2546 CON GX 57.554 0.399998  
1830. 1685 CON GY -1.515 1.8  
1831. 540 CON GY -5.477 5.8  
1832. 1033 CON GY -12.698 5.8

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1833. 499 CON GY -9.509 5.8  
1834. 487 CON GY -13.563 5.8  
1835. \*TUBERIA VAPOR RECALENTADO CALIENTE DE GVRC1 A TURB.\*\*\*\*\*  
1836. 1021 CON GY -19.894 5.325  
1837. 1686 CON GY -3.61 0.47  
1838. 1686 CON GX 48.377 0.47  
1839. 1685 CON GY -4.058 0.47  
1840. 1685 CON GX 25.744 0.47  
1841. 540 CON GY -3.223 4.47  
1842. 1033 CON GY -6.019 4.47  
1843. 499 CON GY -10.153 4.47  
1844. 487 CON GY -8.276 4.47  
1845. \*TUBERIA VAPOR PRINCIPAL\*\*\*\*\*  
1846. MEMBER LOAD  
1847. 889 1021 CON GY -4.549 1  
1848. 1804 CON GY -1.905 3  
1849. 1804 CON GX 20.206 3  
1850. 666 CON GY -0.777 7  
1851. 711 CON GY -0.278 2



1852. 1737 CON GY -0.543 0.3  
 1853. 1988 CON GY -0.553 0.65  
 1854. 642 CON GY -0.22 7  
 1855. 620 CON GY -0.178 7  
 1856. 620 CON GZ 24.855 7  
 1857. 2546 CON GY -3.522 1.6  
 1858. 2546 CON GX 28.645 1.6  
 1859. 1685 CON GY 0.914 3  
 1860. 540 CON GY -0.789 7  
 1861. 1033 CON GY -2.846 7  
 1862. 1034 CON GY -6.411 7  
 1863. 1021 CON GY -3.169 1  
 1864. 1021 CON GX 24.581 1  
 1865. 2549 CON GX 1.63 1  
 1866. JOINT LOAD  
 1867. 905 FY -1.732  
 1868. MEMBER LOAD  
 1869. 1696 CON GY -2.139 7  
 1870. 1023 CON GY 11.461 5.325  
 1871. 2671 CON GY 5.787 2.075  
 1872. \*\*\*\*\*  
 1873. 488 TO 491 500 TO 503 UNI GX 0.69  
 1874. 515 518 521 527 2570 2573 UNI GX 0.46  
 1875. 482 TO 484 492 494 TO 496 504 543 545 547 549 551 553 UNI GX 0.55  
 1876. 506 509 512 524 1034 2571 2574 UNI GX 0.37  
 1877. LOAD 11 LOADTYPE WIND TITLE VIENTO EN DIRECCION Z (WZ)  
 1878. \*TUBERIA VAPOR BAJA PRESION GVRC2 A TURBINA\*\*\*\*\*  
 1879. MEMBER LOAD  
 1880. 889 1021 CON GY -1.081 3  
 1881. 690 CON GY -0.387 2.195  
 1882. 690 CON GX 7.584 2.195  
 1883. 678 CON GY -0.588 2.195  
 1884. 678 CON GZ 1.621 2.195  
 1885. 666 CON GY -0.501 2.195  
 1886. 1621 CON GY -0.242 0.5  
 1887. 1027 2509 CON GY -0.04 1.35  
 1888. 1751 CON GY -0.248 1.4

1889. 642 CON GY -0.291 2.195  
1890. 620 CON GY -0.172 2.195  
1891. 596 CON GY -0.092 2.195  
1892. 584 CON GY -0.005 2.195  
1893. 584 CON GX 8.379 2.195  
1894. 564 CON GY -0.101 2.195  
1895. 540 CON GY -0.347 2.195  
1896. 1033 CON GY -1.27 2.195  
1897. 1034 CON GY -1.832 2.195  
1898. 487 CON GY -0.615 2.195  
1899. \*TUBERIA VAPOR BAJA PRESION GVRC1 A TURBINA\*\*\*\*\*  
1900. 584 CON GY -0.794 1.47  
1901. 584 CON GX 6.295 1.47  
1902. 564 CON GY -0.476 1.47  
1903. 540 CON GY -0.425 1.47  
1904. 1033 CON GY -1.185 1.47  
1905. 1034 CON GY -1.594 1.47  
1906. 487 CON GY -0.632 1.47  
1907. \*TUBERIA VAPOR REC FRIO TURBINA A GVRC2\*\*\*\*\*  
1908. 690 CON GY -4.062 3.195  
1909. 690 CON GX 19.041 3.195  
1910. 678 CON GY -2.737 3.195  
1911. 666 CON GY -1.995 3.195  
1912. 1611 CON GY -1.799 2  
1913. 1667 CON GY -0.291 3.195  
1914. 1027 CON GY -1.183 2.35  
1915. 1697 CON GY -1.256 1.05  
1916. 642 CON GY -1.343 3.195  
1917. 620 CON GY -2.862 3.195  
1918. 596 CON GY -2.648 3.195  
1919. 584 CON GY -1.135 3.195  
1920. 584 CON GX 41.828 3.195  
1921. 564 CON GY -1.599 3.195  
1922. 540 CON GY -2.955 3.195  
1923. 1034 CON GY -4.83 3.195  
1924. 487 CON GY -3.807 3.195  
1925. \*TUBERIA VAPOR REC. CALIENTE DE GVRC2 A TURBINA\*\*\*\*\*

1926. 889 CON GY -2.237 5.325  
1927. 1804 CON GX 19.858 1.8  
1928. 678 CON GY -0.857 5.8  
1929. 666 CON GY -0.876 5.8  
1930. 1611 CON GY -1.111 0.7  
1931. 1611 CON GX 0.121 0.7  
1932. 1667 CON GY -0.281 5.8  
1933. 1667 CON GX 0.147 5.8  
1934. 1981 CON GY -0.183 1.15  
1935. 1981 CON GX 0.018 1.15  
1936. 1697 CON GY -0.491 1.4  
1937. 1697 CON GX 0.102 1.4  
1938. 642 CON GY -0.149 5.8  
1939. 620 CON GY -0.056 5.8  
1940. 1689 CON GY -0.26 1.8  
1941. 2546 CON GY -1.85 0.399998  
1942. 2546 CON GX 57.554 0.399998  
1943. 1685 CON GY -1.515 1.8  
1944. 540 CON GY -5.477 5.8

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1945. 1033 CON GY -12.698 5.8  
1946. 499 CON GY -9.509 5.8  
1947. 487 CON GY -13.563 5.8  
1948. \*TUBERIA VAPOR RECALENTADO CALIENTE DE GVRC1 A TURB.\*\*\*\*\*  
1949. 1021 CON GY -19.894 5.325  
1950. 1686 CON GY -3.61 0.47  
1951. 1686 CON GX 48.377 0.47  
1952. 1685 CON GY -4.058 0.47  
1953. 1685 CON GX 25.744 0.47  
1954. 540 CON GY -3.223 4.47  
1955. 1033 CON GY -6.019 4.47  
1956. 499 CON GY -10.153 4.47  
1957. 487 CON GY -8.276 4.47  
1958. \*TUBERIA VAPOR PRINCIPAL\*\*\*\*\*  
1959. MEMBER LOAD  
1960. 889 1021 CON GY -4.549 1  
1961. 1804 CON GY -1.905 3

1962. 1804 CON GX 20.206 3  
1963. 666 CON GY -0.777 7  
1964. 711 CON GY -0.278 2  
1965. 1737 CON GY -0.543 0.3  
1966. 1988 CON GY -0.553 0.65  
1967. 642 CON GY -0.22 7  
1968. 620 CON GY -0.178 7  
1969. 620 CON GZ 24.855 7  
1970. 2546 CON GY -3.522 1.6  
1971. 2546 CON GX 28.645 1.6  
1972. 1685 CON GY 0.914 3  
1973. 540 CON GY -0.789 7  
1974. 1033 CON GY -2.846 7  
1975. 1034 CON GY -6.411 7  
1976. 1021 CON GY -3.169 1  
1977. 1021 CON GX 24.581 1  
1978. 2549 CON GX 1.63 1  
1979. JOINT LOAD  
1980. 905 FY -1.732  
1981. MEMBER LOAD  
1982. 1696 CON GY -2.139 7  
1983. 1023 CON GY 11.461 5.325  
1984. 2671 CON GY 5.787 2.075  
1985. \*\*\*\*\*  
1986. MEMBER LOAD  
1987. 494 TO 496 500 TO 504 531 534 TO 536 543 547 551 555 559 TO 561 567 -  
1988. 579 TO 581 587 591 TO 593 599 615 TO 617 623 634 637 TO 639 645 649 TO 651 -  
1989. 657 661 TO 663 669 673 TO 675 681 685 TO 687 693 737 738 740 744 TO 746 752 -  
1990. 755 877 879 881 883 885 887 1009 1011 1013 1015 1017 1019 1884 1886 1888 -  
1991. 1890 1892 1894 1896 1898 1900 1902 2059 2064 2090 2092 2304 -  
1992. 2551 UNI GZ 0.49  
1993. 497 TO 499 505 532 544 548 552 570 572 TO 574 604 TO 606 613 626 630 631 635 -  
1994. 700 TO 703 705 TO 708 710 TO 713 726 889 TO 891 1021 TO 1023 1031 1035 1063 -  
1995. 1090 1095 1100 1107 1111 1114 1174 1183 1438 1441 1445 1450 1453 1465 1466 -  
1996. 1472 1473 1478 1479 1502 1505 1508 1605 1606 1611 1615 1616 1621 1625 1626 -  
1997. 1631 1694 1697 1700 1703 1706 1709 1712 1718 1731 1732 1737 1745 1746 1751 -  
1998. 1803 1807 1815 1816 1823 1824 1829 1830 1835 1836 1841 1842 1847 1848 1869 -  
1999. 1870 1879 2065 2126 2540 2557 2572 2635 2664 2667 2677 2678 2685 2688 2693 -

2000. 2696 UNI GZ 0.46

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2001. 725 727 728 758 1493 1496 1499 1511 1514 1517 1520 1523 1599 1880 2103 2134 -  
2002. 2542 UNI GZ 0.46  
2003. 482 TO 484 488 TO 492 530 537 TO 539 545 549 553 556 558 562 563 568 578 582 -  
2004. 583 588 594 595 600 614 618 619 624 636 640 641 646 648 652 653 658 660 664 -  
2005. 665 670 672 676 677 682 684 688 689 694 734 735 741 743 747 748 753 756 1253 -  
2006. 1528 1531 1533 1883 1885 1887 1889 1891 1893 1895 1897 1899 1901 2061 2062 -  
2007. 2091 2094 2098 2100 2109 2117 2121 2127 2290 TO 2293 2295 TO 2298 -  
2008. 2301 TO 2303 2305 2411 2412 2550 2582 2585 2589 2590 2604 2605 2607 2611 -  
2009. 2615 2617 2619 2620 2622 2623 2625 2627 UNI GZ 0.44  
2010. 485 TO 487 493 533 546 550 554 571 575 TO 577 602 603 607 TO 612 627 TO 629 -  
2011. 633 696 TO 699 714 TO 723 731 736 760 TO 762 1032 1439 1443 1446 1451 1455 -  
2012. 1468 1469 1475 1476 1481 1482 1503 1506 1509 1608 1609 1613 1618 1619 1623 -  
2013. 1628 1629 1633 1695 1698 1701 1704 1707 1710 1714 1720 1734 1735 1739 1748 -  
2014. 1749 1753 1806 1812 1813 1820 1821 1826 1827 1832 1833 1838 1839 1844 1845 -  
2015. 2063 2086 2087 2124 2349 2369 2392 2400 2541 2547 2549 2561 2575 2665 2666 -  
2016. 2671 2675 2676 2686 2689 2694 2697 2705 2707 TO 2711 UNI GZ 0.41  
2017. 729 730 732 733 759 1494 1497 1500 1512 1515 1518 1521 1524 1543 1544 1584 -  
2018. 1594 1597 1600 2111 2113 2114 2119 2120 2543 2556 UNI GZ 0.41  
2019. \*\*\*\*\*  
2020. \*COMBINACIONES PARA CARGA DE SERVICIOS  
2021. \*\*\*\*\*  
2022. LOAD COMB 20 (D + CH + Q2) 1.0  
2023. 1 1.0 3 1.0 5 1.0  
2024. LOAD COMB 21 (D + CH + Q3) 1.0  
2025. 1 1.0 3 1.0 6 1.0  
2026. LOAD COMB 22 (D + CH + Q2 + L + T) 1.0  
2027. 1 1.0 3 1.0 5 1.0 2 1.0 4 1.0  
2028. LOAD COMB SRSS 23 (D + CH + Q2 + LO +SISMO) 1.0  
2029. -1 1.0 -3 1.0 -5 1.0 -2 0.5 7 1.0 8 1.0 9 1.0 1  
2030. LOAD COMB SRSS 24 (D + CH + Q2 + LO -SISMO) 1.0  
2031. -1 1.0 -3 1.0 -5 1.0 -2 0.5 7 1.0 8 1.0 9 1.0 -1.  
2032. LOAD COMB 25 (D + CH + Q2 + WX) 1.0  
2033. 1 1.0 3 1.0 5 1.0 10 1.0  
2034. LOAD COMB 26 (D + CH + Q2 - WX) 1.0  
2035. 1 1.0 3 1.0 5 1.0 10 -1.0

2036. LOAD COMB 27 (D + CH + Q2 + WZ) 1.0  
 2037. 1 1.0 3 1.0 5 1.0 11 1.0  
 2038. LOAD COMB 28 (D + CH + Q2 - WZ) 1.0  
 2039. 1 1.0 3 1.0 5 1.0 11 -1.0  
 2040. \*\*\*\*\*  
 2041. \*COMBINACIONES PARA DISEÑO DE CONCRETO  
 2042. \*\*\*\*\*  
 2043. LOAD COMB 30 (D + CH + Q2) 1.4  
 2044. 1 1.4 3 1.4 5 1.4  
 2045. LOAD COMB 31 (D + CH + Q3) 1.4  
 2046. 1 1.4 3 1.4 6 1.4  
 2047. LOAD COMB 32 1.2 D + 1.2 CH + 1.2 Q2 + 1.6 L + 1.6 T  
 2048. 1 1.2 3 1.2 5 1.2 2 1.6 4 1.6  
 2049. LOAD COMB SRSS 33 1.2(D + CH + Q2) + LO + 1.4 SISMO  
 2050. -1 1.2 -3 1.2 -5 1.2 -2 0.5 7 1.0 8 1.0 9 1.0 1.4  
 2051. LOAD COMB SRSS 34 1.2(D + CH + Q2) + LO - 1.4 SISMO  
 2052. -1 1.2 -3 1.2 -5 1.2 -2 0.5 7 1.0 8 1.0 9 1.0 -1.4  
 2053. LOAD COMB 35 1.2 (D + CH + Q2) + L + 1.3 WX  
 2054. 1 1.2 3 1.2 5 1.2 2 1.0 10 1.3  
 2055. LOAD COMB 36 1.2 (D + CH + Q2)+ L - 1.3 WX  
 2056. 1 1.2 3 1.2 5 1.2 2 1.0 10 -1.3

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2057. LOAD COMB 37 1.2 (D + CH + Q2) + L + 1.3 WZ  
 2058. 1 1.2 3 1.2 5 1.2 2 1.0 11 1.3  
 2059. LOAD COMB 38 1.2 (D + CH + Q2)+ L - 1.3 WZ  
 2060. 1 1.2 3 1.2 5 1.2 2 1.0 11 -1.3  
 2061. \*\*\*\*\*  
 2062. \*COMBINACIONES PARA DISEÑO DE ACERO  
 2063. \*\*\*\*\*  
 2064. LOAD COMB 40 (D + CH + Q2) 1.4  
 2065. 1 1.4 3 1.4 5 1.4  
 2066. LOAD COMB 41 (D + CH + Q3) 1.4  
 2067. 1 1.4 3 1.4 6 1.4  
 2068. LOAD COMB 42 1.2 D + 1.2 CH + 1.2 Q2 + 1.6 L + 1.6 T  
 2069. 1 1.2 3 1.2 5 1.2 2 1.6 4 1.6  
 2070. LOAD COMB SRSS 43 1.2(D + CH + Q2) + LR + 1.5 SISMO  
 2071. -1 1.2 -3 1.2 -5 1.2 -2 0.5 7 1.0 8 1.0 9 1.0 1.5

2072. LOAD COMB SRSS 44 1.2(D + CH + Q2) + LR - 1.5 SISMO  
 2073. -1 1.2 -3 1.2 -5 1.2 -2 0.5 7 1.0 8 1.0 9 1.0 -1.5  
 2074. LOAD COMB 45 1.2 (D + CH + Q2) + LR + 1.3 WX  
 2075. 1 1.2 3 1.2 5 1.2 2 0.5 10 1.3  
 2076. LOAD COMB 46 1.2 (D + CH + Q2)+ LR - 1.3 WX  
 2077. 1 1.2 3 1.2 5 1.2 2 0.5 10 -1.3  
 2078. LOAD COMB 47 1.2 (D + CH + Q2) + LR + 1.3 WZ  
 2079. 1 1.2 3 1.2 5 1.2 2 0.5 11 1.3  
 2080. LOAD COMB 48 1.2 (D + CH + Q2)+ LR - 1.3 WZ  
 2081. 1 1.2 3 1.2 5 1.2 2 0.5 11 -1.3  
 2082. PERFORM ANALYSIS

P R O B L E M   S T A T I S T I C S

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NUMBER OF JOINTS/MEMBER+ELEMENTS/SUPPORTS =   673/ 1329/   30

SOLVER USED IS THE IN-CORE ADVANCED SOLVER

TOTAL PRIMARY LOAD CASES =   11, TOTAL DEGREES OF FREEDOM =   3948

ZERO STIFFNESS IN DIRECTION 2 AT JOINT   1321 EQN.NO.   3542

LOADS APPLIED OR DISTRIBUTED HERE FROM ELEMENTS WILL BE IGNORED.

THIS MAY BE DUE TO ALL MEMBERS AT THIS JOINT BEING RELEASED OR

EFFECTIVELY RELEASED IN THIS DIRECTION.

ZERO STIFFNESS IN DIRECTION 4 AT JOINT   1321 EQN.NO.   3544

ZERO STIFFNESS IN DIRECTION 5 AT JOINT   1321 EQN.NO.   3545

ZERO STIFFNESS IN DIRECTION 6 AT JOINT   1321 EQN.NO.   3546

ZERO STIFFNESS IN DIRECTION 2 AT JOINT   1322 EQN.NO.   3548

ZERO STIFFNESS IN DIRECTION 4 AT JOINT   1322 EQN.NO.   3550

ZERO STIFFNESS IN DIRECTION 5 AT JOINT   1322 EQN.NO.   3551

ZERO STIFFNESS IN DIRECTION 6 AT JOINT   1322 EQN.NO.   3552

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NUMBER OF MODES REQUESTED                   =   200

NUMBER OF EXISTING MASSES IN THE MODEL = 1959

NUMBER OF MODES THAT WILL BE USED = 200

ZERO STIFFNESS IN DIRECTION 2 AT JOINT 1321 EQN.NO. 3542

LOADS APPLIED OR DISTRIBUTED HERE FROM ELEMENTS WILL BE IGNORED.

THIS MAY BE DUE TO ALL MEMBERS AT THIS JOINT BEING RELEASED OR

EFFECTIVELY RELEASED IN THIS DIRECTION.

ZERO STIFFNESS IN DIRECTION 4 AT JOINT 1321 EQN.NO. 3544

ZERO STIFFNESS IN DIRECTION 5 AT JOINT 1321 EQN.NO. 3545

ZERO STIFFNESS IN DIRECTION 6 AT JOINT 1321 EQN.NO. 3546

ZERO STIFFNESS IN DIRECTION 2 AT JOINT 1322 EQN.NO. 3548

ZERO STIFFNESS IN DIRECTION 4 AT JOINT 1322 EQN.NO. 3550

ZERO STIFFNESS IN DIRECTION 5 AT JOINT 1322 EQN.NO. 3551

ZERO STIFFNESS IN DIRECTION 6 AT JOINT 1322 EQN.NO. 3552

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CALCULATED FREQUENCIES FOR LOAD CASE 7

MODE	FREQUENCY (CYCLES/SEC)	PERIOD (SEC)
1	0.828	1.20774
2	0.873	1.14592
3	0.993	1.00755
4	1.031	0.97037
5	1.055	0.94785
6	1.100	0.90884
7	1.253	0.79838
8	1.350	0.74088
9	1.459	0.68561
10	1.476	0.67768
11	1.554	0.64362
12	1.645	0.60778
13	1.646	0.60769
14	1.660	0.60245
15	1.695	0.59009
16	1.979	0.50540
17	2.064	0.48445
18	2.103	0.47545



19	2.126	0.47029
20	2.286	0.43754
21	2.375	0.42097
22	2.392	0.41799
23	2.487	0.40206
24	2.545	0.39300
25	2.633	0.37974
26	2.675	0.37383
27	2.746	0.36421
28	2.869	0.34855
29	2.973	0.33635
30	3.049	0.32800
31	3.070	0.32574
32	3.121	0.32038
33	3.148	0.31767
34	3.149	0.31756
35	3.175	0.31497
36	3.190	0.31344
37	3.210	0.31148
38	3.237	0.30894
39	3.252	0.30754
40	3.272	0.30566
41	3.297	0.30333
42	3.304	0.30270
43	3.327	0.30059
44	3.364	0.29725
45	3.385	0.29541
46	3.394	0.29462
47	3.422	0.29221
48	3.440	0.29073
49	3.458	0.28917
50	3.548	0.28188
51	3.574	0.27981

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CALCULATED FREQUENCIES FOR LOAD CASE 7

MODE	FREQUENCY (CYCLES/SEC)	PERIOD (SEC)
------	------------------------	--------------

52	3.659	0.27330
53	3.672	0.27232
54	3.834	0.26080
55	3.860	0.25910
56	3.902	0.25627
57	3.963	0.25234
58	4.010	0.24940
59	4.067	0.24586
60	4.086	0.24473
61	4.110	0.24329
62	4.123	0.24252
63	4.128	0.24223
64	4.161	0.24031
65	4.200	0.23812
66	4.229	0.23645
67	4.245	0.23555
68	4.283	0.23349
69	4.336	0.23065
70	4.374	0.22865
71	4.400	0.22729
72	4.421	0.22620
73	4.424	0.22604
74	4.430	0.22574
75	4.454	0.22452
76	4.519	0.22127
77	4.602	0.21732
78	4.617	0.21660
79	4.651	0.21503
80	4.751	0.21050
81	4.769	0.20967
82	4.829	0.20708
83	4.876	0.20511
84	5.040	0.19842
85	5.114	0.19555
86	5.162	0.19372
87	5.229	0.19125

88	5.245	0.19066
89	5.264	0.18997
90	5.306	0.18847
91	5.323	0.18785
92	5.431	0.18414
93	5.445	0.18364
94	5.455	0.18330
95	5.514	0.18134
96	5.535	0.18067
97	5.547	0.18027
98	5.603	0.17849
99	5.627	0.17773
100	5.669	0.17641
101	5.713	0.17502

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CALCULATED FREQUENCIES FOR LOAD CASE 7

MODE	FREQUENCY (CYCLES/SEC)	PERIOD (SEC)
102	5.795	0.17255
103	5.883	0.16997
104	5.937	0.16843
105	6.014	0.16628
106	6.016	0.16622
107	6.116	0.16349
108	6.153	0.16252
109	6.177	0.16188
110	6.182	0.16175
111	6.303	0.15866
112	6.320	0.15824
113	6.347	0.15755
114	6.382	0.15668
115	6.401	0.15623
116	6.442	0.15524
117	6.482	0.15427
118	6.543	0.15284

119	6.593	0.15168
120	6.624	0.15097
121	6.644	0.15050
122	6.661	0.15012
123	6.711	0.14901
124	6.762	0.14788
125	6.806	0.14694
126	6.910	0.14471
127	6.960	0.14368
128	6.998	0.14290
129	7.042	0.14201
130	7.093	0.14099
131	7.166	0.13955
132	7.185	0.13918
133	7.285	0.13726
134	7.299	0.13701
135	7.373	0.13562
136	7.408	0.13499
137	7.482	0.13366
138	7.486	0.13359
139	7.577	0.13199
140	7.669	0.13040
141	7.697	0.12992
142	7.759	0.12888
143	7.836	0.12762
144	7.859	0.12724
145	7.921	0.12625
146	7.935	0.12602
147	7.962	0.12560
148	7.984	0.12526
149	8.013	0.12479
150	8.024	0.12463
151	8.043	0.12433

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CALCULATED FREQUENCIES FOR LOAD CASE 7

MODE	FREQUENCY (CYCLES/SEC)	PERIOD (SEC)
------	------------------------	--------------

152	8.092	0.12357
153	8.124	0.12310
154	8.134	0.12294
155	8.165	0.12247
156	8.237	0.12140
157	8.291	0.12061
158	8.298	0.12051
159	8.443	0.11844
160	8.496	0.11770
161	8.509	0.11752
162	8.542	0.11707
163	8.553	0.11691
164	8.566	0.11674
165	8.631	0.11586
166	8.660	0.11548
167	8.697	0.11499
168	8.709	0.11482
169	8.768	0.11405
170	8.838	0.11314
171	8.931	0.11197
172	9.051	0.11049
173	9.106	0.10982
174	9.150	0.10929
175	9.245	0.10817
176	9.267	0.10791
177	9.335	0.10712
178	9.365	0.10678
179	9.381	0.10660
180	9.454	0.10578
181	9.468	0.10561
182	9.501	0.10525
183	9.554	0.10467
184	9.608	0.10408
185	9.677	0.10333
186	9.742	0.10265
187	9.763	0.10243

188	9.792	0.10213
189	9.836	0.10167
190	9.842	0.10161
191	9.909	0.10092
192	9.943	0.10057
193	10.002	0.09998
194	10.044	0.09956
195	10.071	0.09929
196	10.081	0.09920
197	10.149	0.09853
198	10.162	0.09840
199	10.257	0.09749
200	10.477	0.09545

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C O M P O S I T E D A M P I N G S U M M A R Y

MODE	STRAIN ENERGY	DAMP*ENERGY	COMPOSITE DAMPING
1	8.681919E+00	3.067572E-01	0.0353
2	4.034432E+00	1.406401E-01	0.0349
3	1.932831E+01	6.807405E-01	0.0352
4	2.708001E+01	9.693521E-01	0.0358
5	4.440585E+00	1.573494E-01	0.0354
6	2.126562E+00	6.893564E-02	0.0324
7	3.528618E+00	1.214743E-01	0.0344
8	1.153158E+01	4.039866E-01	0.0350
9	3.307110E+00	1.005108E-01	0.0304
10	2.602998E+00	7.877011E-02	0.0303
11	8.523600E+00	2.968351E-01	0.0348
12	2.375124E+00	7.125383E-02	0.0300
13	1.864004E+00	5.592012E-02	0.0300
14	3.780964E+00	1.136969E-01	0.0301
15	4.161430E+00	1.286291E-01	0.0309

16	7.165535E+00	2.270423E-01	0.0317
17	4.373654E+00	1.319872E-01	0.0302
18	3.066849E+00	9.257943E-02	0.0302
19	2.090518E+00	6.311120E-02	0.0302
20	5.538111E+00	1.664860E-01	0.0301
21	3.667517E+00	1.112197E-01	0.0303
22	3.199270E+01	9.665493E-01	0.0302
23	2.899564E+00	1.043696E-01	0.0360
24	7.585665E+00	2.323225E-01	0.0306
25	4.621752E+00	1.390101E-01	0.0301
26	1.365569E+01	4.104965E-01	0.0301
27	1.107512E+01	3.326791E-01	0.0300
28	1.038549E+01	3.122379E-01	0.0301
29	6.734867E+00	2.043701E-01	0.0303
30	1.818032E+01	5.481162E-01	0.0301
31	3.808614E+00	1.146810E-01	0.0301
32	1.199953E+01	3.606660E-01	0.0301
33	8.688132E+00	2.606816E-01	0.0300
34	2.219175E+00	6.669801E-02	0.0301
35	1.063895E+01	3.237296E-01	0.0304
36	1.358220E+01	4.078107E-01	0.0300
37	1.029916E+01	3.089852E-01	0.0300
38	1.375579E+01	4.154687E-01	0.0302
39	1.238471E+00	3.734263E-02	0.0302
40	1.167434E+00	3.502871E-02	0.0300
41	1.798183E+01	5.412105E-01	0.0301
42	1.918845E+01	5.847158E-01	0.0305
43	1.799070E+01	5.418628E-01	0.0301
44	1.259101E+01	3.787339E-01	0.0301
45	1.624413E+00	4.888106E-02	0.0301
46	1.227400E+00	3.682829E-02	0.0300
47	1.162757E+01	3.488474E-01	0.0300
48	1.378905E+01	4.177326E-01	0.0303

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MODE STRAIN ENERGY DAMP\*ENERGY COMPOSITE DAMPING

49	1.428784E+01	4.345333E-01	0.0304
50	1.457482E+01	4.476419E-01	0.0307
51	3.431629E+01	1.053624E+00	0.0307
52	2.041747E+01	6.168237E-01	0.0302
53	2.901063E+01	8.783847E-01	0.0303
54	5.359091E+01	1.676195E+00	0.0313
55	6.337737E+01	1.972737E+00	0.0311
56	1.348525E+01	4.201363E-01	0.0312
57	1.358447E+02	4.367081E+00	0.0321
58	4.851554E+01	1.492615E+00	0.0308
59	5.029659E+00	1.520442E-01	0.0302
60	6.339551E+00	1.918796E-01	0.0303
61	8.696707E+00	2.625717E-01	0.0302
62	2.690600E+01	8.348312E-01	0.0310
63	1.919004E+01	5.882723E-01	0.0307
64	1.507635E+01	4.584366E-01	0.0304
65	1.827412E+01	5.499037E-01	0.0301
66	8.131596E+01	2.474009E+00	0.0304
67	3.893215E+01	1.186179E+00	0.0305
68	7.444567E+00	2.252298E-01	0.0303
69	1.842582E+01	5.531549E-01	0.0300
70	1.869799E+01	5.632314E-01	0.0301
71	9.682333E+00	2.913500E-01	0.0301
72	2.165645E+01	6.503207E-01	0.0300
73	6.270663E+00	1.887060E-01	0.0301
74	9.784610E+00	2.935782E-01	0.0300
75	1.478898E+01	4.482933E-01	0.0303
76	4.170505E+01	1.263517E+00	0.0303
77	5.767630E+01	1.766618E+00	0.0306
78	5.400113E+01	1.640044E+00	0.0304
79	4.939282E+01	1.514800E+00	0.0307
80	2.243093E+01	6.922976E-01	0.0309
81	3.482692E+01	1.068103E+00	0.0307
82	6.820476E+01	2.127021E+00	0.0312
83	7.930522E+01	2.463999E+00	0.0311
84	5.328508E+01	1.622318E+00	0.0304
85	5.498407E+01	1.691819E+00	0.0308
86	4.907006E+01	1.502599E+00	0.0306



87	8.855756E+01	2.853995E+00	0.0322
88	8.993381E+01	2.790522E+00	0.0310
89	1.066919E+02	3.319358E+00	0.0311
90	1.817393E+02	5.734850E+00	0.0316
91	5.793915E+01	1.803582E+00	0.0311
92	3.899532E+01	1.266191E+00	0.0325
93	4.388810E+01	1.339923E+00	0.0305
94	4.039694E+01	1.229777E+00	0.0304
95	3.064698E+01	9.452159E-01	0.0308
96	2.245490E+01	6.779724E-01	0.0302
97	3.237999E+01	1.003409E+00	0.0310
98	4.942114E+00	1.490222E-01	0.0302
99	9.546656E+01	2.923886E+00	0.0306
100	1.341789E+02	4.345327E+00	0.0324
101	1.475575E+01	4.566039E-01	0.0309

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MODE	STRAIN ENERGY	DAMP*ENERGY	COMPOSITE DAMPING
102	2.534057E+01	7.768658E-01	0.0307
103	1.946343E+02	6.046277E+00	0.0311
104	1.123834E+02	3.456702E+00	0.0308
105	3.017954E+01	9.129439E-01	0.0303
106	4.706113E+01	1.442017E+00	0.0306
107	5.178853E+01	1.635292E+00	0.0316
108	1.893096E+00	5.769769E-02	0.0305
109	5.382543E+00	1.706056E-01	0.0317
110	2.227882E+01	6.947604E-01	0.0312
111	5.809600E+01	1.764105E+00	0.0304
112	5.499889E+01	1.695527E+00	0.0308
113	3.903349E+01	1.320197E+00	0.0338
114	2.587881E+00	7.870807E-02	0.0304
115	7.667478E+00	2.312786E-01	0.0302
116	1.006398E+01	3.087392E-01	0.0307
117	3.797672E+01	1.185776E+00	0.0312
118	6.456014E+00	1.995824E-01	0.0309
119	3.873540E+01	1.223900E+00	0.0316

120	2.021451E+00	6.065298E-02	0.0300
121	5.407501E+00	1.664051E-01	0.0308
122	8.901038E+00	2.696426E-01	0.0303
123	1.050003E+02	3.374981E+00	0.0321
124	3.054930E+01	9.348453E-01	0.0306
125	7.829250E+01	2.438280E+00	0.0311
126	5.946636E+01	1.838801E+00	0.0309
127	8.699169E+00	2.721366E-01	0.0313
128	4.953293E+01	1.630219E+00	0.0329
129	4.521443E+01	1.505279E+00	0.0333
130	1.512490E+02	4.653013E+00	0.0308
131	3.539636E+00	1.079749E-01	0.0305
132	1.256936E+01	3.860490E-01	0.0307
133	1.151296E+01	3.505402E-01	0.0304
134	2.384607E+01	7.350208E-01	0.0308
135	2.324811E+01	7.176651E-01	0.0309
136	2.907424E+01	8.842002E-01	0.0304
137	3.431931E+01	1.044301E+00	0.0304
138	2.148905E+01	6.742307E-01	0.0314
139	2.524696E+01	7.867735E-01	0.0312
140	1.411387E+02	4.345399E+00	0.0308
141	8.218419E+01	2.562191E+00	0.0312
142	1.866301E+02	5.826624E+00	0.0312
143	1.829049E+02	5.737522E+00	0.0314
144	1.997620E+01	6.097306E-01	0.0305
145	7.957790E+01	2.471415E+00	0.0311
146	1.213312E+02	3.714978E+00	0.0306
147	2.130389E+01	6.437906E-01	0.0302
148	1.222318E+02	3.805868E+00	0.0311
149	1.052255E+02	3.315639E+00	0.0315
150	2.145198E+01	6.505102E-01	0.0303
151	1.889453E+01	5.732242E-01	0.0303
152	9.783878E+01	3.012963E+00	0.0308
153	8.899580E+01	2.735866E+00	0.0307
154	1.400963E+01	4.239022E-01	0.0303

MODE	STRAIN ENERGY	DAMP*ENERGY	COMPOSITE DAMPING
155	3.590483E+01	1.108152E+00	0.0309
156	4.864587E+01	1.472040E+00	0.0303
157	1.809573E+02	5.513120E+00	0.0305
158	2.803867E+01	8.528474E-01	0.0304
159	1.605771E+01	4.907545E-01	0.0306
160	5.678962E+01	1.722739E+00	0.0303
161	2.077878E+01	6.363179E-01	0.0306
162	1.674990E+01	5.062970E-01	0.0302
163	9.386864E+01	2.903209E+00	0.0309
164	5.229266E+00	1.571420E-01	0.0301
165	1.833871E+01	5.523423E-01	0.0301
166	1.527995E+01	4.593954E-01	0.0301
167	4.767898E+01	1.452229E+00	0.0305
168	1.360348E+01	4.104060E-01	0.0302
169	7.907007E+01	2.495566E+00	0.0316
170	1.142550E+02	3.605748E+00	0.0316
171	3.523636E+02	1.069165E+01	0.0303
172	3.704461E+01	1.141885E+00	0.0308
173	6.916825E+01	2.116397E+00	0.0306
174	2.934822E+01	8.852577E-01	0.0302
175	5.467056E+02	1.679449E+01	0.0307
176	6.124024E+01	1.847882E+00	0.0302
177	4.364494E+01	1.320437E+00	0.0303
178	2.113038E+02	6.475242E+00	0.0306
179	1.607146E+02	4.903085E+00	0.0305
180	1.950174E+02	5.934461E+00	0.0304
181	1.029099E+02	3.115575E+00	0.0303
182	1.535862E+02	4.648016E+00	0.0303
183	4.433730E+01	1.334637E+00	0.0301
184	2.926943E+02	8.938984E+00	0.0305
185	1.618523E+02	4.928837E+00	0.0305
186	1.540858E+01	4.633406E-01	0.0301
187	7.715465E+01	2.346161E+00	0.0304
188	1.294540E+02	3.952229E+00	0.0305
189	1.318956E+02	3.981153E+00	0.0302
190	6.481617E+01	1.952263E+00	0.0301

191	1.276640E+02	3.876291E+00	0.0304
192	2.376693E+02	7.576992E+00	0.0319
193	2.070366E+01	6.232804E-01	0.0301
194	9.443843E+01	2.840575E+00	0.0301
195	1.524484E+01	4.573458E-01	0.0300
196	1.706631E+01	5.128473E-01	0.0301
197	9.009227E+01	2.746503E+00	0.0305
198	1.359542E+02	4.156732E+00	0.0306
199	3.018250E+02	9.185206E+00	0.0304
200	1.348435E+02	4.087208E+00	0.0303

STAAD SPACE

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RESPONSE LOAD CASE 7

SRSS MODAL COMBINATION METHOD USED.

DYNAMIC WEIGHT	X	Y	Z	8.303278E+03	9.798368E+03	8.303278E+03	KN
MISSING WEIGHT	X	Y	Z	-5.333425E+02	-8.010067E+01	-3.933308E+02	KN
MODAL WEIGHT	X	Y	Z	7.769936E+03	9.718267E+03	7.909947E+03	KN

MODE	ACCELERATION-G	DAMPING
----	-----	-----
1	0.10268	0.05000
2	0.10611	0.05000
3	0.11379	0.05000
4	0.11663	0.05000
5	0.11846	0.05000
6	0.12164	0.05000
7	0.13076	0.05000
8	0.13921	0.05000
9	0.14732	0.05000
10	0.14849	0.05000
11	0.15349	0.05000
12	0.15875	0.05000

13	0.15876	0.05000
14	0.15953	0.05000
15	0.16199	0.05000
16	0.17996	0.05000
17	0.18567	0.05000
18	0.18831	0.05000
19	0.18983	0.05000
20	0.19945	0.05000
21	0.20431	0.05000
22	0.20519	0.05000
23	0.20986	0.05000
24	0.21340	0.05000
25	0.21895	0.05000
26	0.22143	0.05000
27	0.22546	0.05000
28	0.23201	0.05000
29	0.23712	0.05000
30	0.24062	0.05000
31	0.24157	0.05000
32	0.24381	0.05000
33	0.24494	0.05000
34	0.24499	0.05000
35	0.24608	0.05000
36	0.24672	0.05000
37	0.24754	0.05000
38	0.24860	0.05000
39	0.24919	0.05000
40	0.24997	0.05000
41	0.25095	0.05000
42	0.25122	0.05000

STAAD SPACE

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MODE	ACCELERATION-G	DAMPING
----	-----	-----
43	0.25210	0.05000

44	0.25350	0.05000
45	0.25427	0.05000
46	0.25460	0.05000
47	0.25561	0.05000
48	0.25623	0.05000
49	0.25688	0.05000
50	0.25993	0.05000
51	0.26080	0.05000
52	0.26353	0.05000
53	0.26394	0.05000
54	0.26876	0.05000
55	0.26947	0.05000
56	0.27066	0.05000
57	0.27231	0.05000
58	0.27354	0.05000
59	0.27502	0.05000
60	0.27549	0.05000
61	0.27609	0.05000
62	0.27642	0.05000
63	0.27654	0.05000
64	0.27734	0.05000
65	0.27826	0.05000
66	0.27896	0.05000
67	0.27934	0.05000
68	0.28020	0.05000
69	0.28139	0.05000
70	0.28223	0.05000
71	0.28280	0.05000
72	0.28325	0.05000
73	0.28332	0.05000
74	0.28344	0.05000
75	0.28396	0.05000
76	0.28532	0.05000
77	0.28697	0.05000
78	0.28727	0.05000
79	0.28793	0.05000
80	0.28983	0.05000
81	0.29017	0.05000

82	0.29126	0.05000
83	0.29208	0.05000
84	0.29488	0.05000
85	0.29609	0.05000
86	0.29685	0.05000
87	0.29788	0.05000
88	0.29813	0.05000
89	0.29842	0.05000
90	0.29905	0.05000
91	0.29931	0.05000
92	0.30086	0.05000
93	0.30107	0.05000

STAAD SPACE

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MODE	ACCELERATION-G	DAMPING
----	-----	-----
94	0.30121	0.05000
95	0.30204	0.05000
96	0.30232	0.05000
97	0.30248	0.05000
98	0.30323	0.05000
99	0.30355	0.05000
100	0.30410	0.05000
101	0.30468	0.05000
102	0.30572	0.05000
103	0.30680	0.05000
104	0.30744	0.05000
105	0.30834	0.05000
106	0.30837	0.05000
107	0.30951	0.05000
108	0.30992	0.05000
109	0.31018	0.05000
110	0.31024	0.05000
111	0.31153	0.05000
112	0.31171	0.05000

113	0.31200	0.05000
114	0.31236	0.05000
115	0.31255	0.05000
116	0.31297	0.05000
117	0.31337	0.05000
118	0.31397	0.05000
119	0.31446	0.05000
120	0.31475	0.05000
121	0.31495	0.05000
122	0.31511	0.05000
123	0.31558	0.05000
124	0.31605	0.05000
125	0.31644	0.05000
126	0.31737	0.05000
127	0.31781	0.05000
128	0.31813	0.05000
129	0.31851	0.05000
130	0.31893	0.05000
131	0.31953	0.05000
132	0.31969	0.05000
133	0.32050	0.05000
134	0.32060	0.05000
135	0.32118	0.05000
136	0.32145	0.05000
137	0.32200	0.05000
138	0.32203	0.05000
139	0.32270	0.05000
140	0.32337	0.05000
141	0.32357	0.05000
142	0.32401	0.05000
143	0.32453	0.05000
144	0.32469	0.05000

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MODE	ACCELERATION-G	DAMPING
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145	0.32511	0.05000
146	0.32520	0.05000
147	0.32538	0.05000
148	0.32552	0.05000
149	0.32572	0.05000
150	0.32578	0.05000
151	0.32591	0.05000
152	0.32623	0.05000
153	0.32643	0.05000
154	0.32649	0.05000
155	0.32669	0.05000
156	0.32714	0.05000
157	0.32747	0.05000
158	0.32751	0.05000
159	0.32838	0.05000
160	0.32869	0.05000
161	0.32876	0.05000
162	0.32895	0.05000
163	0.32902	0.05000
164	0.32909	0.05000
165	0.32946	0.05000
166	0.32962	0.05000
167	0.32982	0.05000
168	0.32989	0.05000
169	0.33021	0.05000
170	0.33059	0.05000
171	0.33109	0.05000
172	0.33171	0.05000
173	0.33199	0.05000
174	0.33221	0.05000
175	0.33268	0.05000
176	0.33278	0.05000
177	0.33312	0.05000
178	0.33326	0.05000
179	0.33334	0.05000
180	0.33368	0.05000
181	0.33375	0.05000

182	0.33390	0.05000
183	0.33414	0.05000
184	0.33439	0.05000
185	0.33470	0.05000
186	0.33499	0.05000
187	0.33508	0.05000
188	0.33521	0.05000
189	0.33540	0.05000
190	0.33543	0.05000
191	0.33571	0.05000
192	0.33586	0.05000
193	0.33606	0.05000
194	0.33509	0.05000
195	0.33448	0.05000

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MODE	ACCELERATION-G	DAMPING
----	-----	-----
196	0.33427	0.05000
197	0.33274	0.05000
198	0.33245	0.05000
199	0.33037	0.05000
200	0.32570	0.05000

STAAD SPACE

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MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
1	1.208	0.90	-0.32	-20.88	-256.93	3759.15	-70.39
2	1.146	0.20	-0.08	3.90	54.42	-847.18	-18.54

3	1.008	14.82	-2.29	-6.16	-64.55	2138.88	-623.14
4	0.970	759.07	6.03	-59.73	-951.80	43010.18	-8762.50
5	0.948	76.62	1.25	92.32	1162.69	-18634.45	-737.55
6	0.909	0.00	0.03	-0.38	-6.16	84.13	7.22
7	0.798	3.80	0.04	3.04	34.88	-51.31	-44.55
8	0.741	6.50	-0.11	-12.40	-129.90	2955.06	-117.60
9	0.686	0.07	-0.07	0.17	5.99	-40.76	-15.61
10	0.678	0.04	-0.04	0.54	2.57	-114.25	-9.17
11	0.644	0.35	-0.17	1.02	19.42	-180.20	-44.06
12	0.608	0.00	0.00	0.00	0.01	0.11	-0.11
13	0.608	0.00	0.00	0.00	0.00	0.01	0.00
14	0.602	7.61	-0.02	-0.18	1.02	328.56	-77.91
15	0.590	0.00	0.00	-0.03	-0.42	5.88	0.87
16	0.505	0.13	0.45	-1.97	-37.64	434.18	97.10
17	0.484	0.10	-0.04	0.18	2.93	-41.59	-11.23
18	0.475	9.04	0.71	2.40	-12.11	-101.86	159.20
19	0.470	10.85	0.40	2.44	2.35	-56.75	96.51
20	0.438	0.55	0.32	0.07	-12.66	9.18	19.55
21	0.421	0.13	0.02	0.20	-5.66	-26.06	11.06
22	0.418	4.10	-0.35	0.70	28.46	17.35	35.22
23	0.402	4.64	-0.06	0.27	2.59	72.44	-20.17
24	0.393	0.05	0.30	-0.66	-14.22	136.18	61.15
25	0.380	0.19	-0.07	0.09	3.73	-9.95	-11.09
26	0.374	1.02	-1.22	0.06	50.03	16.70	-195.43
27	0.364	0.70	-0.63	0.27	25.87	-26.86	-124.96
28	0.349	1.38	0.03	-0.19	3.80	76.70	45.00
29	0.336	0.09	-0.63	0.84	21.17	-144.54	-116.55
30	0.328	0.00	0.00	0.00	0.14	0.09	-0.83
31	0.326	0.13	0.38	-0.02	-12.24	12.47	78.00
32	0.320	0.04	-0.39	-0.03	14.25	9.34	-86.05
33	0.318	0.03	-0.20	-0.01	7.15	4.13	-41.65
34	0.318	0.00	0.03	-0.02	-0.93	4.01	4.91
35	0.315	0.47	13.51	-0.42	-518.34	97.38	2526.79
36	0.313	0.03	0.12	0.00	-5.79	2.50	10.49
37	0.311	0.00	-0.04	0.00	1.26	0.48	-7.53
38	0.309	0.05	-2.96	-0.24	105.50	53.43	-643.77
39	0.308	0.00	0.00	0.00	0.01	1.04	-0.06
40	0.306	0.00	0.00	0.00	0.00	0.00	0.00

41	0.303	0.02	-0.38	-0.18	15.98	32.48	-77.21
42	0.303	0.06	-1.01	0.60	36.12	-100.58	-190.75
43	0.301	0.07	0.26	-0.17	-11.20	29.88	42.90
44	0.297	0.05	0.04	0.04	-2.41	-7.11	8.77
45	0.295	0.00	0.01	0.01	-0.41	-1.54	1.60
46	0.295	0.00	0.00	0.00	0.03	0.00	-0.12
47	0.292	0.01	-0.01	0.00	0.24	-0.05	-1.45

STAAD SPACE

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MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
48	0.291	0.58	-2.85	0.22	106.02	-24.35	-546.51
49	0.289	0.00	0.00	0.00	-0.15	-0.29	0.71
50	0.282	0.00	0.03	0.12	-1.07	-27.69	5.24
51	0.280	0.76	-19.47	-0.07	741.82	28.70	-3304.06
52	0.273	0.12	3.82	0.07	-142.93	-13.10	788.61
53	0.272	1.04	-17.41	-0.77	660.58	197.24	-3410.88
54	0.261	1.93	2.29	-5.24	-83.80	1030.51	561.31
55	0.259	0.09	1.36	-0.26	-57.78	52.35	231.56
56	0.256	0.00	-0.04	0.12	0.51	-22.69	-6.89
57	0.252	0.14	0.69	1.48	-34.78	-253.47	143.36
58	0.249	2.61	-8.06	1.30	287.45	-118.48	-1176.91
59	0.246	0.01	0.01	-0.04	0.23	10.20	5.32
60	0.245	0.01	-0.03	0.09	1.31	-18.78	-2.84
61	0.243	0.00	0.02	0.04	-0.75	-7.88	7.14
62	0.243	0.04	0.80	0.35	-28.61	-72.05	167.70
63	0.242	0.02	1.11	-0.06	-39.72	10.95	234.63
64	0.240	0.12	1.33	-0.22	-47.66	53.62	259.14
65	0.238	0.00	0.11	0.04	-4.29	-7.83	25.47
66	0.236	0.00	-0.09	-0.04	4.12	8.69	-15.28
67	0.236	2.06	1.96	1.27	-72.65	-182.33	307.13
68	0.233	0.01	0.03	-0.17	0.39	32.55	7.29
69	0.231	0.02	-0.02	-0.01	0.60	3.58	-6.16

70	0.229	0.02	-0.04	0.05	2.89	-8.67	-11.08
71	0.227	0.07	0.50	0.05	-18.62	-4.15	102.86
72	0.226	0.01	0.32	0.03	-12.85	-5.19	65.17
73	0.226	0.01	-0.10	-0.03	3.65	6.17	-19.65
74	0.226	0.00	-0.01	0.00	0.32	0.04	-1.72
75	0.225	0.14	-0.10	0.13	0.06	-11.13	-16.58
76	0.221	1.00	-2.80	1.19	101.80	-195.02	-512.20
77	0.217	0.00	-0.63	0.00	22.39	1.44	-147.49
78	0.217	0.16	7.36	0.08	-266.44	-18.98	1548.02
79	0.215	1.62	-0.66	-2.08	45.25	452.06	-375.68
80	0.211	1.34	5.05	-0.55	-188.50	162.39	1602.09
81	0.210	0.28	1.07	0.22	-45.73	-31.33	281.97
82	0.207	0.33	4.24	-0.45	-150.04	101.83	897.93
83	0.205	0.23	4.95	0.90	-212.81	-159.10	985.91
84	0.198	0.05	-0.30	0.02	12.93	-4.37	-59.76
85	0.196	0.13	0.49	-0.03	-22.77	18.31	74.14
86	0.194	0.00	-0.01	-0.01	0.26	3.22	-2.20
87	0.191	0.00	-0.04	-0.10	3.43	24.50	-7.81
88	0.191	0.01	-0.16	0.02	2.77	-3.60	-34.49
89	0.190	0.10	0.08	-0.47	-6.21	98.12	15.73
90	0.188	0.17	0.16	-0.50	7.09	93.45	44.08
91	0.188	0.07	-0.32	-0.45	25.77	87.64	-67.46
92	0.184	0.00	-0.03	0.04	1.27	-5.26	-5.08
93	0.184	0.00	-0.02	0.06	0.90	-11.08	-4.14

STAAD SPACE

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MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

MODE	PERIOD	FX	FY	FZ	MOMENTS ARE ABOUT THE ORIGIN		
					MX	MY	MZ
94	0.183	0.00	0.03	-0.06	-0.60	12.71	5.74
95	0.181	0.02	0.46	-0.16	-16.54	32.00	65.38
96	0.181	0.00	-0.03	0.00	0.97	0.51	-3.89
97	0.180	0.16	3.23	0.34	-124.81	-54.83	505.78
98	0.178	0.00	0.01	0.00	-0.29	-0.75	1.30

99	0.178	0.02	-0.27	0.17	10.56	-31.68	-49.01
100	0.176	0.09	-0.66	-0.22	24.72	35.75	-93.37
101	0.175	0.74	-2.60	-1.14	102.52	250.96	-539.58
102	0.173	0.08	0.93	0.27	-35.75	-51.05	207.84
103	0.170	0.40	-0.58	-0.78	32.73	201.39	-106.23
104	0.168	0.24	-2.82	0.91	89.83	-173.09	-591.87
105	0.166	0.03	-0.75	0.10	22.72	-17.35	-153.84
106	0.166	0.11	0.59	0.15	-13.48	-25.69	109.20
107	0.163	0.20	-0.23	0.52	4.40	-91.69	-78.14
108	0.163	0.00	-0.09	0.02	2.63	-5.71	-22.48
109	0.162	0.65	3.45	0.10	-117.44	48.22	822.16
110	0.162	1.84	2.07	1.73	-104.70	-259.77	342.34
111	0.159	0.05	-0.35	0.11	9.85	-20.95	-67.52
112	0.158	0.26	-1.31	0.05	54.67	5.94	-258.21
113	0.158	0.02	-0.03	-0.32	3.23	74.99	-7.74
114	0.157	0.00	0.05	0.04	-1.01	-8.20	10.83
115	0.156	0.03	-0.16	0.05	4.89	-8.70	-27.20
116	0.155	0.35	-0.23	-0.18	24.81	49.32	-18.21
117	0.154	2.56	-0.87	-0.71	29.91	226.11	-76.40
118	0.153	0.00	-0.01	0.00	0.12	-0.81	-0.04
119	0.152	0.98	0.99	-0.23	-31.31	97.00	192.06
120	0.151	0.00	-0.03	0.00	1.24	-0.04	-6.36
121	0.151	0.28	-0.81	0.10	34.48	0.96	-211.34
122	0.150	0.05	-0.52	0.05	21.61	-8.03	-92.78
123	0.149	0.06	-0.11	-0.12	9.73	23.80	-34.39
124	0.148	0.05	-0.86	-0.04	30.00	12.89	-170.78
125	0.147	1.70	0.42	0.25	-44.46	-14.91	144.93
126	0.145	0.17	-0.30	0.10	13.89	-6.78	-75.07
127	0.144	0.03	-0.11	0.42	2.03	-89.90	-18.75
128	0.143	0.12	0.06	0.25	-0.88	-45.61	22.20
129	0.142	0.33	0.17	0.07	-0.61	2.90	51.64
130	0.141	0.15	-1.21	-0.45	51.70	104.69	-249.67
131	0.140	0.15	-0.17	0.42	4.79	-89.14	-20.06
132	0.139	0.02	-0.10	-0.07	4.62	17.88	-25.86
133	0.137	0.01	-0.02	0.11	0.29	-23.70	-5.11
134	0.137	0.01	-0.09	0.17	2.31	-36.36	-18.13
135	0.136	0.14	0.25	0.08	-8.99	-8.81	50.50
136	0.135	0.09	0.27	-0.10	-5.15	20.69	56.11

137	0.134	0.23	-0.04	-0.16	-1.62	51.12	-28.17
138	0.134	0.11	0.28	0.43	-13.07	-87.63	64.00
139	0.132	0.22	0.44	-0.32	-10.81	70.94	99.68

STAAD SPACE -- PAGE NO. 56

MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
140	0.130	0.00	-0.01	0.00	0.29	-1.32	-4.27
141	0.130	0.11	-0.80	0.53	25.45	-94.93	-167.08
142	0.129	0.02	0.16	-0.02	-5.03	5.91	30.30
143	0.128	0.18	0.39	0.16	-15.69	-27.75	91.32
144	0.127	1.80	-0.78	0.78	25.59	-75.42	-134.52
145	0.126	0.62	-0.12	-0.47	5.62	99.21	-31.00
146	0.126	0.05	-0.46	0.11	18.10	-19.94	-100.90
147	0.126	0.96	0.05	-0.23	2.25	77.14	-16.51
148	0.125	4.02	-1.27	-0.78	47.41	265.79	-316.54
149	0.125	0.75	-0.56	0.37	19.63	-48.42	-123.35
150	0.125	1.95	1.28	-0.82	-49.31	237.13	237.48
151	0.124	0.07	0.24	-0.13	-8.30	26.07	43.11
152	0.124	0.96	0.25	-0.48	-14.37	141.54	56.82
153	0.123	0.39	-0.36	0.16	15.47	-18.88	-93.75
154	0.123	0.22	0.62	0.00	-19.93	9.38	129.25
155	0.122	2.08	0.77	-1.05	-27.72	269.93	131.70
156	0.121	0.41	-0.68	-0.13	27.83	39.99	-128.36
157	0.121	0.24	0.02	0.15	-4.04	-15.09	23.84
158	0.121	0.30	0.14	-0.06	-4.61	23.91	15.88
159	0.118	0.18	-0.62	0.00	24.34	8.50	-119.58
160	0.118	0.00	0.01	-0.01	-0.35	1.50	1.26
161	0.118	1.40	0.79	0.15	-29.75	26.64	152.09
162	0.117	0.16	-0.28	0.06	9.66	-7.97	-60.41
163	0.117	0.01	-0.07	-0.01	2.62	2.05	-7.72
164	0.117	0.01	0.04	-0.01	-2.06	2.38	10.91
165	0.116	0.06	0.05	0.04	-1.92	-5.18	2.84

166	0.115	0.04	-0.04	0.02	1.72	-2.35	-6.25
167	0.115	0.27	-0.42	0.13	13.99	-15.20	-75.14
168	0.115	0.02	0.02	-0.01	-1.41	3.01	5.40
169	0.114	0.02	-0.23	0.01	9.53	-1.42	-53.35
170	0.113	0.13	0.62	0.01	-30.79	9.03	131.65
171	0.112	0.04	0.15	-0.09	-5.67	18.75	32.03
172	0.110	0.29	-0.46	-0.44	17.70	96.58	-91.09
173	0.110	1.25	1.06	-0.01	-32.73	54.51	237.78
174	0.109	0.00	0.03	-0.02	-1.15	3.98	5.72
175	0.108	0.04	0.03	0.28	-2.36	-48.17	3.48
176	0.108	0.11	-0.12	-0.10	5.16	23.79	-20.59
177	0.107	0.53	0.08	0.21	-5.27	-15.79	21.48
178	0.107	0.06	-0.33	0.07	12.74	-10.51	-57.63
179	0.107	0.23	-0.15	-0.08	5.96	28.18	-29.40
180	0.106	1.68	1.64	0.18	-64.59	32.64	369.42
181	0.106	0.04	0.32	-0.12	-10.42	23.29	73.02
182	0.105	0.27	-0.70	-0.12	25.87	27.07	-144.69
183	0.105	0.01	-0.04	0.01	1.41	-1.86	-8.67
184	0.104	0.57	-0.50	0.06	19.43	9.23	-96.19
185	0.103	0.23	0.15	0.06	-5.90	1.22	41.87

STAAD SPACE

-- PAGE NO. 57

MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
186	0.103	0.10	-0.06	0.02	2.56	0.46	-7.52
187	0.102	0.14	-0.02	0.19	-0.45	-30.72	7.54
188	0.102	0.76	-0.73	-0.39	34.14	111.42	-133.03
189	0.102	0.01	0.03	0.01	-1.15	-1.76	5.16
190	0.102	0.23	-0.03	0.04	1.92	4.11	-3.75
191	0.101	0.08	0.11	0.00	-4.92	3.31	25.85
192	0.101	0.83	-0.80	0.13	33.44	10.63	-153.27
193	0.100	0.21	-0.02	0.02	0.36	1.62	-5.50
194	0.100	0.01	0.01	0.00	-0.25	0.59	1.21



195	0.099	0.00	0.00	0.00	0.00	0.00	0.00
196	0.099	0.01	0.00	0.03	-0.09	-5.15	0.19
197	0.099	0.53	-0.49	0.26	18.08	-32.98	-95.79
198	0.098	0.08	0.22	0.21	-9.55	-37.70	44.11
199	0.097	0.02	0.04	-0.03	-1.85	9.30	7.81
200	0.095	0.43	0.43	-0.07	-19.44	30.87	76.85

STAAD SPACE

-- PAGE NO. 58

MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
1	0.11	0.01	56.79	0.106	0.011	56.795	0.90	0.00	0.00
2	0.02	0.00	8.41	0.129	0.014	65.210	0.20	0.00	0.00
3	1.57	0.03	0.27	1.698	0.046	65.481	14.82	0.00	0.00
4	78.39	0.00	0.49	80.083	0.050	65.966	759.07	0.00	0.00
5	7.79	0.00	11.31	87.873	0.052	77.274	76.62	0.00	0.00
6	0.00	0.03	4.41	87.874	0.080	81.682	0.00	0.00	0.00
7	0.35	0.00	0.22	88.224	0.080	81.906	3.80	0.00	0.00
8	0.56	0.00	2.05	88.786	0.080	83.952	6.50	0.00	0.00
9	0.01	0.00	0.04	88.792	0.085	83.988	0.07	0.00	0.00
10	0.00	0.00	0.66	88.795	0.088	84.644	0.04	0.00	0.00
11	0.03	0.01	0.24	88.822	0.093	84.882	0.35	0.00	0.00
12	0.00	0.00	0.00	88.822	0.093	84.882	0.00	0.00	0.00
13	0.00	0.00	0.00	88.822	0.093	84.882	0.00	0.00	0.00
14	0.57	0.00	0.00	89.397	0.093	84.882	7.61	0.00	0.00
15	0.00	0.03	1.31	89.397	0.118	86.190	0.00	0.00	0.00
16	0.01	0.09	2.07	89.405	0.207	88.258	0.13	0.00	0.00
17	0.01	0.00	0.02	89.412	0.208	88.279	0.10	0.00	0.00
18	0.58	0.00	0.04	89.990	0.211	88.320	9.04	0.00	0.00
19	0.69	0.00	0.03	90.678	0.212	88.354	10.85	0.00	0.00
20	0.03	0.01	0.00	90.711	0.221	88.355	0.55	0.00	0.00
21	0.01	0.00	0.02	90.719	0.221	88.373	0.13	0.00	0.00
22	0.24	0.00	0.01	90.959	0.223	88.381	4.10	0.00	0.00

23	0.27	0.00	0.00	91.225	0.223	88.381	4.64	0.00	0.00
24	0.00	0.08	0.47	91.228	0.304	88.850	0.05	0.00	0.00
25	0.01	0.00	0.00	91.239	0.305	88.853	0.19	0.00	0.00
26	0.06	0.07	0.00	91.294	0.372	88.853	1.02	0.00	0.00
27	0.04	0.03	0.01	91.332	0.398	88.858	0.70	0.00	0.00
28	0.07	0.00	0.00	91.404	0.398	88.860	1.38	0.00	0.00
29	0.00	0.19	0.39	91.408	0.586	89.246	0.09	0.00	0.00
30	0.00	0.14	0.00	91.408	0.731	89.249	0.00	0.00	0.00
31	0.01	0.05	0.00	91.415	0.777	89.250	0.13	0.00	0.00
32	0.00	0.18	0.00	91.417	0.957	89.251	0.04	0.00	0.00
33	0.00	0.05	0.00	91.418	1.005	89.251	0.03	0.00	0.00
34	0.00	0.06	0.03	91.418	1.064	89.281	0.00	0.00	0.00
35	0.02	16.03	0.02	91.441	17.094	89.299	0.47	0.00	0.00
36	0.00	0.02	0.00	91.443	17.116	89.299	0.03	0.00	0.00
37	0.00	0.03	0.00	91.443	17.148	89.299	0.00	0.00	0.00
38	0.00	6.63	0.05	91.446	23.776	89.352	0.05	0.00	0.00
39	0.00	0.00	0.16	91.446	23.776	89.509	0.00	0.00	0.00
40	0.00	0.00	0.00	91.446	23.776	89.509	0.00	0.00	0.00
41	0.00	0.24	0.06	91.447	24.019	89.573	0.02	0.00	0.00
42	0.00	0.67	0.27	91.450	24.686	89.844	0.06	0.00	0.00
43	0.00	0.04	0.02	91.453	24.725	89.863	0.07	0.00	0.00
44	0.00	0.00	0.00	91.456	24.726	89.864	0.05	0.00	0.00
45	0.00	0.01	0.02	91.456	24.739	89.884	0.00	0.00	0.00
46	0.00	0.00	0.00	91.456	24.739	89.884	0.00	0.00	0.00
47	0.00	0.00	0.00	91.456	24.739	89.884	0.01	0.00	0.00

STAAD SPACE

-- PAGE NO. 59

MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
48	0.03	0.56	0.00	91.483	25.295	89.887	0.58	0.00	0.00
49	0.00	0.21	0.01	91.483	25.505	89.900	0.00	0.00	0.00
50	0.00	0.01	0.24	91.484	25.519	90.136	0.00	0.00	0.00
51	0.04	19.57	0.00	91.519	45.088	90.137	0.76	0.00	0.00

52	0.01	4.75	0.00	91.524	49.839	90.139	0.12	0.00	0.00
53	0.05	11.29	0.03	91.571	61.132	90.165	1.04	0.00	0.00
54	0.09	0.10	0.64	91.658	61.235	90.801	1.93	0.00	0.00
55	0.00	0.74	0.03	91.662	61.979	90.833	0.09	0.00	0.00
56	0.00	0.03	0.25	91.662	62.006	91.084	0.00	0.00	0.00
57	0.01	0.13	0.71	91.668	62.139	91.797	0.14	0.00	0.00
58	0.12	0.93	0.03	91.784	63.065	91.825	2.61	0.00	0.00
59	0.00	0.00	0.01	91.784	63.066	91.832	0.01	0.00	0.00
60	0.00	0.01	0.05	91.784	63.072	91.883	0.01	0.00	0.00
61	0.00	0.01	0.02	91.784	63.078	91.901	0.00	0.00	0.00
62	0.00	0.62	0.14	91.786	63.695	92.042	0.04	0.00	0.00
63	0.00	2.03	0.01	91.787	65.726	92.048	0.02	0.00	0.00
64	0.01	0.52	0.02	91.792	66.251	92.066	0.12	0.00	0.00
65	0.00	0.13	0.02	91.793	66.380	92.083	0.00	0.00	0.00
66	0.00	0.14	0.04	91.793	66.516	92.120	0.00	0.00	0.00
67	0.09	0.07	0.03	91.881	66.584	92.153	2.06	0.00	0.00
68	0.00	0.00	0.13	91.882	66.588	92.287	0.01	0.00	0.00
69	0.00	0.00	0.00	91.883	66.589	92.288	0.02	0.00	0.00
70	0.00	0.00	0.00	91.884	66.591	92.293	0.02	0.00	0.00
71	0.00	0.12	0.00	91.887	66.714	92.294	0.07	0.00	0.00
72	0.00	0.36	0.00	91.887	67.072	92.298	0.01	0.00	0.00
73	0.00	0.05	0.01	91.888	67.121	92.304	0.01	0.00	0.00
74	0.00	0.01	0.00	91.888	67.129	92.304	0.00	0.00	0.00
75	0.01	0.00	0.01	91.893	67.131	92.309	0.14	0.00	0.00
76	0.04	0.28	0.06	91.936	67.411	92.369	1.00	0.00	0.00
77	0.00	3.60	0.00	91.936	71.007	92.370	0.00	0.00	0.00
78	0.01	12.22	0.00	91.942	83.230	92.371	0.16	0.00	0.00
79	0.07	0.01	0.11	92.010	83.240	92.483	1.62	0.00	0.00
80	0.06	0.67	0.01	92.066	83.913	92.492	1.34	0.00	0.00
81	0.01	0.15	0.01	92.077	84.060	92.500	0.28	0.00	0.00
82	0.01	1.89	0.02	92.091	85.945	92.524	0.33	0.00	0.00
83	0.01	3.67	0.14	92.101	89.611	92.669	0.23	0.00	0.00
84	0.00	0.07	0.00	92.103	89.680	92.669	0.05	0.00	0.00
85	0.01	0.06	0.00	92.108	89.742	92.669	0.13	0.00	0.00
86	0.00	0.01	0.05	92.108	89.755	92.715	0.00	0.00	0.00
87	0.00	0.01	0.09	92.108	89.766	92.806	0.00	0.00	0.00
88	0.00	0.08	0.00	92.109	89.846	92.808	0.01	0.00	0.00
89	0.00	0.00	0.09	92.113	89.848	92.895	0.10	0.00	0.00

90	0.01	0.00	0.06	92.120	89.853	92.955	0.17	0.00	0.00
91	0.00	0.05	0.13	92.122	89.905	93.081	0.07	0.00	0.00
92	0.00	0.07	0.09	92.122	89.976	93.176	0.00	0.00	0.00
93	0.00	0.01	0.09	92.122	89.985	93.266	0.00	0.00	0.00

STAAD SPACE

-- PAGE NO. 60

MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
94	0.00	0.02	0.11	92.123	90.006	93.373	0.00	0.00	0.00
95	0.00	0.31	0.05	92.123	90.318	93.418	0.02	0.00	0.00
96	0.00	0.03	0.00	92.124	90.349	93.419	0.00	0.00	0.00
97	0.01	2.17	0.03	92.130	92.515	93.448	0.16	0.00	0.00
98	0.00	0.06	0.02	92.130	92.578	93.463	0.00	0.00	0.00
99	0.00	0.14	0.07	92.131	92.721	93.532	0.02	0.00	0.00
100	0.00	0.16	0.02	92.134	92.884	93.554	0.09	0.00	0.00
101	0.03	0.30	0.07	92.164	93.188	93.623	0.74	0.00	0.00
102	0.00	0.35	0.03	92.167	93.541	93.657	0.08	0.00	0.00
103	0.02	0.03	0.06	92.182	93.569	93.717	0.40	0.00	0.00
104	0.01	1.12	0.14	92.192	94.688	93.854	0.24	0.00	0.00
105	0.00	0.55	0.01	92.193	95.239	93.866	0.03	0.00	0.00
106	0.00	0.10	0.01	92.197	95.342	93.873	0.11	0.00	0.00
107	0.01	0.01	0.05	92.205	95.350	93.924	0.20	0.00	0.00
108	0.00	0.15	0.01	92.205	95.503	93.937	0.00	0.00	0.00
109	0.03	0.61	0.00	92.231	96.110	93.938	0.65	0.00	0.00
110	0.07	0.08	0.06	92.302	96.187	94.001	1.84	0.00	0.00
111	0.00	0.08	0.01	92.304	96.267	94.010	0.05	0.00	0.00
112	0.01	0.21	0.00	92.314	96.477	94.010	0.26	0.00	0.00
113	0.00	0.00	0.18	92.315	96.478	94.191	0.02	0.00	0.00
114	0.00	0.02	0.01	92.315	96.500	94.205	0.00	0.00	0.00
115	0.00	0.03	0.00	92.316	96.526	94.208	0.03	0.00	0.00
116	0.01	0.00	0.00	92.330	96.530	94.211	0.35	0.00	0.00
117	0.10	0.01	0.01	92.428	96.540	94.219	2.56	0.00	0.00
118	0.00	0.00	0.00	92.428	96.544	94.220	0.00	0.00	0.00

119	0.04	0.03	0.00	92.466	96.576	94.223	0.98	0.00	0.00
120	0.00	0.01	0.00	92.466	96.585	94.223	0.00	0.00	0.00
121	0.01	0.07	0.00	92.477	96.659	94.224	0.28	0.00	0.00
122	0.00	0.16	0.00	92.479	96.817	94.226	0.05	0.00	0.00
123	0.00	0.01	0.01	92.481	96.824	94.235	0.06	0.00	0.00
124	0.00	0.52	0.00	92.483	97.348	94.236	0.05	0.00	0.00
125	0.06	0.00	0.00	92.548	97.351	94.238	1.70	0.00	0.00
126	0.01	0.02	0.00	92.554	97.369	94.240	0.17	0.00	0.00
127	0.00	0.01	0.22	92.555	97.381	94.461	0.03	0.00	0.00
128	0.00	0.00	0.02	92.560	97.382	94.482	0.12	0.00	0.00
129	0.01	0.00	0.00	92.572	97.385	94.482	0.33	0.00	0.00
130	0.01	0.31	0.05	92.578	97.692	94.534	0.15	0.00	0.00
131	0.01	0.01	0.05	92.583	97.699	94.579	0.15	0.00	0.00
132	0.00	0.02	0.01	92.584	97.714	94.587	0.02	0.00	0.00
133	0.00	0.00	0.04	92.584	97.715	94.625	0.01	0.00	0.00
134	0.00	0.03	0.15	92.585	97.747	94.772	0.01	0.00	0.00
135	0.01	0.01	0.00	92.590	97.761	94.774	0.14	0.00	0.00
136	0.00	0.03	0.00	92.594	97.787	94.778	0.09	0.00	0.00
137	0.01	0.00	0.00	92.602	97.787	94.782	0.23	0.00	0.00
138	0.00	0.02	0.06	92.606	97.809	94.842	0.11	0.00	0.00
139	0.01	0.03	0.02	92.615	97.836	94.859	0.22	0.00	0.00

STAAD SPACE

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MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
140	0.00	0.00	0.00	92.615	97.841	94.860	0.00	0.00	0.00
141	0.00	0.18	0.09	92.619	98.024	94.954	0.11	0.00	0.00
142	0.00	0.04	0.00	92.620	98.061	94.955	0.02	0.00	0.00
143	0.01	0.03	0.01	92.626	98.088	94.960	0.18	0.00	0.00
144	0.07	0.01	0.01	92.693	98.099	94.973	1.80	0.00	0.00
145	0.02	0.00	0.01	92.716	98.099	94.986	0.62	0.00	0.00
146	0.00	0.13	0.01	92.718	98.230	94.994	0.05	0.00	0.00
147	0.04	0.00	0.00	92.753	98.230	94.996	0.96	0.00	0.00

148	0.15	0.01	0.01	92.902	98.243	95.002	4.02	0.00	0.00
149	0.03	0.01	0.01	92.930	98.256	95.009	0.75	0.00	0.00
150	0.07	0.03	0.01	93.002	98.282	95.022	1.95	0.00	0.00
151	0.00	0.03	0.01	93.005	98.307	95.030	0.07	0.00	0.00
152	0.04	0.00	0.01	93.040	98.309	95.039	0.96	0.00	0.00
153	0.01	0.01	0.00	93.055	98.319	95.041	0.39	0.00	0.00
154	0.01	0.05	0.00	93.063	98.372	95.041	0.22	0.00	0.00
155	0.08	0.01	0.02	93.139	98.381	95.061	2.08	0.00	0.00
156	0.01	0.04	0.00	93.154	98.417	95.062	0.41	0.00	0.00
157	0.01	0.00	0.00	93.163	98.417	95.066	0.24	0.00	0.00
158	0.01	0.00	0.00	93.174	98.419	95.066	0.30	0.00	0.00
159	0.01	0.07	0.00	93.181	98.485	95.066	0.18	0.00	0.00
160	0.00	0.00	0.00	93.181	98.487	95.067	0.00	0.00	0.00
161	0.05	0.01	0.00	93.232	98.500	95.068	1.40	0.00	0.00
162	0.01	0.02	0.00	93.238	98.516	95.069	0.16	0.00	0.00
163	0.00	0.01	0.00	93.239	98.527	95.069	0.01	0.00	0.00
164	0.00	0.00	0.00	93.239	98.531	95.069	0.01	0.00	0.00
165	0.00	0.00	0.00	93.241	98.533	95.070	0.06	0.00	0.00
166	0.00	0.00	0.00	93.243	98.534	95.070	0.04	0.00	0.00
167	0.01	0.02	0.00	93.253	98.554	95.073	0.27	0.00	0.00
168	0.00	0.00	0.00	93.253	98.555	95.073	0.02	0.00	0.00
169	0.00	0.10	0.00	93.254	98.652	95.073	0.02	0.00	0.00
170	0.00	0.09	0.00	93.259	98.741	95.073	0.13	0.00	0.00
171	0.00	0.02	0.01	93.260	98.757	95.081	0.04	0.00	0.00
172	0.01	0.02	0.02	93.271	98.779	95.105	0.29	0.00	0.00
173	0.05	0.03	0.00	93.316	98.807	95.105	1.25	0.00	0.00
174	0.00	0.01	0.00	93.316	98.812	95.109	0.00	0.00	0.00
175	0.00	0.00	0.08	93.318	98.813	95.188	0.04	0.00	0.00
176	0.00	0.00	0.00	93.322	98.817	95.191	0.11	0.00	0.00
177	0.02	0.00	0.00	93.341	98.817	95.194	0.53	0.00	0.00
178	0.00	0.05	0.00	93.343	98.868	95.197	0.06	0.00	0.00
179	0.01	0.00	0.00	93.352	98.871	95.198	0.23	0.00	0.00
180	0.06	0.05	0.00	93.412	98.919	95.199	1.68	0.00	0.00
181	0.00	0.08	0.01	93.414	99.003	95.213	0.04	0.00	0.00
182	0.01	0.06	0.00	93.423	99.059	95.214	0.27	0.00	0.00
183	0.00	0.01	0.00	93.424	99.065	95.215	0.01	0.00	0.00
184	0.02	0.01	0.00	93.444	99.078	95.215	0.57	0.00	0.00
185	0.01	0.00	0.00	93.453	99.081	95.216	0.23	0.00	0.00

MASS PARTICIPATION FACTORS IN PERCENT							BASE SHEAR IN KN		
MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
186	0.00	0.00	0.00	93.456	99.082	95.216	0.10	0.00	0.00
187	0.00	0.00	0.01	93.461	99.082	95.225	0.14	0.00	0.00
188	0.03	0.02	0.01	93.488	99.103	95.232	0.76	0.00	0.00
189	0.00	0.00	0.00	93.489	99.106	95.233	0.01	0.00	0.00
190	0.01	0.00	0.00	93.497	99.106	95.233	0.23	0.00	0.00
191	0.00	0.00	0.00	93.500	99.111	95.233	0.08	0.00	0.00
192	0.03	0.02	0.00	93.530	99.134	95.234	0.83	0.00	0.00
193	0.01	0.00	0.00	93.537	99.134	95.234	0.21	0.00	0.00
194	0.00	0.00	0.00	93.538	99.134	95.234	0.01	0.00	0.00
195	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00
196	0.00	0.00	0.00	93.538	99.134	95.236	0.01	0.00	0.00
197	0.02	0.01	0.00	93.557	99.148	95.241	0.53	0.00	0.00
198	0.00	0.02	0.02	93.560	99.165	95.260	0.08	0.00	0.00
199	0.00	0.00	0.00	93.561	99.169	95.262	0.02	0.00	0.00
200	0.02	0.01	0.00	93.577	99.182	95.263	0.43	0.00	0.00
							-----		
TOTAL SRSS SHEAR							763.35	0.00	0.00
TOTAL 10PCT SHEAR							851.30	0.00	0.00
TOTAL ABS SHEAR							958.31	0.00	0.00

RESPONSE LOAD CASE 8

SRSS MODAL COMBINATION METHOD USED.

DYNAMIC WEIGHT X Y Z 8.303278E+03 9.798368E+03 8.303278E+03 KN

MISSING WEIGHT X Y Z -5.333425E+02 -8.010067E+01 -3.933308E+02 KN

MODAL WEIGHT X Y Z 7.769936E+03 9.718267E+03 7.909947E+03 KN

MODE	ACCELERATION-G	DAMPING
----	-----	-----
1	0.15246	0.05000
2	0.15725	0.05000
3	0.16797	0.05000
4	0.17258	0.05000
5	0.17563	0.05000
6	0.18092	0.05000
7	0.19600	0.05000
8	0.20714	0.05000
9	0.21840	0.05000
10	0.22025	0.05000

STAAD SPACE

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MODE	ACCELERATION-G	DAMPING
----	-----	-----
11	0.22817	0.05000
12	0.23650	0.05000
13	0.23652	0.05000
14	0.23774	0.05000
15	0.24119	0.05000
16	0.26580	0.05000
17	0.27430	0.05000
18	0.27831	0.05000
19	0.28061	0.05000
20	0.29520	0.05000
21	0.30259	0.05000
22	0.30391	0.05000
23	0.31101	0.05000
24	0.31627	0.05000



25	0.32449	0.05000
26	0.32816	0.05000
27	0.33412	0.05000
28	0.34383	0.05000
29	0.35139	0.05000
30	0.35657	0.05000
31	0.35797	0.05000
32	0.36129	0.05000
33	0.36297	0.05000
34	0.36304	0.05000
35	0.36465	0.05000
36	0.36560	0.05000
37	0.36681	0.05000
38	0.36839	0.05000
39	0.36925	0.05000
40	0.37042	0.05000
41	0.37187	0.05000
42	0.37226	0.05000
43	0.37356	0.05000
44	0.37564	0.05000
45	0.37678	0.05000
46	0.37727	0.05000
47	0.37876	0.05000
48	0.37968	0.05000
49	0.38065	0.05000
50	0.38516	0.05000
51	0.38645	0.05000
52	0.39048	0.05000
53	0.39109	0.05000
54	0.39823	0.05000
55	0.39929	0.05000
56	0.40104	0.05000
57	0.40348	0.05000
58	0.40530	0.05000
59	0.40749	0.05000
60	0.40820	0.05000
61	0.40909	0.05000

MODE	ACCELERATION-G	DAMPING
----	-----	-----
62	0.40957	0.05000
63	0.40974	0.05000
64	0.41094	0.05000
65	0.41229	0.05000
66	0.41333	0.05000
67	0.41389	0.05000
68	0.41517	0.05000
69	0.41692	0.05000
70	0.41817	0.05000
71	0.41901	0.05000
72	0.41969	0.05000
73	0.41978	0.05000
74	0.41997	0.05000
75	0.42073	0.05000
76	0.42274	0.05000
77	0.42519	0.05000
78	0.42563	0.05000
79	0.42661	0.05000
80	0.42942	0.05000
81	0.42993	0.05000
82	0.43154	0.05000
83	0.43276	0.05000
84	0.43691	0.05000
85	0.43869	0.05000
86	0.43982	0.05000
87	0.44135	0.05000
88	0.44172	0.05000
89	0.44215	0.05000
90	0.44308	0.05000
91	0.44346	0.05000
92	0.44576	0.05000
93	0.44607	0.05000

94	0.44628	0.05000
95	0.44750	0.05000
96	0.44791	0.05000
97	0.44816	0.05000
98	0.44927	0.05000
99	0.44974	0.05000
100	0.45056	0.05000
101	0.45141	0.05000
102	0.45295	0.05000
103	0.45455	0.05000
104	0.45550	0.05000
105	0.45683	0.05000
106	0.45687	0.05000
107	0.45856	0.05000
108	0.45917	0.05000
109	0.45956	0.05000
110	0.45964	0.05000
111	0.46156	0.05000
112	0.46182	0.05000

STAAD SPACE

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MODE	ACCELERATION-G	DAMPING
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113	0.46225	0.05000
114	0.46278	0.05000
115	0.46307	0.05000
116	0.46368	0.05000
117	0.46428	0.05000
118	0.46517	0.05000
119	0.46589	0.05000
120	0.46633	0.05000
121	0.46662	0.05000
122	0.46685	0.05000
123	0.46754	0.05000
124	0.46824	0.05000

125	0.46883	0.05000
126	0.47021	0.05000
127	0.47085	0.05000
128	0.47133	0.05000
129	0.47188	0.05000
130	0.47252	0.05000
131	0.47340	0.05000
132	0.47364	0.05000
133	0.47483	0.05000
134	0.47498	0.05000
135	0.47584	0.05000
136	0.47624	0.05000
137	0.47706	0.05000
138	0.47710	0.05000
139	0.47810	0.05000
140	0.47908	0.05000
141	0.47938	0.05000
142	0.48002	0.05000
143	0.48080	0.05000
144	0.48104	0.05000
145	0.48165	0.05000
146	0.48180	0.05000
147	0.48205	0.05000
148	0.48227	0.05000
149	0.48256	0.05000
150	0.48266	0.05000
151	0.48284	0.05000
152	0.48331	0.05000
153	0.48361	0.05000
154	0.48371	0.05000
155	0.48399	0.05000
156	0.48466	0.05000
157	0.48515	0.05000
158	0.48521	0.05000
159	0.48649	0.05000
160	0.48696	0.05000
161	0.48706	0.05000
162	0.48734	0.05000

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STAAD SPACE

0.48744 0.05000

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MODE	ACCELERATION-G	DAMPING
----	-----	-----
164	0.48755	0.05000
165	0.48810	0.05000
166	0.48833	0.05000
167	0.48864	0.05000
168	0.48874	0.05000
169	0.48922	0.05000
170	0.48978	0.05000
171	0.49051	0.05000
172	0.49143	0.05000
173	0.49184	0.05000
174	0.49217	0.05000
175	0.49286	0.05000
176	0.49302	0.05000
177	0.49351	0.05000
178	0.49372	0.05000
179	0.49384	0.05000
180	0.49435	0.05000
181	0.49445	0.05000
182	0.49467	0.05000
183	0.49503	0.05000
184	0.49540	0.05000
185	0.49586	0.05000
186	0.49629	0.05000
187	0.49642	0.05000
188	0.49661	0.05000
189	0.49689	0.05000
190	0.49693	0.05000
191	0.49736	0.05000
192	0.49757	0.05000
193	0.49787	0.05000

194	0.49666	0.05000
195	0.49589	0.05000
196	0.49563	0.05000
197	0.49371	0.05000
198	0.49335	0.05000
199	0.49073	0.05000
200	0.48487	0.05000

STAAD SPACE

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MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
1	1.208	-31.00	11.10	718.97	8846.62	-129436.64	2423.75
2	1.146	5.78	-2.20	109.87	1534.22	-23882.72	-522.64
3	1.008	-9.09	1.41	3.78	39.60	-1312.22	382.30
4	0.970	-88.39	-0.70	6.96	110.83	-5008.28	1020.34
5	0.948	136.87	2.24	164.90	2076.88	-33286.32	-1317.46
6	0.909	-0.57	-5.69	66.23	1063.20	-14520.20	-1245.51
7	0.798	4.55	0.05	3.63	41.74	-61.39	-53.31
8	0.741	-18.45	0.30	35.20	368.66	-8386.76	333.76
9	0.686	0.25	-0.25	0.65	22.66	-154.32	-59.09
10	0.678	0.80	-0.88	12.01	57.12	-2539.07	-203.82
11	0.644	1.52	-0.73	4.51	85.43	-792.61	-193.78
12	0.608	0.00	0.00	0.00	0.00	0.01	-0.01
13	0.608	0.00	0.00	0.00	0.00	0.00	0.00
14	0.602	-0.27	0.00	0.01	-0.04	-11.78	2.79
15	0.590	-0.04	-3.94	26.19	409.92	-5743.72	-849.27
16	0.505	-2.91	-10.31	45.64	871.46	-10053.10	-2248.35
17	0.484	0.27	-0.09	0.48	7.60	-107.89	-29.14
18	0.475	3.54	0.28	0.94	-4.75	-39.94	62.43
19	0.470	3.60	0.13	0.81	0.78	-18.85	32.05
20	0.438	0.11	0.06	0.01	-2.45	1.78	3.78
21	0.421	0.29	0.04	0.47	-13.27	-61.12	25.95

22	0.418	1.04	-0.09	0.18	7.22	4.40	8.93
23	0.402	0.41	-0.01	0.02	0.23	6.33	-1.76
24	0.393	-0.98	-5.54	12.31	264.55	-2534.07	-1137.97
25	0.380	0.13	-0.05	0.06	2.63	-7.01	-7.82
26	0.374	0.08	-0.10	0.00	4.03	1.34	-15.72
27	0.364	0.40	-0.36	0.15	14.60	-15.16	-70.54
28	0.349	-0.29	-0.01	0.04	-0.79	-15.95	-9.36
29	0.336	1.24	-8.56	11.28	285.80	-1950.98	-1573.13
30	0.328	0.00	0.69	0.10	-25.06	-15.85	145.41
31	0.326	-0.04	-0.10	0.01	3.32	-3.38	-21.15
32	0.320	-0.04	0.47	0.04	-17.17	-11.25	103.65
33	0.318	-0.02	0.10	0.01	-3.49	-2.01	20.33
34	0.318	-0.03	-1.37	0.89	47.35	-203.60	-249.01
35	0.315	-0.63	-17.90	0.56	686.80	-129.02	-3347.99
36	0.313	0.00	0.00	0.00	-0.10	0.04	0.17
37	0.311	0.00	0.06	0.00	-2.31	-0.88	13.78
38	0.309	-0.36	19.58	1.60	-697.34	-353.13	4255.21
39	0.308	-0.01	0.15	4.82	-7.99	-1084.08	59.10
40	0.306	0.00	0.00	0.00	0.00	0.00	0.00
41	0.303	-0.27	4.18	1.98	-173.66	-352.92	838.83
42	0.303	0.88	-14.29	8.39	508.95	-1417.18	-2687.54
43	0.301	-0.25	-0.90	0.57	38.22	-101.98	-146.42
44	0.297	0.06	0.04	0.04	-2.60	-7.66	9.44
45	0.295	0.01	0.53	0.61	-26.45	-99.82	103.61
46	0.295	0.00	0.00	0.00	0.00	0.00	0.02
47	0.292	0.00	0.00	0.00	0.07	-0.01	-0.39

STAAD SPACE

-- PAGE NO. 68

MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
48	0.291	0.32	-1.58	0.12	58.88	-13.52	-303.52
49	0.289	0.00	1.77	0.40	-61.93	-117.85	292.10
50	0.282	0.18	1.95	7.56	-65.91	-1706.78	322.92

51	0.280	-0.10	2.68	0.01	-102.16	-3.95	455.01
52	0.273	0.11	3.52	0.07	-131.80	-12.08	727.20
53	0.272	-1.14	19.07	0.84	-723.43	-216.00	3735.38
54	0.261	-7.77	-9.20	21.05	336.51	-4137.93	-2253.88
55	0.259	-0.39	-5.57	1.07	236.68	-214.44	-948.51
56	0.256	0.18	-3.01	8.35	35.80	-1590.82	-483.14
57	0.252	2.19	11.20	23.87	-562.26	-4097.88	2317.73
58	0.249	1.93	-5.93	0.96	211.71	-87.26	-866.80
59	0.246	-0.06	-0.04	0.24	-1.39	-60.62	-31.60
60	0.245	0.13	-0.67	1.73	25.28	-362.03	-54.76
61	0.243	0.06	0.38	0.59	-11.70	-123.07	111.57
62	0.243	0.52	10.93	4.83	-390.80	-984.23	2290.85
63	0.242	-0.08	-4.07	0.20	145.03	-40.00	-856.72
64	0.240	-0.33	-3.53	0.59	126.47	-142.31	-687.74
65	0.238	0.05	1.73	0.57	-67.20	-122.70	398.92
66	0.236	-0.06	2.64	1.27	-123.77	-260.79	458.36
67	0.236	1.87	1.78	1.15	-66.17	-166.06	279.73
68	0.233	-0.25	-0.86	4.63	-10.52	-876.12	-196.21
69	0.231	-0.02	0.02	0.01	-0.55	-3.29	5.66
70	0.229	0.07	-0.13	0.16	10.17	-30.51	-39.02
71	0.227	0.08	0.55	0.06	-20.72	-4.62	114.46
72	0.226	0.04	1.39	0.13	-55.58	-22.45	281.92
73	0.226	-0.05	0.65	0.21	-24.32	-41.08	130.89
74	0.226	0.00	0.00	0.00	0.17	0.02	-0.92
75	0.225	0.19	-0.14	0.19	0.08	-15.68	-23.36
76	0.221	1.77	-4.95	2.11	179.83	-344.50	-904.81
77	0.217	0.00	0.73	0.00	-25.90	-1.67	170.64
78	0.217	0.12	5.73	0.06	-207.72	-14.80	1206.84
79	0.215	-3.08	1.25	3.95	-85.98	-858.90	713.79
80	0.211	-0.81	-3.08	0.33	114.81	-98.91	-975.78
81	0.210	0.33	1.27	0.26	-54.28	-37.19	334.72
82	0.207	-0.66	-8.36	0.88	295.57	-200.61	-1768.88
83	0.205	1.34	28.42	5.20	-1222.79	-914.20	5665.05
84	0.198	0.02	-0.16	0.01	6.76	-2.28	-31.23
85	0.196	-0.05	-0.18	0.01	8.40	-6.76	-27.36
86	0.194	-0.02	0.96	1.68	-29.57	-369.37	252.61
87	0.191	-0.15	1.25	3.31	-115.02	-821.77	261.89
88	0.191	0.03	-0.47	0.06	8.13	-10.55	-101.12



89	0.190	-0.70	-0.53	3.22	42.33	-669.06	-107.28
90	0.188	-0.75	-0.69	2.20	-30.96	-407.90	-192.39
91	0.188	-0.67	3.25	4.63	-263.55	-896.31	689.90
92	0.184	0.05	-3.28	3.51	124.83	-515.70	-498.26
93	0.184	0.08	-1.16	3.33	54.34	-667.79	-249.59

STAAD SPACE

-- PAGE NO. 69

MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
94	0.183	-0.09	-1.92	3.98	40.87	-863.39	-389.73
95	0.181	-0.24	-4.81	1.69	171.77	-332.40	-679.02
96	0.181	0.00	0.12	0.01	-3.92	-2.05	15.71
97	0.180	0.51	10.14	1.08	-391.98	-172.18	1588.42
98	0.178	0.01	1.26	0.57	-41.98	-109.45	189.00
99	0.178	0.26	-4.04	2.59	157.54	-472.78	-731.26
100	0.176	-0.33	2.38	0.79	-88.61	-128.13	334.67
101	0.175	-1.69	5.93	2.61	-234.10	-573.04	1232.06
102	0.173	0.39	4.49	1.29	-173.26	-247.44	1007.32
103	0.170	-1.15	1.68	2.25	-94.71	-582.83	307.41
104	0.168	1.35	-16.08	5.18	511.99	-986.60	-3373.54
105	0.166	0.15	-3.29	0.44	99.38	-75.91	-673.06
106	0.166	0.22	1.14	0.28	-25.83	-49.20	209.16
107	0.163	0.77	-0.85	1.94	16.50	-343.91	-293.08
108	0.163	0.03	-1.84	0.49	55.08	-119.43	-470.25
109	0.162	0.16	0.83	0.03	-28.26	11.60	197.82
110	0.162	2.56	2.89	2.41	-145.63	-361.31	476.15
111	0.159	0.16	-1.13	0.36	31.95	-67.97	-219.04
112	0.158	0.07	-0.35	0.01	14.69	1.60	-69.39
113	0.158	-0.48	0.56	6.94	-69.29	-1610.24	166.23
114	0.157	0.05	0.71	0.53	-14.32	-116.64	153.99
115	0.156	0.07	-0.37	0.11	11.51	-20.49	-64.06
116	0.155	-0.26	0.17	0.13	-18.47	-36.71	13.56
117	0.154	-1.05	0.36	0.29	-12.25	-92.61	31.29

118	0.153	0.01	-0.10	0.06	1.98	-13.31	-0.58
119	0.152	-0.35	-0.35	0.08	11.08	-34.34	-68.00
120	0.151	0.00	-0.01	0.00	0.54	-0.02	-2.77
121	0.151	0.15	-0.42	0.05	18.02	0.50	-110.47
122	0.150	0.08	-0.72	0.07	30.26	-11.25	-129.89
123	0.149	-0.17	0.34	0.35	-29.33	-71.77	103.69
124	0.148	-0.06	1.11	0.05	-38.82	-16.68	220.97
125	0.147	0.37	0.09	0.05	-9.53	-3.20	31.07
126	0.145	0.15	-0.27	0.09	12.32	-6.01	-66.57
127	0.144	0.62	-2.19	8.66	42.09	-1859.65	-387.75
128	0.143	0.37	0.19	0.80	-2.77	-144.33	70.25
129	0.142	0.11	0.06	0.02	-0.20	0.96	17.08
130	0.141	-0.67	5.36	2.01	-229.58	-464.89	1108.71
131	0.140	0.62	-0.73	1.77	20.32	-378.18	-85.09
132	0.139	-0.10	0.49	0.33	-23.26	-90.03	130.20
133	0.137	0.16	-0.27	1.48	4.09	-329.31	-70.96
134	0.137	0.25	-2.92	5.81	79.09	-1246.08	-621.31
135	0.136	0.12	0.21	0.07	-7.68	-7.52	43.13
136	0.135	-0.14	-0.42	0.15	8.02	-32.19	-87.30
137	0.134	-0.23	0.04	0.16	1.63	-51.61	28.45
138	0.134	0.63	1.57	2.40	-73.48	-492.54	359.72
139	0.132	-0.47	-0.92	0.67	22.86	-150.00	-210.78

STAAD SPACE

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MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
140	0.130	0.01	-0.07	0.02	1.79	-8.05	-26.02
141	0.130	0.78	-5.69	3.76	181.74	-677.88	-1193.11
142	0.129	-0.03	-0.23	0.03	7.20	-8.47	-43.42
143	0.128	0.24	0.53	0.22	-21.48	-38.00	125.05
144	0.127	1.16	-0.50	0.51	16.50	-48.62	-86.73
145	0.126	-0.69	0.13	0.52	-6.28	-110.81	34.63
146	0.126	0.16	-1.44	0.34	57.07	-62.86	-318.06

147	0.126	-0.35	-0.02	0.08	-0.81	-27.72	5.93
148	0.125	-1.15	0.36	0.22	-13.57	-76.09	90.62
149	0.125	0.55	-0.41	0.28	14.52	-35.82	-91.27
150	0.125	-1.22	-0.80	0.52	30.88	-148.48	-148.70
151	0.124	-0.19	-0.63	0.34	21.76	-68.29	-112.94
152	0.124	-0.71	-0.18	0.35	10.52	-103.59	-41.58
153	0.123	0.24	-0.22	0.10	9.40	-11.47	-56.98
154	0.123	0.00	0.00	0.00	-0.10	0.05	0.63
155	0.122	-1.56	-0.58	0.79	20.78	-202.38	-98.74
156	0.121	-0.19	0.32	0.06	-13.02	-18.71	60.03
157	0.121	0.22	0.02	0.13	-3.67	-13.70	21.64
158	0.121	-0.09	-0.04	0.02	1.37	-7.09	-4.71
159	0.118	0.00	-0.01	0.00	0.42	0.15	-2.09
160	0.118	-0.01	-0.07	0.05	2.06	-8.79	-7.41
161	0.118	0.22	0.13	0.02	-4.77	4.27	24.40
162	0.117	0.09	-0.17	0.04	5.81	-4.79	-36.32
163	0.117	-0.01	0.05	0.00	-1.67	-1.31	4.93
164	0.117	-0.01	-0.05	0.01	2.22	-2.57	-11.78
165	0.116	0.05	0.04	0.03	-1.71	-4.62	2.52
166	0.115	0.03	-0.03	0.01	1.23	-1.68	-4.46
167	0.115	0.20	-0.31	0.10	10.35	-11.24	-55.58
168	0.115	-0.02	-0.01	0.01	1.05	-2.24	-4.03
169	0.114	0.01	-0.14	0.00	5.77	-0.86	-32.29
170	0.113	0.01	0.07	0.00	-3.25	0.95	13.91
171	0.112	-0.14	-0.49	0.32	19.09	-63.09	-107.79
172	0.110	-0.65	1.02	0.98	-39.70	-216.59	204.27
173	0.110	-0.01	-0.01	0.00	0.37	-0.61	-2.67
174	0.109	-0.03	-0.19	0.15	8.24	-28.47	-40.87
175	0.108	0.42	0.34	3.24	-26.97	-549.67	39.71
176	0.108	-0.15	0.15	0.13	-6.62	-30.52	26.42
177	0.107	0.32	0.05	0.13	-3.14	-9.40	12.80
178	0.107	0.11	-0.55	0.12	21.38	-17.63	-96.68
179	0.107	-0.12	0.07	0.04	-3.06	-14.46	15.09
180	0.106	0.26	0.26	0.03	-10.18	5.15	58.23
181	0.106	-0.18	-1.51	0.57	49.43	-110.51	-346.40
182	0.105	-0.18	0.46	0.08	-17.10	-17.89	95.64
183	0.105	0.01	-0.06	0.02	2.18	-2.88	-13.38
184	0.104	0.08	-0.07	0.01	2.84	1.35	-14.05



4	78.39	0.00	0.49	80.083	0.050	65.966	0.00	0.00	6.96
5	7.79	0.00	11.31	87.873	0.052	77.274	0.00	0.00	164.90
6	0.00	0.03	4.41	87.874	0.080	81.682	0.00	0.00	66.23
7	0.35	0.00	0.22	88.224	0.080	81.906	0.00	0.00	3.63
8	0.56	0.00	2.05	88.786	0.080	83.952	0.00	0.00	35.20
9	0.01	0.00	0.04	88.792	0.085	83.988	0.00	0.00	0.65
10	0.00	0.00	0.66	88.795	0.088	84.644	0.00	0.00	12.01
11	0.03	0.01	0.24	88.822	0.093	84.882	0.00	0.00	4.51
12	0.00	0.00	0.00	88.822	0.093	84.882	0.00	0.00	0.00
13	0.00	0.00	0.00	88.822	0.093	84.882	0.00	0.00	0.00
14	0.57	0.00	0.00	89.397	0.093	84.882	0.00	0.00	0.01
15	0.00	0.03	1.31	89.397	0.118	86.190	0.00	0.00	26.19
16	0.01	0.09	2.07	89.405	0.207	88.258	0.00	0.00	45.64
17	0.01	0.00	0.02	89.412	0.208	88.279	0.00	0.00	0.48
18	0.58	0.00	0.04	89.990	0.211	88.320	0.00	0.00	0.94
19	0.69	0.00	0.03	90.678	0.212	88.354	0.00	0.00	0.81
20	0.03	0.01	0.00	90.711	0.221	88.355	0.00	0.00	0.01
21	0.01	0.00	0.02	90.719	0.221	88.373	0.00	0.00	0.47
22	0.24	0.00	0.01	90.959	0.223	88.381	0.00	0.00	0.18
23	0.27	0.00	0.00	91.225	0.223	88.381	0.00	0.00	0.02
24	0.00	0.08	0.47	91.228	0.304	88.850	0.00	0.00	12.31
25	0.01	0.00	0.00	91.239	0.305	88.853	0.00	0.00	0.06
26	0.06	0.07	0.00	91.294	0.372	88.853	0.00	0.00	0.00
27	0.04	0.03	0.01	91.332	0.398	88.858	0.00	0.00	0.15
28	0.07	0.00	0.00	91.404	0.398	88.860	0.00	0.00	0.04
29	0.00	0.19	0.39	91.408	0.586	89.246	0.00	0.00	11.28
30	0.00	0.14	0.00	91.408	0.731	89.249	0.00	0.00	0.10
31	0.01	0.05	0.00	91.415	0.777	89.250	0.00	0.00	0.01
32	0.00	0.18	0.00	91.417	0.957	89.251	0.00	0.00	0.04
33	0.00	0.05	0.00	91.418	1.005	89.251	0.00	0.00	0.01
34	0.00	0.06	0.03	91.418	1.064	89.281	0.00	0.00	0.89
35	0.02	16.03	0.02	91.441	17.094	89.299	0.00	0.00	0.56
36	0.00	0.02	0.00	91.443	17.116	89.299	0.00	0.00	0.00
37	0.00	0.03	0.00	91.443	17.148	89.299	0.00	0.00	0.00
38	0.00	6.63	0.05	91.446	23.776	89.352	0.00	0.00	1.60
39	0.00	0.00	0.16	91.446	23.776	89.509	0.00	0.00	4.82
40	0.00	0.00	0.00	91.446	23.776	89.509	0.00	0.00	0.00
41	0.00	0.24	0.06	91.447	24.019	89.573	0.00	0.00	1.98

42	0.00	0.67	0.27	91.450	24.686	89.844	0.00	0.00	8.39
43	0.00	0.04	0.02	91.453	24.725	89.863	0.00	0.00	0.57
44	0.00	0.00	0.00	91.456	24.726	89.864	0.00	0.00	0.04
45	0.00	0.01	0.02	91.456	24.739	89.884	0.00	0.00	0.61
46	0.00	0.00	0.00	91.456	24.739	89.884	0.00	0.00	0.00
47	0.00	0.00	0.00	91.456	24.739	89.884	0.00	0.00	0.00

STAAD SPACE

-- PAGE NO. 73

MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
48	0.03	0.56	0.00	91.483	25.295	89.887	0.00	0.00	0.12
49	0.00	0.21	0.01	91.483	25.505	89.900	0.00	0.00	0.40
50	0.00	0.01	0.24	91.484	25.519	90.136	0.00	0.00	7.56
51	0.04	19.57	0.00	91.519	45.088	90.137	0.00	0.00	0.01
52	0.01	4.75	0.00	91.524	49.839	90.139	0.00	0.00	0.07
53	0.05	11.29	0.03	91.571	61.132	90.165	0.00	0.00	0.84
54	0.09	0.10	0.64	91.658	61.235	90.801	0.00	0.00	21.05
55	0.00	0.74	0.03	91.662	61.979	90.833	0.00	0.00	1.07
56	0.00	0.03	0.25	91.662	62.006	91.084	0.00	0.00	8.35
57	0.01	0.13	0.71	91.668	62.139	91.797	0.00	0.00	23.87
58	0.12	0.93	0.03	91.784	63.065	91.825	0.00	0.00	0.96
59	0.00	0.00	0.01	91.784	63.066	91.832	0.00	0.00	0.24
60	0.00	0.01	0.05	91.784	63.072	91.883	0.00	0.00	1.73
61	0.00	0.01	0.02	91.784	63.078	91.901	0.00	0.00	0.59
62	0.00	0.62	0.14	91.786	63.695	92.042	0.00	0.00	4.83
63	0.00	2.03	0.01	91.787	65.726	92.048	0.00	0.00	0.20
64	0.01	0.52	0.02	91.792	66.251	92.066	0.00	0.00	0.59
65	0.00	0.13	0.02	91.793	66.380	92.083	0.00	0.00	0.57
66	0.00	0.14	0.04	91.793	66.516	92.120	0.00	0.00	1.27
67	0.09	0.07	0.03	91.881	66.584	92.153	0.00	0.00	1.15
68	0.00	0.00	0.13	91.882	66.588	92.287	0.00	0.00	4.63
69	0.00	0.00	0.00	91.883	66.589	92.288	0.00	0.00	0.01
70	0.00	0.00	0.00	91.884	66.591	92.293	0.00	0.00	0.16

71	0.00	0.12	0.00	91.887	66.714	92.294	0.00	0.00	0.06
72	0.00	0.36	0.00	91.887	67.072	92.298	0.00	0.00	0.13
73	0.00	0.05	0.01	91.888	67.121	92.304	0.00	0.00	0.21
74	0.00	0.01	0.00	91.888	67.129	92.304	0.00	0.00	0.00
75	0.01	0.00	0.01	91.893	67.131	92.309	0.00	0.00	0.19
76	0.04	0.28	0.06	91.936	67.411	92.369	0.00	0.00	2.11
77	0.00	3.60	0.00	91.936	71.007	92.370	0.00	0.00	0.00
78	0.01	12.22	0.00	91.942	83.230	92.371	0.00	0.00	0.06
79	0.07	0.01	0.11	92.010	83.240	92.483	0.00	0.00	3.95
80	0.06	0.67	0.01	92.066	83.913	92.492	0.00	0.00	0.33
81	0.01	0.15	0.01	92.077	84.060	92.500	0.00	0.00	0.26
82	0.01	1.89	0.02	92.091	85.945	92.524	0.00	0.00	0.88
83	0.01	3.67	0.14	92.101	89.611	92.669	0.00	0.00	5.20
84	0.00	0.07	0.00	92.103	89.680	92.669	0.00	0.00	0.01
85	0.01	0.06	0.00	92.108	89.742	92.669	0.00	0.00	0.01
86	0.00	0.01	0.05	92.108	89.755	92.715	0.00	0.00	1.68
87	0.00	0.01	0.09	92.108	89.766	92.806	0.00	0.00	3.31
88	0.00	0.08	0.00	92.109	89.846	92.808	0.00	0.00	0.06
89	0.00	0.00	0.09	92.113	89.848	92.895	0.00	0.00	3.22
90	0.01	0.00	0.06	92.120	89.853	92.955	0.00	0.00	2.20
91	0.00	0.05	0.13	92.122	89.905	93.081	0.00	0.00	4.63
92	0.00	0.07	0.09	92.122	89.976	93.176	0.00	0.00	3.51
93	0.00	0.01	0.09	92.122	89.985	93.266	0.00	0.00	3.33

STAAD SPACE

-- PAGE NO. 74

MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
94	0.00	0.02	0.11	92.123	90.006	93.373	0.00	0.00	3.98
95	0.00	0.31	0.05	92.123	90.318	93.418	0.00	0.00	1.69
96	0.00	0.03	0.00	92.124	90.349	93.419	0.00	0.00	0.01
97	0.01	2.17	0.03	92.130	92.515	93.448	0.00	0.00	1.08
98	0.00	0.06	0.02	92.130	92.578	93.463	0.00	0.00	0.57
99	0.00	0.14	0.07	92.131	92.721	93.532	0.00	0.00	2.59

100	0.00	0.16	0.02	92.134	92.884	93.554	0.00	0.00	0.79
101	0.03	0.30	0.07	92.164	93.188	93.623	0.00	0.00	2.61
102	0.00	0.35	0.03	92.167	93.541	93.657	0.00	0.00	1.29
103	0.02	0.03	0.06	92.182	93.569	93.717	0.00	0.00	2.25
104	0.01	1.12	0.14	92.192	94.688	93.854	0.00	0.00	5.18
105	0.00	0.55	0.01	92.193	95.239	93.866	0.00	0.00	0.44
106	0.00	0.10	0.01	92.197	95.342	93.873	0.00	0.00	0.28
107	0.01	0.01	0.05	92.205	95.350	93.924	0.00	0.00	1.94
108	0.00	0.15	0.01	92.205	95.503	93.937	0.00	0.00	0.49
109	0.03	0.61	0.00	92.231	96.110	93.938	0.00	0.00	0.03
110	0.07	0.08	0.06	92.302	96.187	94.001	0.00	0.00	2.41
111	0.00	0.08	0.01	92.304	96.267	94.010	0.00	0.00	0.36
112	0.01	0.21	0.00	92.314	96.477	94.010	0.00	0.00	0.01
113	0.00	0.00	0.18	92.315	96.478	94.191	0.00	0.00	6.94
114	0.00	0.02	0.01	92.315	96.500	94.205	0.00	0.00	0.53
115	0.00	0.03	0.00	92.316	96.526	94.208	0.00	0.00	0.11
116	0.01	0.00	0.00	92.330	96.530	94.211	0.00	0.00	0.13
117	0.10	0.01	0.01	92.428	96.540	94.219	0.00	0.00	0.29
118	0.00	0.00	0.00	92.428	96.544	94.220	0.00	0.00	0.06
119	0.04	0.03	0.00	92.466	96.576	94.223	0.00	0.00	0.08
120	0.00	0.01	0.00	92.466	96.585	94.223	0.00	0.00	0.00
121	0.01	0.07	0.00	92.477	96.659	94.224	0.00	0.00	0.05
122	0.00	0.16	0.00	92.479	96.817	94.226	0.00	0.00	0.07
123	0.00	0.01	0.01	92.481	96.824	94.235	0.00	0.00	0.35
124	0.00	0.52	0.00	92.483	97.348	94.236	0.00	0.00	0.05
125	0.06	0.00	0.00	92.548	97.351	94.238	0.00	0.00	0.05
126	0.01	0.02	0.00	92.554	97.369	94.240	0.00	0.00	0.09
127	0.00	0.01	0.22	92.555	97.381	94.461	0.00	0.00	8.66
128	0.00	0.00	0.02	92.560	97.382	94.482	0.00	0.00	0.80
129	0.01	0.00	0.00	92.572	97.385	94.482	0.00	0.00	0.02
130	0.01	0.31	0.05	92.578	97.692	94.534	0.00	0.00	2.01
131	0.01	0.01	0.05	92.583	97.699	94.579	0.00	0.00	1.77
132	0.00	0.02	0.01	92.584	97.714	94.587	0.00	0.00	0.33
133	0.00	0.00	0.04	92.584	97.715	94.625	0.00	0.00	1.48
134	0.00	0.03	0.15	92.585	97.747	94.772	0.00	0.00	5.81
135	0.01	0.01	0.00	92.590	97.761	94.774	0.00	0.00	0.07
136	0.00	0.03	0.00	92.594	97.787	94.778	0.00	0.00	0.15
137	0.01	0.00	0.00	92.602	97.787	94.782	0.00	0.00	0.16



138	0.00	0.02	0.06	92.606	97.809	94.842	0.00	0.00	2.40
139	0.01	0.03	0.02	92.615	97.836	94.859	0.00	0.00	0.67

STAAD SPACE

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MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
140	0.00	0.00	0.00	92.615	97.841	94.860	0.00	0.00	0.02
141	0.00	0.18	0.09	92.619	98.024	94.954	0.00	0.00	3.76
142	0.00	0.04	0.00	92.620	98.061	94.955	0.00	0.00	0.03
143	0.01	0.03	0.01	92.626	98.088	94.960	0.00	0.00	0.22
144	0.07	0.01	0.01	92.693	98.099	94.973	0.00	0.00	0.51
145	0.02	0.00	0.01	92.716	98.099	94.986	0.00	0.00	0.52
146	0.00	0.13	0.01	92.718	98.230	94.994	0.00	0.00	0.34
147	0.04	0.00	0.00	92.753	98.230	94.996	0.00	0.00	0.08
148	0.15	0.01	0.01	92.902	98.243	95.002	0.00	0.00	0.22
149	0.03	0.01	0.01	92.930	98.256	95.009	0.00	0.00	0.28
150	0.07	0.03	0.01	93.002	98.282	95.022	0.00	0.00	0.52
151	0.00	0.03	0.01	93.005	98.307	95.030	0.00	0.00	0.34
152	0.04	0.00	0.01	93.040	98.309	95.039	0.00	0.00	0.35
153	0.01	0.01	0.00	93.055	98.319	95.041	0.00	0.00	0.10
154	0.01	0.05	0.00	93.063	98.372	95.041	0.00	0.00	0.00
155	0.08	0.01	0.02	93.139	98.381	95.061	0.00	0.00	0.79
156	0.01	0.04	0.00	93.154	98.417	95.062	0.00	0.00	0.06
157	0.01	0.00	0.00	93.163	98.417	95.066	0.00	0.00	0.13
158	0.01	0.00	0.00	93.174	98.419	95.066	0.00	0.00	0.02
159	0.01	0.07	0.00	93.181	98.485	95.066	0.00	0.00	0.00
160	0.00	0.00	0.00	93.181	98.487	95.067	0.00	0.00	0.05
161	0.05	0.01	0.00	93.232	98.500	95.068	0.00	0.00	0.02
162	0.01	0.02	0.00	93.238	98.516	95.069	0.00	0.00	0.04
163	0.00	0.01	0.00	93.239	98.527	95.069	0.00	0.00	0.00
164	0.00	0.00	0.00	93.239	98.531	95.069	0.00	0.00	0.01
165	0.00	0.00	0.00	93.241	98.533	95.070	0.00	0.00	0.03
166	0.00	0.00	0.00	93.243	98.534	95.070	0.00	0.00	0.01

167	0.01	0.02	0.00	93.253	98.554	95.073	0.00	0.00	0.10
168	0.00	0.00	0.00	93.253	98.555	95.073	0.00	0.00	0.01
169	0.00	0.10	0.00	93.254	98.652	95.073	0.00	0.00	0.00
170	0.00	0.09	0.00	93.259	98.741	95.073	0.00	0.00	0.00
171	0.00	0.02	0.01	93.260	98.757	95.081	0.00	0.00	0.32
172	0.01	0.02	0.02	93.271	98.779	95.105	0.00	0.00	0.98
173	0.05	0.03	0.00	93.316	98.807	95.105	0.00	0.00	0.00
174	0.00	0.01	0.00	93.316	98.812	95.109	0.00	0.00	0.15
175	0.00	0.00	0.08	93.318	98.813	95.188	0.00	0.00	3.24
176	0.00	0.00	0.00	93.322	98.817	95.191	0.00	0.00	0.13
177	0.02	0.00	0.00	93.341	98.817	95.194	0.00	0.00	0.13
178	0.00	0.05	0.00	93.343	98.868	95.197	0.00	0.00	0.12
179	0.01	0.00	0.00	93.352	98.871	95.198	0.00	0.00	0.04
180	0.06	0.05	0.00	93.412	98.919	95.199	0.00	0.00	0.03
181	0.00	0.08	0.01	93.414	99.003	95.213	0.00	0.00	0.57
182	0.01	0.06	0.00	93.423	99.059	95.214	0.00	0.00	0.08
183	0.00	0.01	0.00	93.424	99.065	95.215	0.00	0.00	0.02
184	0.02	0.01	0.00	93.444	99.078	95.215	0.00	0.00	0.01
185	0.01	0.00	0.00	93.453	99.081	95.216	0.00	0.00	0.02

STAAD SPACE

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MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
186	0.00	0.00	0.00	93.456	99.082	95.216	0.00	0.00	0.01
187	0.00	0.00	0.01	93.461	99.082	95.225	0.00	0.00	0.38
188	0.03	0.02	0.01	93.488	99.103	95.232	0.00	0.00	0.29
189	0.00	0.00	0.00	93.489	99.106	95.233	0.00	0.00	0.03
190	0.01	0.00	0.00	93.497	99.106	95.233	0.00	0.00	0.01
191	0.00	0.00	0.00	93.500	99.111	95.233	0.00	0.00	0.00
192	0.03	0.02	0.00	93.530	99.134	95.234	0.00	0.00	0.03
193	0.01	0.00	0.00	93.537	99.134	95.234	0.00	0.00	0.00
194	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00
195	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00

196	0.00	0.00	0.00	93.538	99.134	95.236	0.00	0.00	0.10
197	0.02	0.01	0.00	93.557	99.148	95.241	0.00	0.00	0.19
198	0.00	0.02	0.02	93.560	99.165	95.260	0.00	0.00	0.80
199	0.00	0.00	0.00	93.561	99.169	95.262	0.00	0.00	0.08
200	0.02	0.01	0.00	93.577	99.182	95.263	0.00	0.00	0.02

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TOTAL SRSS SHEAR      0.00      0.00      752.86
TOTAL 10PCT SHEAR    0.00      0.00      872.49
TOTAL ABS SHEAR      0.00      0.00     1438.02

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RESPONSE LOAD CASE 9

SRSS MODAL COMBINATION METHOD USED.

DYNAMIC WEIGHT X Y Z 8.303278E+03 9.798368E+03 8.303278E+03 KN  
MISSING WEIGHT X Y Z -5.333425E+02 -8.010067E+01 -3.933308E+02 KN  
MODAL WEIGHT X Y Z 7.769936E+03 9.718267E+03 7.909947E+03 KN

MODE	ACCELERATION-G	DAMPING
----	-----	-----
1	0.03509	0.05000
2	0.03623	0.05000
3	0.03877	0.05000
4	0.03974	0.05000
5	0.04037	0.05000
6	0.04147	0.05000
7	0.04518	0.05000
8	0.04760	0.05000
9	0.05009	0.05000
10	0.05052	0.05000

MODE	ACCELERATION-G	DAMPING
----	-----	-----
11	0.05236	0.05000
12	0.05430	0.05000
13	0.05430	0.05000
14	0.05459	0.05000
15	0.05542	0.05000
16	0.06144	0.05000
17	0.06336	0.05000
18	0.06425	0.05000
19	0.06476	0.05000
20	0.06801	0.05000
21	0.06965	0.05000
22	0.06994	0.05000
23	0.07152	0.05000
24	0.07273	0.05000
25	0.07462	0.05000
26	0.07547	0.05000
27	0.07684	0.05000
28	0.07908	0.05000
29	0.08083	0.05000
30	0.08202	0.05000
31	0.08235	0.05000
32	0.08311	0.05000
33	0.08350	0.05000
34	0.08352	0.05000
35	0.08389	0.05000
36	0.08411	0.05000
37	0.08439	0.05000
38	0.08475	0.05000
39	0.08495	0.05000
40	0.08522	0.05000
41	0.08555	0.05000
42	0.08564	0.05000
43	0.08594	0.05000

44	0.08642	0.05000
45	0.08669	0.05000
46	0.08680	0.05000
47	0.08714	0.05000
48	0.08736	0.05000
49	0.08758	0.05000
50	0.08862	0.05000
51	0.08892	0.05000
52	0.08985	0.05000
53	0.08999	0.05000
54	0.09164	0.05000
55	0.09188	0.05000
56	0.09229	0.05000
57	0.09285	0.05000
58	0.09327	0.05000
59	0.09377	0.05000
60	0.09394	0.05000
61	0.09414	0.05000

STAAD SPACE

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MODE	ACCELERATION-G	DAMPING
----	-----	-----
62	0.09425	0.05000
63	0.09429	0.05000
64	0.09457	0.05000
65	0.09488	0.05000
66	0.09512	0.05000
67	0.09525	0.05000
68	0.09554	0.05000
69	0.09595	0.05000
70	0.09624	0.05000
71	0.09643	0.05000
72	0.09659	0.05000
73	0.09661	0.05000
74	0.09665	0.05000

75	0.09683	0.05000
76	0.09729	0.05000
77	0.09786	0.05000
78	0.09796	0.05000
79	0.09818	0.05000
80	0.09883	0.05000
81	0.09895	0.05000
82	0.09932	0.05000
83	0.09960	0.05000
84	0.10056	0.05000
85	0.10097	0.05000
86	0.10123	0.05000
87	0.10159	0.05000
88	0.10167	0.05000
89	0.10177	0.05000
90	0.10198	0.05000
91	0.10207	0.05000
92	0.10260	0.05000
93	0.10267	0.05000
94	0.10272	0.05000
95	0.10300	0.05000
96	0.10310	0.05000
97	0.10316	0.05000
98	0.10341	0.05000
99	0.10352	0.05000
100	0.10371	0.05000
101	0.10391	0.05000
102	0.10426	0.05000
103	0.10463	0.05000
104	0.10485	0.05000
105	0.10516	0.05000
106	0.10517	0.05000
107	0.10556	0.05000
108	0.10570	0.05000
109	0.10579	0.05000
110	0.10581	0.05000
111	0.10625	0.05000
112	0.10631	0.05000

MODE	ACCELERATION-G	DAMPING
----	-----	-----
113	0.10641	0.05000
114	0.10653	0.05000
115	0.10660	0.05000
116	0.10674	0.05000
117	0.10688	0.05000
118	0.10708	0.05000
119	0.10725	0.05000
120	0.10735	0.05000
121	0.10741	0.05000
122	0.10747	0.05000
123	0.10763	0.05000
124	0.10779	0.05000
125	0.10792	0.05000
126	0.10824	0.05000
127	0.10839	0.05000
128	0.10850	0.05000
129	0.10863	0.05000
130	0.10878	0.05000
131	0.10898	0.05000
132	0.10903	0.05000
133	0.10931	0.05000
134	0.10935	0.05000
135	0.10954	0.05000
136	0.10963	0.05000
137	0.10982	0.05000
138	0.10983	0.05000
139	0.11006	0.05000
140	0.11029	0.05000
141	0.11036	0.05000
142	0.11051	0.05000
143	0.11069	0.05000

144	0.11074	0.05000
145	0.11088	0.05000
146	0.11092	0.05000
147	0.11098	0.05000
148	0.11103	0.05000
149	0.11109	0.05000
150	0.11112	0.05000
151	0.11116	0.05000
152	0.11127	0.05000
153	0.11134	0.05000
154	0.11136	0.05000
155	0.11142	0.05000
156	0.11158	0.05000
157	0.11169	0.05000
158	0.11171	0.05000
159	0.11200	0.05000
160	0.11211	0.05000
161	0.11213	0.05000
162	0.11220	0.05000
163	0.11222	0.05000

STAAD SPACE

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MODE	ACCELERATION-G	DAMPING
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164	0.11224	0.05000
165	0.11237	0.05000
166	0.11243	0.05000
167	0.11250	0.05000
168	0.11252	0.05000
169	0.11263	0.05000
170	0.11276	0.05000
171	0.11293	0.05000
172	0.11314	0.05000
173	0.11323	0.05000
174	0.11331	0.05000



175	0.11347	0.05000
176	0.11351	0.05000
177	0.11362	0.05000
178	0.11367	0.05000
179	0.11370	0.05000
180	0.11381	0.05000
181	0.11384	0.05000
182	0.11389	0.05000
183	0.11397	0.05000
184	0.11406	0.05000
185	0.11416	0.05000
186	0.11426	0.05000
187	0.11429	0.05000
188	0.11434	0.05000
189	0.11440	0.05000
190	0.11441	0.05000
191	0.11451	0.05000
192	0.11456	0.05000
193	0.11462	0.05000
194	0.11430	0.05000
195	0.11409	0.05000
196	0.11402	0.05000
197	0.11350	0.05000
198	0.11340	0.05000
199	0.11269	0.05000
200	0.11111	0.05000

STAAD SPACE

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MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
1	1.208	-0.11	0.04	2.56	31.45	-460.08	8.62
2	1.146	-0.03	0.01	-0.51	-7.07	110.04	2.41

3	1.008	-0.78	0.12	0.32	3.40	-112.81	32.87
4	0.970	2.06	0.02	-0.16	-2.58	116.44	-23.72
5	0.948	0.43	0.01	0.51	6.48	-103.89	-4.11
6	0.909	0.01	0.11	-1.30	-20.95	286.07	24.54
7	0.798	0.01	0.00	0.01	0.13	-0.20	-0.17
8	0.741	-0.04	0.00	0.07	0.73	-16.59	0.66
9	0.686	-0.02	0.02	-0.06	-2.04	13.88	5.32
10	0.678	-0.01	0.01	-0.20	-0.96	42.59	3.42
11	0.644	-0.06	0.03	-0.17	-3.17	29.42	7.19
12	0.608	0.00	0.00	0.00	0.00	0.00	0.00
13	0.608	0.00	0.00	0.00	0.00	0.00	0.00
14	0.602	-0.01	0.00	0.00	0.00	-0.36	0.08
15	0.590	0.00	0.14	-0.90	-14.16	198.46	29.34
16	0.505	0.15	0.54	-2.38	-45.52	525.09	117.44
17	0.484	-0.01	0.00	-0.02	-0.34	4.83	1.30
18	0.475	0.24	0.02	0.06	-0.32	-2.72	4.25
19	0.470	0.14	0.01	0.03	0.03	-0.72	1.22
20	0.438	0.11	0.06	0.01	-2.52	1.83	3.89
21	0.421	0.01	0.00	0.01	-0.29	-1.32	0.56
22	0.418	-0.12	0.01	-0.02	-0.84	-0.51	-1.03
23	0.402	-0.02	0.00	0.00	-0.01	-0.34	0.09
24	0.393	0.10	0.57	-1.27	-27.38	262.29	117.78
25	0.380	-0.02	0.01	-0.01	-0.45	1.19	1.32
26	0.374	-0.42	0.50	-0.02	-20.45	-6.82	79.87
27	0.364	-0.21	0.19	-0.08	-7.90	8.20	38.15
28	0.349	0.01	0.00	0.00	0.03	0.64	0.38
29	0.336	-0.22	1.49	-1.97	-49.88	340.48	274.54
30	0.328	0.00	1.16	0.16	-42.03	-26.59	243.90
31	0.326	0.13	0.37	-0.02	-11.98	12.21	76.35
32	0.320	-0.13	1.46	0.11	-52.90	-34.66	319.36
33	0.318	-0.07	0.39	0.02	-14.39	-8.31	83.84
34	0.318	0.01	0.48	-0.32	-16.69	71.75	87.75
35	0.315	4.61	131.77	-4.12	-5055.37	949.70	24643.64
36	0.313	0.04	0.17	0.00	-8.36	3.61	15.14
37	0.311	-0.01	0.27	0.01	-9.69	-3.67	57.76
38	0.309	-1.01	55.04	4.50	-1960.20	-992.64	11961.21
39	0.308	0.00	0.00	0.03	-0.06	-7.80	0.43
40	0.306	0.00	0.00	0.00	0.00	0.00	0.03

41	0.303	-0.13	2.03	0.96	-84.49	-171.71	408.13
42	0.303	-0.35	5.60	-3.29	-199.44	555.35	1053.18
43	0.301	0.09	0.33	-0.21	-13.82	36.86	52.93
44	0.297	0.01	0.01	0.01	-0.63	-1.87	2.30
45	0.295	0.00	0.11	0.12	-5.33	-20.11	20.87
46	0.295	0.00	0.01	0.00	-0.23	-0.02	1.03
47	0.292	0.00	0.00	0.00	-0.07	0.01	0.41

STAAD SPACE

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MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
48	0.291	-0.97	4.75	-0.36	-176.87	40.63	911.72
49	0.289	0.00	1.81	0.41	-63.03	-119.95	297.30
50	0.282	0.01	0.12	0.45	-3.90	-101.11	19.13
51	0.280	-6.64	170.50	0.62	-6494.71	-251.23	28927.29
52	0.273	1.30	41.83	0.81	-1564.71	-143.41	8633.07
53	0.272	-5.94	99.57	4.39	-3777.10	-1127.78	19502.90
54	0.261	0.78	0.93	-2.12	-33.85	416.30	226.75
55	0.259	0.46	6.70	-1.28	-284.59	257.85	1140.52
56	0.256	-0.01	0.25	-0.69	-2.97	131.86	40.05
57	0.252	0.24	1.21	2.58	-60.70	-442.40	250.22
58	0.249	-2.75	8.47	-1.37	-302.06	124.50	1236.72
59	0.246	0.00	0.00	-0.01	0.05	2.20	1.15
60	0.245	-0.01	0.06	-0.15	-2.26	32.32	4.89
61	0.243	0.01	0.06	0.09	-1.72	-18.14	16.45
62	0.243	0.27	5.69	2.51	-203.65	-512.89	1193.77
63	0.242	0.38	18.77	-0.94	-669.19	184.55	3952.96
64	0.240	0.45	4.86	-0.81	-173.94	195.72	945.83
65	0.238	0.04	1.20	0.40	-46.68	-85.24	277.13
66	0.236	-0.03	1.27	0.61	-59.33	-125.02	219.73
67	0.236	0.67	0.63	0.41	-23.56	-59.12	99.60
68	0.233	0.01	0.04	-0.20	0.45	37.37	8.37
69	0.231	-0.01	0.00	0.00	-0.15	-0.88	1.52

70	0.229	-0.01	0.02	-0.03	-1.84	5.52	7.05
71	0.227	0.17	1.16	0.13	-43.30	-9.64	239.16
72	0.226	0.11	3.39	0.32	-135.75	-54.83	688.55
73	0.226	-0.03	0.46	0.15	-17.33	-29.26	93.25
74	0.226	0.00	0.07	0.00	-2.75	-0.31	14.58
75	0.225	-0.03	0.02	-0.03	-0.01	2.68	3.99
76	0.221	-0.95	2.67	-1.14	-97.10	186.02	488.56
77	0.217	-0.21	34.48	0.17	-1229.82	-79.05	8101.35
78	0.217	2.51	117.33	1.32	-4250.04	-302.76	24692.70
79	0.215	-0.22	0.09	0.29	-6.24	-62.36	51.83
80	0.211	1.72	6.52	-0.71	-243.12	209.45	2066.40
81	0.210	0.37	1.42	0.29	-60.64	-41.55	373.94
82	0.207	1.45	18.35	-1.92	-648.71	440.30	3882.34
83	0.205	1.69	35.77	6.54	-1538.90	-1150.54	7129.55
84	0.198	-0.10	0.69	-0.04	-29.19	9.86	134.97
85	0.196	0.17	0.61	-0.04	-28.34	22.79	92.28
86	0.194	0.00	0.13	0.22	-3.87	-48.37	33.08
87	0.191	-0.01	0.11	0.29	-10.00	-71.42	22.76
88	0.191	-0.05	0.80	-0.11	-13.93	18.07	173.31
89	0.190	0.03	0.02	-0.12	-1.59	25.21	4.04
90	0.188	0.05	0.05	-0.16	2.22	29.30	13.82
91	0.188	-0.11	0.52	0.75	-42.55	-144.70	111.38
92	0.184	-0.01	0.71	-0.75	-26.86	110.95	107.20
93	0.184	-0.01	0.09	-0.27	-4.38	53.80	20.11

STAAD SPACE

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MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

MODE	PERIOD	MOMENTS ARE ABOUT THE ORIGIN					
		FX	FY	FZ	MX	MY	MZ
94	0.183	0.01	0.21	-0.44	-4.54	95.99	43.33
95	0.181	0.16	3.15	-1.11	-112.50	217.72	444.75
96	0.181	-0.01	0.31	0.03	-10.14	-5.29	40.59
97	0.180	1.10	21.90	2.33	-846.29	-371.75	3429.44
98	0.178	0.00	0.64	0.29	-21.25	-55.41	95.68

99	0.178	-0.09	1.45	-0.93	-56.58	169.79	262.62
100	0.176	-0.23	1.65	0.55	-61.57	-89.03	232.54
101	0.175	-0.89	3.10	1.37	-122.44	-299.70	644.38
102	0.173	0.32	3.60	1.03	-139.11	-198.68	808.80
103	0.170	-0.20	0.29	0.39	-16.25	-100.02	52.75
104	0.168	-0.96	11.50	-3.70	-365.90	705.09	2410.96
105	0.166	-0.26	5.68	-0.76	-171.54	131.03	1161.78
106	0.166	0.20	1.06	0.26	-24.08	-45.87	194.98
107	0.163	-0.08	0.09	-0.20	-1.66	34.53	29.42
108	0.163	-0.03	1.58	-0.42	-47.37	102.72	404.46
109	0.162	1.18	6.29	0.19	-214.10	87.90	1498.90
110	0.162	0.71	0.80	0.66	-40.22	-99.80	131.52
111	0.159	-0.12	0.83	-0.26	-23.35	49.68	160.10
112	0.158	-0.45	2.19	-0.08	-91.94	-9.99	434.21
113	0.158	-0.01	0.01	0.13	-1.29	-30.09	3.11
114	0.157	0.02	0.22	0.16	-4.45	-36.28	47.90
115	0.156	-0.05	0.27	-0.08	-8.48	15.09	47.17
116	0.155	-0.08	0.05	0.04	-5.47	-10.88	4.02
117	0.154	-0.30	0.10	0.08	-3.47	-26.20	8.85
118	0.153	0.00	0.04	-0.02	-0.74	4.99	0.22
119	0.152	0.34	0.34	-0.08	-10.85	33.61	66.54
120	0.151	-0.01	0.09	0.00	-3.27	0.10	16.81
121	0.151	-0.28	0.78	-0.10	-33.42	-0.93	204.89
122	0.150	-0.18	1.66	-0.17	-69.51	25.84	298.40
123	0.149	-0.04	0.08	0.08	-6.59	-16.12	23.28
124	0.148	-0.29	5.53	0.26	-193.65	-83.21	1102.28
125	0.147	0.14	0.03	0.02	-3.71	-1.24	12.08
126	0.145	-0.10	0.18	-0.06	-8.38	4.09	45.29
127	0.144	-0.04	0.13	-0.50	-2.45	108.30	22.58
128	0.143	0.02	0.01	0.04	-0.15	-7.70	3.75
129	0.142	0.06	0.03	0.01	-0.11	0.51	9.14
130	0.141	-0.41	3.28	1.23	-140.57	-284.65	678.85
131	0.140	-0.06	0.07	-0.17	-1.91	35.62	8.01
132	0.139	-0.03	0.17	0.11	-7.94	-30.74	44.46
133	0.137	-0.01	0.01	-0.06	-0.17	13.99	3.02
134	0.137	-0.03	0.34	-0.67	-9.15	144.20	71.90
135	0.136	0.09	0.15	0.05	-5.33	-5.22	29.94
136	0.135	0.09	0.28	-0.10	-5.27	21.17	57.41

137	0.134	-0.02	0.00	0.01	0.11	-3.34	1.84
138	0.134	0.10	0.24	0.36	-11.10	-74.41	54.35
139	0.132	0.15	0.29	-0.21	-7.26	47.66	66.97
STAAD SPACE					-- PAGE NO. 84		

MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

					MOMENTS ARE ABOUT THE ORIGIN		
MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
140	0.130	0.00	0.05	-0.02	-1.26	5.67	18.33
141	0.130	-0.27	1.98	-1.31	-63.35	236.29	415.88
142	0.129	0.05	0.40	-0.05	-12.67	14.89	76.38
143	0.128	0.13	0.30	0.12	-11.91	-21.08	69.37
144	0.127	-0.26	0.11	-0.12	-3.75	11.06	19.73
145	0.126	-0.04	0.01	0.03	-0.36	-6.28	1.96
146	0.126	-0.16	1.42	-0.33	-56.30	62.02	313.80
147	0.126	0.02	0.00	0.00	0.04	1.32	-0.28
148	0.125	-0.43	0.14	0.08	-5.11	-28.65	34.12
149	0.125	-0.19	0.14	-0.10	-5.02	12.38	31.53
150	0.125	0.44	0.29	-0.18	-11.02	52.99	53.07
151	0.124	0.08	0.27	-0.15	-9.41	29.53	48.84
152	0.124	0.08	0.02	-0.04	-1.25	12.32	4.95
153	0.123	-0.12	0.11	-0.05	-4.85	5.92	29.39
154	0.123	0.21	0.58	0.00	-18.67	8.79	121.12
155	0.122	0.26	0.10	-0.13	-3.52	34.26	16.71
156	0.121	-0.23	0.39	0.07	-15.97	-22.94	73.62
157	0.121	0.01	0.00	0.00	-0.11	-0.39	0.62
158	0.121	0.05	0.02	-0.01	-0.76	3.95	2.62
159	0.118	-0.21	0.72	0.00	-28.38	-9.91	139.41
160	0.118	0.00	0.02	-0.02	-0.59	2.53	2.13
161	0.118	0.27	0.15	0.03	-5.69	5.09	29.07
162	0.117	-0.09	0.17	-0.04	-5.84	4.82	36.56
163	0.117	-0.02	0.12	0.01	-4.63	-3.63	13.66
164	0.117	0.02	0.05	-0.01	-2.33	2.69	12.34
165	0.116	0.02	0.01	0.01	-0.51	-1.36	0.75

166	0.115	-0.01	0.01	-0.01	-0.52	0.71	1.88
167	0.115	-0.14	0.23	-0.07	-7.54	8.19	40.50
168	0.115	0.01	0.00	0.00	-0.38	0.80	1.44
169	0.114	-0.08	1.08	-0.03	-44.67	6.67	250.03
170	0.113	0.21	0.98	0.02	-48.68	14.28	208.10
171	0.112	0.05	0.18	-0.11	-6.83	22.59	38.59
172	0.110	-0.16	0.25	0.24	-9.56	-52.15	49.18
173	0.110	0.36	0.31	0.00	-9.48	15.79	68.86
174	0.109	0.01	0.06	-0.04	-2.45	8.45	12.13
175	0.108	0.01	0.01	0.08	-0.65	-13.35	0.96
176	0.108	-0.04	0.04	0.04	-1.86	-8.57	7.42
177	0.107	0.03	0.00	0.01	-0.28	-0.83	1.14
178	0.107	-0.11	0.56	-0.13	-21.97	18.12	99.33
179	0.107	-0.05	0.03	0.02	-1.28	-6.03	6.30
180	0.106	0.56	0.54	0.06	-21.50	10.87	122.99
181	0.106	0.11	0.93	-0.35	-30.33	67.80	212.54
182	0.105	-0.24	0.63	0.11	-23.23	-24.30	129.91
183	0.105	-0.01	0.06	-0.01	-2.09	2.75	12.80
184	0.104	-0.17	0.15	-0.02	-5.76	-2.74	28.49
185	0.103	0.05	0.03	0.01	-1.33	0.27	9.42

STAAD SPACE

-- PAGE NO. 85

MODAL BASE ACTIONS                      FORCES IN KN      LENGTH IN METE

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MOMENTS ARE ABOUT THE ORIGIN

MODE	PERIOD	FX	FY	FZ	MX	MY	MZ
186	0.103	-0.02	0.01	0.00	-0.51	-0.09	1.50
187	0.102	-0.01	0.00	-0.01	0.02	1.25	-0.31
188	0.102	-0.25	0.24	0.13	-11.17	-36.45	43.53
189	0.102	0.01	0.03	0.01	-1.04	-1.59	4.65
190	0.102	-0.01	0.00	0.00	-0.08	-0.18	0.16
191	0.101	0.04	0.05	0.00	-2.36	1.59	12.40
192	0.101	-0.27	0.26	-0.04	-11.05	-3.51	50.66
193	0.100	-0.01	0.00	0.00	-0.01	-0.05	0.17
194	0.100	0.00	0.00	0.00	-0.08	0.18	0.36

195	0.099	0.00	0.00	0.00	0.00	0.00	0.00
196	0.099	0.00	0.00	0.00	0.00	0.00	0.00
197	0.099	-0.17	0.15	-0.08	-5.67	10.34	30.02
198	0.098	0.07	0.19	0.19	-8.39	-33.14	38.78
199	0.097	0.01	0.04	-0.03	-1.71	8.61	7.23
200	0.095	0.15	0.15	-0.02	-6.65	10.57	26.31

STAAD SPACE

-- PAGE NO. 86

MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
1	0.11	0.01	56.79	0.106	0.011	56.795	0.00	0.04	0.00
2	0.02	0.00	8.41	0.129	0.014	65.210	0.00	0.01	0.00
3	1.57	0.03	0.27	1.698	0.046	65.481	0.00	0.12	0.00
4	78.39	0.00	0.49	80.083	0.050	65.966	0.00	0.02	0.00
5	7.79	0.00	11.31	87.873	0.052	77.274	0.00	0.01	0.00
6	0.00	0.03	4.41	87.874	0.080	81.682	0.00	0.11	0.00
7	0.35	0.00	0.22	88.224	0.080	81.906	0.00	0.00	0.00
8	0.56	0.00	2.05	88.786	0.080	83.952	0.00	0.00	0.00
9	0.01	0.00	0.04	88.792	0.085	83.988	0.00	0.02	0.00
10	0.00	0.00	0.66	88.795	0.088	84.644	0.00	0.01	0.00
11	0.03	0.01	0.24	88.822	0.093	84.882	0.00	0.03	0.00
12	0.00	0.00	0.00	88.822	0.093	84.882	0.00	0.00	0.00
13	0.00	0.00	0.00	88.822	0.093	84.882	0.00	0.00	0.00
14	0.57	0.00	0.00	89.397	0.093	84.882	0.00	0.00	0.00
15	0.00	0.03	1.31	89.397	0.118	86.190	0.00	0.14	0.00
16	0.01	0.09	2.07	89.405	0.207	88.258	0.00	0.54	0.00
17	0.01	0.00	0.02	89.412	0.208	88.279	0.00	0.00	0.00
18	0.58	0.00	0.04	89.990	0.211	88.320	0.00	0.02	0.00
19	0.69	0.00	0.03	90.678	0.212	88.354	0.00	0.01	0.00
20	0.03	0.01	0.00	90.711	0.221	88.355	0.00	0.06	0.00
21	0.01	0.00	0.02	90.719	0.221	88.373	0.00	0.00	0.00
22	0.24	0.00	0.01	90.959	0.223	88.381	0.00	0.01	0.00



23	0.27	0.00	0.00	91.225	0.223	88.381	0.00	0.00	0.00
24	0.00	0.08	0.47	91.228	0.304	88.850	0.00	0.57	0.00
25	0.01	0.00	0.00	91.239	0.305	88.853	0.00	0.01	0.00
26	0.06	0.07	0.00	91.294	0.372	88.853	0.00	0.50	0.00
27	0.04	0.03	0.01	91.332	0.398	88.858	0.00	0.19	0.00
28	0.07	0.00	0.00	91.404	0.398	88.860	0.00	0.00	0.00
29	0.00	0.19	0.39	91.408	0.586	89.246	0.00	1.49	0.00
30	0.00	0.14	0.00	91.408	0.731	89.249	0.00	1.16	0.00
31	0.01	0.05	0.00	91.415	0.777	89.250	0.00	0.37	0.00
32	0.00	0.18	0.00	91.417	0.957	89.251	0.00	1.46	0.00
33	0.00	0.05	0.00	91.418	1.005	89.251	0.00	0.39	0.00
34	0.00	0.06	0.03	91.418	1.064	89.281	0.00	0.48	0.00
35	0.02	16.03	0.02	91.441	17.094	89.299	0.00	131.77	0.00
36	0.00	0.02	0.00	91.443	17.116	89.299	0.00	0.17	0.00
37	0.00	0.03	0.00	91.443	17.148	89.299	0.00	0.27	0.00
38	0.00	6.63	0.05	91.446	23.776	89.352	0.00	55.04	0.00
39	0.00	0.00	0.16	91.446	23.776	89.509	0.00	0.00	0.00
40	0.00	0.00	0.00	91.446	23.776	89.509	0.00	0.00	0.00
41	0.00	0.24	0.06	91.447	24.019	89.573	0.00	2.03	0.00
42	0.00	0.67	0.27	91.450	24.686	89.844	0.00	5.60	0.00
43	0.00	0.04	0.02	91.453	24.725	89.863	0.00	0.33	0.00
44	0.00	0.00	0.00	91.456	24.726	89.864	0.00	0.01	0.00
45	0.00	0.01	0.02	91.456	24.739	89.884	0.00	0.11	0.00
46	0.00	0.00	0.00	91.456	24.739	89.884	0.00	0.01	0.00
47	0.00	0.00	0.00	91.456	24.739	89.884	0.00	0.00	0.00

STAAD SPACE

-- PAGE NO. 87

MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
48	0.03	0.56	0.00	91.483	25.295	89.887	0.00	4.75	0.00
49	0.00	0.21	0.01	91.483	25.505	89.900	0.00	1.81	0.00
50	0.00	0.01	0.24	91.484	25.519	90.136	0.00	0.12	0.00
51	0.04	19.57	0.00	91.519	45.088	90.137	0.00	170.50	0.00

52	0.01	4.75	0.00	91.524	49.839	90.139	0.00	41.83	0.00
53	0.05	11.29	0.03	91.571	61.132	90.165	0.00	99.57	0.00
54	0.09	0.10	0.64	91.658	61.235	90.801	0.00	0.93	0.00
55	0.00	0.74	0.03	91.662	61.979	90.833	0.00	6.70	0.00
56	0.00	0.03	0.25	91.662	62.006	91.084	0.00	0.25	0.00
57	0.01	0.13	0.71	91.668	62.139	91.797	0.00	1.21	0.00
58	0.12	0.93	0.03	91.784	63.065	91.825	0.00	8.47	0.00
59	0.00	0.00	0.01	91.784	63.066	91.832	0.00	0.00	0.00
60	0.00	0.01	0.05	91.784	63.072	91.883	0.00	0.06	0.00
61	0.00	0.01	0.02	91.784	63.078	91.901	0.00	0.06	0.00
62	0.00	0.62	0.14	91.786	63.695	92.042	0.00	5.69	0.00
63	0.00	2.03	0.01	91.787	65.726	92.048	0.00	18.77	0.00
64	0.01	0.52	0.02	91.792	66.251	92.066	0.00	4.86	0.00
65	0.00	0.13	0.02	91.793	66.380	92.083	0.00	1.20	0.00
66	0.00	0.14	0.04	91.793	66.516	92.120	0.00	1.27	0.00
67	0.09	0.07	0.03	91.881	66.584	92.153	0.00	0.63	0.00
68	0.00	0.00	0.13	91.882	66.588	92.287	0.00	0.04	0.00
69	0.00	0.00	0.00	91.883	66.589	92.288	0.00	0.00	0.00
70	0.00	0.00	0.00	91.884	66.591	92.293	0.00	0.02	0.00
71	0.00	0.12	0.00	91.887	66.714	92.294	0.00	1.16	0.00
72	0.00	0.36	0.00	91.887	67.072	92.298	0.00	3.39	0.00
73	0.00	0.05	0.01	91.888	67.121	92.304	0.00	0.46	0.00
74	0.00	0.01	0.00	91.888	67.129	92.304	0.00	0.07	0.00
75	0.01	0.00	0.01	91.893	67.131	92.309	0.00	0.02	0.00
76	0.04	0.28	0.06	91.936	67.411	92.369	0.00	2.67	0.00
77	0.00	3.60	0.00	91.936	71.007	92.370	0.00	34.48	0.00
78	0.01	12.22	0.00	91.942	83.230	92.371	0.00	117.33	0.00
79	0.07	0.01	0.11	92.010	83.240	92.483	0.00	0.09	0.00
80	0.06	0.67	0.01	92.066	83.913	92.492	0.00	6.52	0.00
81	0.01	0.15	0.01	92.077	84.060	92.500	0.00	1.42	0.00
82	0.01	1.89	0.02	92.091	85.945	92.524	0.00	18.35	0.00
83	0.01	3.67	0.14	92.101	89.611	92.669	0.00	35.77	0.00
84	0.00	0.07	0.00	92.103	89.680	92.669	0.00	0.69	0.00
85	0.01	0.06	0.00	92.108	89.742	92.669	0.00	0.61	0.00
86	0.00	0.01	0.05	92.108	89.755	92.715	0.00	0.13	0.00
87	0.00	0.01	0.09	92.108	89.766	92.806	0.00	0.11	0.00
88	0.00	0.08	0.00	92.109	89.846	92.808	0.00	0.80	0.00
89	0.00	0.00	0.09	92.113	89.848	92.895	0.00	0.02	0.00

90	0.01	0.00	0.06	92.120	89.853	92.955	0.00	0.05	0.00
91	0.00	0.05	0.13	92.122	89.905	93.081	0.00	0.52	0.00
92	0.00	0.07	0.09	92.122	89.976	93.176	0.00	0.71	0.00
93	0.00	0.01	0.09	92.122	89.985	93.266	0.00	0.09	0.00

STAAD SPACE

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MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
94	0.00	0.02	0.11	92.123	90.006	93.373	0.00	0.21	0.00
95	0.00	0.31	0.05	92.123	90.318	93.418	0.00	3.15	0.00
96	0.00	0.03	0.00	92.124	90.349	93.419	0.00	0.31	0.00
97	0.01	2.17	0.03	92.130	92.515	93.448	0.00	21.90	0.00
98	0.00	0.06	0.02	92.130	92.578	93.463	0.00	0.64	0.00
99	0.00	0.14	0.07	92.131	92.721	93.532	0.00	1.45	0.00
100	0.00	0.16	0.02	92.134	92.884	93.554	0.00	1.65	0.00
101	0.03	0.30	0.07	92.164	93.188	93.623	0.00	3.10	0.00
102	0.00	0.35	0.03	92.167	93.541	93.657	0.00	3.60	0.00
103	0.02	0.03	0.06	92.182	93.569	93.717	0.00	0.29	0.00
104	0.01	1.12	0.14	92.192	94.688	93.854	0.00	11.50	0.00
105	0.00	0.55	0.01	92.193	95.239	93.866	0.00	5.68	0.00
106	0.00	0.10	0.01	92.197	95.342	93.873	0.00	1.06	0.00
107	0.01	0.01	0.05	92.205	95.350	93.924	0.00	0.09	0.00
108	0.00	0.15	0.01	92.205	95.503	93.937	0.00	1.58	0.00
109	0.03	0.61	0.00	92.231	96.110	93.938	0.00	6.29	0.00
110	0.07	0.08	0.06	92.302	96.187	94.001	0.00	0.80	0.00
111	0.00	0.08	0.01	92.304	96.267	94.010	0.00	0.83	0.00
112	0.01	0.21	0.00	92.314	96.477	94.010	0.00	2.19	0.00
113	0.00	0.00	0.18	92.315	96.478	94.191	0.00	0.01	0.00
114	0.00	0.02	0.01	92.315	96.500	94.205	0.00	0.22	0.00
115	0.00	0.03	0.00	92.316	96.526	94.208	0.00	0.27	0.00
116	0.01	0.00	0.00	92.330	96.530	94.211	0.00	0.05	0.00
117	0.10	0.01	0.01	92.428	96.540	94.219	0.00	0.10	0.00
118	0.00	0.00	0.00	92.428	96.544	94.220	0.00	0.04	0.00

119	0.04	0.03	0.00	92.466	96.576	94.223	0.00	0.34	0.00
120	0.00	0.01	0.00	92.466	96.585	94.223	0.00	0.09	0.00
121	0.01	0.07	0.00	92.477	96.659	94.224	0.00	0.78	0.00
122	0.00	0.16	0.00	92.479	96.817	94.226	0.00	1.66	0.00
123	0.00	0.01	0.01	92.481	96.824	94.235	0.00	0.08	0.00
124	0.00	0.52	0.00	92.483	97.348	94.236	0.00	5.53	0.00
125	0.06	0.00	0.00	92.548	97.351	94.238	0.00	0.03	0.00
126	0.01	0.02	0.00	92.554	97.369	94.240	0.00	0.18	0.00
127	0.00	0.01	0.22	92.555	97.381	94.461	0.00	0.13	0.00
128	0.00	0.00	0.02	92.560	97.382	94.482	0.00	0.01	0.00
129	0.01	0.00	0.00	92.572	97.385	94.482	0.00	0.03	0.00
130	0.01	0.31	0.05	92.578	97.692	94.534	0.00	3.28	0.00
131	0.01	0.01	0.05	92.583	97.699	94.579	0.00	0.07	0.00
132	0.00	0.02	0.01	92.584	97.714	94.587	0.00	0.17	0.00
133	0.00	0.00	0.04	92.584	97.715	94.625	0.00	0.01	0.00
134	0.00	0.03	0.15	92.585	97.747	94.772	0.00	0.34	0.00
135	0.01	0.01	0.00	92.590	97.761	94.774	0.00	0.15	0.00
136	0.00	0.03	0.00	92.594	97.787	94.778	0.00	0.28	0.00
137	0.01	0.00	0.00	92.602	97.787	94.782	0.00	0.00	0.00
138	0.00	0.02	0.06	92.606	97.809	94.842	0.00	0.24	0.00
139	0.01	0.03	0.02	92.615	97.836	94.859	0.00	0.29	0.00

STAAD SPACE

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MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KN

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
140	0.00	0.00	0.00	92.615	97.841	94.860	0.00	0.05	0.00
141	0.00	0.18	0.09	92.619	98.024	94.954	0.00	1.98	0.00
142	0.00	0.04	0.00	92.620	98.061	94.955	0.00	0.40	0.00
143	0.01	0.03	0.01	92.626	98.088	94.960	0.00	0.30	0.00
144	0.07	0.01	0.01	92.693	98.099	94.973	0.00	0.11	0.00
145	0.02	0.00	0.01	92.716	98.099	94.986	0.00	0.01	0.00
146	0.00	0.13	0.01	92.718	98.230	94.994	0.00	1.42	0.00
147	0.04	0.00	0.00	92.753	98.230	94.996	0.00	0.00	0.00

148	0.15	0.01	0.01	92.902	98.243	95.002	0.00	0.14	0.00
149	0.03	0.01	0.01	92.930	98.256	95.009	0.00	0.14	0.00
150	0.07	0.03	0.01	93.002	98.282	95.022	0.00	0.29	0.00
151	0.00	0.03	0.01	93.005	98.307	95.030	0.00	0.27	0.00
152	0.04	0.00	0.01	93.040	98.309	95.039	0.00	0.02	0.00
153	0.01	0.01	0.00	93.055	98.319	95.041	0.00	0.11	0.00
154	0.01	0.05	0.00	93.063	98.372	95.041	0.00	0.58	0.00
155	0.08	0.01	0.02	93.139	98.381	95.061	0.00	0.10	0.00
156	0.01	0.04	0.00	93.154	98.417	95.062	0.00	0.39	0.00
157	0.01	0.00	0.00	93.163	98.417	95.066	0.00	0.00	0.00
158	0.01	0.00	0.00	93.174	98.419	95.066	0.00	0.02	0.00
159	0.01	0.07	0.00	93.181	98.485	95.066	0.00	0.72	0.00
160	0.00	0.00	0.00	93.181	98.487	95.067	0.00	0.02	0.00
161	0.05	0.01	0.00	93.232	98.500	95.068	0.00	0.15	0.00
162	0.01	0.02	0.00	93.238	98.516	95.069	0.00	0.17	0.00
163	0.00	0.01	0.00	93.239	98.527	95.069	0.00	0.12	0.00
164	0.00	0.00	0.00	93.239	98.531	95.069	0.00	0.05	0.00
165	0.00	0.00	0.00	93.241	98.533	95.070	0.00	0.01	0.00
166	0.00	0.00	0.00	93.243	98.534	95.070	0.00	0.01	0.00
167	0.01	0.02	0.00	93.253	98.554	95.073	0.00	0.23	0.00
168	0.00	0.00	0.00	93.253	98.555	95.073	0.00	0.00	0.00
169	0.00	0.10	0.00	93.254	98.652	95.073	0.00	1.08	0.00
170	0.00	0.09	0.00	93.259	98.741	95.073	0.00	0.98	0.00
171	0.00	0.02	0.01	93.260	98.757	95.081	0.00	0.18	0.00
172	0.01	0.02	0.02	93.271	98.779	95.105	0.00	0.25	0.00
173	0.05	0.03	0.00	93.316	98.807	95.105	0.00	0.31	0.00
174	0.00	0.01	0.00	93.316	98.812	95.109	0.00	0.06	0.00
175	0.00	0.00	0.08	93.318	98.813	95.188	0.00	0.01	0.00
176	0.00	0.00	0.00	93.322	98.817	95.191	0.00	0.04	0.00
177	0.02	0.00	0.00	93.341	98.817	95.194	0.00	0.00	0.00
178	0.00	0.05	0.00	93.343	98.868	95.197	0.00	0.56	0.00
179	0.01	0.00	0.00	93.352	98.871	95.198	0.00	0.03	0.00
180	0.06	0.05	0.00	93.412	98.919	95.199	0.00	0.54	0.00
181	0.00	0.08	0.01	93.414	99.003	95.213	0.00	0.93	0.00
182	0.01	0.06	0.00	93.423	99.059	95.214	0.00	0.63	0.00
183	0.00	0.01	0.00	93.424	99.065	95.215	0.00	0.06	0.00
184	0.02	0.01	0.00	93.444	99.078	95.215	0.00	0.15	0.00
185	0.01	0.00	0.00	93.453	99.081	95.216	0.00	0.03	0.00

MASS PARTICIPATION FACTORS IN PERCENT							BASE SHEAR IN KN		
MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
186	0.00	0.00	0.00	93.456	99.082	95.216	0.00	0.01	0.00
187	0.00	0.00	0.01	93.461	99.082	95.225	0.00	0.00	0.00
188	0.03	0.02	0.01	93.488	99.103	95.232	0.00	0.24	0.00
189	0.00	0.00	0.00	93.489	99.106	95.233	0.00	0.03	0.00
190	0.01	0.00	0.00	93.497	99.106	95.233	0.00	0.00	0.00
191	0.00	0.00	0.00	93.500	99.111	95.233	0.00	0.05	0.00
192	0.03	0.02	0.00	93.530	99.134	95.234	0.00	0.26	0.00
193	0.01	0.00	0.00	93.537	99.134	95.234	0.00	0.00	0.00
194	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00
195	0.00	0.00	0.00	93.538	99.134	95.234	0.00	0.00	0.00
196	0.00	0.00	0.00	93.538	99.134	95.236	0.00	0.00	0.00
197	0.02	0.01	0.00	93.557	99.148	95.241	0.00	0.15	0.00
198	0.00	0.02	0.02	93.560	99.165	95.260	0.00	0.19	0.00
199	0.00	0.00	0.00	93.561	99.169	95.262	0.00	0.04	0.00
200	0.02	0.01	0.00	93.577	99.182	95.263	0.00	0.15	0.00
							-----		
TOTAL SRSS SHEAR							0.00	281.34	0.00
TOTAL 10PCT SHEAR							0.00	461.20	0.00
TOTAL ABS SHEAR							0.00	892.82	0.00

2083. LOAD LIST 30 TO 38

2084. START CONCRETE DESIGN

2085. CODE ACI

2086. CONCRETE TAKE

2087. FC 24525 MEMB 2137 TO 2164 2166 TO 2169 2171 2173 TO 2176 2178 TO 2181 2183 -

2088. 2185 TO 2201 2306 2307 2323 2324 2432 2433 2435 2470 TO 2472 2523 2524  
 2089. MINSEC 10 MEMB 2137 TO 2164 2166 TO 2169 2171 2173 TO 2176 2178 TO 2181 2183 -  
 2090. 2185 TO 2201 2306 2307 2323 2324 2432 2433 2435 2470 TO 2472 2523 2524  
 2091. TRACK 2 MEMB 2137 TO 2164 2166 TO 2169 2171 2173 TO 2176 2178 TO 2181 2183 -  
 2092. 2185 TO 2201 2306 2307 2323 2324 2432 2433 2435 2470 TO 2472 2523 2524  
 2093. DESIGN BEAM 2164 2166 TO 2169 2171 2173 TO 2176 2178 TO 2181 2183 -  
 2094. 2185 TO 2201 2323 2324 2432 2470 TO 2472 2524

STAAD SPACE

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BEAM NO. 2164 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
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1	54.	6 - 12MM	0.	8000.	YES YES
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|-----|
| CRITICAL POS MOMENT= 124.31 KN-MET AT 5333.MM, LOAD 34|
| REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |
| REQD. DEVELOPMENT LENGTH = 566. MMS |
|-----|
  
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Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

2	646.	6 - 12MM	0.	8000.	YES YES
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|-----|
| CRITICAL NEG MOMENT= 123.66 KN-MET AT 0.MM, LOAD 33|
| REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |
  
```

REQD. DEVELOPMENT LENGTH = 566. MMS

Cracked Moment of Inertia I<sub>z</sub> at above location = 175566.9 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

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SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	200./	536.	47./	124.	34/	33
667.	282./	448.	66./	104.	34/	33
1333.	353./	373.	82./	87.	34/	33
2000.	413./	310.	96./	72.	34/	33
2667.	461./	259.	107./	61.	34/	33
3333.	498./	219.	115./	51.	34/	33
4000.	523./	191.	121./	45.	34/	33
4667.	537./	174.	124./	41.	34/	33
5333.	539./	169.	124./	40.	34/	33
6000.	529./	175.	122./	41.	34/	33
6667.	507./	192.	117./	45.	34/	33
7333.	474./	221.	110./	52.	34/	33
8000.	429./	261.	100./	61.	33/	34

B E A M N O. 2164 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

-- PAGE NO. 92

AT START SUPPORT - V<sub>u</sub>= 47.84 KNS V<sub>c</sub>= 154.68 KNS V<sub>s</sub>= 0.00 KNS  
 T<sub>u</sub>= 12.29 KN-MET T<sub>c</sub>= 6.8 KN-MET T<sub>s</sub>= 16.4 KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

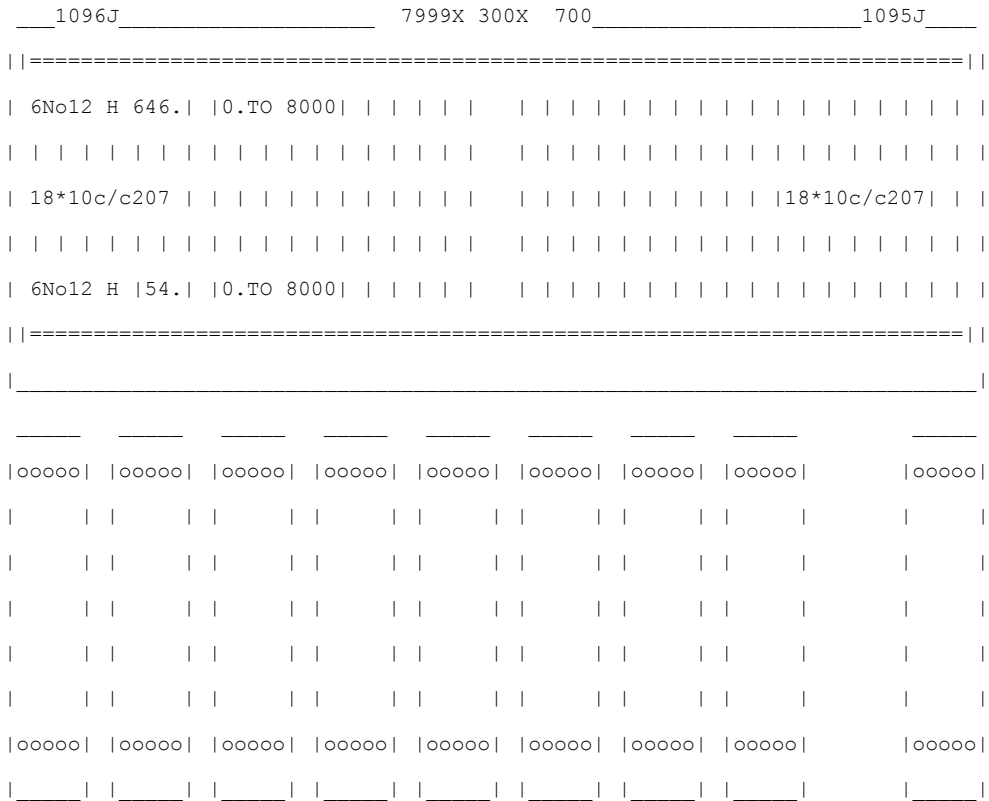
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 2.94 SQ.CM.



AT END SUPPORT - Vu= 31.92 KNS Vc= 153.61 KNS Vs= 0.00 KNS  
 Tu= 12.29 KN-MET Tc= 6.8 KN-MET Ts= 16.4 KN-MET LOAD 34  
 STIRRUPS ARE REQUIRED FOR TORSION.  
 REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.  
 PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM  
 ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 2.94 SQ.CM.



=====

BEAM NO. 2166 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5250. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT	BAR INFO	FROM	TO	ANCHOR
	(MM)		(MM)	(MM)	STA END

1 56. 3 - 16MM 0. 4833. YES NO

-----  
| CRITICAL POS MOMENT= 111.84 KN-MET AT 0.MM, LOAD 34 |  
| REQD STEEL= 572.MM2, ROW=0.0035, ROWMX=0.0190 ROWMN=0.0033 |  
| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 94. MMS |  
REQD. DEVELOPMENT LENGTH = 483. MMS

Cracked Moment of Inertia Iz at above location = 109760.7 cm<sup>4</sup>

2 546. 5 - 12MM 0. 5250. YES YES

STAAD SPACE

-- PAGE NO. 93

-----  
| CRITICAL NEG MOMENT= 107.86 KN-MET AT 5250.MM, LOAD 32 |  
| REQD STEEL= 549.MM2, ROW=0.0034, ROWMX=0.0190 ROWMN=0.0033 |  
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 48. MMS |  
REQD. DEVELOPMENT LENGTH = 453. MMS

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION	REINF STEEL (+VE/-VE)		MOMENTS (+VE/-VE)		LOAD (+VE/-VE)	
( MM )	(SQ.	MM )	(KNS-MET )			
0.	579./	340.	112./	67.	34/	33
438.	547./	330.	106./	65.	34/	33
875.	509./	325.	99./	64.	34/	33
1313.	466./	324.	91./	64.	34/	33
1750.	419./	329.	82./	65.	34/	33
2188.	366./	339.	72./	66.	34/	33
2625.	309./	354.	61./	69.	34/	33

3063.	247./	374.	49./	73.	34/	33
3500.	181./	400.	36./	78.	34/	33
3938.	111./	430.	22./	84.	34/	33
4375.	36./	467.	7./	91.	34/	33
4813.	0./	508.	0./	99.	0/	33
5250.	0./	558.	0./	108.	0/	32

B E A M N O . 2166 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 17.99 KNS Vc= 128.98 KNS Vs= 0.00 KNS  
 Tu= 15.23 KN-MET Tc= 5.6 KN-MET Ts= 20.3 KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2083. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.83 SQ.CM.

AT END SUPPORT - Vu= 51.84 KNS Vc= 137.35 KNS Vs= 0.00 KNS  
 Tu= 15.23 KN-MET Tc= 5.6 KN-MET Ts= 20.3 KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2083. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.83 SQ.CM.

STAAD SPACE

-- PAGE NO. 94

1099J	5250X 300X 600	1101J
=====		
5No12 H 546.	0.TO 5250	
13*10c/c182		13*10c/c182
3No16 H 56.	0.TO 4833	
=====		

oooo	oooo	oooo	oooo	oooo	oooo	oooo	oooo	oooo	
5#12	5#12	5#12	5#12	5#12	5#12	5#12	5#12	5#12	
3#16	3#16	3#16	3#16	3#16	3#16	3#16	3#16		
ooo	ooo	ooo	ooo	ooo	ooo	ooo	ooo		

=====

BEAM NO. 2167 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5250. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	61.	4 - 25MM	0.	5250.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 330.97 KN-MET AT 0.MM, LOAD 34|
| REQD STEEL= 1860.MM2, ROW=0.0115, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 60. MMS |
| REQD. DEVELOPMENT LENGTH = 1582. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 272812.6 cm^4

2	539.	4 - 25MM	0.	5250.	YES YES
---	------	----------	----	-------	---------

```

|-----|
| CRITICAL NEG MOMENT= 330.47 KN-MET AT 0.MM, LOAD 33|
| REQD STEEL= 1857.MM2, ROW=0.0115, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 60. MMS |

```

| REQD. DEVELOPMENT LENGTH = 1582. MMS |

|-----|

Cracked Moment of Inertia I<sub>z</sub> at above location = 272812.6 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL(+VE/-VE) (SQ. MM )		MOMENTS(+VE/-VE) (KNS-MET )		LOAD(+VE/-VE)	
0.	1868./	1865.	331./	330.	34/	33
STAAD SPACE					-- PAGE NO.	95
437.	1834./	1772.	326./	316.	34/	33
875.	1794./	1687.	319./	303.	34/	33
1312.	1748./	1609.	312./	290.	34/	33
1750.	1696./	1538.	304./	279.	34/	33
2187.	1638./	1473.	295./	268.	34/	33
2625.	1575./	1416.	285./	259.	34/	33
3062.	1507./	1364.	274./	250.	34/	33
3500.	1434./	1319.	262./	243.	34/	33
3937.	1356./	1281.	249./	236.	34/	33
4375.	1273./	1248.	235./	231.	34/	33
4812.	1185./	1221.	220./	226.	34/	33
5250.	1093./	1200.	204./	223.	33/	34

B E A M N O. 2167 D E S I G N R E S U L T S - S H E A R

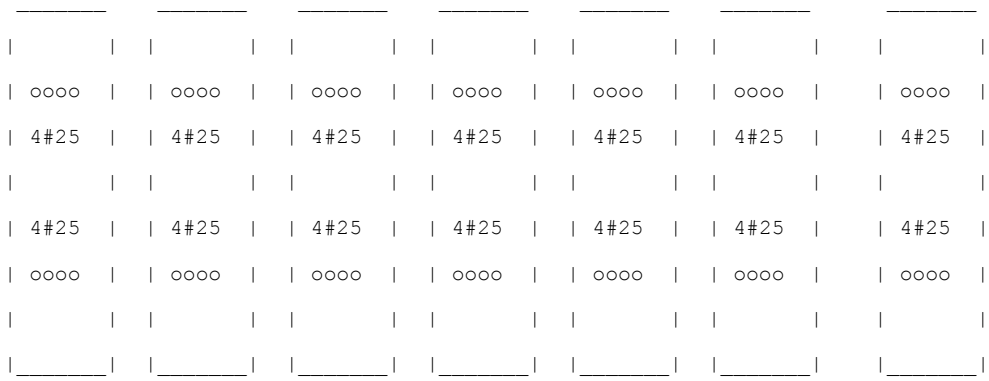
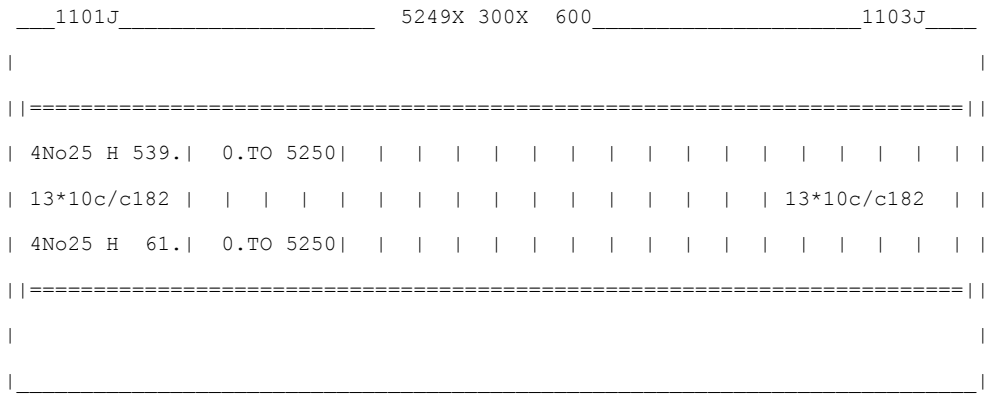
AT START SUPPORT - V<sub>u</sub>= 112.29 KNS V<sub>c</sub>= 132.64 KNS V<sub>s</sub>= 17.08 KNS  
T<sub>u</sub>= 8.87 KN-MET T<sub>c</sub>= 5.6 KN-MET T<sub>s</sub>= 11.8 KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR SHEAR AND TORSION.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2083. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 2.23 SQ.CM.

AT END SUPPORT - Vu= 115.90 KNS Vc= 135.40 KNS Vs= 19.13 KNS  
 Tu= 8.87 KN-MET Tc= 5.6 KN-MET Ts= 11.8 KN-MET LOAD 34  
 STIRRUPS ARE REQUIRED FOR SHEAR AND TORSION.  
 PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2083. MM  
 ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 2.23 SQ.CM.



STAAD SPACE

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=====

BEAM NO. 2168 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 1500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT	BAR INFO	FROM	TO	ANCHOR
	(MM)		(MM)	(MM)	STA END

```

1          54.      5 - 12MM      0.          1500.      YES  YES
|-----|
| CRITICAL POS MOMENT=      81.21 KN-MET AT      625.MM, LOAD  34|
| REQD STEEL=      546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING=  261./  37./  48. MMS      |
| REQD. DEVELOPMENT LENGTH =  453. MMS      |
|-----|

```

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

```

2          546.     5 - 12MM      0.          1500.      YES  YES
|-----|
| CRITICAL NEG MOMENT=      94.85 KN-MET AT      0.MM, LOAD  33|
| REQD STEEL=      546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING=  261./  37./  48. MMS      |
| REQD. DEVELOPMENT LENGTH =  453. MMS      |
|-----|

```

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION      REINF STEEL (+VE/-VE)      MOMENTS (+VE/-VE)      LOAD (+VE/-VE)
( MM )      (SQ. MM )      (KNS-MET )

```

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	411./ 489.	80./ 95.	34/ 33
125.	413./ 452.	81./ 88.	34/ 33
250.	414./ 415.	81./ 81.	34/ 33
375.	415./ 380.	81./ 74.	34/ 33
500.	416./ 345.	81./ 68.	34/ 33
625.	416./ 310.	81./ 61.	34/ 33
750.	416./ 276.	81./ 54.	34/ 33
875.	416./ 243.	81./ 48.	34/ 33
1000.	415./ 211.	81./ 42.	34/ 33

1125.	414./	179.	81./	35.	34/	33
1250.	413./	147.	81./	29.	34/	33
1375.	411./	117.	80./	23.	34/	33
1500.	410./	87.	80./	17.	33/	34

B E A M N O . 2168 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

-- PAGE NO. 97

AT START SUPPORT - Vu= 55.24 KNS Vc= 132.30 KNS Vs= 0.00 KNS  
 Tu= 4.17 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 33

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

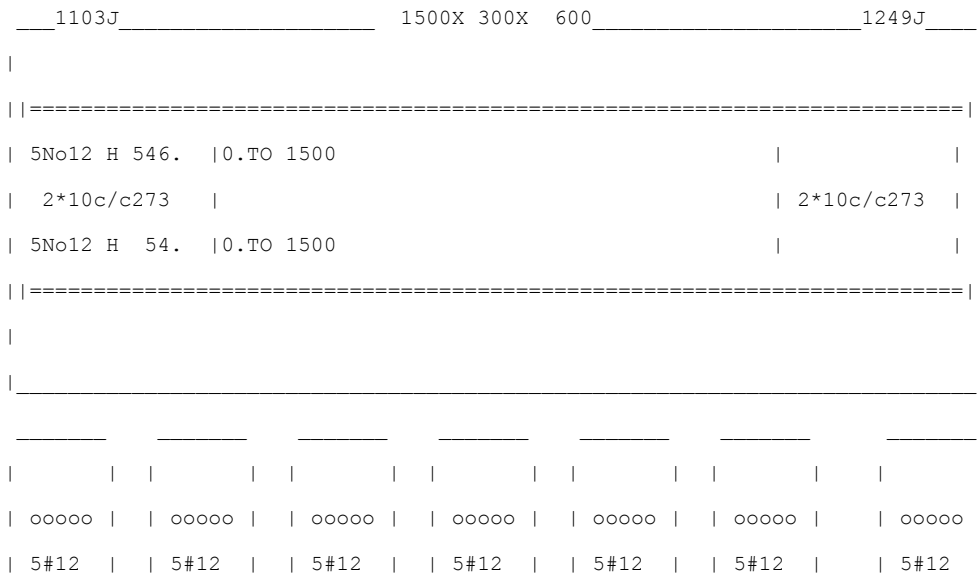
PROVIDE 10 MM 2-LEGGED STIRRUPS AT 273. MM C/C FOR 208. MM

AT END SUPPORT - Vu= 53.13 KNS Vc= 133.65 KNS Vs= 0.00 KNS  
 Tu= 0.82 KN-MET Tc= 5.5 KN-MET Ts= 0.0 KN-MET LOAD 33

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 273. MM C/C FOR 208. MM





```

|      | |      | |      | |      | |      | |      | |      |
| 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 |
| ooooo | | ooooo | | ooooo | | ooooo | | ooooo | | ooooo | | ooooo |
|      | |      | |      | |      | |      | |      | |      |
|_____ | |_____ | |_____ | |_____ | |_____ | |_____ | |_____ |

```

=====

BEAM NO. 2169 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 2700. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	54.	5 - 12MM	0.	2700.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 87.92 KN-MET AT 2700.MM, LOAD 33|
| REQD STEEL= 546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 48. MMS |
| REQD. DEVELOPMENT LENGTH = 453. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

2	542.	4 - 20MM	0.	2700.	YES YES
---	------	----------	----	-------	---------

STAAD SPACE -- PAGE NO. 98

```

|-----|
| CRITICAL NEG MOMENT= 212.45 KN-MET AT 0.MM, LOAD 32|
| REQD STEEL= 1131.MM2, ROW=0.0070, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |
| REQD. DEVELOPMENT LENGTH = 985. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 197182.2 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION      REINF STEEL (+VE/-VE)      MOMENTS (+VE/-VE)      LOAD (+VE/-VE)
( MM )              (SQ. MM )              (KNS-MET )

```

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	124./ 1142.	25./ 212.	34/ 32
225.	156./ 1018.	31./ 191.	34/ 33
450.	188./ 935.	37./ 176.	34/ 33
675.	218./ 855.	43./ 162.	34/ 33
900.	247./ 778.	49./ 148.	34/ 33
1125.	275./ 703.	54./ 135.	34/ 33
1350.	303./ 630.	59./ 121.	34/ 33
1575.	329./ 560.	65./ 108.	34/ 33
1800.	355./ 493.	69./ 96.	34/ 33
2025.	380./ 428.	74./ 83.	34/ 33
2250.	404./ 366.	79./ 72.	34/ 33
2475.	428./ 306.	83./ 60.	34/ 33
2700.	452./ 249.	88./ 49.	33/ 34

B E A M N O. 2169 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 110.26 KNS Tu= 61.9 KN-MET

Vc= 131.7 KNS, ACI 318:CLAUSE 11.6.3.1

LOAD 32 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.

AT END SUPPORT - Vu= 102.03 KNS Tu= 61.9 KN-MET

Vc= 137.4 KNS, ACI 318:CLAUSE 11.6.3.1

LOAD 32 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.

\_\_\_\_\_1105J\_\_\_\_\_ 2700X 300X 600\_\_\_\_\_1107J\_\_\_\_\_

|  
|=====|

4No20 H 542. 0.TO 2700									
5No12 H 54. 0.TO 2700									
=====									
_____									
_____									
0000	0000	0000	0000	0000	0000	0000	0000	0000	
4#20	4#20	4#20	4#20	4#20	4#20	4#20	4#20	4#20	
5#12	5#12	5#12	5#12	5#12	5#12	5#12	5#12	5#12	
00000	00000	00000	00000	00000	00000	00000	00000	00000	
_____	_____	_____	_____	_____	_____	_____	_____	_____	

STAAD SPACE

-- PAGE NO. 99

=====

BEAM NO. 2171 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8200. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT	BAR INFO	FROM	TO	ANCHOR
	(MM)		(MM)	(MM)	STA END

1	54.	6 - 12MM	0.	8200.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 98.73 KN-MET AT 4100.MM, LOAD 34 |
| REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING=- 261./ 37./ 38. MMS |
| REQD. DEVELOPMENT LENGTH = 566. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

2            646.            6 - 12MM            0.            8200.            YES YES

```

|-----|
| CRITICAL NEG MOMENT=    70.15 KN-MET AT 8200.MM, LOAD 34|
| REQD STEEL=    646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING=    261./    37./    38. MMS    |
| REQD. DEVELOPMENT LENGTH =    566. MMS                    |
|-----|

```

Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION            REINF STEEL (+VE/-VE)            MOMENTS (+VE/-VE)            LOAD (+VE/-VE)
( MM )            (SQ. MM )                            (KNS-MET )

```

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	208./ 283.	49./ 66.	34/ 33
683.	274./ 219.	64./ 51.	34/ 33
1367.	328./ 166.	77./ 39.	34/ 33
2050.	371./ 126.	86./ 30.	34/ 33
2733.	401./ 98.	93./ 23.	34/ 33
3417.	419./ 82.	97./ 19.	34/ 33
4100.	425./ 77.	99./ 18.	34/ 33
4783.	419./ 85.	97./ 20.	34/ 33
5467.	401./ 104.	93./ 24.	34/ 33
6150.	370./ 135.	86./ 32.	34/ 33
6833.	328./ 178.	77./ 42.	34/ 33
7517.	274./ 233.	64./ 55.	34/ 33
8200.	208./ 300.	49./ 70.	33/ 34



=====

BEAM NO. 2173 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	58.	4 - 20MM	0.	5500.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 216.47 KN-MET AT 5500.MM, LOAD 33 |
| REQD STEEL= 1154.MM2, ROW=0.0071, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |
| REQD. DEVELOPMENT LENGTH = 985. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 197182.2 cm^4

2	539.	4 - 25MM	0.	5500.	YES YES
---	------	----------	----	-------	---------

STAAD SPACE

-- PAGE NO. 101

```

|-----|
| CRITICAL NEG MOMENT= 315.75 KN-MET AT 5500.MM, LOAD 34 |
| REQD STEEL= 1763.MM2, ROW=0.0109, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 60. MMS |
| REQD. DEVELOPMENT LENGTH = 1582. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 272812.6 cm^4

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
-------------------	------------------------------------	---------------------------------	----------------

0.	570./	409.	110./	80.	34/	33
458.	650./	482.	125./	94.	34/	33
917.	724./	561.	139./	108.	34/	33
1375.	794./	647.	151./	124.	34/	33
1833.	858./	740.	163./	141.	34/	33
2292.	917./	840.	173./	159.	34/	33
2750.	970./	947.	183./	179.	34/	33
3208.	1017./	1062.	191./	199.	34/	33
3667.	1059./	1186.	198./	220.	34/	33
4125.	1094./	1317.	204./	242.	34/	33
4583.	1124./	1458.	209./	266.	34/	33
5042.	1148./	1609.	213./	290.	34/	33
5500.	1165./	1770.	216./	316.	33/	34

B E A M N O. 2173 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT -  $V_u = 65.09$  KNS  $V_c = 134.30$  KNS  $V_s = 0.00$  KNS  
 $T_u = 16.86$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 22.5$  KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2208. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 4.24 SQ.CM.

AT END SUPPORT -  $V_u = 88.66$  KNS  $V_c = 131.51$  KNS  $V_s = 0.00$  KNS  
 $T_u = 16.86$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 22.5$  KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2208. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 4.24 SQ.CM.

STAAD SPACE

-- PAGE NO. 102

```

|
|=====|
| 4No25 H 539.| 0.TO 5500| | | | | | | | | | | | | | | | | |
| 14*10c/c182 | | | | | | | | | | | | | | | | | 14*10c/c182 | |
| 4No20 H 58.| 0.TO 5500| | | | | | | | | | | | | | | | |
|=====|
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|_____|

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| | | | | | | | | | | | | |
| oooo | | oooo | | oooo | | oooo | | oooo | | oooo | |
| 4#25 | | 4#25 | | 4#25 | | 4#25 | | 4#25 | | 4#25 | |
| | | | | | | | | | | | |
| 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 | |
| oooo | | oooo | | oooo | | oooo | | oooo | | oooo | |
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BEAM NO. 2174 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
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1	54.	5 - 12MM	0.	5500.	YES YES
---	-----	----------	----	-------	---------

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|-----|
| CRITICAL POS MOMENT= 54.61 KN-MET AT 5500.MM, LOAD 33|
| REQD STEEL= 546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 48. MMS |
| REQD. DEVELOPMENT LENGTH = 453. MMS |
|-----|

```





AT START SUPPORT - Vu= 36.59 KNS Vc= 130.47 KNS Vs= 0.00 KNS  
 Tu= 12.83 KN-MET Tc= 5.6 KN-MET Ts= 17.1 KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2208. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.22 SQ.CM.

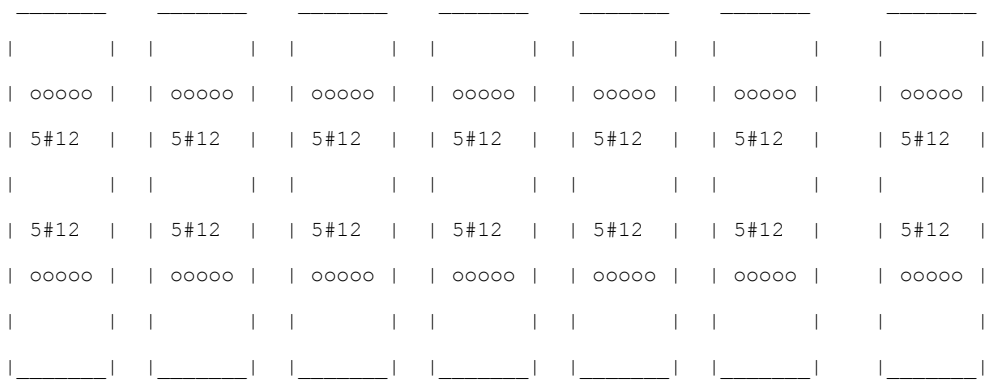
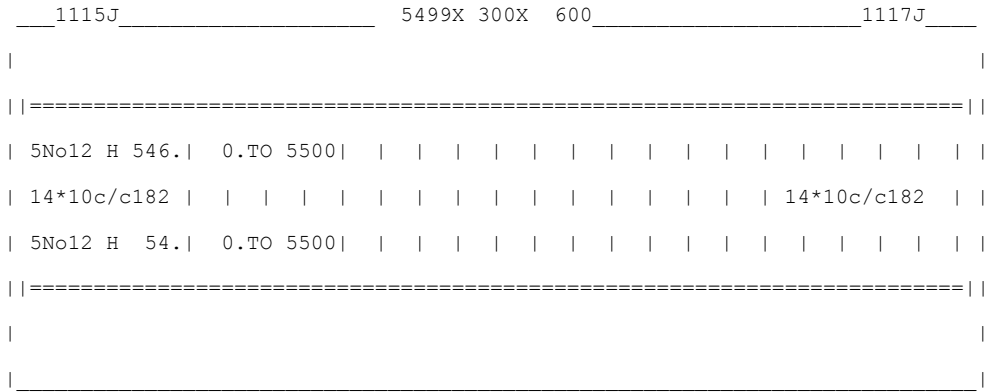
AT END SUPPORT - Vu= 12.02 KNS Vc= 129.10 KNS Vs= 0.00 KNS  
 Tu= 12.83 KN-MET Tc= 5.6 KN-MET Ts= 17.1 KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2208. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.22 SQ.CM.



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BEAM NO. 2175 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 6500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR	
					STA	END

1	58.	4 - 20MM	0.	6500.	YES	YES
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|-----|

| CRITICAL POS MOMENT= 224.19 KN-MET AT 0.MM, LOAD 34 |

| REQD STEEL= 1199.MM2, ROW=0.0074, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |

| REQD. DEVELOPMENT LENGTH = 985. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 197182.2 cm<sup>4</sup>

2	542.	3 - 20MM	0.	6500.	YES	YES
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|-----|

| CRITICAL NEG MOMENT= 168.51 KN-MET AT 0.MM, LOAD 33 |

| REQD STEEL= 883.MM2, ROW=0.0054, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 92. MMS |

| REQD. DEVELOPMENT LENGTH = 657. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 157123.0 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
-------------------	------------------------------------	---------------------------------	----------------

0.	1210./	891.	224./	169.	34/	33
542.	1173./	813.	218./	155.	34/	33
1083.	1127./	744.	210./	142.	34/	33
1625.	1073./	684.	201./	131.	34/	33
2167.	1010./	632.	190./	122.	34/	33
2708.	940./	589.	177./	114.	34/	33
3250.	862./	554.	163./	107.	34/	33
3792.	777./	527.	148./	102.	34/	33
4333.	684./	508.	131./	99.	34/	33
4875.	585./	498.	113./	97.	34/	33
5417.	479./	495.	93./	96.	34/	33
5958.	367./	500.	72./	97.	34/	33
6500.	249./	514.	49./	100.	33/	34

B E A M N O . 2 1 7 5 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

-- PAGE NO. 105

AT START SUPPORT -  $V_u = 47.22$  KNS  $V_c = 130.44$  KNS  $V_s = 0.00$  KNS

$T_u = 6.32$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 8.4$  KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2708. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.59 SQ.CM.

AT END SUPPORT -  $V_u = 63.57$  KNS  $V_c = 136.93$  KNS  $V_s = 0.00$  KNS

$T_u = 6.32$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 8.4$  KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2708. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.59 SQ.CM.

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|=====|
| 3No20 H 542.| 0.TO 6500| | | | | | | | | | | | | | | | | | | | | |
| 16*10c/c182 | | | | | | | | | | | | | | | | | 16*10c/c182 | | |
| 4No20 H 58.| 0.TO 6500| | | | | | | | | | | | | | | | | | | | | |
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| 000 | | 000 | | 000 | | 000 | | 000 | | 000 | | 000 |
| 3#20 | | 3#20 | | 3#20 | | 3#20 | | 3#20 | | 3#20 | | 3#20 |
| | | | | | | | | | | | | | | | | | | | | |
| 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 |
| 0000 | | 0000 | | 0000 | | 0000 | | 0000 | | 0000 | | 0000 |
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BEAM NO. 2176 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
1	54.	6 - 12MM	0.	8000.	YES YES

```

|-----|
| CRITICAL POS MOMENT= 144.63 KN-MET AT 5333.MM, LOAD 34 |
| REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |
| REQD. DEVELOPMENT LENGTH = 566. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

2            646.          6 - 12MM            0.            8000.            YES YES

STAAD SPACE

-- PAGE NO. 106

```
|-----|
| CRITICAL NEG MOMENT= 146.16 KN-MET AT 0.MM, LOAD 33|
| REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |
| REQD. DEVELOPMENT LENGTH = 566. MMS |
|-----|
```

Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

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```

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	286./	637.	67./	146.	34/	33
667.	369./	547.	86./	126.	34/	33
1333.	441./	470.	102./	109.	34/	33
2000.	501./	404.	116./	94.	34/	33
2667.	550./	351.	127./	82.	34/	33
3333.	588./	310.	135./	72.	34/	33
4000.	614./	280.	141./	66.	34/	33
4667.	628./	262.	144./	61.	34/	33
5333.	630./	255.	145./	60.	34/	33
6000.	620./	260.	142./	61.	34/	33
6667.	599./	276.	138./	65.	34/	33
7333.	565./	304.	130./	71.	34/	33
8000.	520./	343.	120./	80.	33/	34



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BEAM NO. 2178 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5250. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR	
					STA	END

1	54.	6 - 12MM	0.	5250.	YES	YES
---	-----	----------	----	-------	-----	-----

|-----|

| CRITICAL POS MOMENT= 127.54 KN-MET AT 0.MM, LOAD 34 |

| REQD STEEL= 653.MM2, ROW=0.0040, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |

| REQD. DEVELOPMENT LENGTH = 566. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 122044.6 cm<sup>4</sup>

2	544.	4 - 16MM	0.	5250.	YES	YES
---	------	----------	----	-------	-----	-----

|-----|

| CRITICAL NEG MOMENT= 141.59 KN-MET AT 0.MM, LOAD 33 |

| REQD STEEL= 732.MM2, ROW=0.0045, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |

| REQD. DEVELOPMENT LENGTH = 617. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 139329.5 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
-------------------	------------------------------------	---------------------------------	----------------



0.	664./	741.	128./	142.	34/	33
STAAD SPACE					-- PAGE NO.	108
438.	654./	708.	126./	136.	34/	33
875.	638./	680.	123./	130.	34/	33
1313.	617./	658.	119./	126.	34/	33
1750.	592./	641.	114./	123.	34/	33
2188.	561./	630.	108./	121.	34/	33
2625.	525./	625.	102./	120.	34/	33
3063.	485./	625.	94./	120.	34/	33
3500.	440./	630.	86./	121.	34/	33
3938.	391./	641.	76./	123.	34/	33
4375.	337./	658.	66./	126.	34/	33
4813.	279./	681.	55./	131.	34/	33
5250.	217./	710.	43./	136.	33/	34

B E A M N O. 2178 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT -  $V_u = 45.28$  KNS  $V_c = 130.03$  KNS  $V_s = 0.00$  KNS  
 $T_u = 15.17$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 20.2$  KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2083. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.81 SQ.CM.

AT END SUPPORT -  $V_u = 60.32$  KNS  $V_c = 134.40$  KNS  $V_s = 0.00$  KNS  
 $T_u = 15.17$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 20.2$  KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2083. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.81 SQ.CM.

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|=====|
| 4No16 H 544. | 0.TO 5250| | | | | | | | | | | | | | | | | | | | |
| 13*10c/c182 | | | | | | | | | | | | | | | | | 13*10c/c182 | |
| 6No12 H 54. | 0.TO 5250| | | | | | | | | | | | | | | | | | | |
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| oooo | | oooo | | oooo | | oooo | | oooo | | oooo | |
| 4#16 | | 4#16 | | 4#16 | | 4#16 | | 4#16 | | 4#16 | |
| | | | | | | | | | | | | | | | | | | |
| 6#12 | | 6#12 | | 6#12 | | 6#12 | | 6#12 | | 6#12 | |
|oooooo | |oooooo | |oooooo | |oooooo | |oooooo | |oooooo |
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BEAM NO. 2179 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5250. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
1	61.	3 - 25MM	0.	5250.	YES YES

```

|-----|
| CRITICAL POS MOMENT= 265.30 KN-MET AT 0.MM, LOAD 34|
| REQD STEEL= 1450.MM2, ROW=0.0090, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 89. MMS |

```

| REQD. DEVELOPMENT LENGTH = 1055. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 220403.6 cm<sup>4</sup>

2            536.            3 - 32MM            0.            5250.            YES YES

|-----|

| CRITICAL NEG MOMENT= 385.00 KN-MET AT 0.MM, LOAD 33 |

| REQD STEEL= 2239.MM2, ROW=0.0139, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 64./ 86. MMS |

| REQD. DEVELOPMENT LENGTH = 1799. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 311255.6 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	1456./	2229.	265./	385.	34/	33
437.	1435./	2033.	262./	356.	34/	33
875.	1409./	1849.	258./	328.	34/	33
1312.	1377./	1676.	252./	301.	34/	33
1750.	1339./	1514.	246./	275.	34/	33
2187.	1295./	1361.	239./	250.	34/	33
2625.	1246./	1218.	230./	225.	34/	33
3062.	1192./	1083.	221./	202.	34/	33
3500.	1133./	956.	211./	180.	34/	33
3937.	1068./	837.	200./	159.	34/	33
4375.	999./	725.	188./	139.	34/	33
4812.	924./	620.	175./	119.	34/	33
5250.	845./	522.	160./	101.	33/	34

AT START SUPPORT - Vu= 114.42 KNS Vc= 130.85 KNS Vs= 21.72 KNS  
 Tu= 4.29 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 33  
 NO STIRRUPS ARE REQUIRED FOR TORSION.  
 REINFORCEMENT IS REQUIRED FOR SHEAR.  
 PROVIDE 10 MM 2-LEGGED STIRRUPS AT 270. MM C/C FOR 2083. MM

AT END SUPPORT - Vu= 93.22 KNS Vc= 136.56 KNS Vs= 0.00 KNS  
 Tu= 2.45 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 33  
 NO STIRRUPS ARE REQUIRED FOR TORSION.  
 REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.  
 PROVIDE 10 MM 2-LEGGED STIRRUPS AT 270. MM C/C FOR 2083. MM

1102J		5249X 300X 600				1104J	
=====							
3No32 H 536.	0.TO 5250						
9*10c/c270						9*10c/c270	
3No25 H 61.	0.TO 5250						
=====							
000	000	000	000	000	000	000	000
3#32	3#32	3#32	3#32	3#32	3#32	3#32	3#32
3#25	3#25	3#25	3#25	3#25	3#25	3#25	3#25
ooo	ooo	ooo	ooo	ooo	ooo	ooo	ooo

=====

BEAM NO. 2180 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 6500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	56.	4 - 16MM	0.	6500.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 134.82 KN-MET AT 6500.MM, LOAD 32 |
| REQD STEEL= 695.MM2, ROW=0.0043, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |
| REQD. DEVELOPMENT LENGTH = 617. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 139329.5 cm^4

2	546.	5 - 12MM	0.	6500.	YES YES
---	------	----------	----	-------	---------

STAAD SPACE

-- PAGE NO. 111

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|-----|
| CRITICAL NEG MOMENT= 60.51 KN-MET AT 6500.MM, LOAD 34 |
| REQD STEEL= 546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 48. MMS |
| REQD. DEVELOPMENT LENGTH = 453. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 104738.6 cm^4

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
-------------------	------------------------------------	---------------------------------	----------------

0.	260./	96.	51./	19.	34/	33
542.	313./	70.	61./	14.	34/	33
1083.	359./	52.	70./	10.	34/	33
1625.	398./	42.	78./	8.	34/	33
2167.	430./	40.	84./	8.	34/	33
2708.	454./	46.	88./	9.	34/	33
3250.	504./	59.	98./	12.	32/	33
3792.	557./	81.	108./	16.	32/	33
4333.	602./	110.	116./	22.	32/	33
4875.	640./	148.	123./	29.	32/	33
5417.	669./	193.	128./	38.	32/	33
5958.	691./	247.	132./	49.	32/	33
6500.	704./	308.	135./	61.	32/	34

B E A M N O . 2 1 8 0 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 13.06 KNS Vc= 131.07 KNS Vs= 0.00 KNS  
 Tu= 0.19 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 30

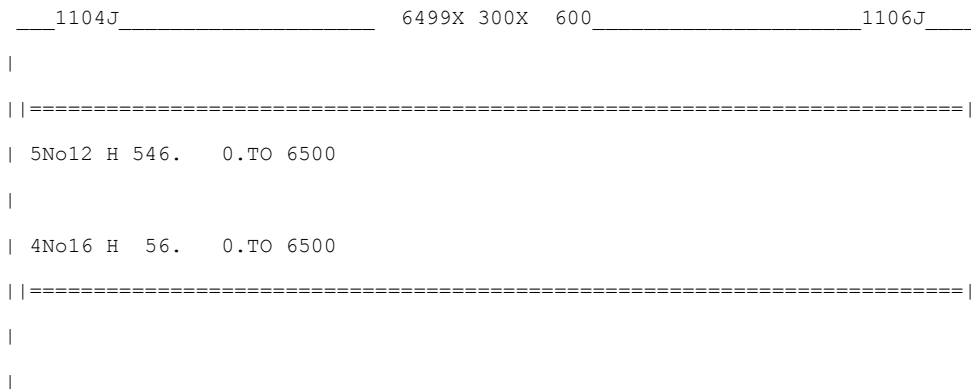
STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 19.10 KNS Vc= 139.47 KNS Vs= 0.00 KNS  
 Tu= 0.19 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 30

STIRRUPS ARE NOT REQUIRED.

STAAD SPACE

-- PAGE NO. 112



oooo	oooo	oooo	oooo	oooo	oooo	oooo	oooo	oooo
5#12	5#12	5#12	5#12	5#12	5#12	5#12	5#12	5#12
4#16	4#16	4#16	4#16	4#16	4#16	4#16	4#16	4#16
oooo	oooo	oooo	oooo	oooo	oooo	oooo	oooo	oooo

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BEAM NO. 2181 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 2700. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	58.	4 - 20MM	0.	2700.	YES YES
---	-----	----------	----	-------	---------

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|-----|
| CRITICAL POS MOMENT= 224.32 KN-MET AT 0.MM, LOAD 32|
| REQD STEEL= 1200.MM2, ROW=0.0074, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |
| REQD. DEVELOPMENT LENGTH = 985. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 197182.2 cm^4

2	544.	4 - 16MM	0.	2700.	YES YES
---	------	----------	----	-------	---------

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|-----|
| CRITICAL NEG MOMENT= 132.43 KN-MET AT 2700.MM, LOAD 32|
| REQD STEEL= 682.MM2, ROW=0.0042, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |

```

| REQD. DEVELOPMENT LENGTH = 617. MMS |

|-----|

Cracked Moment of Inertia I<sub>z</sub> at above location = 139329.5 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	1211./ 317.	224./ 62.	32/ 33
STAAD SPACE			-- PAGE NO. 113
225.	1047./ 324.	196./ 64.	32/ 33
450.	885./ 333.	167./ 65.	32/ 33
675.	725./ 343.	139./ 67.	32/ 33
900.	567./ 355.	110./ 70.	32/ 33
1125.	457./ 370.	89./ 72.	34/ 33
1350.	414./ 386.	81./ 75.	34/ 33
1575.	371./ 404.	73./ 79.	34/ 33
1800.	328./ 424.	64./ 83.	34/ 33
2025.	284./ 447.	56./ 87.	34/ 33
2250.	240./ 472.	47./ 92.	34/ 33
2475.	196./ 523.	39./ 101.	34/ 32
2700.	152./ 691.	30./ 132.	33/ 32

B E A M N O. 2181 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - V<sub>u</sub>= 128.02 KNS Tu= 58.0 KN-MET  
V<sub>c</sub>= 136.2 KNS, ACI 318:CLAUSE 11.6.3.1

LOAD 32 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.

AT END SUPPORT - V<sub>u</sub>= 136.24 KNS Tu= 58.0 KN-MET  
V<sub>c</sub>= 147.7 KNS, ACI 318:CLAUSE 11.6.3.1

LOAD 32 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



```

_____1106J_____ 2700X 300X 600_____1108J_____
|
|=====|
| 4No16 H 544. 0.TO 2700 |
|
| 4No20 H 58. 0.TO 2700 |
|=====|
|
|_____|

```

```

_____
| | | | | | | | | | | | | |
| oooo | | oooo | | oooo | | oooo | | oooo | | oooo | | oooo |
| 4#16 | | 4#16 | | 4#16 | | 4#16 | | 4#16 | | 4#16 | | 4#16 |
| | | | | | | | | | | | |
| 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 |
| oooo | | oooo | | oooo | | oooo | | oooo | | oooo | | oooo |
| | | | | | | | | | | | |
|_____| |_____| |_____| |_____| |_____| |_____| |_____|

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BEAM NO. 2183 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8200. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	54.	6 - 12MM	0.	8200.	YES YES
---	-----	----------	----	-------	---------

STAAD SPACE -- PAGE NO. 114

```

|-----|
| CRITICAL POS MOMENT= 115.42 KN-MET AT 4100.MM, LOAD 34|

```

```

|  REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
|  MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |
|  REQD. DEVELOPMENT LENGTH = 566. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

```

2      646.      6 - 12MM      0.      8200.      YES YES

```

```

|-----|
|  CRITICAL NEG MOMENT= 91.18 KN-MET AT 8200.MM, LOAD 34 |
|  REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
|  MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |
|  REQD. DEVELOPMENT LENGTH = 566. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION      REINF STEEL(+VE/-VE)      MOMENTS(+VE/-VE)      LOAD(+VE/-VE)
( MM )      (SQ. MM )      (KNS-MET )

```

SECTION ( MM )	REINF STEEL(+VE/-VE) (SQ. MM )	MOMENTS(+VE/-VE) (KNS-MET )	LOAD(+VE/-VE)
0.	270./ 345.	63./ 80.	34/ 33
683.	338./ 282.	79./ 66.	34/ 33
1367.	394./ 232.	92./ 54.	34/ 33
2050.	439./ 194.	102./ 46.	34/ 33
2733.	471./ 168.	109./ 40.	34/ 33
3417.	491./ 154.	114./ 36.	34/ 33
4100.	499./ 152.	115./ 36.	34/ 33
4783.	495./ 162.	114./ 38.	34/ 33
5467.	478./ 184.	111./ 43.	34/ 33
6150.	449./ 217.	104./ 51.	34/ 33
6833.	408./ 263.	95./ 62.	34/ 33
7517.	355./ 321.	83./ 75.	34/ 33
8200.	290./ 392.	68./ 91.	33/ 34

B E A M N O. 2183 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 22.46 KNS Vc= 162.53 KNS Vs= 0.00 KNS  
 Tu= 28.60 KN-MET Tc= 6.8 KN-MET Ts= 38.1 KN-MET LOAD 32

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 190. MM C/C FOR 3458. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 6.84 SQ.CM.

AT END SUPPORT - Vu= 18.61 KNS Vc= 162.53 KNS Vs= 0.00 KNS  
 Tu= 28.60 KN-MET Tc= 6.8 KN-MET Ts= 38.1 KN-MET LOAD 32

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 190. MM C/C FOR 3458. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 6.84 SQ.CM.

STAAD SPACE

-- PAGE NO. 115

1110J	8199X	300X	700	1112J
6No12 H 646.	0.TO 8200			
20*10c/c190				20*10c/c190
6No12 H 54.	0.TO 8200			
ooooo	ooooo	ooooo	ooooo	ooooo
ooooo	ooooo	ooooo	ooooo	ooooo

|\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_|

=====

BEAM NO. 2185 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

-----

1	61.	4 - 25MM	0.	5500.	YES YES
---	-----	----------	----	-------	---------

|-----|  
| CRITICAL POS MOMENT= 293.85 KN-MET AT 5500.MM, LOAD 33 |  
| REQD STEEL= 1625.MM2, ROW=0.0100, ROWMX=0.0190 ROWMN=0.0033 |  
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 60. MMS |  
REQD. DEVELOPMENT LENGTH = 1582. MMS

Cracked Moment of Inertia Iz at above location = 272812.6 cm<sup>4</sup>

2	539.	4 - 25MM	0.	5500.	YES YES
---	------	----------	----	-------	---------

|-----|  
| CRITICAL NEG MOMENT= 317.95 KN-MET AT 5500.MM, LOAD 34 |  
| REQD STEEL= 1777.MM2, ROW=0.0110, ROWMX=0.0190 ROWMN=0.0033 |  
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 60. MMS |  
REQD. DEVELOPMENT LENGTH = 1582. MMS

Cracked Moment of Inertia Iz at above location = 272812.6 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	639./	644.	123./	124.	34/	33
STAAD SPACE					-- PAGE NO. 116	
458.	749./	701.	143./	134.	34/	33
917.	854./	763.	162./	146.	34/	33
1375.	955./	832.	180./	158.	34/	33
1833.	1052./	907.	197./	171.	34/	33
2292.	1143./	990.	213./	186.	34/	33
2750.	1230./	1079.	228./	202.	34/	33
3208.	1311./	1176.	241./	218.	34/	33
3667.	1387./	1281.	254./	236.	34/	33
4125.	1457./	1393.	266./	255.	34/	33
4583.	1521./	1515.	276./	275.	34/	33
5042.	1580./	1645.	285./	296.	34/	33
5500.	1632./	1784.	294./	318.	33/	34

B E A M N O. 2185 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT -  $V_u = 87.15$  KNS  $V_c = 136.10$  KNS  $V_s = 0.00$  KNS

$T_u = 17.15$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 22.9$  KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2208. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 4.31 SQ.CM.

AT END SUPPORT -  $V_u = 91.34$  KNS  $V_c = 131.29$  KNS  $V_s = 0.00$  KNS

$T_u = 17.15$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 22.9$  KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2208. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 4.31 SQ.CM.

1114J 5499X 300X 600 1116J

```

|
|=====|
| 4No25 H 539. | 0.TO 5500| | | | | | | | | | | | | | | | | |
| 14*10c/c182 | | | | | | | | | | | | | | | 14*10c/c182 | |
| 4No25 H 61. | 0.TO 5500| | | | | | | | | | | | | | | | | |
|=====|
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|_____|

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| | | | | | | | | | | | | | | |
| oooo | | oooo | | oooo | | oooo | | oooo | | oooo | | oooo |
| 4#25 | | 4#25 | | 4#25 | | 4#25 | | 4#25 | | 4#25 | | 4#25 |
| | | | | | | | | | | | | | | |
| 4#25 | | 4#25 | | 4#25 | | 4#25 | | 4#25 | | 4#25 | | 4#25 |
| oooo | | oooo | | oooo | | oooo | | oooo | | oooo | | oooo |
| | | | | | | | | | | | | | | |
|_____| |_____| |_____| |_____| |_____| |_____| |_____|

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BEAM NO. 2186 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

-----

1	54.	5 - 12MM	338.	5500.	NO YES
---	-----	----------	------	-------	--------

-----

| CRITICAL POS MOMENT= 55.40 KN-MET AT 5042.MM, LOAD 34 |

```

|  REQD STEEL= 546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
|  MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 48. MMS |
|  REQD. DEVELOPMENT LENGTH = 453. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

```

2      546.      5 - 12MM      0.      5500.      YES YES

```

```

|-----|
|  CRITICAL NEG MOMENT= 96.27 KN-MET AT 0.MM, LOAD 33|
|  REQD STEEL= 546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
|  MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 48. MMS |
|  REQD. DEVELOPMENT LENGTH = 453. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION      REINF STEEL (+VE/-VE)      MOMENTS (+VE/-VE)      LOAD (+VE/-VE)
( MM )      (SQ. MM )      (KNS-MET )

```

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	0./ 496.	0./ 96.	0/ 33
458.	0./ 431.	0./ 84.	0/ 33
917.	46./ 371.	9./ 73.	34/ 33
1375.	93./ 318.	18./ 62.	34/ 33
1833.	135./ 271.	27./ 53.	34/ 33
2292.	172./ 230.	34./ 45.	34/ 33
2750.	203./ 194.	40./ 38.	34/ 33
3208.	230./ 164.	45./ 33.	34/ 33
3667.	251./ 140.	49./ 28.	34/ 33
4125.	266./ 122.	52./ 24.	34/ 33
4583.	277./ 109.	54./ 22.	34/ 33
5042.	282./ 101.	55./ 20.	34/ 33
5500.	281./ 99.	55./ 20.	33/ 34

B E A M N O . 2 1 8 6 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

-- PAGE NO. 118

AT START SUPPORT - Vu= 38.27 KNS Vc= 130.36 KNS Vs= 0.00 KNS  
 Tu= 12.84 KN-MET Tc= 5.6 KN-MET Ts= 17.1 KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2208. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.22 SQ.CM.

AT END SUPPORT - Vu= 11.99 KNS Vc= 129.07 KNS Vs= 0.00 KNS  
 Tu= 12.84 KN-MET Tc= 5.6 KN-MET Ts= 17.1 KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 2208. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.22 SQ.CM.

1116J	5499X	300X	600	1118J
=====				
5No12 H 546.	0.TO	5500		
14*10c/c182				14*10c/c182
5No12 H 54.	338.TO	5500		
=====				
ooooo	ooooo	ooooo	ooooo	ooooo
5#12	5#12	5#12	5#12	5#12
	5#12	5#12	5#12	5#12
	ooooo	ooooo	ooooo	ooooo



|\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_| |\_\_\_\_\_|

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BEAM NO. 2187 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 6500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
-------	----------------	----------	--------------	------------	-------------------	--

-----

1	61.	3 - 25MM	0.	6500.	YES	YES
---	-----	----------	----	-------	-----	-----

-----|  
| CRITICAL POS MOMENT= 246.88 KN-MET AT 0.MM, LOAD 34|  
| REQD STEEL= 1339.MM2, ROW=0.0083, ROWMX=0.0190 ROWMN=0.0033 |  
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 89. MMS |  
REQD. DEVELOPMENT LENGTH = 1055. MMS

Cracked Moment of Inertia Iz at above location = 220403.6 cm^4

2	542.	4 - 20MM	0.	6500.	YES	YES
---	------	----------	----	-------	-----	-----

STAAD SPACE -- PAGE NO. 119

-----|  
| CRITICAL NEG MOMENT= 228.72 KN-MET AT 0.MM, LOAD 33|  
| REQD STEEL= 1225.MM2, ROW=0.0075, ROWMX=0.0190 ROWMN=0.0033 |  
| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |  
REQD. DEVELOPMENT LENGTH = 985. MMS

Cracked Moment of Inertia Iz at above location = 197182.2 cm^4

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	1345./	1237.	247./	229.	34/	33
542.	1301./	1120.	240./	209.	34/	33
1083.	1250./	1014.	231./	190.	34/	33
1625.	1190./	918.	221./	173.	34/	33
2167.	1121./	831.	209./	158.	34/	33
2708.	1045./	754.	196./	144.	34/	33
3250.	961./	685.	181./	131.	34/	33
3792.	870./	626.	165./	120.	34/	33
4333.	772./	574.	147./	111.	34/	33
4875.	667./	532.	128./	103.	34/	33
5417.	555./	497.	107./	96.	34/	33
5958.	437./	470.	85./	91.	34/	33
6500.	313./	451.	61./	88.	33/	34

B E A M N O. 2187 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 58.41 KNS Vc= 129.91 KNS Vs= 0.00 KNS

Tu= 4.02 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 33

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 270. MM C/C FOR 2708. MM

AT END SUPPORT - Vu= 65.27 KNS Vc= 135.98 KNS Vs= 0.00 KNS

Tu= 4.02 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 34

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 270. MM C/C FOR 2708. MM

```

    1118J _____ 6500X 300X 600 _____ 1120J _____
|
|=====|
| 4No20 H 542. | 0.TO 6500| | | | | | | | | | | | | | | | |
| 12*10c/c270 | | | | | | | | | | | | 12*10c/c270 | |
| 3No25 H |61. | 0.TO 6500| | | | | | | | | | | | | | | |
|=====|
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|_____|

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| | | | | | | | | | | | | | | |
| oooo | | oooo | | oooo | | oooo | | oooo | | oooo | | oooo |
| 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 | | 4#20 |
| | | | | | | | | | | | | | | |
| 3#25 | | 3#25 | | 3#25 | | 3#25 | | 3#25 | | 3#25 | | 3#25 |
| ooo | | ooo | | ooo | | ooo | | ooo | | ooo | | ooo |
| | | | | | | | | | | | | | | |
|_____| |_____| |_____| |_____| |_____| |_____| |_____|

```

=====

BEAM NO. 2188 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	64.	3 - 32MM	0.	8000.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 420.53 KN-MET AT 4667.MM, LOAD 34|
| REQD STEEL= 1980.MM2, ROW=0.0104, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 64./ 86. MMS |

```

| REQD. DEVELOPMENT LENGTH = 1799. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 460654.4 cm<sup>4</sup>

2            636.          3 - 32MM            0.            8000.            YES YES

|-----|

| CRITICAL NEG MOMENT= 430.33 KN-MET AT 0.MM, LOAD 33 |

| REQD STEEL= 2032.MM2, ROW=0.0107, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 64./ 86. MMS |

| REQD. DEVELOPMENT LENGTH = 1799. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 460654.4 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION	REINF STEEL (+VE/-VE)		MOMENTS (+VE/-VE)		LOAD (+VE/-VE)	
( MM )	(SQ. MM )		(KNS-MET )			

0.	1669./	2026.	362./	430.	34/	33
----	--------	-------	-------	------	-----	----

STAAD SPACE -- PAGE NO. 121

667.	1752./	1939.	378./	414.	34/	33
------	--------	-------	-------	------	-----	----

1333.	1823./	1867.	392./	400.	34/	33
-------	--------	-------	-------	------	-----	----

2000.	1880./	1810.	403./	389.	34/	33
-------	--------	-------	-------	------	-----	----

2667.	1924./	1767.	411./	381.	34/	33
-------	--------	-------	-------	------	-----	----

3333.	1955./	1737.	417./	375.	34/	33
-------	--------	-------	-------	------	-----	----

4000.	1971./	1721.	420./	372.	34/	33
-------	--------	-------	-------	------	-----	----

4667.	1973./	1719.	421./	372.	34/	33
-------	--------	-------	-------	------	-----	----

5333.	1962./	1730.	418./	374.	34/	33
-------	--------	-------	-------	------	-----	----

6000.	1936./	1755.	414./	379.	34/	33
-------	--------	-------	-------	------	-----	----

6667.	1897./	1793.	406./	386.	34/	33
-------	--------	-------	-------	------	-----	----

7333.	1844./	1846.	396./	396.	34/	33
-------	--------	-------	-------	------	-----	----

8000.	1778./	1912.	383./	409.	33/	34
-------	--------	-------	-------	------	-----	----

B E A M N O. 2188 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 121.64 KNS Vc= 155.42 KNS Vs= 6.77 KNS

Tu= 4.44 KN-MET Tc= 6.8 KN-MET Ts= 0.0 KN-MET LOAD 33

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT IS REQUIRED FOR SHEAR.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 318. MM C/C FOR 3358. MM

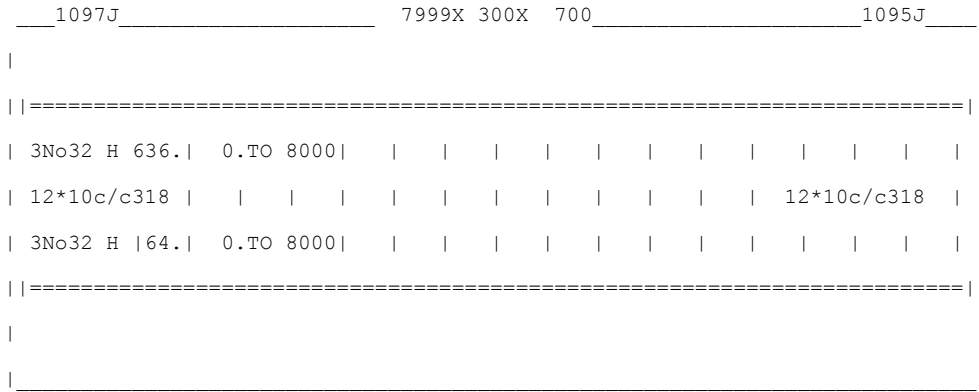
AT END SUPPORT - Vu= 74.40 KNS Vc= 153.45 KNS Vs= 0.00 KNS

Tu= 4.55 KN-MET Tc= 6.8 KN-MET Ts= 0.0 KN-MET LOAD 33

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 318. MM C/C FOR 3358. MM



000	000	000	000	000	000	000	000
3#32	3#32	3#32	3#32	3#32	3#32	3#32	3#32
3#32	3#32	3#32	3#32	3#32	3#32	3#32	3#32
000	000	000	000	000	000	000	000

=====

BEAM NO. 2189 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
-------	----------------	----------	--------------	------------	-------------------	--

1	58.	4 - 20MM	0.	8000.	YES	YES
---	-----	----------	----	-------	-----	-----

|-----|

| CRITICAL POS MOMENT= 247.33 KN-MET AT 4000.MM, LOAD 34 |

| REQD STEEL= 1097.MM2, ROW=0.0057, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |

| REQD. DEVELOPMENT LENGTH = 985. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 286994.1 cm<sup>4</sup>

2	642.	4 - 20MM	0.	8000.	YES	YES
---	------	----------	----	-------	-----	-----

|-----|

| CRITICAL NEG MOMENT= 250.41 KN-MET AT 0.MM, LOAD 33 |

| REQD STEEL= 1111.MM2, ROW=0.0058, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |

| REQD. DEVELOPMENT LENGTH = 985. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 286994.1 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
-------------------	------------------------------------	---------------------------------	----------------

0.	869./	1120.	197./	250.	34/	33
667.	939./	1049.	212./	235.	34/	33
1333.	996./	991.	224./	223.	34/	33
2000.	1042./	945.	234./	213.	34/	33
2667.	1076./	912.	241./	206.	34/	33
3333.	1097./	892.	246./	202.	34/	33
4000.	1105./	883.	247./	200.	34/	33
4667.	1101./	887.	246./	201.	34/	33
5333.	1085./	903.	243./	204.	34/	33
6000.	1056./	932.	237./	211.	34/	33
6667.	1014./	973.	228./	219.	34/	33
7333.	961./	1026.	217./	231.	34/	33
8000.	895./	1093.	203./	245.	33/	34

B E A M N O. 2189 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

-- PAGE NO. 123

AT START SUPPORT -  $V_u = 76.57$  KNS  $V_c = 154.39$  KNS  $V_s = 0.00$  KNS

$T_u = 4.69$  KN-MET  $T_c = 6.8$  KN-MET  $T_s = 0.0$  KN-MET LOAD 33

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 321. MM C/C FOR 3358. MM

AT END SUPPORT -  $V_u = 71.00$  KNS  $V_c = 154.77$  KNS  $V_s = 0.00$  KNS

$T_u = 4.69$  KN-MET  $T_c = 6.8$  KN-MET  $T_s = 0.0$  KN-MET LOAD 34

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 321. MM C/C FOR 3358. MM

\_\_\_\_\_1098J\_\_\_\_\_ 7999X 300X 700\_\_\_\_\_1096J\_\_\_\_\_

||=====||

| 4N<sub>o</sub>20 H 642. | 0.TO 8000 | | | | | | | | | | | | | | | | |

```

| | | | | | | | | | | | | | | | |
| 12*10c/c321 | | | | | | | | | | | | | 12*10c/c321 | |
| | | | | | | | | | | | | | | | |
| 4No20 H |58.| 0.TO 8000| | | | | | | | | | | |
|=====|
|-----|
|ooo | |ooo | |ooo | |ooo | |ooo | |ooo |
| 4#20| | 4#20| | 4#20| | 4#20| | 4#20| | 4#20|
| | | | | | | | | | | | | | |
| | | | | | | | | | | | | | |
| 4#20| | 4#20| | 4#20| | 4#20| | 4#20| | 4#20|
|ooo | |ooo | |ooo | |ooo | |ooo | |ooo |
|_____| |_____| |_____| |_____| |_____| |_____| |_____|

```

=====

BEAM NO. 2190 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	64.	3 - 32MM	0.	8000.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 487.05 KN-MET AT 4000.MM, LOAD 34|
| REQD STEEL= 2343.MM2, ROW=0.0123, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 64./ 86. MMS |
| REQD. DEVELOPMENT LENGTH = 1799. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 460654.4 cm^4



2            632.        2 - 40MM            0.            8000.            YES YES

STAAD SPACE

-- PAGE NO. 124

```

|-----|
| CRITICAL NEG MOMENT= 513.40 KN-MET AT 0.MM, LOAD 33|
| REQD STEEL= 2513.MM2, ROW=0.0133, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 80./ 164. MMS |
| REQD. DEVELOPMENT LENGTH = 1773. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 467463.3 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION        REINF STEEL (+VE/-VE)        MOMENTS (+VE/-VE)        LOAD (+VE/-VE)
( MM )            (SQ. MM )                    (KNS-MET )

```

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	2068./ 2482.	438./ 513.	34/ 33
667.	2148./ 2399.	453./ 499.	34/ 33
1333.	2213./ 2332.	465./ 487.	34/ 33
2000.	2265./ 2279.	475./ 477.	34/ 33
2667.	2303./ 2242.	481./ 470.	34/ 33
3333.	2326./ 2219.	486./ 466.	34/ 33
4000.	2334./ 2210.	487./ 465.	34/ 33
4667.	2328./ 2216.	486./ 466.	34/ 33
5333.	2307./ 2237.	482./ 469.	34/ 33
6000.	2272./ 2272.	476./ 476.	34/ 33
6667.	2222./ 2322.	467./ 485.	34/ 33
7333.	2159./ 2387.	455./ 496.	34/ 33
8000.	2082./ 2467.	441./ 511.	33/ 34

AT START SUPPORT - Vu= 139.21 KNS Vc= 156.21 KNS Vs= 29.41 KNS  
Tu= 7.48 KN-MET Tc= 6.8 KN-MET Ts= 10.0 KN-MET LOAD 33  
STIRRUPS ARE REQUIRED FOR SHEAR AND TORSION.  
PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM  
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.79 SQ.CM.

AT END SUPPORT - Vu= 129.73 KNS Vc= 156.99 KNS Vs= 15.98 KNS  
Tu= 7.48 KN-MET Tc= 6.8 KN-MET Ts= 10.0 KN-MET LOAD 34  
STIRRUPS ARE REQUIRED FOR SHEAR AND TORSION.  
PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM  
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.79 SQ.CM.

1100J	7999X	300X	700	1099J				
=====								
2No40 H 632.	0.	TO 8000						
18*10c/c207							18*10c/c207	
=====								
OO		OO		OO		OO		OO
2#40		2#40		2#40		2#40		2#40
3#32		3#32		3#32		3#32		3#32
OOO		OOO		OOO		OOO		OOO
_____		_____		_____		_____		_____

=====

BEAM NO. 2191 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	58.	4 - 20MM	0.	8000.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 237.95 KN-MET AT 6000.MM, LOAD 34|
| REQD STEEL= 1053.MM2, ROW=0.0055, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |
| REQD. DEVELOPMENT LENGTH = 985. MMS |
|-----|
    
```

Cracked Moment of Inertia Iz at above location = 286994.1 cm<sup>4</sup>

2	639.	3 - 25MM	0.	8000.	YES YES
---	------	----------	----	-------	---------

```

|-----|
| CRITICAL NEG MOMENT= 300.43 KN-MET AT 0.MM, LOAD 33|
| REQD STEEL= 1357.MM2, ROW=0.0071, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 89. MMS |
| REQD. DEVELOPMENT LENGTH = 1055. MMS |
|-----|
    
```

Cracked Moment of Inertia Iz at above location = 322220.8 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
-------------------	------------------------------------	---------------------------------	----------------

0.	549./ 1362.	127./ 300.	34/ 33
----	-------------	------------	--------

STAAD SPACE

667.	652./	1250.	150./	277.	34/	33
1333.	744./	1152.	170./	257.	34/	33
2000.	825./	1067.	188./	239.	34/	33
2667.	895./	996.	203./	224.	34/	33
3333.	952./	937.	215./	212.	34/	33
4000.	998./	892.	225./	202.	34/	33
4667.	1031./	858.	232./	195.	34/	33
5333.	1052./	837.	236./	190.	34/	33
6000.	1061./	829.	238./	188.	34/	33
6667.	1057./	832.	237./	189.	34/	33
7333.	1040./	848.	234./	192.	34/	33
8000.	1012./	875.	228./	198.	33/	34

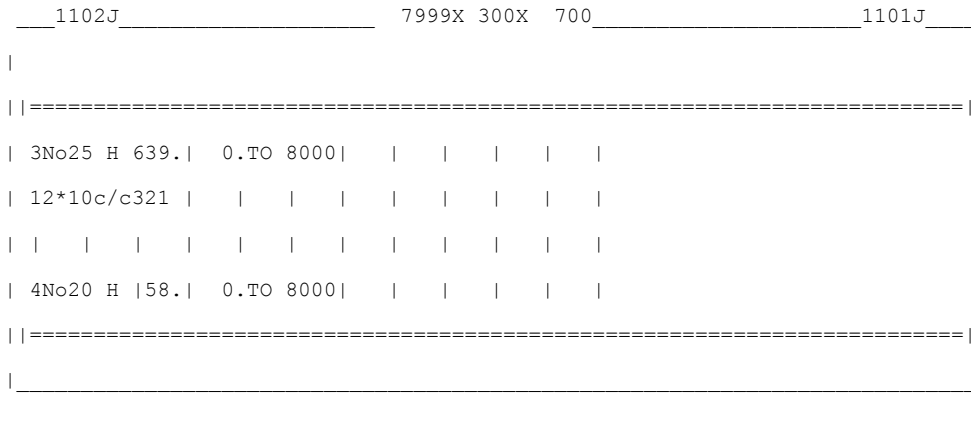
B E A M N O. 2191 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 85.93 KNS Vc= 154.05 KNS Vs= 0.00 KNS  
 Tu= 4.43 KN-MET Tc= 6.8 KN-MET Ts= 0.0 KN-MET LOAD 33  
 NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 321. MM C/C FOR 3358. MM

AT END SUPPORT - Vu= 8.47 KNS Vc= 154.60 KNS Vs= 0.00 KNS  
 Tu= 0.63 KN-MET Tc= 6.8 KN-MET Ts= 0.0 KN-MET LOAD 30  
 STIRRUPS ARE NOT REQUIRED.



ooo	ooo	ooo	ooo	ooo	ooo	ooo
3#25	3#25	3#25	3#25	3#25	3#25	3#25
4#20	4#20	4#20	4#20	4#20	4#20	4#20
oooo	oooo	oooo	oooo	oooo	oooo	oooo
_____	_____	_____	_____	_____	_____	_____

STAAD SPACE

-- PAGE NO. 127

=====

BEAM NO. 2192 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	61.	3 - 25MM	0.	8000.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 290.90 KN-MET AT 5333.MM, LOAD 34 |
| REQD STEEL= 1311.MM2, ROW=0.0068, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 89. MMS |
| REQD. DEVELOPMENT LENGTH = 1055. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 322220.8 cm<sup>4</sup>

2	642.	5 - 20MM	0.	8000.	YES YES
---	------	----------	----	-------	---------

```

|-----|
| CRITICAL NEG MOMENT= 337.72 KN-MET AT 0.MM, LOAD 33 |
| REQD STEEL= 1534.MM2, ROW=0.0080, ROWMX=0.0190 ROWMN=0.0033 |

```

| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 46. MMS |  
REQD. DEVELOPMENT LENGTH = 1314. MMS

Cracked Moment of Inertia Iz at above location = 341631.0 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	880./ 1547.	199./ 338.	34/ 33
667.	976./ 1431.	220./ 314.	34/ 33
1333.	1061./ 1330.	238./ 294.	34/ 33
2000.	1135./ 1242.	253./ 276.	34/ 33
2667.	1196./ 1169.	266./ 261.	34/ 33
3333.	1245./ 1108.	276./ 248.	34/ 33
4000.	1281./ 1061.	284./ 238.	34/ 33
4667.	1304./ 1026.	289./ 231.	34/ 33
5333.	1315./ 1003.	291./ 226.	34/ 33
6000.	1313./ 993.	290./ 224.	34/ 33
6667.	1298./ 996.	287./ 224.	34/ 33
7333.	1270./ 1011.	282./ 227.	34/ 33
8000.	1230./ 1038.	273./ 233.	33/ 34

B E A M N O. 2192 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

-- PAGE NO. 128

AT START SUPPORT - Vu= 34.12 KNS Vc= 152.12 KNS Vs= 0.00 KNS  
 Tu= 9.43 KN-MET Tc= 6.8 KN-MET Ts= 12.6 KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 2.25 SQ.CM.

AT END SUPPORT - Vu= 51.59 KNS Vc= 153.44 KNS Vs= 0.00 KNS

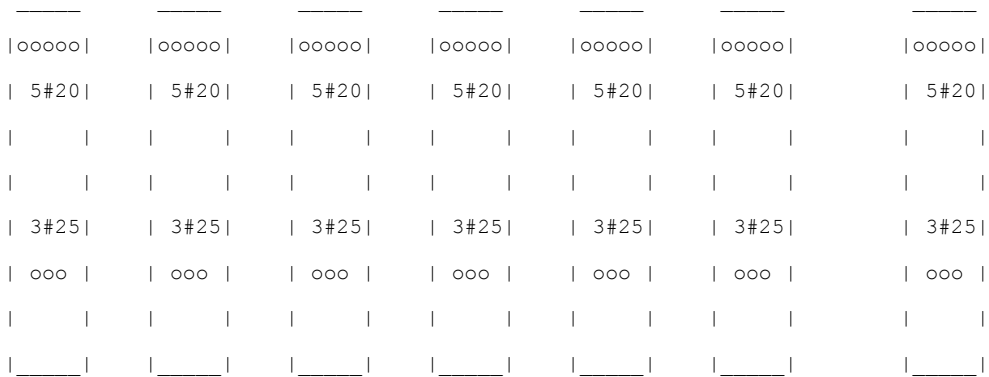
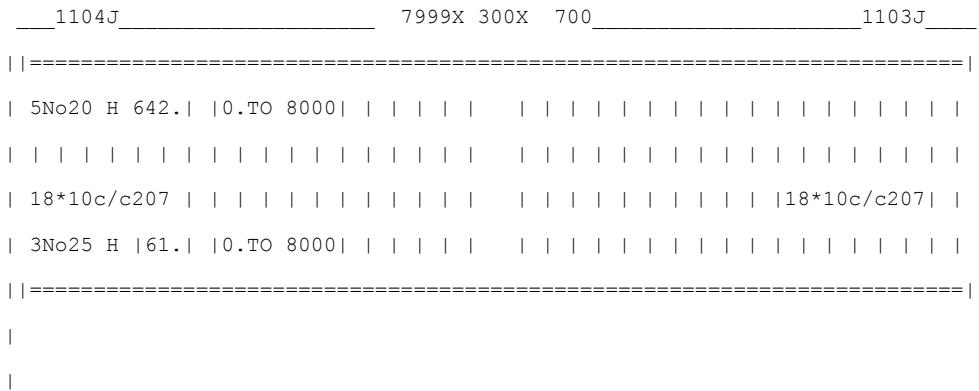
Tu= 9.43 KN-MET Tc= 6.8 KN-MET Ts= 12.6 KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 2.25 SQ.CM.



=====

BEAM NO. 2193 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL HEIGHT BAR INFO FROM TO ANCHOR

(MM) (MM) (MM) STA END

1 61. 3 - 25MM 0. 8000. YES YES

```

|-----|
| CRITICAL POS MOMENT= 290.54 KN-MET AT 5333.MM, LOAD 34 |
| REQD STEEL= 1309.MM2, ROW=0.0068, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 89. MMS |
| REQD. DEVELOPMENT LENGTH = 1055. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 322220.8 cm<sup>4</sup>

2 642. 5 - 20MM 0. 8000. YES YES

STAAD SPACE -- PAGE NO. 129

```

|-----|
| CRITICAL NEG MOMENT= 336.51 KN-MET AT 0.MM, LOAD 33 |
| REQD STEEL= 1528.MM2, ROW=0.0079, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 46. MMS |
| REQD. DEVELOPMENT LENGTH = 1314. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 341631.0 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	905./ 1541.	205./ 337.	34/ 33
667.	998./ 1427.	225./ 314.	34/ 33
1333.	1080./ 1328.	242./ 294.	34/ 33
2000.	1150./ 1243.	257./ 276.	34/ 33
2667.	1208./ 1172.	269./ 261.	34/ 33
3333.	1253./ 1113.	278./ 249.	34/ 33



4000.	1286./	1068.	285./	239.	34/	33
4667.	1306./	1035.	289./	232.	34/	33
5333.	1313./	1015.	291./	228.	34/	33
6000.	1308./	1007.	289./	226.	34/	33
6667.	1289./	1011.	286./	227.	34/	33
7333.	1258./	1028.	279./	231.	34/	33
8000.	1214./	1058.	270./	237.	33/	34

B E A M N O . 2193 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 36.83 KNS Vc= 158.59 KNS Vs= 0.00 KNS  
 Tu= 24.22 KN-MET Tc= 6.8 KN-MET Ts= 32.3 KN-MET LOAD 32

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 5.79 SQ.CM.

AT END SUPPORT - Vu= 3.05 KNS Vc= 150.71 KNS Vs= 0.00 KNS  
 Tu= 24.22 KN-MET Tc= 6.8 KN-MET Ts= 32.3 KN-MET LOAD 32

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 5.79 SQ.CM.

STAAD SPACE

-- PAGE NO. 130

1106J				7999X 300X 700	1105J			
=====								
5Nø20 H 642.	0.TO 8000							
18*10c/c207						18*10c/c207		
3Nø25 H 61.	0.TO 8000							
=====								

ooooo	ooooo	ooooo	ooooo	ooooo	ooooo	ooooo
5#20	5#20	5#20	5#20	5#20	5#20	5#20
3#25	3#25	3#25	3#25	3#25	3#25	3#25
ooo	ooo	ooo	ooo	ooo	ooo	ooo
_____	_____	_____	_____	_____	_____	_____

=====

BEAM NO. 2194 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT	BAR INFO	FROM	TO	ANCHOR
	(MM)		(MM)	(MM)	STA END

1	58.	4 - 20MM	0.	8000.	YES YES
---	-----	----------	----	-------	---------

-----|

| CRITICAL POS MOMENT= 260.03 KN-MET AT 4000.MM, LOAD 34|

| REQD STEEL= 1157.MM2, ROW=0.0060, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |

| REQD. DEVELOPMENT LENGTH = 985. MMS |

-----|

Cracked Moment of Inertia Iz at above location = 286994.1 cm^4

2	642.	4 - 20MM	0.	8000.	YES YES
---	------	----------	----	-------	---------

-----|

| CRITICAL NEG MOMENT= 272.35 KN-MET AT 0.MM, LOAD 33|

| REQD STEEL= 1216.MM2, ROW=0.0063, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |  
REQD. DEVELOPMENT LENGTH = 985. MMS

Cracked Moment of Inertia Iz at above location = 286994.1 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	954./	1225.	215./	272.	32/	33
STAAD SPACE					-- PAGE NO. 131	
667.	980./	1148.	221./	256.	34/	33
1333.	1041./	1085.	234./	243.	34/	33
2000.	1091./	1034.	244./	232.	34/	33
2667.	1129./	997.	252./	224.	34/	33
3333.	1153./	971.	257./	219.	34/	33
4000.	1166./	958.	260./	216.	34/	33
4667.	1166./	958.	260./	216.	34/	33
5333.	1153./	969.	257./	219.	34/	33
6000.	1127./	994.	252./	224.	34/	33
6667.	1089./	1030.	244./	231.	34/	33
7333.	1039./	1080.	233./	242.	34/	33
8000.	977./	1193.	220./	266.	33/	32

B E A M N O. 2194 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 40.19 KNS Vc= 153.15 KNS Vs= 0.00 KNS

Tu= 38.37 KN-MET Tc= 6.8 KN-MET Ts= 51.2 KN-MET LOAD 32

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 12 MM 2-LEGGED STIRRUPS AT 204. MM C/C FOR 3358. MM



LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
-------	----------------	----------	--------------	------------	-------------------	--

1	61.	4 - 25MM	0.	8000.	YES	YES
---	-----	----------	----	-------	-----	-----

```

|-----|
| CRITICAL POS MOMENT= 361.50 KN-MET AT 0.MM, LOAD 32 |
| REQD STEEL= 1661.MM2, ROW=0.0087, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 60. MMS |
| REQD. DEVELOPMENT LENGTH = 1582. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 401032.4 cm<sup>4</sup>

2	636.	3 - 32MM	0.	8000.	YES	YES
---	------	----------	----	-------	-----	-----

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|-----|
| CRITICAL NEG MOMENT= 428.57 KN-MET AT 8000.MM, LOAD 32 |
| REQD STEEL= 2023.MM2, ROW=0.0106, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 64./ 86. MMS |
| REQD. DEVELOPMENT LENGTH = 1799. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 460654.4 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	1667./	1190.	361./	265.	32/	33
667.	1410./	1122.	310./	251.	32/	33
1333.	1148./	1068.	256./	239.	32/	33
2000.	1056./	1026.	237./	231.	34/	33
2667.	1085./	997.	243./	224.	34/	33
3333.	1103./	980.	247./	221.	34/	33

4000.	1107./	975.	248./	220.	34/	33
4667.	1099./	983.	246./	221.	34/	33
5333.	1079./	1003.	242./	226.	34/	33
6000.	1046./	1036.	235./	233.	34/	33
6667.	1001./	1216.	225./	271.	34/	32
7333.	944./	1600.	213./	348.	34/	32
8000.	875./	2016.	198./	429.	33/	32

B E A M N O . 2195 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

-- PAGE NO. 133

AT START SUPPORT - Vu= 78.82 KNS Vc= 154.49 KNS Vs= 0.00 KNS  
 Tu= 7.12 KN-MET Tc= 6.8 KN-MET Ts= 9.5 KN-MET LOAD 32

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM

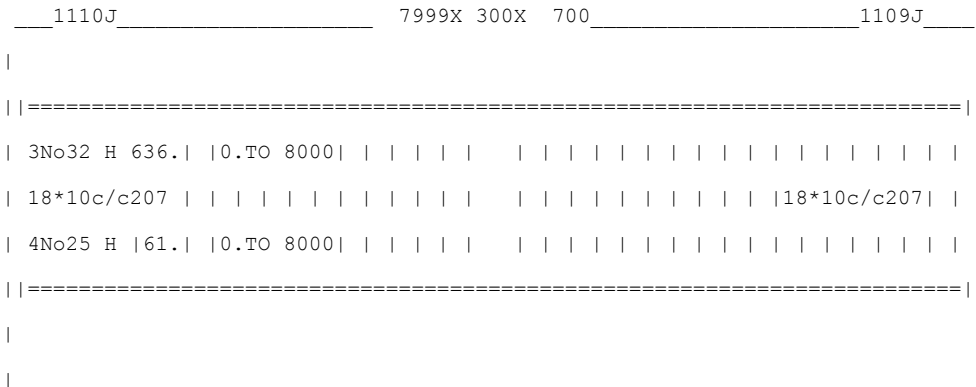
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.70 SQ.CM.

AT END SUPPORT - Vu= 118.70 KNS Vc= 156.06 KNS Vs= 2.21 KNS  
 Tu= 7.12 KN-MET Tc= 6.8 KN-MET Ts= 9.5 KN-MET LOAD 32

STIRRUPS ARE REQUIRED FOR SHEAR AND TORSION.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.70 SQ.CM.



000	000	000	000	000	000	000	000
3#32	3#32	3#32	3#32	3#32	3#32	3#32	3#32
4#25	4#25	4#25	4#25	4#25	4#25	4#25	4#25
0000	0000	0000	0000	0000	0000	0000	0000

=====

BEAM NO. 2196 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 6500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	56.	4 - 16MM	0.	6500.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 172.07 KN-MET AT 6500.MM, LOAD 33|
| REQD STEEL= 746.MM2, ROW=0.0039, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |
| REQD. DEVELOPMENT LENGTH = 617. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 201146.2 cm^4

2	642.	3 - 20MM	0.	6500.	YES YES
---	------	----------	----	-------	---------

STAAD SPACE

-- PAGE NO. 134

```

|-----|
| CRITICAL NEG MOMENT= 199.91 KN-MET AT 0.MM, LOAD 33|

```

| REQD STEEL= 876.MM2, ROW=0.0045, ROWMX=0.0190 ROWMN=0.0033 |  
 | MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 92. MMS |  
REQD. DEVELOPMENT LENGTH = 657. MMS

Cracked Moment of Inertia Iz at above location = 227647.7 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	78./ 882.	19./ 200.	34/ 33
542.	174./ 768.	41./ 175.	34/ 33
1083.	263./ 662.	62./ 152.	34/ 33
1625.	345./ 566.	80./ 130.	34/ 33
2167.	420./ 478.	98./ 111.	34/ 33
2708.	488./ 399.	113./ 93.	34/ 33
3250.	549./ 328.	127./ 77.	34/ 33
3792.	602./ 266.	139./ 62.	34/ 33
4333.	648./ 211.	149./ 49.	34/ 33
4875.	687./ 164.	157./ 38.	34/ 33
5417.	717./ 124.	164./ 29.	34/ 33
5958.	740./ 92.	169./ 22.	34/ 33
6500.	754./ 68.	172./ 16.	33/ 34

B E A M N O. 2196 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 10.22 KNS Vc= 152.82 KNS Vs= 0.00 KNS  
 Tu= 13.01 KN-MET Tc= 6.8 KN-MET Ts= 17.3 KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 2608. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.11 SQ.CM.



AT END SUPPORT - Vu= 41.68 KNS Vc= 163.79 KNS Vs= 0.00 KNS  
 Tu= 13.01 KN-MET Tc= 6.8 KN-MET Ts= 17.3 KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 2608. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.11 SQ.CM.

STAAD SPACE

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```

_____1121J_____ 6500X 300X 700_____1112J_____
|=====|
| 3No20 H 642.| 0.TO 6500| | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 14*10c/c207 | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 4No16 H 56.| 0.TO 6500| | | | | | | | | | | | | | | | | | |
|=====|
|_____|
|_____|
| 000 | | 000 | | 000 | | 000 | | 000 | | 000 | | 000 | | | | | | | | | |
| 3#20| | 3#20| | 3#20| | 3#20| | 3#20| | 3#20| | 3#20|
| | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | |
| 4#16| | 4#16| | 4#16| | 4#16| | 4#16| | 4#16| | 4#16|
|0000 | |0000 | |0000 | |0000 | |0000 | |0000 |
|_____| |_____| |_____| |_____| |_____| |_____| |_____|
  
```

=====

BEAM NO. 2197 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT	BAR INFO	FROM	TO	ANCHOR	
	(MM)		(MM)	(MM)	STA	END

1	56.	4 - 16MM	0.	8000.	YES	YES
---	-----	----------	----	-------	-----	-----

```

|-----|
| CRITICAL POS MOMENT= 170.46 KN-MET AT 1333.MM, LOAD 34 |
| REQD STEEL= 739.MM2, ROW=0.0038, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |
| REQD. DEVELOPMENT LENGTH = 617. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 201146.2 cm<sup>4</sup>

2	642.	4 - 20MM	0.	8000.	YES	YES
---	------	----------	----	-------	-----	-----

```

|-----|
| CRITICAL NEG MOMENT= 233.23 KN-MET AT 8000.MM, LOAD 34 |
| REQD STEEL= 1031.MM2, ROW=0.0054, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |
| REQD. DEVELOPMENT LENGTH = 985. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 286994.1 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION	REINF STEEL (+VE/-VE)		MOMENTS (+VE/-VE)		LOAD (+VE/-VE)	
( MM )	(SQ.	MM )	(KNS-MET )			
0.	731./	776.	167./	177.	34/	32
667.	745./	599.	170./	138.	34/	32
1333.	747./	437.	170./	101.	34/	32
2000.	737./	356.	168./	83.	34/	33
2667.	715./	383.	163./	89.	34/	33

STAAD SPACE

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oooo	oooo	oooo	oooo	oooo	oooo	oooo
4#20	4#20	4#20	4#20	4#20	4#20	4#20
4#16	4#16	4#16	4#16	4#16	4#16	4#16
oooo	oooo	oooo	oooo	oooo	oooo	oooo
____	____	____	____	____	____	____

STAAD SPACE

-- PAGE NO. 137

=====

BEAM NO. 2198 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	54.	6 - 12MM	0.	8000.	YES YES
---	-----	----------	----	-------	---------

-----|

| CRITICAL POS MOMENT= 139.69 KN-MET AT 2000.MM, LOAD 34|

| REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |

| REQD. DEVELOPMENT LENGTH = 566. MMS |

-----|

Cracked Moment of Inertia Iz at above location = 175566.9 cm^4

2	642.	3 - 20MM	0.	8000.	YES YES
---	------	----------	----	-------	---------

-----|

| CRITICAL NEG MOMENT= 196.50 KN-MET AT 8000.MM, LOAD 34|

```

|   REQD STEEL=   860.MM2, ROW=0.0045, ROWMX=0.0190 ROWMN=0.0033 |
|   MAX/MIN/ACTUAL BAR SPACING=   261./   45./   92. MMS       |
|   REQD. DEVELOPMENT LENGTH =   657. MMS                       |
|-----|

```

Cracked Moment of Inertia Iz at above location = 227647.7 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION          REINF STEEL (+VE/-VE)      MOMENTS (+VE/-VE)      LOAD (+VE/-VE)
( MM )          (SQ. MM )              (KNS-MET )

```

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	550./ 390.	127./ 91.	34/ 33
667.	581./ 365.	134./ 85.	34/ 33
1333.	600./ 352.	138./ 82.	34/ 33
2000.	608./ 350.	140./ 82.	34/ 33
2667.	603./ 360.	139./ 84.	34/ 33
3333.	587./ 381.	135./ 89.	34/ 33
4000.	559./ 414.	129./ 96.	34/ 33
4667.	519./ 459.	120./ 106.	34/ 33
5333.	468./ 515.	108./ 119.	34/ 33
6000.	405./ 584.	94./ 134.	34/ 33
6667.	331./ 665.	77./ 152.	34/ 33
7333.	247./ 759.	58./ 173.	34/ 33
8000.	152./ 867.	36./ 197.	33/ 34

B E A M N O. 2198 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

-- PAGE NO. 138

AT START SUPPORT - Vu= 20.48 KNS Vc= 152.50 KNS Vs= 0.00 KNS

Tu= 12.69 KN-MET Tc= 6.8 KN-MET Ts= 16.9 KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM  
 ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.03 SQ.CM.

AT END SUPPORT - Vu= 6.22 KNS Vc= 151.66 KNS Vs= 0.00 KNS  
 Tu= 12.69 KN-MET Tc= 6.8 KN-MET Ts= 16.9 KN-MET LOAD 33  
 STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 3358. MM  
 ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.03 SQ.CM.

1114J		7999X 300X 700				1113J	
=====							
3No20 H 642.	0.TO 8000						
18*10c/c207						18*10c/c207	
6No12 H 54.	0.TO 8000						
=====							
-----							
ooo	ooo	ooo	ooo	ooo	ooo	ooo	ooo
3#20	3#20	3#20	3#20	3#20	3#20	3#20	3#20
ooooo	ooooo	ooooo	ooooo	ooooo	ooooo	ooooo	ooooo
_____	_____	_____	_____	_____	_____	_____	_____

=====

BEAM NO. 2199 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
-------	----------------	----------	--------------	------------	-------------------	--

1	54.	6 - 12MM	0.	8000.	YES	YES
---	-----	----------	----	-------	-----	-----

```

|-----|
| CRITICAL POS MOMENT= 153.81 KN-MET AT 4000.MM, LOAD 34 |
| REQD STEEL= 662.MM2, ROW=0.0034, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |
| REQD. DEVELOPMENT LENGTH = 566. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

2	644.	4 - 16MM	0.	8000.	YES	YES
---	------	----------	----	-------	-----	-----

STAAD SPACE

-- PAGE NO. 139

```

|-----|
| CRITICAL NEG MOMENT= 175.51 KN-MET AT 8000.MM, LOAD 34 |
| REQD STEEL= 762.MM2, ROW=0.0039, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |
| REQD. DEVELOPMENT LENGTH = 617. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 201146.2 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	473./	740.	109./	169.	34/	33
667.	535./	676.	123./	155.	34/	33
1333.	586./	625.	135./	144.	34/	33
2000.	625./	586.	143./	135.	34/	33
2667.	652./	559.	150./	129.	34/	33

3333.	668./	544.	153./	125.	34/	33
4000.	671./	540.	154./	125.	34/	33
4667.	663./	549.	152./	127.	34/	33
5333.	643./	569.	147./	131.	34/	33
6000.	611./	601.	140./	138.	34/	33
6667.	567./	645.	131./	148.	34/	33
7333.	512./	701.	118./	160.	34/	33
8000.	445./	770.	103./	176.	33/	34

B E A M N O. 2199 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 22.35 KNS Vc= 160.04 KNS Vs= 0.00 KNS  
 Tu= 0.89 KN-MET Tc= 6.8 KN-MET Ts= 0.0 KN-MET LOAD 30

STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 24.18 KNS Vc= 158.46 KNS Vs= 0.00 KNS  
 Tu= 0.89 KN-MET Tc= 6.8 KN-MET Ts= 0.0 KN-MET LOAD 30

STIRRUPS ARE NOT REQUIRED.

STAAD SPACE

-- PAGE NO. 140

1116J		7999X 300X 700			1115J	
=====						
4No16 H 644. 0.TO 8000						
6No12 H 54. 0.TO 8000						
=====						
-----						
oooo	oooo	oooo	oooo	oooo	oooo	oooo
4#16	4#16	4#16	4#16	4#16	4#16	4#16



```

|   |   |   |   |   |   |   |   | | | | | |
|ooooo| |ooooo| |ooooo| |ooooo| |ooooo| |ooooo| |ooooo|
|_____| |_____| |_____| |_____| |_____| |_____| |_____|

```

=====

BEAM NO. 2200 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	61.	3 - 25MM	0.	8000.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 309.51 KN-MET AT 4000.MM, LOAD 34|
| REQD STEEL= 1402.MM2, ROW=0.0073, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 50./ 89. MMS |
| REQD. DEVELOPMENT LENGTH = 1055. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 322220.8 cm<sup>4</sup>

2	642.	5 - 20MM	0.	8000.	YES YES
---	------	----------	----	-------	---------

```

|-----|
| CRITICAL NEG MOMENT= 332.60 KN-MET AT 0.MM, LOAD 33|
| REQD STEEL= 1509.MM2, ROW=0.0078, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 46. MMS |
| REQD. DEVELOPMENT LENGTH = 1314. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 341631.0 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	1152./	1521.	257./	333.	34/	33
STAAD SPACE					-- PAGE NO. 141	
667.	1225./	1445.	272./	317.	34/	33
1333.	1287./	1382.	285./	305.	34/	33
2000.	1336./	1332.	295./	294.	34/	33
2667.	1372./	1296.	303./	287.	34/	33
3333.	1396./	1273.	307./	282.	34/	33
4000.	1406./	1262.	310./	280.	34/	33
4667.	1404./	1265.	309./	281.	34/	33
5333.	1388./	1280.	306./	284.	34/	33
6000.	1360./	1308.	300./	289.	34/	33
6667.	1319./	1349.	292./	298.	34/	33
7333.	1265./	1403.	281./	309.	34/	33
8000.	1199./	1471.	267./	322.	33/	34

B E A M N O. 2200 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT -  $V_u = 94.88$  KNS  $V_c = 154.48$  KNS  $V_s = 0.00$  KNS

$T_u = 3.42$  KN-MET  $T_c = 6.8$  KN-MET  $T_s = 0.0$  KN-MET LOAD 33

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 320. MM C/C FOR 3358. MM

AT END SUPPORT -  $V_u = 86.91$  KNS  $V_c = 155.07$  KNS  $V_s = 0.00$  KNS

$T_u = 3.42$  KN-MET  $T_c = 6.8$  KN-MET  $T_s = 0.0$  KN-MET LOAD 34

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 320. MM C/C FOR 3358. MM

```

    1118J          7999X 300X 700          1117J
|=====|
| 5No20 H 642. | 0.TO 8000 | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 12*10c/c320 | | | | | | | | | | | | | | | | | | | | | |
| 3No25 H |61. | 0.TO 8000 | | | | | | | | | | | | | | | | | |
|=====|
|
|
|

```

ooooo	ooooo	ooooo	ooooo	ooooo	ooooo	ooooo
5#20	5#20	5#20	5#20	5#20	5#20	5#20
3#25	3#25	3#25	3#25	3#25	3#25	3#25
ooo	ooo	ooo	ooo	ooo	ooo	ooo
_____	_____	_____	_____	_____	_____	_____

STAAD SPACE

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=====

BEAM NO. 2201 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 8000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

---

1	58.	4 - 20MM	0.	8000.	YES YES
---	-----	----------	----	-------	---------

|-----|

| CRITICAL POS MOMENT= 256.36 KN-MET AT 4000.MM, LOAD 34 |

```

|  REQD STEEL= 1140.MM2, ROW=0.0059, ROWMX=0.0190 ROWMN=0.0033 |
|  MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |
|  REQD. DEVELOPMENT LENGTH = 985. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 286994.1 cm<sup>4</sup>

```

2      642.      4 - 20MM      0.      8000.      YES YES

```

```

|-----|
|  CRITICAL NEG MOMENT= 260.98 KN-MET AT 8000.MM, LOAD 34 |
|  REQD STEEL= 1162.MM2, ROW=0.0060, ROWMX=0.0190 ROWMN=0.0033 |
|  MAX/MIN/ACTUAL BAR SPACING= 261./ 45./ 61. MMS |
|  REQD. DEVELOPMENT LENGTH = 985. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 286994.1 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION      REINF STEEL (+VE/-VE)      MOMENTS (+VE/-VE)      LOAD (+VE/-VE)
( MM )      (SQ. MM )      (KNS-MET )

```

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	957./ 1103.	216./ 247.	34/ 33
667.	1020./ 1040.	229./ 233.	34/ 33
1333.	1070./ 989.	240./ 223.	34/ 33
2000.	1108./ 951.	248./ 215.	34/ 33
2667.	1134./ 926.	253./ 209.	34/ 33
3333.	1148./ 913.	256./ 207.	34/ 33
4000.	1148./ 913.	256./ 206.	34/ 33
4667.	1136./ 924.	254./ 209.	34/ 33
5333.	1112./ 948.	249./ 214.	34/ 33
6000.	1075./ 985.	241./ 222.	34/ 33
6667.	1026./ 1034.	231./ 232.	34/ 33
7333.	964./ 1095.	218./ 245.	34/ 33
8000.	891./ 1170.	202./ 261.	33/ 34

B E A M N O. 2201 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

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AT START SUPPORT - Vu= 76.02 KNS Vc= 154.55 KNS Vs= 0.00 KNS  
 Tu= 2.11 KN-MET Tc= 6.8 KN-MET Ts= 0.0 KN-MET LOAD 33

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 321. MM C/C FOR 3358. MM

AT END SUPPORT - Vu= 75.25 KNS Vc= 155.21 KNS Vs= 0.00 KNS  
 Tu= 2.11 KN-MET Tc= 6.8 KN-MET Ts= 0.0 KN-MET LOAD 34

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 321. MM C/C FOR 3358. MM

1120J	7999X	300X	700	1119J
=====				
4No20 H 642.	0.TO 8000			
12*10c/c321				12*10c/c321
4No20 H 58.	0.TO 8000			
=====				
-----				
oooo	oooo	oooo	oooo	oooo
4#20	4#20	4#20	4#20	4#20
4#20	4#20	4#20	4#20	4#20
oooo	oooo	oooo	oooo	oooo
_____	_____	_____	_____	_____

=====

BEAM NO. 2323 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 2850. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	56.	4 - 16MM	0.	2850.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 175.69 KN-MET AT 0.MM, LOAD 34|
| REQD STEEL= 763.MM2, ROW=0.0039, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |
| REQD. DEVELOPMENT LENGTH = 617. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 201146.2 cm^4

2	644.	4 - 16MM	0.	2850.	YES YES
---	------	----------	----	-------	---------

STAAD SPACE

-- PAGE NO. 144

```

|-----|
| CRITICAL NEG MOMENT= 182.08 KN-MET AT 2850.MM, LOAD 34|
| REQD STEEL= 792.MM2, ROW=0.0041, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |
| REQD. DEVELOPMENT LENGTH = 617. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 201146.2 cm^4

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	771./	28.	176./	7.	34/	33
237.	733./	82.	167./	19.	34/	33
475.	694./	138.	159./	33.	34/	33
712.	653./	195.	150./	46.	34/	33
950.	611./	255.	140./	60.	34/	33
1187.	568./	316.	131./	74.	34/	33
1425.	523./	379.	121./	88.	34/	33
1662.	478./	444.	111./	103.	34/	33
1900.	431./	511.	100./	118.	34/	33
2137.	383./	580.	89./	134.	34/	33
2375.	334./	651.	78./	149.	34/	33
2612.	283./	725.	66./	166.	34/	33
2850.	232./	800.	54./	182.	33/	34

B E A M N O. 2323 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT -  $V_u = 25.87$  KNS  $V_c = 156.14$  KNS  $V_s = 0.00$  KNS  
 $T_u = 15.92$  KN-MET  $T_c = 6.8$  KN-MET  $T_s = 21.2$  KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 783. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.81 SQ.CM.

AT END SUPPORT -  $V_u = 129.99$  KNS  $V_c = 163.76$  KNS  $V_s = 9.56$  KNS  
 $T_u = 15.92$  KN-MET  $T_c = 6.8$  KN-MET  $T_s = 21.2$  KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR SHEAR AND TORSION.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 783. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 3.81 SQ.CM.

```

_____1103J_____ 2849X 300X 700_____1181J_____
|=====|
| 4No16 H 644.| 0.TO 2850| | | | | | | | |
| | | | | | | | | | |
| 5*10c/c207 | | | | | | 5*10c/c207 | |
| | | | | | | | | | |
| 4No16 H 56.| 0.TO 2850| | | | | |
|=====|
|_____|

```

```

_____
|oooo | |oooo | |oooo | |oooo | |oooo | |oooo | | |
| 4#16| | 4#16| | 4#16| | 4#16| | 4#16| | 4#16|
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| 4#16| | 4#16| | 4#16| | 4#16| | 4#16| | 4#16|
|oooo | |oooo | |oooo | |oooo | |oooo | |oooo |
|_____| |_____| |_____| |_____| |_____| |_____| |_____|

```

=====

BEAM NO. 2324 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 2850. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	56.	4 - 16MM	0.	2850.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 177.35 KN-MET AT 0.MM, LOAD 34|
| REQD STEEL= 770.MM2, ROW=0.0040, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |
| REQD. DEVELOPMENT LENGTH = 617. MMS |

```



|-----|

Cracked Moment of Inertia Iz at above location = 201146.2 cm<sup>4</sup>

2            644.            4 - 16MM            0.            2850.            YES YES

|-----|

| CRITICAL NEG MOMENT= 158.77 KN-MET AT 2850.MM, LOAD 34 |

| REQD STEEL= 686.MM2, ROW=0.0036, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 41./ 63. MMS |

| REQD. DEVELOPMENT LENGTH = 617. MMS |

|-----|

Cracked Moment of Inertia Iz at above location = 201146.2 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	779./ 23.	177./ 5.	34/ 33
237.	745./ 69.	170./ 16.	34/ 33
475.	710./ 117.	162./ 28.	34/ 33
712.	673./ 167.	154./ 39.	34/ 33
950.	635./ 218.	146./ 51.	34/ 33
1187.	596./ 271.	137./ 63.	34/ 33
1425.	556./ 326.	128./ 76.	34/ 33
1662.	514./ 382.	119./ 89.	34/ 33
1900.	471./ 441.	109./ 102.	34/ 33
2137.	426./ 501.	99./ 116.	34/ 33
2375.	381./ 563.	89./ 130.	34/ 33
2612.	334./ 628.	78./ 144.	34/ 33
2850.	286./ 694.	67./ 159.	33/ 34

STAAD SPACE

-- PAGE NO. 146

B E A M N O. 2324 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 66.62 KNS Vc= 156.50 KNS Vs= 0.00 KNS  
 Tu= 19.88 KN-MET Tc= 6.8 KN-MET Ts= 26.5 KN-MET LOAD 32

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 783. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 4.75 SQ.CM.

AT END SUPPORT - Vu= 75.92 KNS Vc= 164.20 KNS Vs= 0.00 KNS  
 Tu= 19.88 KN-MET Tc= 6.8 KN-MET Ts= 26.5 KN-MET LOAD 32

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 207. MM C/C FOR 783. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 4.75 SQ.CM.

\_\_\_\_1105J\_\_\_\_\_ 2849X 300X 700\_\_\_\_\_1182J\_\_\_\_\_

```

|=====|
| 4No16 H 644.| 0.TO 2850| | | | | | | | | |
| | | | | | | | | | | |
| 5*10c/c207 | | | | | | 5*10c/c207 | |
| | | | | | | | | | | |
| 4No16 H 56.| 0.TO 2850| | | | | |
|=====|
|_____|
    
```

```

_____|_____|_____|_____|_____|_____|_____|
|oooo | |oooo | |oooo | |oooo | |oooo | |oooo | | | | |
| 4#16| | 4#16| | 4#16| | 4#16| | 4#16| | 4#16|
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| 4#16| | 4#16| | 4#16| | 4#16| | 4#16| | 4#16|
|oooo | |oooo | |oooo | |oooo | |oooo | |oooo |
|_____| |_____| |_____| |_____| |_____| |_____| |_____|
    
```

=====

BEAM NO. 2432 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	54.	5 - 12MM	0.	4516.	YES NO
---	-----	----------	----	-------	--------

```

|-----|
| CRITICAL POS MOMENT=      82.59 KN-MET AT      0.MM, LOAD  34|
| REQD STEEL=    546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING=  261./  37./  48. MMS          |
| REQD. DEVELOPMENT LENGTH =  453. MMS                      |
|-----|
  
```

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

2	544.	4 - 16MM	0.	5000.	YES YES
---	------	----------	----	-------	---------

```

|-----|
| CRITICAL NEG MOMENT=    145.48 KN-MET AT 5000.MM, LOAD  32|
| REQD STEEL=    753.MM2, ROW=0.0046, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING=  261./  41./  63. MMS          |
| REQD. DEVELOPMENT LENGTH =  617. MMS                      |
|-----|
  
```

Cracked Moment of Inertia Iz at above location = 139329.5 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION      REINF STEEL (+VE/-VE)      MOMENTS (+VE/-VE)      LOAD (+VE/-VE)
  
```

( MM )	(SQ. MM )		(KNS-MET )			
0.	424./	0.	83./	0.	34/	0
417.	402./	14.	78./	3.	34/	33
833.	376./	47.	74./	9.	34/	33
1250.	345./	85.	68./	17.	34/	33
1667.	310./	128.	61./	25.	34/	33
2083.	271./	175.	53./	35.	34/	33
2500.	227./	228.	45./	45.	34/	33
2917.	179./	285.	35./	56.	34/	33
3333.	127./	347.	25./	68.	34/	33
3750.	71./	414.	14./	81.	34/	33
4167.	10./	512.	2./	99.	34/	32
4583.	0./	634.	0./	122.	0/	32
5000.	0./	763.	0./	145.	0/	32

B E A M N O. 2432 D E S I G N R E S U L T S - S H E A R

STAAD SPACE

-- PAGE NO. 148

AT START SUPPORT - Vu= 15.27 KNS Vc= 130.44 KNS Vs= 0.00 KNS  
 Tu= 0.55 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 30

STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 54.77 KNS Vc= 130.40 KNS Vs= 0.00 KNS  
 Tu= 4.70 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 32

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 273. MM C/C FOR 1958. MM

1249J	4999X 300X 600	1105J
=====		
4No16 H 544. 0.TO 5000		
		9*10c/c273

5No12	H	54.	0.TO	4516						
=====										
oooo		oooo		oooo		oooo		oooo		oooo
4#16		4#16		4#16		4#16		4#16		4#16
5#12		5#12		5#12		5#12		5#12		
ooooo		ooooo		ooooo		ooooo		ooooo		

=====

BEAM NO. 2470 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 1500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
-------	----------------	----------	--------------	------------	-------------------	--

1	54.	5 - 12MM	0.	1500.	YES	YES
---	-----	----------	----	-------	-----	-----

-----|

| CRITICAL POS MOMENT= 57.12 KN-MET AT 1500.MM, LOAD 32|

| REQD STEEL= 546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |

| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 48. MMS |

| REQD. DEVELOPMENT LENGTH = 453. MMS |

-----|

Cracked Moment of Inertia Iz at above location = 104738.6 cm^4

2	546.	5 - 12MM	0.	1096.	YES	NO
---	------	----------	----	-------	-----	----

```

|-----|
| CRITICAL NEG MOMENT=    40.12 KN-MET AT    0.MM, LOAD  33|
| REQD STEEL=    546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING=    261./    37./    48. MMS    |
| REQD. DEVELOPMENT LENGTH =    453. MMS                    |
|-----|
    
```

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION      REINF STEEL(+VE/-VE)      MOMENTS(+VE/-VE)      LOAD(+VE/-VE)
( MM )          (SQ. MM )                (KNS-MET )
    
```

SECTION ( MM )	REINF STEEL(+VE/-VE) (SQ. MM )	MOMENTS(+VE/-VE) (KNS-MET )	LOAD(+VE/-VE)
0.	65./ 203.	13./ 40.	34/ 33
125.	82./ 168.	16./ 33.	34/ 33
250.	100./ 133.	20./ 26.	34/ 33
375.	117./ 98.	23./ 20.	34/ 33
500.	134./ 65.	27./ 13.	34/ 33
625.	151./ 31.	30./ 6.	34/ 33
750.	167./ 0.	33./ 0.	34/ 0
875.	183./ 0.	36./ 0.	34/ 0
1000.	199./ 0.	39./ 0.	34/ 0
1125.	214./ 0.	42./ 0.	34/ 0
1250.	230./ 0.	45./ 0.	34/ 0
1375.	255./ 0.	50./ 0.	32/ 0
1500.	291./ 0.	57./ 0.	32/ 0

B E A M N O. 2470 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 38.92 KNS Vc= 134.67 KNS Vs= 0.00 KNS  
 Tu= 0.68 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 30

STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 56.74 KNS Vc= 139.29 KNS Vs= 0.00 KNS

Tu= 0.54 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 32

NO STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 273. MM C/C FOR 208. MM

STAAD SPACE

-- PAGE NO. 150

```
_____1181J_____ 1500X 300X 600_____1252J_____
|
|=====
| 5No12 H 546. 0.TO 1096
|
|                                     2*10c/c273
| 5No12 H 54. 0.TO 1500
|=====
|
|_____
```

```
_____
| | | | | | | | | | | | | |
| ooooo | | ooooo | | ooooo | | ooooo | | ooooo | | | | |
| 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 | | | | |
| | | | | | | | | | | |
| 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 |
| ooooo | | ooooo | | ooooo | | ooooo | | ooooo | | ooooo |
| | | | | | | | | | | |
|_____ | |_____ | |_____ | |_____ | |_____ | |_____ |
```

=====

BEAM NO. 2471 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 5000. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL HEIGHT BAR INFO FROM TO ANCHOR

(MM) (MM) (MM) STA END

```

1      54.      5 - 12MM      0.      5000.      YES YES
|-----|
| CRITICAL POS MOMENT=      89.13 KN-MET AT      0.MM, LOAD 34 |
| REQD STEEL=      546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING=      261./      37./      48. MMS      |
| REQD. DEVELOPMENT LENGTH =      453. MMS      |
|-----|

```

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

```

2      546.      5 - 12MM      0.      5000.      YES YES
|-----|
| CRITICAL NEG MOMENT=      55.93 KN-MET AT 5000.MM, LOAD 34 |
| REQD STEEL=      546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING=      261./      37./      48. MMS      |
| REQD. DEVELOPMENT LENGTH =      453. MMS      |
|-----|

```

Cracked Moment of Inertia Iz at above location = 104738.6 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	458./ 30.	89./ 6.	34/ 33
417.	446./ 26.	87./ 5.	34/ 33
833.	430./ 27.	84./ 5.	34/ 33
1250.	409./ 32.	80./ 6.	34/ 33
1667.	384./ 42.	75./ 8.	34/ 33
2083.	353./ 56.	69./ 11.	34/ 33

STAAD SPACE



2500.	319./	75.	63./	15.	34/	33
2917.	280./	98.	55./	20.	34/	33
3333.	237./	126.	47./	25.	34/	33
3750.	189./	159.	37./	31.	34/	33
4167.	138./	196.	27./	39.	34/	33
4583.	82./	238.	16./	47.	34/	33
5000.	22./	284.	4./	56.	33/	34

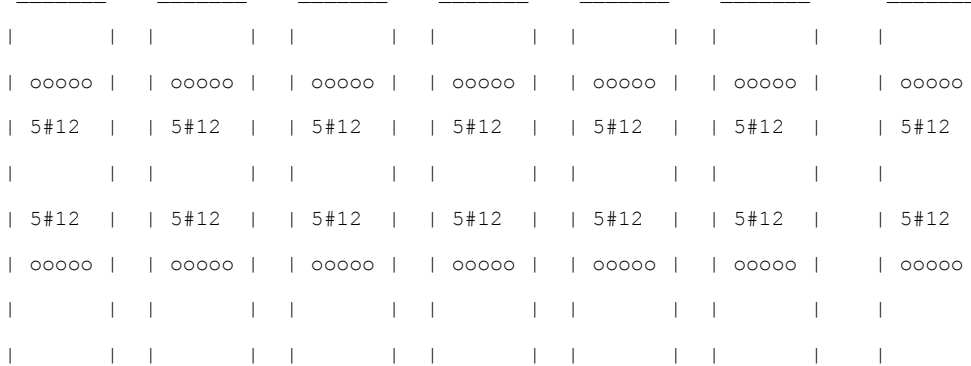
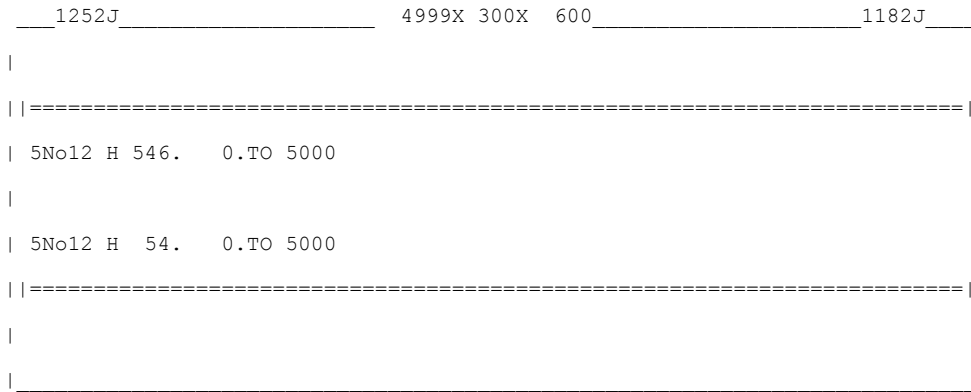
B E A M N O. 2471 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 2.01 KNS Vc= 128.22 KNS Vs= 0.00 KNS  
 Tu= 1.23 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 30

STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 25.26 KNS Vc= 137.10 KNS Vs= 0.00 KNS  
 Tu= 1.23 KN-MET Tc= 5.6 KN-MET Ts= 0.0 KN-MET LOAD 30

STIRRUPS ARE NOT REQUIRED.



=====

BEAM NO. 2472 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 2850. MM FY - 414. FC - 25. MPA, SIZE - 300. X 600. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	54.	5 - 12MM	0.	2850.	YES YES
---	-----	----------	----	-------	---------

STAAD SPACE

-- PAGE NO. 152

```

|-----|
| CRITICAL POS MOMENT= 16.40 KN-MET AT 950.MM, LOAD 34|
| REQD STEEL= 546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 48. MMS |
| REQD. DEVELOPMENT LENGTH = 453. MMS |
|-----|

```

Cracked Moment of Inertia I<sub>z</sub> at above location = 104738.6 cm<sup>4</sup>

2	546.	5 - 12MM	0.	2850.	YES YES
---	------	----------	----	-------	---------

```

|-----|
| CRITICAL NEG MOMENT= 12.21 KN-MET AT 2850.MM, LOAD 34|
| REQD STEEL= 546.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 48. MMS |
| REQD. DEVELOPMENT LENGTH = 453. MMS |
|-----|

```

Cracked Moment of Inertia I<sub>z</sub> at above location = 104738.6 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

-----

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )		MOMENTS (+VE/-VE) (KNS-MET )		LOAD (+VE/-VE)	
0.	69./	18.	14./	4.	34/	33
237.	74./	13.	15./	3.	34/	33
475.	78./	10.	16./	2.	34/	33
712.	81./	9.	16./	2.	34/	33
950.	82./	9.	16./	2.	34/	33
1187.	82./	10.	16./	2.	34/	33
1425.	81./	13.	16./	3.	34/	33
1662.	77./	18.	15./	4.	34/	33
1900.	73./	23.	15./	5.	34/	33
2137.	67./	31.	13./	6.	34/	33
2375.	60./	39.	12./	8.	34/	33
2612.	51./	50.	10./	10.	34/	33
2850.	41./	61.	8./	12.	33/	34

B E A M N O. 2472 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT -  $V_u = 8.41$  KNS  $V_c = 134.67$  KNS  $V_s = 0.00$  KNS  
 $T_u = 6.53$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 8.7$  KN-MET LOAD 33

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 883. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.64 SQ.CM.

AT END SUPPORT -  $V_u = 13.40$  KNS  $V_c = 134.67$  KNS  $V_s = 0.00$  KNS  
 $T_u = 6.53$  KN-MET  $T_c = 5.6$  KN-MET  $T_s = 8.7$  KN-MET LOAD 34

STIRRUPS ARE REQUIRED FOR TORSION.

REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.

PROVIDE 10 MM 2-LEGGED STIRRUPS AT 182. MM C/C FOR 883. MM

ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.64 SQ.CM.

\_\_\_\_\_1249J\_\_\_\_\_ 2849X 300X 600\_\_\_\_\_1252J\_\_\_\_\_

```

|
|=====|
| 5No12|H 54. 0.TO 2850 | | | | | | | |
| 6*10c/c182 | | | | | | | 6*10c/c182 | |
| 5No12|H 54. 0.TO 2850 | | | | | | | |
|=====|
|
|_____|

```

```

| | | | | | | | | | | | | |
| ooooo | | ooooo | | ooooo | | ooooo | | ooooo | | ooooo |
| 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 |
| | | | | | | | | | | | | |
| 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 | | 5#12 |
| ooooo | | ooooo | | ooooo | | ooooo | | ooooo | | ooooo |
| | | | | | | | | | | | | |
|_____| |_____| |_____| |_____| |_____| |_____| |_____|

```

=====

BEAM NO. 2524 DESIGN RESULTS - FLEXURE PER CODE ACI 318-05

LEN - 6500. MM FY - 414. FC - 25. MPA, SIZE - 300. X 700. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	54.	6 - 12MM	0.	6500.	YES YES
---	-----	----------	----	-------	---------

```

|-----|
| CRITICAL POS MOMENT= 115.30 KN-MET AT 4875.MM, LOAD 34|
| REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |

```

```

| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |
| REQD. DEVELOPMENT LENGTH = 566. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

```

2      646.      6 - 12MM      0.      6500.      YES YES
|-----|
| CRITICAL NEG MOMENT= 145.94 KN-MET AT 0.MM, LOAD 33 |
| REQD STEEL= 646.MM2, ROW=0.0033, ROWMX=0.0190 ROWMN=0.0033 |
| MAX/MIN/ACTUAL BAR SPACING= 261./ 37./ 38. MMS |
| REQD. DEVELOPMENT LENGTH = 566. MMS |
|-----|

```

Cracked Moment of Inertia Iz at above location = 175566.9 cm<sup>4</sup>

REQUIRED REINF. STEEL SUMMARY :

```

-----
SECTION      REINF STEEL (+VE/-VE)      MOMENTS (+VE/-VE)      LOAD (+VE/-VE)
( MM )      (SQ. MM )      (KNS-MET )

```

SECTION ( MM )	REINF STEEL (+VE/-VE) (SQ. MM )	MOMENTS (+VE/-VE) (KNS-MET )	LOAD (+VE/-VE)
0.	162./ 636.	38./ 146.	34/ 33
542.	229./ 553.	54./ 128.	34/ 33
1083.	289./ 480.	67./ 111.	34/ 33
1625.	341./ 414.	80./ 96.	34/ 33
2167.	386./ 357.	90./ 83.	34/ 33
2708.	424./ 307.	98./ 72.	34/ 33
3250.	454./ 265.	105./ 62.	34/ 33
3792.	477./ 231.	110./ 54.	34/ 33
4333.	491./ 205.	114./ 48.	34/ 33
4875.	499./ 186.	115./ 44.	34/ 33
5417.	498./ 175.	115./ 41.	34/ 33
5958.	490./ 171.	113./ 40.	34/ 33
6500.	474./ 174.	110./ 41.	33/ 34

STAAD SPACE

-- PAGE NO. 154



\*\*\*\*\*END OF BEAM DESIGN\*\*\*\*\*

2095. DESIGN COLUMN 2137 TO 2163 2306 2307 2433 2435 2523

STAAD SPACE

-- PAGE NO. 155

=====

COLUMN NO. 2137 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----

28 - 16 MM 1.083 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	734.27	17.53	0.01038

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)	
13051.65	10441.32	5230.10	1633.12	312.3	
M0	P-tens.	Des.Pn	Des.Mn	e/h	
824.17	-2328.96	734.27	14.32	0.00848	
-----					
		Pn	Mn	Pn	Mn (@ Z )
		9638.15	828.21	4819.07	1307.51
P0	*	8834.97	962.17	4015.89	1281.31
	*	8031.79	1071.09	3212.72	1222.65
Pn,max	__*	7228.61	1159.29	2409.54	1132.41
	*	6425.43	1229.16	1606.36	1010.05
Pn	*	5622.25	1283.60	803.18	850.73
NOMINAL	*	Pn	Mn	Pn	Mn (@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93
Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76
	*	7228.61	1444.09	2409.54	1410.64
	__*	6425.43	1532.75	1606.36	1258.98
	* M0 Mn,	5622.25	1604.04	803.18	1065.43
	* BENDING				
P-tens	* MOMENT				

=====

COLUMN NO. 2138 DESIGN PER ACI 318-05 - AXIAL + BENDING

STAAD SPACE

-- PAGE NO. 156

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----



28 - 16 MM                      1.083              30              END              0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER    10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5139.01  1310.81  255.1

M0          P-tens.    Des.Pn    Des.Mn    e/h
656.99   -2328.96  355.96   39.39   0.04811
-----

```

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5230.10  1633.12  312.3

M0          P-tens.    Des.Pn    Des.Mn    e/h
824.17   -2328.96  355.96   22.92   0.02799
-----

```

```

-----
                Pn      Mn      Pn      Mn      (@ Z )
                |
                |      9638.15  828.21  4819.07  1307.51
P0 | *
                | *
                |      8031.79  1071.09  3212.72  1222.65
Pn,max|__*
                | *
                |      7228.61  1159.29  2409.54  1132.41
                | *
                |      6425.43  1229.16  1606.36  1010.05
Pn      | *
                |      5622.25  1283.60  803.18   850.73
NOMINAL| *
                |      Pn      Mn      Pn      Mn      (@ Y )
AXIAL  | *
                |      9638.15  1030.09  4819.07  1629.04
COMPRESSION| *
                |      8834.97  1196.79  4015.89  1595.93
                |      Pb|-----*Mb
                |      8031.79  1333.11  3212.72  1522.76
                | *
                |      7228.61  1444.09  2409.54  1410.64
                | *
                |      6425.43  1532.75  1606.36  1258.98
                | * M0  Mn,
                | * BENDING
                |      P-tens|*
                |      MOMENT
                |
-----

```

=====

COLUMN NO. 2139 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

STAAD SPACE

-- PAGE NO. 157

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----

28 - 16 MM 1.083 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	679.71	17.99	0.01150

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	679.71	37.34	0.02389

-----

Pn Mn Pn Mn (@ Z )

			9638.15	828.21	4819.07	1307.51
P0	*		8834.97	962.17	4015.89	1281.31
		*	8031.79	1071.09	3212.72	1222.65
Pn,max	__*		7228.61	1159.29	2409.54	1132.41
		*	6425.43	1229.16	1606.36	1010.05
Pn		*	5622.25	1283.60	803.18	850.73
NOMINAL		*	Pn	Mn	Pn	Mn (@ Y )
AXIAL		*	9638.15	1030.09	4819.07	1629.04
COMPRESSION		*	8834.97	1196.79	4015.89	1595.93
	Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76
		*	7228.61	1444.09	2409.54	1410.64
		*_____	6425.43	1532.75	1606.36	1258.98
		* M0 Mn,	5622.25	1604.04	803.18	1065.43
		* BENDING				
P-tens	*	MOMENT				

=====

COLUMN NO. 2140 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
-----				
28 - 16 MM	1.083	30	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

STAAD SPACE

-- PAGE NO. 158

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5139.01  1310.81  255.1
M0          P-tens.    Des.Pn    Des.Mn    e/h
656.99   -2328.96  305.46   40.21   0.05723
-----

```

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5230.10  1633.12  312.3
M0          P-tens.    Des.Pn    Des.Mn    e/h
824.17   -2328.96  305.46   27.94   0.03977
-----

```

```

-----
                Pn          Mn          Pn          Mn          (@ Z )
                |          |          |          |
                |          |          |          |
P0 | *          |          |          |          |
                |          |          |          |
                | *          |          |          |
Pn,max|__*      |          |          |          |
                | *          |          |          |
Pn    | *          |          |          |          |
NOMINAL| *          |          |          |          |
                |          |          |          |
AXIAL| *          |          |          |          |
COMPRESSION| *          |          |          |          |
                |          |          |          |
Pb|-----*Mb   |          |          |          |
                | *          |          |          | |
                | *          |          |          |
                | * MO Mn, |          |          |          |
                | *          |          |          |          |
                | *          |          |          |          |
                | *          |          |          |          |
                | *          |          |          |          |
P-tens|*          |          |          |          |
                |          |          |          |
                |          |          |          |
-----

```

=====

COLUMN NO. 2141 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI  
-----

28 - 16 MM 1.083 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----  
P0 Pn max P-bal. M-bal. e-bal. (MM)  
13051.65 10441.32 5139.01 1310.81 255.1  
M0 P-tens. Des.Pn Des.Mn e/h  
656.99 -2328.96 592.46 1.56 0.00114  
-----

STAAD SPACE

-- PAGE NO. 159

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----  
P0 Pn max P-bal. M-bal. e-bal. (MM)  
13051.65 10441.32 5230.10 1633.12 312.3  
M0 P-tens. Des.Pn Des.Mn e/h  
824.17 -2328.96 592.46 21.56 0.01582  
-----

-----  
Pn Mn Pn Mn (@ Z )  
| 9638.15 828.21 4819.07 1307.51  
P0 | \* 8834.97 962.17 4015.89 1281.31  
| \* 8031.79 1071.09 3212.72 1222.65  
Pn,max | \* 7228.61 1159.29 2409.54 1132.41  
| \* 6425.43 1229.16 1606.36 1010.05  
Pn | \* 5622.25 1283.60 803.18 850.73  
-----

NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04	
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93	
Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76	
	*	7228.61	1444.09	2409.54	1410.64	
	*	6425.43	1532.75	1606.36	1258.98	
	* M0 Mn,	5622.25	1604.04	803.18	1065.43	
	* BENDING					
P-tens	* MOMENT					

=====

COLUMN NO. 2142 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

AREA OF STEEL REQUIRED = 6292.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
8 - 32 MM	1.237	34	STA	0.650

-----

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13367.56	10694.05	4991.22	1402.60	281.0
M0	P-tens.	Des.Pn	Des.Mn	e/h
742.39	-2661.63	255.03	228.70	0.38990

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
      P0          Pn max      P-bal.      M-bal.      e-bal. (MM)
13367.56  10694.05  5091.20  1752.95  344.3

      M0          P-tens.      Des.Pn      Des.Mn      e/h
935.70   -2661.63  255.03  806.74  1.37534
-----

```

STAAD SPACE

-- PAGE NO. 160

```

              Pn          Mn          Pn          Mn      (@ Z )
      |
      |          9871.43  853.66  4935.71  1401.35
      |
      | *          9048.81  990.93  4113.09  1366.49
      |
      | *          8226.19  1108.51  3290.48  1309.19
      |
      | *          7403.57  1205.24  2467.86  1235.71
      |
      | *          6580.95  1284.53  1645.24  1125.80
      |
      | *          5758.33  1350.16  822.62   947.72
      |
      | *          Pn          Mn          Pn          Mn      (@ Y )
      |
      | *          9871.43  1060.00  4935.71  1749.03
      |
      | *          9048.81  1237.22  4113.09  1706.12
      |
      | *          Pb|-----*Mb  8226.19  1384.54  3290.48  1635.58
      |
      | *          7403.57  1506.92  2467.86  1545.17
      |
      | *          6580.95  1608.16  1645.24  1411.79
      |
      | * M0   Mn,  5758.33  1692.91  822.62  1194.05
      |
      | *      BENDING
      |
      | *      P-tens|*      MOMENT
      |

```

```

=====
COLUMN NO. 2143 DESIGN PER ACI 318-05 - AXIAL + BENDING

```

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

AREA OF STEEL REQUIRED = 5564.0 SQ. MM

```

BAR CONFIGURATION      REINF PCT.      LOAD      LOCATION      PHI
-----

```

28 - 16 MM                      1.083              34              STA              0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER    10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5139.01  1310.81  255.1

M0          P-tens.    Des.Pn    Des.Mn    e/h
656.99   -2328.96    3.73     451.50  52.61004
-----

```

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5230.10  1633.12  312.3

M0          P-tens.    Des.Pn    Des.Mn    e/h
824.17   -2328.96    3.73     341.07  39.74303
-----

```

STAAD SPACE

-- PAGE NO. 161

```

          Pn      Mn      Pn      Mn      (@ Z )
          |
          |      9638.15  828.21  4819.07  1307.51
P0 | *
          |      8834.97  962.17  4015.89  1281.31
          | *
          |      8031.79  1071.09  3212.72  1222.65
Pn,max|__*
          |      7228.61  1159.29  2409.54  1132.41
          | *
          |      6425.43  1229.16  1606.36  1010.05
Pn   | *
          |      5622.25  1283.60  803.18   850.73
NOMINAL| *
          |      Pn      Mn      Pn      Mn      (@ Y )
          |
          |      9638.15  1030.09  4819.07  1629.04
          | *
          |      8834.97  1196.79  4015.89  1595.93
          |
          |      8031.79  1333.11  3212.72  1522.76
          |-----*Mb
          |      7228.61  1444.09  2409.54  1410.64
          |
          |      6425.43  1532.75  1606.36  1258.98
          |-----*
          | * MO   Mn, 5622.25  1604.04  803.18  1065.43
          | * BENDING

```



P-tens|\*      MOMENT

|

=====

COLUMN NO. 2144 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

AREA OF STEEL REQUIRED = 5564.0 SQ. MM

BAR CONFIGURATION      REINF PCT.      LOAD      LOCATION      PHI

-----

28 - 16 MM                      1.083              34              STA              0.900

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER      10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	-9.57	466.28	-21.18749

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	-9.57	328.78	-14.93973

-----

STAAD SPACE

-- PAGE NO. 162

		Pn	Mn	Pn	Mn	(@ Z )
		9638.15	828.21	4819.07	1307.51	
P0	*	8834.97	962.17	4015.89	1281.31	
	*	8031.79	1071.09	3212.72	1222.65	
Pn,max	**	7228.61	1159.29	2409.54	1132.41	
	*	6425.43	1229.16	1606.36	1010.05	
Pn	*	5622.25	1283.60	803.18	850.73	
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04	
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93	
Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76	
	*	7228.61	1444.09	2409.54	1410.64	
	* _____	6425.43	1532.75	1606.36	1258.98	
	* M0 Mn,	5622.25	1604.04	803.18	1065.43	
	* BENDING					
P-tens	* MOMENT					

=====

COLUMN NO. 2145 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
28 - 16 MM	1.083	30	END	0.650

-----

28 - 16 MM 1.083 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	519.30	3.78	0.00316

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	519.30	40.91	0.03425

STAAD SPACE

-- PAGE NO. 163

	Pn	Mn	Pn	Mn	(@ Z )	
	9638.15	828.21	4819.07	1307.51		
P0   *	8834.97	962.17	4015.89	1281.31		
*	8031.79	1071.09	3212.72	1222.65		
Pn,max __*	7228.61	1159.29	2409.54	1132.41		
*	6425.43	1229.16	1606.36	1010.05		
Pn   *	5622.25	1283.60	803.18	850.73		
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04	
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93	
Pb -----*Mb		8031.79	1333.11	3212.72	1522.76	
*		7228.61	1444.09	2409.54	1410.64	
_____   ____*		6425.43	1532.75	1606.36	1258.98	
* M0 Mn,		5622.25	1604.04	803.18	1065.43	
*	BENDING					
P-tens *	MOMENT					

=====

COLUMN NO. 2146 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----

28 - 16 MM 1.083 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	709.45	9.90	0.00607

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	709.45	46.00	0.02819

-----

STAAD SPACE

-- PAGE NO. 164

	Pn	Mn	Pn	Mn	(@ Z )
	9638.15	828.21	4819.07	1307.51	
P0   *	8834.97	962.17	4015.89	1281.31	
*	8031.79	1071.09	3212.72	1222.65	
Pn,max __*	7228.61	1159.29	2409.54	1132.41	
*	6425.43	1229.16	1606.36	1010.05	

Pn		*	5622.25	1283.60	803.18	850.73
NOMINAL		*	Pn	Mn	Pn	Mn (@ Y )
AXIAL		*	9638.15	1030.09	4819.07	1629.04
COMPRESSION		*	8834.97	1196.79	4015.89	1595.93
Pb		-----*Mb	8031.79	1333.11	3212.72	1522.76
		*	7228.61	1444.09	2409.54	1410.64
		*	6425.43	1532.75	1606.36	1258.98
		* M0 Mn,	5622.25	1604.04	803.18	1065.43
		* BENDING				
P-tens		* MOMENT				

=====

COLUMN NO. 2147 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
28 - 16 MM	1.083	30	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	363.34	9.18	0.01098

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5230.10  1633.12  312.3

M0          P-tens.    Des.Pn    Des.Mn    e/h
824.17   -2328.96  363.34   45.72    0.05472
-----

```

STAAD SPACE

-- PAGE NO. 165

```

          Pn      Mn      Pn      Mn      (@ Z )
          |
          |      9638.15  828.21  4819.07  1307.51
P0 | *
          |      8834.97  962.17  4015.89  1281.31
          | *
          |      8031.79  1071.09  3212.72  1222.65
Pn,max|__*
          | *
          |      6425.43  1229.16  1606.36  1010.05
Pn   | *
NOMINAL| *      Pn      Mn      Pn      Mn      (@ Y )
AXIAL| *
COMPRESSION| *
          |      8834.97  1196.79  4015.89  1595.93
          |      Pb|-----*Mb
          |      8031.79  1333.11  3212.72  1522.76
          | *
          |      7228.61  1444.09  2409.54  1410.64
          | *
          |      6425.43  1532.75  1606.36  1258.98
          | * MO Mn,
          | * BENDING
          |      5622.25  1604.04  803.18  1065.43
          | *
          |      P-tens|*
          |      MOMENT
          |

```

=====

COLUMN NO. 2148 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION          REINF PCT.      LOAD      LOCATION      PHI  
-----  
28 - 16 MM                    1.083            30            END            0.650  
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)  
TIE BAR NUMBER    10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----  
P0            Pn max      P-bal.      M-bal.      e-bal. (MM)  
13051.65    10441.32    5139.01    1310.81    255.1  
M0            P-tens.      Des.Pn      Des.Mn      e/h  
656.99    -2328.96    631.03      13.34      0.00919  
-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----  
P0            Pn max      P-bal.      M-bal.      e-bal. (MM)  
13051.65    10441.32    5230.10    1633.12    312.3  
M0            P-tens.      Des.Pn      Des.Mn      e/h  
824.17    -2328.96    631.03      56.94      0.03923  
-----

STAAD SPACE

-- PAGE NO. 166

		Pn	Mn	Pn	Mn	(@ Z )
		9638.15	828.21	4819.07	1307.51	
P0	*	8834.97	962.17	4015.89	1281.31	
	*	8031.79	1071.09	3212.72	1222.65	
Pn,max	**	7228.61	1159.29	2409.54	1132.41	
	*	6425.43	1229.16	1606.36	1010.05	
Pn	*	5622.25	1283.60	803.18	850.73	
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04	
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93	
	Pb -----*Mb	8031.79	1333.11	3212.72	1522.76	
	*	7228.61	1444.09	2409.54	1410.64	

	*	6425.43	1532.75	1606.36	1258.98
	* M0 Mn,	5622.25	1604.04	803.18	1065.43
	* BENDING				
P-tens *	MOMENT				

=====

COLUMN NO. 2149 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED  
 ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
28 - 16 MM	1.083	30	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	676.82	21.12	0.01357

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h



824.17 -2328.96 676.82 29.08 0.01868

-----  
STAAD SPACE

-- PAGE NO. 167

	Pn	Mn	Pn	Mn	(@ Z )	
	9638.15	828.21	4819.07	1307.51		
P0   *	8834.97	962.17	4015.89	1281.31		
*	8031.79	1071.09	3212.72	1222.65		
Pn,max __*	7228.61	1159.29	2409.54	1132.41		
*	6425.43	1229.16	1606.36	1010.05		
Pn   *	5622.25	1283.60	803.18	850.73		
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04	
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93	
Pb -----*Mb		8031.79	1333.11	3212.72	1522.76	
*		7228.61	1444.09	2409.54	1410.64	
----- _*-----		6425.43	1532.75	1606.36	1258.98	
* M0 Mn,		5622.25	1604.04	803.18	1065.43	
* BENDING						
P-tens * MOMENT						

=====

COLUMN NO. 2150 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----

28 - 16 MM 1.083 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65   10441.32   5139.01   1310.81   255.1

M0          P-tens.    Des.Pn    Des.Mn    e/h
656.99    -2328.96   511.88    19.42     0.01649
-----

```

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65   10441.32   5230.10   1633.12   312.3

M0          P-tens.    Des.Pn    Des.Mn    e/h
824.17    -2328.96   511.88    19.47     0.01654
-----

```

STAAD SPACE

-- PAGE NO. 168

```

          Pn      Mn      Pn      Mn      (@ Z )
          |
          |      9638.15  828.21  4819.07  1307.51
P0 | *
          |      8834.97  962.17  4015.89  1281.31
          | *
          |      8031.79  1071.09  3212.72  1222.65
Pn,max|__*
          |      7228.61  1159.29  2409.54  1132.41
          | *
          |      6425.43  1229.16  1606.36  1010.05
Pn   | *
NOMINAL| *
          |      Pn      Mn      Pn      Mn      (@ Y )
          |
          |      9638.15  1030.09  4819.07  1629.04
AXIAL| *
COMPRESSION| *
          |      8834.97  1196.79  4015.89  1595.93
          |
          |      Pb|-----*Mb  8031.79  1333.11  3212.72  1522.76
          |      *
          |      7228.61  1444.09  2409.54  1410.64
          |
          |      *
          |      6425.43  1532.75  1606.36  1258.98
          |
          | * M0 Mn, 5622.25  1604.04  803.18  1065.43
          | * BENDING
          |
          |
P-tens| * MOMENT
          |
          |

```

=====

COLUMN NO. 2151 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED  
 ONLY MINIMUM STEEL IS REQUIRED.  
 AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
28 - 16 MM	1.083	30	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)  
 TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	735.34	10.26	0.00607

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	735.34	20.89	0.01235

STAAD SPACE

-- PAGE NO. 169

	Pn	Mn	Pn	Mn	(@ Z )
	9638.15	828.21	4819.07	1307.51	

P0	*	8834.97	962.17	4015.89	1281.31
	*	8031.79	1071.09	3212.72	1222.65
Pn,max	__*	7228.61	1159.29	2409.54	1132.41
	*	6425.43	1229.16	1606.36	1010.05
Pn	*	5622.25	1283.60	803.18	850.73
NOMINAL	*	Pn	Mn	Pn	Mn (@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93
Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76
	*	7228.61	1444.09	2409.54	1410.64
	__*	6425.43	1532.75	1606.36	1258.98
	* M0 Mn,	5622.25	1604.04	803.18	1065.43
	* BENDING				
P-tens	* MOMENT				

=====

COLUMN NO. 2152 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
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-----

28 - 16 MM	1.083	30	END	0.650
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(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1

M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	692.43	10.39	0.00652

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3

M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	692.43	22.71	0.01426

-----

STAAD SPACE

-- PAGE NO. 170

		Pn	Mn	Pn	Mn	(@ Z )
		9638.15	828.21	4819.07	1307.51	
P0	*	8834.97	962.17	4015.89	1281.31	
	*	8031.79	1071.09	3212.72	1222.65	
Pn,max	**	7228.61	1159.29	2409.54	1132.41	
	*	6425.43	1229.16	1606.36	1010.05	
Pn	*	5622.25	1283.60	803.18	850.73	
NOMINAL	*					(@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04	
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93	
Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76	
	*	7228.61	1444.09	2409.54	1410.64	
	*	6425.43	1532.75	1606.36	1258.98	
	* M0 Mn,	5622.25	1604.04	803.18	1065.43	
	* BENDING					
P-tens	* MOMENT					

=====

COLUMN NO. 2153 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI  
 -----  
 28 - 16 MM 1.083 30 END 0.650  
 (PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)  
 TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----  
 P0 Pn max P-bal. M-bal. e-bal. (MM)  
 13051.65 10441.32 5139.01 1310.81 255.1  
 M0 P-tens. Des.Pn Des.Mn e/h  
 656.99 -2328.96 651.32 11.69 0.00780  
 -----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----  
 P0 Pn max P-bal. M-bal. e-bal. (MM)  
 13051.65 10441.32 5230.10 1633.12 312.3  
 M0 P-tens. Des.Pn Des.Mn e/h  
 824.17 -2328.96 651.32 82.96 0.05538  
 -----

STAAD SPACE

-- PAGE NO. 171

	Pn	Mn	Pn	Mn	(@ Z )
	9638.15	828.21	4819.07	1307.51	
P0   *	8834.97	962.17	4015.89	1281.31	
*	8031.79	1071.09	3212.72	1222.65	
Pn,max __*	7228.61	1159.29	2409.54	1132.41	
*	6425.43	1229.16	1606.36	1010.05	
Pn   *	5622.25	1283.60	803.18	850.73	
NOMINAL  *	Pn	Mn	Pn	Mn	(@ Y )

AXIAL	*	9638.15	1030.09	4819.07	1629.04
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93
Pb -----*	Mb	8031.79	1333.11	3212.72	1522.76
	*	7228.61	1444.09	2409.54	1410.64
-----	*	6425.43	1532.75	1606.36	1258.98
	* M0 Mn,	5622.25	1604.04	803.18	1065.43
	* BENDING				
P-tens *	MOMENT				

=====

COLUMN NO. 2154 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
-------------------	------------	------	----------	-----

-----

28 - 16 MM	1.083	30	END	0.650
------------	-------	----	-----	-------

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	698.41	10.82	0.00674

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5230.10  1633.12  312.3

M0          P-tens.    Des.Pn    Des.Mn    e/h
824.17   -2328.96  698.41   15.99    0.00996
-----

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STAAD SPACE

-- PAGE NO. 172

```

          Pn      Mn      Pn      Mn      (@ Z )
          |
          | 9638.15  828.21  4819.07  1307.51
P0 | *
          | 8834.97  962.17  4015.89  1281.31
          | *
          | 8031.79  1071.09  3212.72  1222.65
Pn,max|_*
          | *
          | 7228.61  1159.29  2409.54  1132.41
          | *
          | 6425.43  1229.16  1606.36  1010.05
Pn | *
          | 5622.25  1283.60  803.18   850.73
NOMINAL| *
          | Pn      Mn      Pn      Mn      (@ Y )
          |
          | 9638.15  1030.09  4819.07  1629.04
AXIAL| *
COMPRESSION| *
          | 8834.97  1196.79  4015.89  1595.93
          |
          | Pb|-----*Mb 8031.79  1333.11  3212.72  1522.76
          | *
          | 7228.61  1444.09  2409.54  1410.64
          | *
          | 6425.43  1532.75  1606.36  1258.98
          |
          | * M0 Mn, 5622.25  1604.04  803.18  1065.43
          | * BENDING
          |
          | P-tens|* MOMENT
          |

```

=====

COLUMN NO. 2155 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED  
 ONLY MINIMUM STEEL IS REQUIRED.  
 AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----



28 - 16 MM                    1.083            30            END            0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER    10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

```

-----
      P0          Pn max      P-bal.      M-bal.      e-bal. (MM)
13051.65  10441.32  5139.01  1310.81  255.1
      M0          P-tens.     Des.Pn     Des.Mn     e/h
656.99   -2328.96  599.03   11.54   0.00838
-----

```

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
      P0          Pn max      P-bal.      M-bal.      e-bal. (MM)
13051.65  10441.32  5230.10  1633.12  312.3
      M0          P-tens.     Des.Pn     Des.Mn     e/h
824.17   -2328.96  599.03   66.32   0.04813
-----

```

STAAD SPACE

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```

          Pn      Mn      Pn      Mn      (@ Z )
          |
          |      9638.15  828.21  4819.07  1307.51
P0 | *      8834.97  962.17  4015.89  1281.31
          | *      8031.79  1071.09  3212.72  1222.65
Pn,max|__*      7228.61  1159.29  2409.54  1132.41
          | *      6425.43  1229.16  1606.36  1010.05
Pn   | *      5622.25  1283.60  803.18   850.73
NOMINAL| *      Pn      Mn      Pn      Mn      (@ Y )
AXIAL| *      9638.15  1030.09  4819.07  1629.04
COMPRESSION| *      8834.97  1196.79  4015.89  1595.93
Pb|-----*Mb  8031.79  1333.11  3212.72  1522.76
          | *      7228.61  1444.09  2409.54  1410.64
          |__*_____ 6425.43  1532.75  1606.36  1258.98
          | * M0 Mn, 5622.25  1604.04  803.18  1065.43

```

| \* BENDING  
P-tens|\* MOMENT  
|

=====

COLUMN NO. 2156 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED  
ONLY MINIMUM STEEL IS REQUIRED.  
AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----

28 - 16 MM 1.083 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	493.32	2.04	0.00180

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	493.32	1.35	0.00119

-----

		Pn	Mn	Pn	Mn	(@ Z )
		9638.15	828.21	4819.07	1307.51	
P0	*	8834.97	962.17	4015.89	1281.31	
	*	8031.79	1071.09	3212.72	1222.65	
Pn,max	**	7228.61	1159.29	2409.54	1132.41	
	*	6425.43	1229.16	1606.36	1010.05	
Pn	*	5622.25	1283.60	803.18	850.73	
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04	
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93	
Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76	
	*	7228.61	1444.09	2409.54	1410.64	
	*-----	6425.43	1532.75	1606.36	1258.98	
	* M0 Mn,	5622.25	1604.04	803.18	1065.43	
	* BENDING					
P-tens	* MOMENT					

=====

COLUMN NO. 2157 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
28 - 16 MM	1.083	30	END	0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)				
TIE BAR NUMBER	10	SPACING	256.00	MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5139.01  1310.81  255.1

M0          P-tens.    Des.Pn    Des.Mn    e/h
656.99   -2328.96  344.14   9.15    0.01155
-----

```

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5230.10  1633.12  312.3

M0          P-tens.    Des.Pn    Des.Mn    e/h
824.17   -2328.96  344.14   20.88   0.02638
-----

```

STAAD SPACE

-- PAGE NO. 175

```

          Pn      Mn      Pn      Mn      (@ Z )
          |
          9638.15  828.21  4819.07  1307.51
P0 | *
          8834.97  962.17  4015.89  1281.31
          | *
          8031.79  1071.09  3212.72  1222.65
Pn,max|___*
          7228.61  1159.29  2409.54  1132.41
          | *
          6425.43  1229.16  1606.36  1010.05
Pn | *
          5622.25  1283.60  803.18   850.73
NOMINAL| *
          Pn      Mn      Pn      Mn      (@ Y )
AXIAL| *
          9638.15  1030.09  4819.07  1629.04
COMPRESSION| *
          8834.97  1196.79  4015.89  1595.93
          Pb|-----*Mb
          8031.79  1333.11  3212.72  1522.76
          | *
          7228.61  1444.09  2409.54  1410.64
          |___*
          6425.43  1532.75  1606.36  1258.98
          | * M0 Mn,
          5622.25  1604.04  803.18   1065.43
          | * BENDING
          P-tens|*
          MOMENT
          |

```

=====

COLUMN NO. 2158 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----  
 28 - 16 MM 1.083 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----  

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	390.85	11.85	0.01318

 -----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----  

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	390.85	22.99	0.02557

 -----

STAAD SPACE

-- PAGE NO. 176

	Pn	Mn	Pn	Mn	(@ Z )
	9638.15	828.21	4819.07	1307.51	
P0   *	8834.97	962.17	4015.89	1281.31	
*	8031.79	1071.09	3212.72	1222.65	

Pn,max	__*	7228.61	1159.29	2409.54	1132.41
	*	6425.43	1229.16	1606.36	1010.05
Pn	*	5622.25	1283.60	803.18	850.73
NOMINAL	*	Pn	Mn	Pn	Mn (@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93
Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76
	*	7228.61	1444.09	2409.54	1410.64
_____	*_____	6425.43	1532.75	1606.36	1258.98
	* M0 Mn,	5622.25	1604.04	803.18	1065.43
	* BENDING				
P-tens	* MOMENT				

=====

COLUMN NO. 2159 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
-------------------	------------	------	----------	-----

-----

28 - 16 MM	1.083	30	END	0.650
------------	-------	----	-----	-------

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	412.86	3.76	0.00396

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	412.86	18.00	0.01896

-----

STAAD SPACE

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		Pn	Mn	Pn	Mn	(@ Z )
		9638.15	828.21	4819.07	1307.51	
P0	*	8834.97	962.17	4015.89	1281.31	
	*	8031.79	1071.09	3212.72	1222.65	
Pn,max	**	7228.61	1159.29	2409.54	1132.41	
	*	6425.43	1229.16	1606.36	1010.05	
Pn	*	5622.25	1283.60	803.18	850.73	
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04	
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93	
Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76	
	*	7228.61	1444.09	2409.54	1410.64	
	**	6425.43	1532.75	1606.36	1258.98	
	* M0 Mn,	5622.25	1604.04	803.18	1065.43	
	* BENDING					
P-tens	* MOMENT					

=====

COLUMN NO. 2160 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

AREA OF STEEL REQUIRED = 6292.0 SQ. MM

BAR CONFIGURATION            REINF PCT.    LOAD    LOCATION    PHI  
 -----  
 8 - 32 MM                    1.237        34        STA        0.650  
 (PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)  
 TIE BAR NUMBER    10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----  
 P0            Pn max    P-bal.    M-bal.    e-bal. (MM)  
 13367.56    10694.05    4991.22    1402.60    281.0  
 M0            P-tens.    Des.Pn    Des.Mn    e/h  
 742.39    -2661.63    3.19    371.12    50.57240  
 -----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----  
 P0            Pn max    P-bal.    M-bal.    e-bal. (MM)  
 13367.56    10694.05    5091.20    1752.95    344.3  
 M0            P-tens.    Des.Pn    Des.Mn    e/h  
 935.70    -2661.63    3.19    540.67    73.67786  
 -----

STAAD SPACE

-- PAGE NO. 178

		Pn	Mn	Pn	Mn	(@ Z )
		9871.43	853.66	4935.71	1401.35	
P0	*	9048.81	990.93	4113.09	1366.49	
	*	8226.19	1108.51	3290.48	1309.19	
Pn,max	__*	7403.57	1205.24	2467.86	1235.71	
	*	6580.95	1284.53	1645.24	1125.80	
Pn	*	5758.33	1350.16	822.62	947.72	
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	9871.43	1060.00	4935.71	1749.03	
COMPRESSION	*	9048.81	1237.22	4113.09	1706.12	
	Pb -----*Mb	8226.19	1384.54	3290.48	1635.58	



		*		7403.57	1506.92	2467.86	1545.17
-----		*	-----	6580.95	1608.16	1645.24	1411.79
		*	M0 Mn,	5758.33	1692.91	822.62	1194.05
		*	BENDING				
P-tens	*		MOMENT				

=====

COLUMN NO. 2161 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED  
 ONLY MINIMUM STEEL IS REQUIRED.  
 AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
28 - 16 MM	1.083	30	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)  
 TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	131.94	24.82	0.08178

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3

M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	131.94	31.47	0.10371

-----

STAAD SPACE

-- PAGE NO. 179

		Pn	Mn	Pn	Mn	(@ Z )
		9638.15	828.21	4819.07	1307.51	
P0	*	8834.97	962.17	4015.89	1281.31	
	*	8031.79	1071.09	3212.72	1222.65	
Pn,max	__*	7228.61	1159.29	2409.54	1132.41	
	*	6425.43	1229.16	1606.36	1010.05	
Pn	*	5622.25	1283.60	803.18	850.73	
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	9638.15	1030.09	4819.07	1629.04	
COMPRESSION	*	8834.97	1196.79	4015.89	1595.93	
Pb	-----*Mb	8031.79	1333.11	3212.72	1522.76	
	*	7228.61	1444.09	2409.54	1410.64	
	__*_____	6425.43	1532.75	1606.36	1258.98	
	* M0 Mn,	5622.25	1604.04	803.18	1065.43	
	* BENDING					
P-tens	* MOMENT					

=====

COLUMN NO. 2162 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
-------------------	------------	------	----------	-----

-----

28 - 16 MM	1.083	30	END	0.650
------------	-------	----	-----	-------

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5139.01  1310.81  255.1

M0          P-tens.    Des.Pn    Des.Mn    e/h
656.99   -2328.96  301.12   15.85    0.02289
-----

```

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
P0          Pn max    P-bal.    M-bal.    e-bal. (MM)
13051.65  10441.32  5230.10  1633.12  312.3

M0          P-tens.    Des.Pn    Des.Mn    e/h
824.17   -2328.96  301.12   18.49    0.02670
-----

```

STAAD SPACE

-- PAGE NO. 180

```

          Pn      Mn      Pn      Mn      (@ Z )
          |
          |      9638.15  828.21  4819.07  1307.51
P0 | *
          | *
          |      8031.79  1071.09  3212.72  1222.65
Pn,max|__*
          | *
          |      6425.43  1229.16  1606.36  1010.05
Pn   | *
NOMINAL| *
          |      Pn      Mn      Pn      Mn      (@ Y )
          |      *
          |      9638.15  1030.09  4819.07  1629.04
AXIAL| *
COMPRESSION| *
          |      8834.97  1196.79  4015.89  1595.93
          |      Pb|-----*Mb
          |      8031.79  1333.11  3212.72  1522.76
          |      *
          |      7228.61  1444.09  2409.54  1410.64
          |      *
          |      6425.43  1532.75  1606.36  1258.98
          |      * M0 Mn,
          |      5622.25  1604.04  803.18  1065.43
          |      *
          |      BENDING
          |      P-tens|*
          |      MOMENT
          |

```

=====

COLUMN NO. 2163 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 5200.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----

28 - 16 MM 1.083 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 256.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5139.01	1310.81	255.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
656.99	-2328.96	248.91	2.42	0.00422

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
13051.65	10441.32	5230.10	1633.12	312.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
824.17	-2328.96	248.91	81.08	0.14163

-----

STAAD SPACE

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Pn Mn Pn Mn (@ Z )

			9638.15	828.21	4819.07	1307.51
P0	*		8834.97	962.17	4015.89	1281.31
	*		8031.79	1071.09	3212.72	1222.65
Pn,max	__*		7228.61	1159.29	2409.54	1132.41
	*		6425.43	1229.16	1606.36	1010.05
Pn	*		5622.25	1283.60	803.18	850.73
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*		9638.15	1030.09	4819.07	1629.04
COMPRESSION	*		8834.97	1196.79	4015.89	1595.93
	Pb -----*Mb		8031.79	1333.11	3212.72	1522.76
	*		7228.61	1444.09	2409.54	1410.64
	__*_____		6425.43	1532.75	1606.36	1258.98
	* M0 Mn,		5622.25	1604.04	803.18	1065.43
	* BENDING					
P-tens	* MOMENT					

=====

COLUMN NO. 2306 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 650.0 X 500.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 3250.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
-----				
32 - 12 MM	1.114	30	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 192.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
----	--------	--------	--------	-------------

8196.75	6557.40	3137.33	625.43	199.4
M0	P-tens.	Des.Pn	Des.Mn	e/h
316.93	-1497.16	258.93	3.86	0.00648

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
8196.75	6557.40	3225.91	829.21	257.0
M0	P-tens.	Des.Pn	Des.Mn	e/h
422.86	-1497.16	258.93	28.57	0.04797

-----

STAAD SPACE

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		Pn	Mn	Pn	Mn	(@ Z )
		6052.99	396.28	3026.49	624.45	
P0	*	5548.57	458.62	2522.08	612.47	
	*	5044.16	510.22	2017.66	585.45	
Pn,max	__*	4539.74	551.77	1513.25	542.28	
	*	4035.32	584.37	1008.83	482.85	
Pn	*	3530.91	609.64	504.42	406.68	
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	6052.99	521.77	3026.49	827.60	
COMPRESSION	*	5548.57	606.17	2522.08	811.47	
Pb	-----*Mb	5044.16	674.96	2017.66	775.60	
	*	4539.74	730.68	1513.25	718.53	
	* _____	4035.32	775.35	1008.83	642.18	
	* M0 Mn,	3530.91	811.14	504.42	542.96	
	* BENDING					
P-tens	* MOMENT					

=====

COLUMN NO. 2307 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 650.0 X 500.0 MMS, TIED  
 ONLY MINIMUM STEEL IS REQUIRED.  
 AREA OF STEEL REQUIRED = 3250.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI  
 -----

32 - 12 MM 1.114 30 END 0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 192.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----  
 P0 Pn max P-bal. M-bal. e-bal. (MM)  
 8196.75 6557.40 3137.33 625.43 199.4  
 M0 P-tens. Des.Pn Des.Mn e/h  
 316.93 -1497.16 212.60 8.87 0.01813  
 -----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----  
 P0 Pn max P-bal. M-bal. e-bal. (MM)  
 8196.75 6557.40 3225.91 829.21 257.0  
 M0 P-tens. Des.Pn Des.Mn e/h  
 422.86 -1497.16 212.60 36.11 0.07384  
 -----

STAAD SPACE

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	Pn	Mn	Pn	Mn	(@ Z )
	6052.99	396.28	3026.49	624.45	
P0   *	5548.57	458.62	2522.08	612.47	
*	5044.16	510.22	2017.66	585.45	
Pn,max   ___*	4539.74	551.77	1513.25	542.28	
*	4035.32	584.37	1008.83	482.85	
Pn   *	3530.91	609.64	504.42	406.68	

NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	6052.99	521.77	3026.49	827.60	
COMPRESSION	*	5548.57	606.17	2522.08	811.47	
Pb	-----*Mb	5044.16	674.96	2017.66	775.60	
	*	4539.74	730.68	1513.25	718.53	
	*	4035.32	775.35	1008.83	642.18	
	* M0 Mn,	3530.91	811.14	504.42	542.96	
	* BENDING					
P-tens	* MOMENT					

=====

COLUMN NO. 2433 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, SQRE SIZE - 500.0 X 500.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 2500.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
-------------------	------------	------	----------	-----

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8 - 20 MM	1.005	30	END	0.650
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(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 10 SPACING 320.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
6198.88	4959.10	2361.37	476.52	201.8
M0	P-tens.	Des.Pn	Des.Mn	e/h
221.29	-1039.71	14.09	3.57	0.11029

-----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)



P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
6198.88	4959.10	2361.37	476.52	201.8
M0	P-tens.	Des.Pn	Des.Mn	e/h
221.29	-1039.71	14.09	0.71	0.02184

STAAD SPACE

-- PAGE NO. 184

	Pn	Mn	Pn	Mn	(@ Z )
	4577.63	301.28	2288.82	474.90	
P0   *	4196.16	348.74	1907.35	459.98	
*	3814.70	388.04	1525.88	437.01	
Pn,max _*	3433.23	419.63	1144.41	407.95	
*	3051.76	444.44	762.94	359.24	
Pn   *	2670.29	463.80	381.47	294.91	
NOMINAL  *	Pn	Mn	Pn	Mn	(@ Y )
AXIAL  *	4577.63	301.28	2288.82	474.90	
COMPRESSION  *	4196.16	348.74	1907.35	459.98	
Pb -----*Mb	3814.70	388.04	1525.88	437.01	
*	3433.23	419.63	1144.41	407.95	
----- _*-----	3051.76	444.44	762.94	359.24	
* M0 Mn,	2670.29	463.80	381.47	294.91	
* BENDING					
P-tens * MOMENT					

=====

COLUMN NO. 2435 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, SQRE SIZE - 500.0 X 500.0 MMS, TIED

ONLY MINIMUM STEEL IS REQUIRED.

AREA OF STEEL REQUIRED = 2500.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

-----  
 8 - 20 MM                    1.005            30            END            0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER    10 SPACING 320.00 MM

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

-----  

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
6198.88	4959.10	2361.37	476.52	201.8
M0	P-tens.	Des.Pn	Des.Mn	e/h
221.29	-1039.71	5.45	0.76	0.06073

  
 -----

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

-----  

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
6198.88	4959.10	2361.37	476.52	201.8
M0	P-tens.	Des.Pn	Des.Mn	e/h
221.29	-1039.71	5.45	1.33	0.10634

  
 -----

STAAD SPACE

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		Pn	Mn	Pn	Mn	(@ Z )
		4577.63	301.28	2288.82	474.90	
P0	*	4196.16	348.74	1907.35	459.98	
	*	3814.70	388.04	1525.88	437.01	
Pn,max	__*	3433.23	419.63	1144.41	407.95	
	*	3051.76	444.44	762.94	359.24	
Pn	*	2670.29	463.80	381.47	294.91	
NOMINAL	*	Pn	Mn	Pn	Mn	(@ Y )
AXIAL	*	4577.63	301.28	2288.82	474.90	
COMPRESSION	*	4196.16	348.74	1907.35	459.98	
	Pb -----*Mb	3814.70	388.04	1525.88	437.01	
	*	3433.23	419.63	1144.41	407.95	
	_*	3051.76	444.44	762.94	359.24	

```

      | * M0 Mn, 2670.29 463.80 381.47 294.91
      | * BENDING
P-tens|* MOMENT
      |

```

=====

COLUMN NO. 2523 DESIGN PER ACI 318-05 - AXIAL + BENDING

FY - 413.7 FC - 24.5 MPA, RECT SIZE - 800.0 X 650.0 MMS, TIED  
 ONLY MINIMUM STEEL IS REQUIRED.  
 AREA OF STEEL REQUIRED = 5200.0 SQ. MM

```

BAR CONFIGURATION      REINF PCT.  LOAD  LOCATION  PHI
-----
28 - 16 MM             1.083     30    END        0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 10 SPACING 256.00 MM

```

COLUMN INTERACTION: MOMENT ABOUT Z -AXIS (KN-MET)

```

-----
P0      Pn max  P-bal.  M-bal.  e-bal. (MM)
13051.65 10441.32 5139.01 1310.81 255.1
      M0      P-tens.  Des.Pn  Des.Mn  e/h
656.99 -2328.96 152.00  1.18   0.00337
-----

```

COLUMN INTERACTION: MOMENT ABOUT Y -AXIS (KN-MET)

```

-----
P0      Pn max  P-bal.  M-bal.  e-bal. (MM)
13051.65 10441.32 5230.10 1633.12 312.3
      M0      P-tens.  Des.Pn  Des.Mn  e/h
824.17 -2328.96 152.00  49.44  0.14143
-----

```

	Pn	Mn	Pn	Mn	(@ Z )
	9638.15	828.21	4819.07	1307.51	
P0   *	8834.97	962.17	4015.89	1281.31	
*	8031.79	1071.09	3212.72	1222.65	
Pn,max __*	7228.61	1159.29	2409.54	1132.41	
*	6425.43	1229.16	1606.36	1010.05	
Pn   *	5622.25	1283.60	803.18	850.73	
NOMINAL  *	Pn	Mn	Pn	Mn	(@ Y )
AXIAL  *	9638.15	1030.09	4819.07	1629.04	
COMPRESSION  *	8834.97	1196.79	4015.89	1595.93	
Pb -----*Mb	8031.79	1333.11	3212.72	1522.76	
*	7228.61	1444.09	2409.54	1410.64	
_____   ____*	6425.43	1532.75	1606.36	1258.98	
* MO Mn,	5622.25	1604.04	803.18	1065.43	
* BENDING					
P-tens * MOMENT					

\*\*\*\*\*END OF COLUMN DESIGN RESULTS\*\*\*\*\*

2096. CONCRETE TAKE

2097. END CONCRETE DESIGN

\*\*\*\*\* CONCRETE TAKE OFF \*\*\*\*\*

(FOR BEAMS AND COLUMNS DESIGNED ABOVE)

TOTAL VOLUME OF CONCRETE = 2958.63 CU.FT

BAR SIZE	WEIGHT
NUMBER	(in lbs)
-----	-----

10	2207.94
12	2193.29
16	6605.57
20	3099.81
25	2299.44
32	2063.62
40	347.23

-----

\*\*\* TOTAL= 18816.90

2098. LOAD LIST 40 TO 48

2099. PARAMETER 1

2100. CODE LRFD

2101. BEAM 1 MEMB 482 TO 506 509 512 515 518 521 524 527 530 TO 589 591 TO 631 633 -

2102. 634 TO 694 696 TO 703 705 TO 708 710 TO 723 725 TO 738 740 TO 753 755 TO 762 -

2103. 877 879 881 883 885 887 889 TO 891 1009 1011 1013 1015 1017 1019 -

2104. 1021 TO 1023 1027 TO 1035 1051 TO 1053 1055 1056 1061 TO 1065 1067 1070 1073 -

2105. 1074 TO 1081 1083 1085 TO 1087 1089 TO 1107 1109 TO 1114 1165 1174 1183 1253 -

2106. 1438 TO 1456 1465 TO 1484 1493 TO 1525 1528 1531 1533 1543 1544 1584 1594 -

2107. 1597 1599 1600 1605 TO 1611 1613 1615 TO 1621 1623 1625 TO 1631 1633 1639 -

2108. 1640 TO 1646 1651 TO 1669 1685 1686 1689 TO 1727 1729 TO 1772 1777 1778 1781 -

2109. 1782 TO 1784 1787 1788 1793 1794 1797 TO 1800 1803 TO 1849 1851 TO 1853 1855 -

2110. 1856 TO 1865 1869 1870 1872 1873 1879 1880 1883 TO 1924 1926 TO 1945 1981 -

2111. 1982 TO 1990 1995 1996 2005 TO 2024 2029 2030 2037 TO 2055 2059 2061 TO 2067 -

2112. 2069 2070 2072 2073 2075 2076 2078 2079 2081 TO 2087 2089 TO 2101 2103 2106 -

2113. 2107 TO 2111 2113 TO 2121 2124 TO 2127 2129 2131 TO 2135 2229 TO 2260 2269 -

2114. 2270 TO 2284 2290 TO 2293 2295 TO 2298 2301 TO 2305 2310 2311 2313 TO 2315 -

2115. 2317 TO 2319 2321 2322 2326 TO 2340 2342 TO 2374 2376 TO 2378 2381 2383 2385 -

2116. 2387 2388 2390 TO 2394 2397 TO 2431 2445 TO 2469 2473 TO 2480 2485 TO 2500 -

2117. 2509 TO 2522 2532 TO 2534 2540 TO 2543 2546 2547 2549 TO 2551 2554 TO 2557 -

2118. 2559

2119. BEAM 1 MEMB 2723 TO 2726

2120. BEAM 1 MEMB 2685 TO 2718

2121. BEAM 1 MEMB 2664 TO 2667 2671 2675 TO 2678

2122. BEAM 1 MEMB 2604 TO 2608 2610 TO 2612 2615 2617 TO 2623 2625 TO 2627 2631 -

2123. 2633 TO 2638 2643 TO 2646 2655 2656 2659 2660

2124. BEAM 1 MEMB 2561 2562 2564 2566 2568 2582 TO 2585 2588 TO 2590 2593 2600 2603

2125. BEAM 1 MEMB 2569 TO 2575

2126. KY 1 MEMB 482 TO 484 488 TO 492 494 TO 496 500 TO 504 530 531 534 TO 539 543 -  
2127. 545 547 549 551 553 555 556 558 TO 563 567 568 578 TO 583 587 588 -  
2128. 591 TO 595 599 600 614 TO 619 623 624 634 636 TO 641 645 646 648 TO 653 657 -  
2129. 658 660 TO 665 669 670 672 TO 677 681 682 684 TO 689 693 694 734 735 737 -  
2130. 738 740 741 743 TO 748 752 753 755 756 877 879 881 883 885 887 1009 1011 -  
2131. 1013 1015 1017 1019 1253 1528 1531 1533 1883 TO 1902 2059 2061 2062 2064 -  
2132. 2090 2094 2121 2290 TO 2293 2295 TO 2298 2301 TO 2305 2411 2412 2550 2551 -  
2133. 2582 2590 2604 2605 2607 2611 2615 2617 2619 2620 2622 2623 2625 2627  
2134. KZ 1.9 MEMB 482 TO 484 488 TO 492 494 TO 496 500 TO 504 530 531 534 TO 539 -  
2135. 543 545 547 549 551 553 555 556 558 TO 563 567 568 578 TO 583 587 588 591 -  
2136. 592 TO 595 599 600 614 TO 619 623 624 634 636 TO 641 645 646 648 TO 653 657 -  
2137. 658 660 TO 665 669 670 672 TO 677 681 682 684 TO 689 693 694 734 735 737 -  
2138. 738 740 741 743 TO 748 752 753 755 756 877 879 881 883 885 887 1009 1011 -  
2139. 1013 1015 1017 1019 1253 1528 1531 1533 1883 TO 1902 2059 2061 2062 2064 -  
2140. 2090 2094 2121 2290 TO 2293 2295 TO 2298 2301 TO 2305 2411 2412 2550 2551 -  
2141. 2582 2590 2604 2605 2607 2611 2615 2617 2619 2620 2622 2623 2625 2627  
2142. UNB 2.7 MEMB 626 TO 631 633 635 1584 2063 2065 2069 2072 2075 2078 2081 2103 -  
2143. 2556 2557 2561  
2144. UNT 2.7 MEMB 626 TO 631 633 635 1584 2063 2065 2069 2072 2075 2078 2081 2103 -  
2145. 2556 2557 2561  
2146. UNB 5 MEMB 570 TO 577 726 731 1106 1107 1109 1111 1112 1502 1503 1505 1506 -  
2147. 1508 1509 2124 2634 2635 2637 2664 2665 2693 2694  
2148. UNT 5 MEMB 570 TO 577 726 731 1106 1107 1109 1111 1112 1502 1503 1505 1506 -  
2149. 1508 1509 2124 2634 2635 2637 2664 2665 2693 2694  
2150. UNB 5.5 MEMB 532 533 602 604 TO 607 609 611 613 698 699 702 703 707 708 712 -  
2151. 713 717 718 722 723 727 728 732 733 1090 1095 1100 1438 1439 1445 1446 1450 -  
2152. 1451 1520 1521 1523 1524 1543 1594 2119 2120 2547 2666 2667 2675 TO 2678 -  
2153. 2685 2686 2688 2689 2696 2697

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2154. UNT 5.5 MEMB 532 533 602 604 TO 607 609 611 613 698 699 702 703 707 708 712 -  
2155. 713 717 718 722 723 727 728 732 733 1090 1095 1100 1438 1439 1445 1446 1450 -  
2156. 1451 1520 1521 1523 1524 1543 1594 2119 2120 2547 2666 2667 2675 TO 2678 -  
2157. 2685 2686 2688 2689 2696 2697  
2158. UNB 6.5 MEMB 603 608 610 758 759 761 762 889 TO 891 1022 1023 1027 1030 1544 -  
2159. 1597 1600 1981 1982 1987 1988 2086 2087 2095 2097 2326 TO 2329 2332 2336 -  
2160. 2337 2349 2350 2369 2370 2383 2392 2399 TO 2401 2414 2415 2509 2512 TO 2514 -  
2161. 2519 2520 2530 2549 2671 2705 2707 TO 2711

2162. UNT 6.5 MEMB 603 608 610 758 759 761 762 889 TO 891 1022 1023 1027 1030 1981 -  
2163. 1982 1987 1988 2326 TO 2330 2332 2334 2336 2337 2349 2350 2369 2370 2383 -  
2164. 2392 2400 2401 2415 2509 2512 TO 2514 2519 2520 2530 2549 2671 2705 2707 -  
2165. 2708 TO 2711  
2166. UNB 10 MEMB 697 701 706 711 716 721 1605 1606 1608 1609 1611 1613 1615 1616 -  
2167. 1618 1619 1621 1623 1625 1626 1628 1629 1631 1633 2526 TO 2529  
2168. UNT 10 MEMB 697 701 706 711 716 721 1605 1606 1608 1609 1611 1613 1615 1616 -  
2169. 1618 1619 1621 1623 1625 1626 1628 1629 1631 1633 2526 TO 2529  
2170. UNB 8.2 MEMB 696 700 705 710 715 720 1697 1698 1703 1704 1706 1707 1731 1732 -  
2171. 1734 1735 1737 1739 1745 1746 1748 1749 1751 1753  
2172. UNT 8.2 MEMB 696 700 705 710 715 720 1697 1698 1703 1704 1706 1707 1731 1732 -  
2173. 1734 1735 1737 1739 1745 1746 1748 1749 1751 1753  
2174. UNB 11 MEMB 544 546 548 550 552 554 1031 1032 1441 1443 1453 1455 1465 1466 -  
2175. 1468 1469 1472 1473 1475 1476 1478 1479 1481 1482 2037 TO 2052 2229 TO 2236  
2176. UNT 11 MEMB 544 546 548 550 552 554 1031 1032 1441 1443 1453 1455 1465 1466 -  
2177. 1468 1469 1472 1473 1475 1476 1478 1479 1481 1482 2037 TO 2052 2229 TO 2236  
2178. UNB 11.6 MEMB 714 719 736 1694 1695 1700 1701 1709 1710 1712 1714 1718 1720 -  
2179. 1812 1813 1815 1816 1820 1821 1823 1824 1826 1827 1829 1830 1832 1833 1835 -  
2180. 1836 1838 1839 1841 1842 1844 1845 1847 1848 1869 1870 1879 2540 2541  
2181. UNT 11.6 MEMB 714 719 736 1694 1695 1700 1701 1709 1710 1712 1714 1718 1720 -  
2182. 1812 1813 1815 1816 1820 1821 1823 1824 1826 1827 1829 1830 1832 1833 1835 -  
2183. 1836 1838 1839 1841 1842 1844 1845 1847 1848 1869 1870 1879 2540 2541  
2184. UNB 5.3 MEMB 729 1493 1494 1496 1497 1499 1500 1880 2542 2543  
2185. UNT 5.3 MEMB 729 1493 1494 1496 1497 1499 1500 1880 2542 2543  
2186. UNB 4.1 MEMB 725 730 1511 1512 1514 1515 1517 1518  
2187. UNT 4.1 MEMB 725 730 1511 1512 1514 1515 1517 1518  
2188. UNB 8 MEMB 485 TO 487 493 497 TO 499 505 506 509 512 515 518 521 524 527 540 -  
2189. 541 TO 542 557 564 TO 566 569 585 586 589 596 TO 598 620 TO 622 625 -  
2190. 642 TO 644 647 654 TO 656 659 666 TO 668 671 678 TO 680 683 691 692 750 751 -  
2191. 1011 1033 1034 1051 1074 1077 1080 1083 1086 1105 1447 1471 1484 1498 1507 -  
2192. 1516 1667 TO 1669 1685 1689 1696 1699 1702 1705 1708 1903 TO 1924 2053 2092 -  
2193. 2093 TO 2094 2107 2115 2135 2305 2554 2555 2570 TO 2575 2584 2588 2593 2606 -  
2194. 2608 2612 2618 2621 2626 2631 2643 TO 2646 2655 2656 2659 2660 2687 2690 -  
2195. 2691 TO 2692 2695 2698 TO 2704 2706 2712 TO 2718  
2196. UNT 8 MEMB 485 TO 487 493 497 TO 499 505 506 509 512 515 518 521 524 527 540 -  
2197. 541 TO 542 555 TO 557 564 TO 566 569 585 586 589 596 TO 598 620 TO 622 625 -  
2198. 642 TO 644 647 654 TO 656 659 666 TO 668 671 678 TO 680 683 691 692 750 751 -  
2199. 1011 1033 1034 1051 1074 1077 1080 1083 1086 1105 1447 1471 1484 1498 1507 -

2200. 1516 1522 1525 1667 TO 1669 1685 1689 1696 1699 1702 1705 1708 1903 TO 1924 -  
2201. 2053 2092 TO 2094 2107 2115 2135 2305 2554 2555 2570 TO 2575 2584 2588 2593 -  
2202. 2606 2608 2612 2618 2621 2626 2631 2643 TO 2646 2655 2656 2659 2660 2687 -  
2203. 2690 TO 2692 2695 2698 TO 2704 2706 2712 TO 2718  
2204. UNB 4 MEMB 1440 1452 1495 1501 1504 1510 1513 1519 1995 1996 2005 2006 2019 -  
2205. 2020 2029 2030  
2206. UNT 4 MEMB 1440 1452 1495 1501 1504 1510 1513 1519 1995 1996 2005 2006 2019 -  
2207. 2020 2029 2030  
2208. UNB 2.85 MEMB 2311 2314 2317 2319 2321 2331  
2209. UNT 2.85 MEMB 2314 2317 2319 2321 2331

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2210. CHECK CODE MEMB 482 TO 506 509 512 515 518 521 524 527 530 TO 589 591 TO 631 -  
2211. 633 TO 694 696 TO 703 705 TO 708 710 TO 723 725 TO 738 740 TO 753 -  
2212. 755 TO 762 877 879 881 883 885 887 889 TO 891 1009 1011 1013 1015 1017 1019 -  
2213. 1021 TO 1023 1027 TO 1035 1051 TO 1053 1055 1056 1061 TO 1065 1067 1070 1073 -  
2214. 1074 TO 1081 1083 1085 TO 1087 1089 TO 1107 1109 TO 1114 1165 1174 1183 1253 -  
2215. 1438 TO 1456 1465 TO 1484 1493 TO 1525 1528 1531 1533 1543 1544 1584 1594 -  
2216. 1597 1599 1600 1605 TO 1611 1613 1615 TO 1621 1623 1625 TO 1631 1633 1639 -  
2217. 1640 TO 1646 1651 TO 1669 1685 1686 1689 TO 1727 1729 TO 1772 1777 1778 1781 -  
2218. 1782 TO 1784 1787 1788 1793 1794 1797 TO 1800 1803 TO 1849 1851 TO 1853 1855 -  
2219. 1856 TO 1865 1869 1870 1872 1873 1879 1880 1883 TO 1924 1926 TO 1945 1981 -  
2220. 1982 TO 1990 1995 1996 2005 TO 2024 2029 2030 2037 TO 2055 2059 2061 TO 2067 -  
2221. 2069 2070 2072 2073 2075 2076 2078 2079 2081 TO 2087 2089 TO 2101 2103 2106 -  
2222. 2107 TO 2111 2113 TO 2121 2124 TO 2127 2129 2131 TO 2135 2229 TO 2260 2269 -  
2223. 2270 TO 2293 2295 TO 2298 2301 TO 2305 2310 2311 2313 TO 2315 2317 TO 2319 -  
2224. 2321 2322 2326 TO 2340 2342 TO 2374 2376 TO 2378 2381 2383 2385 2387 2388 -  
2225. 2390 TO 2394 2397 TO 2431 2445 TO 2469 2473 TO 2480 2485 TO 2500 -  
2226. 2509 TO 2522 2526 TO 2547 2549 TO 2551 2554 TO 2557 2559 2561 2562 2564 2566 -  
2227. 2568 TO 2585 2588

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STAAD.Pro CODE CHECKING - (LRFD 3RD EDITION)

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
--------	-------	---------------	----------------------	--------------	----------------------

=====

482	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.802	44
		233.26 C	-56.72	-265.46	0.00
483	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.372	44
		535.87 C	-38.11	-48.96	1.38
484	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.423	43
		176.00 C	-50.14	-72.95	1.50
485	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.523	43
		6.33 C	9.31	75.06	0.00
486	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.676	43
		9.88 T	12.85	95.76	0.00
487	ST	W14X68	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.809	42
		33.22 C	2.44	-258.31	4.67
488	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.671	44
		112.73 C	-70.50	-149.91	0.00
489	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.397	44
		237.10 C	41.34	82.07	3.00
490	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.713	42
		198.07 C	115.67	-10.16	2.50
491	ST	W18X86	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.526	42
		60.97	C	-87.96	-5.17	0.00
492	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.640	42
		81.47	C	106.17	-10.70	0.00
493	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.556	43
		25.32	C	17.83	56.74	0.00
494	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.884	43
		587.60	C	48.80	279.10	0.00
495	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.440	44
		520.84	C	-45.69	-66.91	1.38
496	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.452	44
		415.34	C	50.90	65.34	1.50
497	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.592	43
		11.90	C	9.30	87.04	0.00

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498	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.658	43
		9.05	T	12.92	92.25	0.00
499	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.792	43
		32.86	C	15.08	107.04	0.00

500	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.699	44
			90.88 C	-83.97	-121.57	0.00
501	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.381	44
			200.60 C	40.48	78.42	3.00
502	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.465	44
			116.01 C	60.45	66.87	2.50
503	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.289	41
			32.64 C	-46.25	10.59	0.00
504	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.519	43
			90.28 C	-41.40	-181.56	1.50
505	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.425	43
			16.60 C	17.60	34.01	0.00
506	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.823	43
			2.72 T	11.71	226.49	0.00
509	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.952	43
			5.33 T	16.55	256.17	0.00
512	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.852	43
			12.89 T	15.76	121.82	0.00
515	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.654	43
			4.70 C	9.72	98.99	0.00
518	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.620	43
			0.40 T	13.64	83.65	0.00
521	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.577	43
			5.41 T	16.13	69.09	0.00
524	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.957	43

52.32 C            18.35            133.10            0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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527	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.604	43
		15.56 C	18.54	67.64	0.00
530	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.642	44
		561.78 C	8.77	291.22	4.20
531	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.601	44
		521.37 C	-11.93	-286.38	3.85
532	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.050	44
		79.43 C	-0.10	-5.66	2.52
533	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.046	44
		75.29 C	-0.15	-5.03	2.52
534	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.461	44
		435.60 C	37.29	119.76	3.00
535	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.540	43
		91.20 C	-47.12	-172.69	2.50
536	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.372	44
		154.84 C	52.83	29.41	2.50
537	ST	W18X86	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.485	44
		467.00 C		37.49	131.90	3.00
538	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.467	44
		281.68 C		29.37	172.69	2.50
539	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.475	44
		177.39 C		52.41	98.61	2.50
540	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.690	44
		4.83 T		0.30	228.72	8.00
541	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.651	43
		2.90 C		0.19	122.44	0.00
542	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.994	43
		313.13 T		-17.41	-269.02	2.50
543	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.413	43
		138.66 C		45.14	88.89	0.00
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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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544	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.058	44
		59.30 C		-1.04	-3.85	2.52
545	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.353	44
		63.68 C		-41.67	-69.87	0.00

546	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.052	43
		70.55	T	-1.13	-2.93	2.75
547	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.521	44
		275.83	C	-44.85	-151.04	0.92
548	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.040	41
		72.33	C	-0.05	-2.54	2.29
549	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.810	42
		281.81	C	-128.41	-18.76	1.00
550	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.055	42
		108.42	C	0.07	-3.13	2.75
551	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.493	44
		515.83	C	-24.63	-186.88	1.38
552	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.032	44
		64.64	C	-0.11	-1.70	2.52
553	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.408	44
		530.87	C	-25.02	-125.79	1.38
554	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.022	44
		35.56	C	0.02	-1.55	2.52
555	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.356	43
		56.42	C	52.83	29.42	0.00
556	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.458	43
		72.73	C	52.41	98.62	0.00
557	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.585	43
		44.62	C	0.19	106.97	0.00
558	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.289	44

419.23 C                    6.97                    113.36                    4.20

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559	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.370	44
		431.37 C	-10.87	-150.17	3.85
560	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.364	43
		271.81 C	18.54	145.25	0.00
561	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.298	44
		215.24 C	-20.09	-101.39	2.29
562	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.240	44
		231.99 C	25.56	37.40	3.00
563	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.275	44
		197.65 C	22.73	77.13	2.50
564	ST	W14X68	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.588	43
		26.85 T	0.25	193.45	0.00
565	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.507	43
		4.37 C	0.11	95.07	0.00
566	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.575	44
		52.43 C	14.93	142.29	2.50
567	ST	W18X86	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.297	43
		161.06 C		22.13	97.38	0.00
568	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.388	44
		112.74 C		23.01	161.04	3.00
569	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.963	43
		83.59 C		0.19	176.68	0.00
570	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.402	42
		23.33 T		6.03	-34.71	1.25
571	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.419	44
		42.24 C		11.92	-15.40	2.50
572	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.117	48
		56.41 C		2.06	-2.11	2.63
573	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.276	43
		203.11 T		-4.40	-14.16	2.63

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574	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.281	44
		144.07 C		5.47	-11.62	2.63
575	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.138	48
		85.11 C		1.84	-2.11	2.63



576	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.267	43	
		176.06	T	-4.39	-14.13	2.63	
577	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.284	44	
		147.51	C	5.54	-11.64	2.63	
578	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.579	44	
		262.65	C	-51.89	-163.75	0.00	
579	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.753	43	
		660.85	C	56.77	166.19	0.00	
580	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.583	43	
		472.71	C	49.06	156.48	0.00	
581	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.350	44	
		404.14	C	39.75	41.16	1.25	
582	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.423	44	
		263.33	C	-28.49	-147.04	0.00	
583	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.510	44	
		383.22	C	23.12	215.32	2.50	
584	ST	W14X68		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.734	47	
		56.71	T	-63.99	-97.57	4.00	
585	ST	W12X50		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.536	43	
		9.74	T	0.10	100.93	0.00	
586	ST	W12X65		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.657	44	
		137.92	C	13.40	164.64	2.50	
587	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.313	43	
		296.37	C	16.94	116.33	0.00	
588	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.283	44	

111.48 C            -24.08            -86.61            0.00

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589	ST	W14X68	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.531	43
		119.18 C	0.33	167.16	0.00
591	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.752	43
		610.00 C	55.48	183.44	0.00
592	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.466	42
		556.35 C	-61.40	19.30	1.50
593	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.246	44
		316.91 C	-22.95	-45.27	1.15
594	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.385	44
		140.22 C	-27.41	-138.68	0.00
595	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.359	44
		200.84 C	19.70	145.33	2.50
596	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.499	43
		19.95 T	0.45	148.90	0.00
597	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.521	43
		4.96 C	0.09	97.56	0.00
598	ST	W12X65	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.680	44
		119.03	C	15.36	169.24	2.50
599	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.405	43
		260.26	C	47.82	61.50	0.00
600	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.281	44
		170.06	C	17.96	103.72	1.55
601	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.520	43
		21.97	C	0.22	82.26	0.00
602	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.235	44
		84.48	C	4.39	-11.94	2.62
603	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.547	43
		20.58	T	-7.47	-41.39	1.50
604	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.225	44
		69.63	C	4.33	-11.76	2.62

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605	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.256	43
		171.27	T	-3.82	-14.02	2.62
606	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.082	47
		17.18	C	-2.06	-2.11	2.62

607	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.244	43
		141.19	T	-3.78	-13.82	2.62
608	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.684	43
		105.98	T	-7.62	-53.52	3.25
609	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.095	48
		37.16	C	1.84	-2.11	2.62
610	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.504	42
		91.35	T	1.91	-49.39	1.50
611	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.073	47
		36.11	T	-1.84	-2.11	2.62
612	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.440	42
		24.99	T	-2.79	68.06	0.00
613	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.659	43
		82.67	T	-3.36	-72.93	1.40
614	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.573	42
		608.46	C	-25.49	-165.45	0.00
615	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.631	43
		437.33	C	57.57	143.85	0.00
616	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.477	42
		208.16	C	-34.97	-166.53	0.00
617	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.448	44
		112.92	C	-62.17	-48.46	0.00
618	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.420	44
		428.18	C	53.94	30.38	1.50
619	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.431	44

207.22 C            -51.53            -68.97            0.00

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620	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.852	41
		2.02 C	-0.91	-255.69	3.33
621	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.653	42
		12.80 T	-0.01	123.12	8.00
622	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.454	42
		178.93 C	-0.14	-126.00	2.50
623	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.330	43
		165.29 C	47.10	23.00	0.00
624	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.282	43
		230.88 C	35.80	27.90	0.00
625	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.057	44
		7.08 T	-4.21	-8.85	2.99
626	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.031	47
		32.20 C	-0.54	-0.56	1.35
627	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.027	48
		25.45 C	0.49	-0.56	1.35
628	ST	W12X35	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.037	48
		66.37 T		0.49	-0.56	1.35
629	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.059	43
		127.78 C		0.00	0.00	0.00
630	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.043	42
		118.02 T		0.00	-0.56	1.35
631	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.051	47
		75.68 C		-0.54	-0.56	1.35
633	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.039	42
		23.70 T		0.00	-5.45	1.35
634	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.284	44
		76.50 C		-38.36	-35.12	0.00
635	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.043	42
		33.32 T		0.00	-5.64	1.35

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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636	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.515	44
		309.45 C		-42.06	-141.47	0.00
637	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.542	43
		488.33 C		40.83	142.77	0.00

638	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.500		43
			363.81 C	43.34	133.48		0.00
639	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.380		44
			190.71 C	-27.97	-128.53		2.29
640	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.389		44
			312.24 C	-27.59	-124.86		0.00
641	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.517		42
			193.44 C	-12.01	284.34		2.50
642	ST	W14X68		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.816		42
			84.69 C	-0.33	258.49		8.00
643	ST	W12X50		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.733		42
			16.07 T	0.14	137.81		8.00
644	ST	W12X65		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.765		42
			212.72 C	0.01	-218.22		2.50
645	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.342		43
			141.36 C	36.38	74.73		0.00
646	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.334		44
			93.10 C	-37.28	-71.03		0.00
647	ST	W12X50		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.556		43
			30.63 C	0.08	102.65		0.00
648	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.409		44
			299.99 C	-30.29	-117.63		0.00
649	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.556		43
			460.26 C	51.32	115.71		0.00
650	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.379		44

315.07 C            -44.37            -49.30            0.00

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651	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.296	44
		294.58 C	29.15	55.63	2.50
652	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.403	44
		217.49 C	-37.70	-102.13	0.00
653	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.288	43
		204.78 C	37.25	27.79	0.00
654	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.374	42
		47.03 T	-11.84	39.50	0.00
655	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.356	44
		31.21 C	0.11	61.79	8.00
656	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.420	43
		44.12 T	-12.03	-101.74	2.50
657	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.279	43
		231.34 C	29.17	50.88	0.00
658	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.235	44
		103.77 C	-31.82	-24.28	0.00
659	ST	W12X50	(AISC SECTIONS)		



			PASS	LRFD-H1-1B-C	0.432	42
		19.86 C		-0.32	77.51	0.00
660	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.518	43
		432.63 C		69.49	25.61	0.00
661	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.624	43
		375.93 C		71.17	95.38	0.00
662	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.424	43
		343.56 C		45.71	71.34	0.00
663	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.256	43
		147.07 C		35.23	20.38	0.00
664	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.424	44
		198.48 C		-48.56	-75.84	0.00
665	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.270	43
		154.03 C		-31.52	43.86	2.50

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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666	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.723	41
		4.05 C		0.02	136.36	8.00
667	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.263	44
		1.62 C		0.08	49.31	8.00

668	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.332	43
		47.56	T	-11.24	-76.55	2.50
669	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.242	43
		103.64	C	22.20	66.69	0.00
670	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.269	44
		85.61	C	-31.52	-50.44	0.00
671	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.385	44
		33.28	C	0.03	66.99	8.00
672	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.242	44
		335.19	C	5.96	96.49	4.20
673	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.291	43
		342.64	C	16.03	89.42	0.00
674	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.207	43
		148.25	C	10.46	83.74	0.00
675	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.188	43
		63.69	C	-17.54	-52.54	2.50
676	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.231	44
		88.02	C	-16.63	-82.33	0.00
677	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.243	42
		191.39	C	-17.95	74.37	2.50
678	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.729	42
		15.90	T	0.11	137.10	8.00
679	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.302	43
		2.81	C	0.09	56.54	0.00
680	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.345	43

108.02 T            -11.29            -76.95            2.50

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681	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.146	43
		73.50 C	16.40	27.14	0.00
682	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.207	42
		69.09 C	-17.95	-62.73	0.00
683	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.431	43
		25.27 C	0.01	77.30	0.00
684	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.376	44
		384.98 C	7.95	170.50	4.20
685	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.413	43
		487.46 C	9.23	177.85	0.00
686	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.319	43
		309.02 C	8.22	153.49	0.00
687	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.286	43
		147.00 C	-30.83	-58.82	1.25
688	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.261	43
		258.09 C	12.52	99.94	0.00
689	ST	W18X86	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.412	44
		207.05	C	21.93	172.11	2.50
690	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.606	42
		11.36	T	55.33	-75.32	3.00
691	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.584	43
		12.65	T	0.10	109.85	0.00
692	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.701	44
		146.39	C	15.66	173.17	2.50
693	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.367	41
		165.66	C	51.44	31.14	1.50
694	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.172	44
		26.01	C	-6.44	-89.30	0.00
696	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.214	43
		15.87	T	-14.61	-3.52	2.05

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697	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.048	42
		108.97	C	0.23	-2.28	2.50
698	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.395	44
		236.27	C	4.30	-12.54	2.75

699	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.248	44
		133.38	C	3.82	-12.44	2.75
700	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.217	43
		25.70	T	-14.70	-3.53	2.05
701	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.059	43
		132.01	T	-0.03	-4.36	2.50
702	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.377	44
		217.35	C	4.26	-12.59	2.75
703	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.250	43
		134.29	T	-3.82	-14.78	2.75
705	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.143	43
		66.73	T	-8.05	-4.06	2.05
706	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.088	44
		155.59	C	-0.18	-6.59	2.29
707	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.241	43
		118.63	T	-3.73	-14.62	2.75
708	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.262	43
		86.46	T	-4.98	-14.80	2.75
710	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.227	43
		91.37	T	-14.10	-4.05	2.05
711	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.345	43
		67.58	T	-14.42	-66.66	2.50
712	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.090	48
		17.69	C	2.26	-2.32	2.75
713	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.094	48

21.30 C                    2.26                    -2.32                    2.75

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714	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.089	43
		64.79 T	-0.07	-3.77	1.45
715	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.144	43
		82.98 T	-7.97	-3.87	2.05
716	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.043	44
		64.82 C	-0.20	-3.52	2.29
717	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.233	43
		93.88 T	-3.73	-14.66	2.75
718	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.252	43
		59.91 T	-4.95	-14.76	2.75
719	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.693	42
		69.51 T	-60.09	-77.99	1.00
720	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.281	43
		117.79 T	-17.76	-3.91	2.05
721	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.040	44
		50.39 C	-0.45	-3.14	2.29
722	ST	W12X35	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.080	48
		13.80 C		2.02	-2.32	2.75
723	ST	W12X35	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.092	48
		24.97 C		2.02	-2.32	2.75
725	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.295	44
		177.85 C		17.49	-4.24	2.05
726	ST	W14X68	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.744	43
		43.21 T		-49.90	-139.12	2.50
727	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.352	43
		13.84 T		-10.55	-47.27	2.75
728	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.432	43
		3.58 T		-16.82	-47.35	2.75
729	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.234	44
		151.33 C		13.02	-5.66	1.00

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730	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.279	44
		148.77 C		17.32	-2.93	2.05
731	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.345	44
		91.55 C		23.25	-3.40	2.50

732	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.330	43
		24.43	T	-10.76	-41.16	2.75
733	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.410	43
		7.52	T	-17.14	-41.20	2.75
734	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.419	43
		421.81	C	53.94	30.38	0.00
735	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.406	44
		401.37	C	44.13	62.45	1.50
736	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.121	42
		80.60	C	-0.02	-5.11	1.45
737	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.449	43
		194.09	C	-62.17	-41.72	1.50
738	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.368	43
		297.61	C	-43.26	-50.52	1.50
740	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.223	41
		47.57	C	24.97	-48.62	1.50
741	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.181	44
		38.42	C	6.02	95.93	1.50
742	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.600	43
		29.88	C	1.28	105.37	0.00
743	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.655	43
		156.76	C	65.36	136.57	0.00
744	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.705	43
		278.43	C	60.55	164.40	0.00
745	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.347	44



221.24 C            -46.94            -29.46            1.38

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746	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.291	43
		67.93 C	-40.49	-32.15	1.25
747	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.254	44
		98.90 C	19.02	87.67	3.00
748	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.340	43
		2.29 T	-42.11	-65.72	2.50
749	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.431	42
		6.24 T	-30.90	-6.33	4.00
750	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.507	44
		3.16 T	0.15	95.59	8.00
751	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.462	44
		2.14 C	1.37	136.97	8.00
752	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.418	41
		157.77 C	-67.50	3.59	1.50
753	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.080	43
		2.10 T	-6.63	-28.10	2.50
755	ST	W18X86	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.252	43
		8.62 C		-31.94	-45.42	1.50
756	ST	W18X86	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.124	43
		4.28 T		-11.83	-37.35	1.50
757	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.276	44
		8.29 C		1.09	48.16	8.00
758	ST	W12X35	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.339	44
		22.81 C		1.27	31.94	6.50
759	ST	W12X35	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.138	44
		3.61 C		1.27	11.84	6.50
760	ST	W12X35	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.475	42
		48.53 T		-16.72	-8.50	0.00
761	ST	W12X35	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.318	43
		35.67 T		-6.08	-18.20	3.25

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762	ST	W12X35	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.485	44
		25.05 C		1.93	46.66	6.50
877	ST	W18X86	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.292	41
		28.57 C		48.97	3.59	0.00

879	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.277	41	
			68.34 C	-38.26	31.13	0.00	
881	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.276	43	
			61.76 C	-28.77	-68.98	1.25	
883	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.438	43	
			135.20 C	-33.03	-155.11	1.25	
885	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.228	44	
			214.97 C	14.83	74.84	1.50	
887	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.252	43	
			298.37 C	25.05	42.87	0.00	
889	ST	W12X50		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.570	41	
			24.89 C	0.01	116.46	6.50	
890	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.571	44	
			1.12 C	0.24	65.25	6.50	
891	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.480	44	
			18.91 C	0.13	52.15	6.50	
1009	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.225	44	
			197.75 C	-7.54	-103.97	0.05	
1011	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.492	43	
			40.49 C	-4.97	-242.18	1.50	
1013	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.424	44	
			318.06 C	47.82	69.06	1.25	
1015	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.405	44	
			382.40 C	-20.57	-156.13	1.15	
1017	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.395	42	

419.52 C            52.76            19.30            0.00

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1019	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.362	44
		459.48 C	32.32	72.98	1.50
1021	ST	W14X68	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.471	41
		2.44 C	0.00	-169.12	3.79
1022	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.546	43
		33.13 C	0.03	57.67	0.00
1023	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.996	42
		21.53 T	0.10	114.16	6.50
1027	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.533	42
		160.47 T	13.68	66.48	2.50
1028	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.250	43
		222.91 C	9.96	99.98	0.00
1029	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.243	42
		134.56 C	0.54	-148.21	3.00
1030	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.380	42
		152.43 T	0.43	71.13	2.50
1031	ST	W12X50	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.103	47
		85.16 T		2.20	-7.69	0.00
1032	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.140	47
		145.14 T		1.99	-11.34	0.00
1033	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.449	42
		7.38 C		-2.31	78.08	6.00
1034	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.887	41
		18.57 C		-0.05	292.28	8.00
1035	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.380	43
		63.79 T		-7.87	-69.60	1.30
1051	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.442	44
		107.83 C		-15.68	-101.22	0.32
1052	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.400	44
		31.07 T		-8.29	-35.08	0.00

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1053	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.388	42
		0.61 T		-0.47	-70.48	1.30
1055	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.527	42
		0.61 T		-2.65	-86.69	0.50

1056	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.275	42
			3.25 T	0.00	-17.01	1.75
1061	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.552	42
			1.29 C	2.47	-92.01	0.00
1062	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.274	42
			1.80 T	0.00	-17.01	1.75
1063	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.431	42
			20.08 T	1.75	-107.01	0.00
1064	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.345	42
			1.29 C	1.20	-59.05	0.00
1065	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.274	42
			1.49 T	0.00	-17.01	1.75
1067	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.561	42
			1.29 C	1.40	-98.66	0.80
1070	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.111	44
			5.18 C	3.90	-2.68	0.60
1073	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.275	42
			4.25 T	0.00	-17.01	1.75
1074	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.554	44
			93.84 C	22.67	121.62	1.00
1075	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.123	44
			31.70 T	-2.57	-9.22	0.00
1076	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.069	42
			2.88 T	0.00	-4.92	0.90
1077	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.596	44

115.06 C            -26.15            -125.68            0.34

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1078	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.228	43
		6.36 C	8.07	-5.38	0.00
1079	ST W8X21		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.059	42
		2.72 C	0.00	-4.21	0.90
1080	ST W14X68		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.545	43
		48.49 C	-25.08	-115.40	0.65
1081	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.227	44
		5.24 C	8.07	-5.38	0.65
1083	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.751	44
		24.94 C	-11.91	-111.56	0.00
1085	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.187	44
		41.20 T	-7.09	-0.15	0.00
1086	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.752	44
		66.64 C	-11.43	-110.92	0.83
1087	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.585	43
		38.37 C	12.40	50.26	0.00
1089	ST W12X35		(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.561	42
		1.29 C		1.40	-98.66	0.00
1090	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.542	44
		80.27 T		-1.32	-64.13	0.00
1091	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.162	44
		58.10 T		-2.95	-13.30	0.00
1092	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.369	43
		6.02 C		3.85	10.37	0.00
1093	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.101	42
		35.57 C		0.71	-13.17	0.33
1094	ST	W8X21		(AISC SECTIONS)		
			PASS	SHEAR-Y	0.029	42
		0.88 T		0.00	0.00	0.00
1095	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.414	44
		78.25 T		-2.20	-44.42	0.00

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1096	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.132	44
		58.69 T		-1.84	-12.69	0.00
1097	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.198	44
		2.66 T		-4.00	-0.02	0.00



1098	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.100		42
			35.57 C	0.06	-15.87		0.00
1099	ST	W8X21		(AISC SECTIONS)			
			PASS	SHEAR-Y	0.029		42
			0.05 C	0.00	0.00		0.00
1100	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.266		44
			75.95 T	-2.56	-23.60		0.00
1101	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.155		43
			59.38 T	-5.33	0.98		1.30
1102	ST	W8X21		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.235		44
			3.40 C	4.72	-0.01		3.85
1103	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.258		44
			63.71 C	9.48	0.48		1.30
1104	ST	W8X21		(AISC SECTIONS)			
			PASS	SHEAR-Y	0.029		42
			0.49 C	0.00	0.00		0.00
1105	ST	W12X50		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.645		44
			42.49 C	0.21	119.27		3.25
1106	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.405		43
			3.92 T	-4.24	-42.26		1.25
1107	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.478		44
			94.66 T	-1.48	-57.69		0.00
1109	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.391		44
			44.33 T	-1.65	-47.28		0.00
1110	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.263		43
			9.35 T	-9.17	-5.80		3.85
1111	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.655		42

24.63 T            11.57            -51.31            0.00

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1112	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.712	44
		20.11 T	-7.05	-74.95	0.00
1113	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.320	43
		7.59 T	-7.87	-19.87	3.85
1114	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.569	43
		62.29 T	-6.01	-126.10	1.30
1165	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.187	43
		6.40 C	6.17	-6.51	0.00
1174	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.782	42
		20.08 T	5.44	-187.27	0.00
1183	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.643	43
		60.92 T	-2.08	-159.53	1.02
1253	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.582	44
		187.40 C	-39.98	-207.77	0.00
1438	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.042	47
		68.90 C	0.38	-3.87	2.75
1439	ST	W12X50	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.050	47
		68.00	C	0.25	-6.08	2.75
1440	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.367	43
		3.85	T	0.03	-89.40	4.00
1441	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.104	47
		100.37	T	2.20	-7.26	2.75
1442	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.107	44
		31.36	C	0.01	2.44	2.50
1443	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.130	47
		125.78	T	1.99	-10.49	2.75
1444	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.092	44
		28.89	C	0.00	2.02	2.50
1445	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.059	44
		112.90	C	-0.18	-5.19	2.52

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1446	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.049	44
		96.25	C	-0.10	-4.25	2.52
1447	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.514	42
		10.56	T	0.00	-96.85	4.00

1448	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.050	43
			18.16 T	-0.01	-1.40	2.50
1449	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.062	43
			31.58 T	-0.01	-1.48	2.50
1450	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.049	43
			125.02 C	0.09	-2.58	0.00
1451	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.044	43
			106.53 C	0.18	-2.22	0.00
1452	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.384	43
			3.80 T	-0.06	-93.48	4.00
1453	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.067	43
			143.98 C	1.10	-1.42	0.00
1454	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.119	43
			32.97 C	0.00	2.84	0.00
1455	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.060	43
			130.80 C	1.11	-0.98	0.00
1456	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.124	43
			30.96 C	0.01	3.10	0.00
1465	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.111	44
			190.86 T	-0.18	-9.11	0.00
1466	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.072	44
			145.50 C	-0.12	-3.93	2.52
1467	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.094	43
			60.51 T	0.00	-1.80	3.00
1468	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.107	44

179.27 T            -0.15            -8.99            0.00

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1469	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.061	42
		123.05 C	0.05	-3.43	1.37
1470	ST	W6X16	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.088	43
		61.04 T	0.00	-1.56	3.00
1471	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.103	40
		8.37 T	0.00	-19.15	4.00
1472	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.126	43
		206.24 T	-0.18	-10.59	2.75
1473	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.061	44
		70.05 C	-0.02	-5.64	2.52
1474	ST	W6X16	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.101	44
		10.20 C	-0.01	-3.01	0.00
1475	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.108	43
		160.32 T	-0.15	-9.76	2.75
1476	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.058	44
		74.50 C	-0.07	-4.98	2.52
1477	ST	W6X16	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.082	44
		10.07	C	-0.01	-2.30	0.00
1478	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.101	43
		319.37	C	0.17	0.73	0.00
1479	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.055	43
		167.36	C	0.05	-0.80	0.00
1480	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.108	43
		14.16	C	0.02	2.99	0.00
1481	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.099	43
		318.76	C	0.17	0.42	0.00
1482	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.061	42
		122.37	C	0.20	-3.20	0.00
1483	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.103	43
		14.04	C	0.03	2.80	0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1484	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.439	40
		12.44	C	0.00	-131.46	4.00
1493	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.048	43
		143.51	C	0.18	-0.63	0.00

1494	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.062	43
			177.66 C	0.15	-1.48	0.00
1495	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.062	41
			0.08 C	-0.06	-14.82	4.00
1496	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.054	44
			131.34 C	-0.08	-3.03	2.66
1497	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.073	41
			132.72 C	0.01	-7.47	0.00
1498	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.442	43
			24.59 T	-1.97	-128.57	5.12
1499	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.059	44
			111.30 C	-0.19	-5.27	2.66
1500	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.089	44
			139.84 C	-0.23	-9.97	2.66
1501	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.057	40
			1.18 C	-0.03	-13.77	4.00
1502	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.738	44
			26.89 T	-49.35	-139.69	0.00
1503	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.340	43
			89.09 C	22.98	-3.41	0.00
1504	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.709	43
			9.05 T	-23.99	-95.98	4.00
* 1505	ST	W14X68		(AISC SECTIONS)		
			FAIL	LRFD-H1-1B-T	1.019	44
			32.45 T	-69.57	-188.88	0.00
1506	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.462	43

95.33 C            32.43            -2.62            0.00

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ALL UNITS ARE - KN    METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1507	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.511	42
		0.12 T	0.00	-96.85	4.00
* 1508	ST W14X68		(AISC SECTIONS)		
		FAIL	LRFD-H1-1B-T	1.020	43
		37.23 T	-69.57	-188.88	2.50
1509	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.460	44
		88.90 C	32.43	-2.48	2.50
1510	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.703	43
		9.34 T	-24.05	-94.41	4.00
1511	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.319	43
		157.85 C	17.80	-10.55	0.00
1512	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.266	43
		74.23 C	17.67	-3.47	0.00
1513	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.722	43
		4.38 T	-24.22	-98.71	4.00
1514	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.375	43
		165.63 C	24.61	-2.05	0.00
1515	ST W12X50		(AISC SECTIONS)		



			PASS	LRFD-H1-1B-C	0.346	43
		79.52	C	24.68	-0.37	0.00
1516	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.511	42
		0.16	T	0.00	-96.85	4.00
1517	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.380	44
		162.93	C	-24.06	-5.12	1.88
1518	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.364	44
		148.03	C	24.68	-0.25	2.05
1519	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.578	43
		2.71	T	-23.81	-64.94	4.00
1520	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.351	44
		9.98	T	-10.55	-47.26	0.00
1521	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.329	44
		20.65	T	-10.76	-41.16	0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1522	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.511	42
		0.12	C	0.00	-96.85	4.00
1523	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.432	44
		3.33	T	-16.82	-47.34	0.00

1524	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.409	44
			4.33 T	-17.14	-41.20	0.00
1525	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.511	42
			0.03 T	0.00	-96.85	4.00
1528	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.233	43
			46.79 C	23.08	62.91	0.00
1531	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.279	44
			49.97 C	-34.44	-49.34	0.00
1533	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.262	44
			55.26 C	-29.65	-56.08	0.00
1543	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.445	43
			7.09 T	-12.06	-19.77	2.50
1544	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.410	43
			8.36 T	-0.06	-76.00	1.81
1584	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.016	48
			3.37 T	0.49	-0.56	1.35
1594	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.444	44
			5.02 T	-12.06	-19.77	0.00
1597	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.681	43
			48.73 T	-0.64	-121.73	1.81
1599	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.068	48
			3.50 T	0.60	-9.08	1.20
1600	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.734	44
			46.70 T	-0.64	-121.75	0.00
1605	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.193	41

270.94 T            -0.15            -20.20            2.50

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1606	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.289	44
		156.80 C	-0.02	-38.67	2.29
1607	ST	W6X16	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.142	44
		29.14 C	0.00	3.35	3.00
1608	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.119	43
		210.89 T	-0.13	-10.69	2.50
1609	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.059	44
		65.64 C	-0.09	-6.20	2.29
1610	ST	W6X16	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.086	44
		9.92 C	0.01	-2.49	0.00
1611	ST	W14X68	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.346	44
		74.10 T	-14.42	-66.66	0.00
1613	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.100	41
		95.44 T	0.00	-12.33	2.50
1615	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.196	41
		278.34 T	-0.14	-20.26	0.00
1616	ST	W12X50	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.323	44
		5.18	T	-0.01	-50.94	0.00
1617	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.115	43
		46.79	T	-0.35	-1.85	3.00
1618	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.117	44
		225.34	T	-0.13	-9.85	0.00
1619	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.053	48
		78.54	C	-0.31	-4.23	0.00
1620	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.065	43
		45.71	T	0.00	-1.15	3.00
1621	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.192	43
		65.32	C	6.47	38.52	0.00
1623	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.100	41
		95.50	T	0.00	-12.25	0.00

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1625	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.085	44
		276.59	T	-0.06	-3.01	0.00
1626	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.066	43
		161.11	C	-0.02	3.20	0.00

1627	ST	W6X16		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.170	43	
		36.42	C	0.03	3.83	0.00	
1628	ST	W12X50		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.083	43	
		258.64	C	0.21	-0.96	0.00	
1629	ST	W12X50		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.046	48	
		78.03	C	-0.26	-3.27	0.00	
1630	ST	W6X16		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.093	43	
		12.75	C	0.01	2.55	0.00	
1631	ST	W14X68		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.100	44	
		52.52	T	-3.37	-19.45	0.00	
1633	ST	W12X50		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.039	43	
		83.55	C	0.52	-1.31	0.00	
1639	LD	L35358		(AISC SECTIONS)			
			PASS	TENSION	0.247	44	
		231.75	T	0.00	0.00	0.00	
1640	LD	L35358		(AISC SECTIONS)			
			PASS	COMPRESSION	0.582	43	
		169.78	C	0.00	0.00	0.00	
1641	LD	L35358		(AISC SECTIONS)			
			PASS	COMPRESSION	0.729	44	
		212.74	C	0.00	0.00	3.91	
1642	LD	L35358		(AISC SECTIONS)			
			PASS	TENSION	0.301	43	
		282.42	T	0.00	0.00	3.91	
1643	LD	L30306		(AISC SECTIONS)			
			PASS	TENSION	0.211	44	
		128.56	T	0.00	0.00	0.00	
1644	LD	L30306		(AISC SECTIONS)			
			PASS	COMPRESSION	0.636	43	
		89.88	C	0.00	0.00	0.00	
1645	LD	L30306		(AISC SECTIONS)			
			PASS	COMPRESSION	0.700	44	

98.98 C            0.00            0.00            3.91

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1646	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.222	43
		134.67 T	0.00	0.00	3.91
1651	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.218	44
		132.48 T	0.00	0.00	0.00
1652	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.559	43
		87.24 C	0.00	0.00	0.00
1653	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.647	44
		100.96 C	0.00	0.00	3.72
1654	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.244	43
		148.49 T	0.00	0.00	3.72
1655	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.180	44
		109.19 T	0.00	0.00	0.00
1656	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.420	43
		65.50 C	0.00	0.00	0.00
1657	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.698	44
		108.90 C	0.00	0.00	3.72
1658	LD	L30306	(AISC SECTIONS)		

			PASS	TENSION	0.257	43
		155.96	T	0.00	0.00	3.72
1659	LD	L35358	(AISC SECTIONS)			
			PASS	TENSION	0.180	44
		168.75	T	0.00	0.00	0.00
1660	LD	L35358	(AISC SECTIONS)			
			PASS	COMPRESSION	0.471	43
		126.46	C	0.00	0.00	0.00
1661	LD	L35358	(AISC SECTIONS)			
			PASS	COMPRESSION	0.657	44
		176.52	C	0.00	0.00	4.07
1662	LD	L35358	(AISC SECTIONS)			
			PASS	TENSION	0.232	43
		217.15	T	0.00	0.00	4.07
1663	LD	L35358	(AISC SECTIONS)			
			PASS	TENSION	0.244	44
		228.58	T	0.00	0.00	0.00
1664	LD	L35358	(AISC SECTIONS)			
			PASS	COMPRESSION	0.689	43
		185.30	C	0.00	0.00	0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1665	LD	L35358	(AISC SECTIONS)			
			PASS	COMPRESSION	0.721	44
		193.73	C	0.00	0.00	4.07
1666	LD	L35358	(AISC SECTIONS)			
			PASS	TENSION	0.251	43
		235.36	T	0.00	0.00	4.07

1667	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.961	41
			0.01 T	0.00	-182.12	3.33
1668	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.104	41
			11.70 T	0.00	-19.15	4.00
1669	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.405	43
			9.55 T	-13.37	-96.67	2.50
1685	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.455	44
			26.69 T	0.31	149.03	4.00
1686	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.721	45
			32.59 T	-64.02	-97.56	0.00
1689	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.406	44
			14.21 T	3.71	114.89	4.00
1690	ST	W12X30		(AISC SECTIONS)		
			PASS	COMPRESSION	0.364	46
			103.81 C	0.00	0.00	0.00
1691	ST	W12X30		(AISC SECTIONS)		
			PASS	COMPRESSION	0.277	48
			79.08 C	0.00	0.00	0.00
1692	ST	W12X30		(AISC SECTIONS)		
			PASS	COMPRESSION	0.433	45
			92.29 C	0.00	0.00	0.00
1693	ST	W12X30		(AISC SECTIONS)		
			PASS	COMPRESSION	0.432	47
			92.27 C	0.00	0.00	0.00
1694	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.112	48
			111.45 T	-2.35	-7.34	0.00
1695	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.145	42
			249.51 T	7.29	-13.83	0.00
1696	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.182	41



8.87 C                    0.00                    -32.91                    6.67

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1697	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.552	44
		64.93 T	-32.97	-19.73	1.37
1698	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.483	44
		83.66 T	-33.58	-4.63	0.00
1699	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.043	40
		0.01 T	0.00	-8.16	4.00
1700	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.214	40
		137.12 C	0.04	-9.19	0.00
1701	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.224	40
		154.75 C	-0.05	-9.37	0.00
1702	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.105	40
		18.60 T	0.00	-19.15	4.00
1703	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.195	43
		110.64 C	11.43	-2.86	0.00
1704	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.186	43
		74.18 C	11.43	-2.99	0.00
1705	ST	W12X50	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.101	40
		0.02	T	0.00	-19.15	4.00
1706	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.413	44
		121.59	T	-25.13	-10.41	0.00
1707	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.402	44
		91.22	T	-25.17	-9.55	0.00
1708	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.405	43
		6.81	T	-14.46	-94.75	2.50
1709	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.302	40
		203.37	T	-0.07	-13.27	0.00
1710	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.306	40
		208.10	T	0.07	-13.46	0.00
1711	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.527	40
		22.22	C	0.00	-96.06	4.00
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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1712	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.149	41
		90.47	T	-0.61	16.02	0.00
1713	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.084	44
		8.23	C	-0.31	-1.81	0.00

1714	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.226	42
		268.61	T	-17.29	17.76	0.00
1715	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.092	44
		10.68	C	0.01	2.90	2.50
1716	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.063	44
		20.44	T	-0.02	-1.79	0.00
1717	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.180	42
		4.92	C	-1.82	0.82	2.50
1718	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.053	48
		94.37	T	1.47	-1.47	0.73
1719	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.066	43
		9.09	C	0.00	2.06	0.00
1720	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.072	42
		212.69	T	3.60	-2.68	0.00
1721	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.081	43
		10.45	C	0.01	2.54	0.00
1722	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.282	44
		171.53	T	0.00	0.00	0.00
1723	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.720	44
		105.88	C	0.00	0.00	3.83
1724	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.492	44
		72.28	C	0.00	0.00	3.83
1725	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.156	43
		94.99	T	0.00	0.00	3.83
1726	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.372	41

226.27 T                    0.00                    0.00                    0.00

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ALL UNITS ARE - KN    METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1727	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.798	43
		117.32 C	0.00	0.00	0.00
1729	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.189	43
		114.77 T	0.00	0.00	3.83
1730	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.585	44
		85.94 C	0.00	0.00	3.83
1731	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.250	44
		170.30 T	-14.51	-3.79	0.00
1732	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.150	43
		110.57 C	8.29	-2.11	0.00
1733	ST	W6X16	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.086	43
		11.33 C	0.12	2.06	0.00
1734	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.229	44
		105.43 T	-14.41	-2.91	0.00
1735	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.145	44
		62.01 T	-8.30	-4.05	0.00
1736	ST	W6X16	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.095	43
		11.19 C		0.14	2.33	0.00
1737	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.534	43
		132.82 C		36.95	2.73	0.00
1738	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.155	43
		25.14 C		0.29	3.68	0.00
1739	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.268	44
		42.50 T		-17.67	-5.09	0.00
1740	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.128	42
		29.64 C		0.02	3.31	0.00
1741	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.094	44
		44.04 T		-0.19	-1.66	0.00
1742	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.075	43
		40.81 T		0.19	1.06	2.75

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1743	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.091	42
		31.01 T		0.02	-2.53	0.00
1744	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.070	43
		5.84 T		-0.31	-1.50	2.50

1745	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.416	43
		117.33	T	-25.13	-11.25	2.05
1746	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.194	43
		66.26	T	-11.43	-5.46	2.05
1747	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.110	44
		9.34	C	-0.13	-3.00	0.00
1748	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.403	43
		86.96	T	-25.17	-10.05	2.05
1749	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.193	43
		82.56	T	-11.41	-4.63	2.05
1750	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.092	44
		8.48	C	-0.12	-2.44	0.00
1751	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.381	43
		152.69	T	-25.04	-3.37	2.05
1752	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.125	42
		23.24	C	-0.02	-3.54	0.00
1753	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.480	43
		83.94	T	-33.58	-4.00	2.05
1754	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.080	44
		18.50	C	0.20	1.47	2.50
1755	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.273	42
		166.05	T	0.00	0.00	0.00
1756	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.622	42
		128.26	C	0.00	0.00	0.00
1757	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.397	44

81.89 C 0.00 0.00 3.23

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1758	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.318	41
		193.09 T	0.00	0.00	3.23
1759	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.241	42
		49.71 C	0.00	0.00	3.23
1760	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.255	43
		52.56 C	0.00	0.00	0.00
1761	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.586	42
		120.72 C	0.00	0.00	3.23
1762	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.267	42
		162.47 T	0.00	0.00	3.23
1763	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.237	44
		143.83 T	0.00	0.00	0.00
1764	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.690	43
		112.55 C	0.00	0.00	0.00
1765	LD	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.382	44
		62.38 C	0.00	0.00	3.63
1766	LD	L30306	(AISC SECTIONS)		

			PASS	COMPRESSION	0.190	43
		31.02	C	0.00	0.00	0.00
1767	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.186	44
		113.27	T	0.00	0.00	0.00
1768	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.504	43
		82.29	C	0.00	0.00	0.00
1769	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.496	44
		80.99	C	0.00	0.00	3.63
1770	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.181	43
		109.75	T	0.00	0.00	3.63
1771	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.271	44
		115.01	C	2.34	0.58	4.14
1772	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.576	44
		284.18	C	1.76	0.32	4.30

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1777	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.527	44
		230.52	C	3.48	0.41	4.30
1778	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.397	44
		152.22	C	3.78	1.13	4.30



1781	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.549	43
		231.62	C	3.45	1.48	0.00
1782	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.415	43
		153.43	C	3.75	2.12	0.00
1783	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.318	43
		130.77	C	2.26	0.69	0.00
1784	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.572	43
		283.49	C	1.82	1.46	0.00
1787	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.157	43
		80.61	T	-2.85	-3.73	4.14
1788	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.615	44
		305.24	C	1.76	0.34	4.30
1793	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.453	44
		190.89	C	3.49	0.35	4.30
1794	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.389	44
		148.72	C	3.79	0.97	4.30
1797	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.473	43
		191.43	C	3.45	1.42	0.00
1798	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.408	43
		150.36	C	3.76	2.02	0.00
1799	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.136	43
		59.98	C	2.26	0.79	0.00
1800	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.612	43
		305.31	C	1.82	1.53	0.00
1803	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.481	42

79.60 T            16.60            -8.23            3.25

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ALL UNITS ARE - KN    METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1804	ST W14X68		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.772	42
		11.04 T	92.48	28.99	2.00
1805	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.444	42
		5.05 C	-30.87	-9.55	0.00
1806	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.409	42
		28.07 C	-15.44	-3.11	0.00
1807	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.412	42
		19.70 T	16.11	-2.09	0.00
1808	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.627	42
		27.36 T	45.84	-4.85	0.00
1809	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.623	42
		41.98 T	46.23	2.08	5.15
1810	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.644	42
		68.75 C	45.34	4.88	5.15
1811	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.780	42
		76.11 C	53.81	9.97	0.00
1812	ST W12X35		(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.203	44
		142.92	T	-0.12	-8.71	0.00
1813	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.180	42
		181.88	C	0.02	-6.28	1.45
1814	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.099	41
		33.18	T	0.00	3.10	1.50
1815	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.197	44
		138.72	T	-0.04	-8.59	0.00
1816	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.159	44
		151.47	C	-0.05	-5.72	1.33
1817	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.103	41
		32.53	T	0.00	3.31	1.50
1818	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.042	44
		36.17	T	0.00	-0.66	0.00

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1819	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.050	44
		35.86	T	0.00	-0.99	0.00
1820	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.257	40
		148.52	T	-0.08	-11.78	0.00

1821	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.169	41
		43.86	C	0.05	-8.67	0.00
1822	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.088	43
		33.19	T	0.00	-2.68	1.50
1823	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.258	40
		151.76	T	0.08	-11.74	0.00
1824	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.163	41
		30.28	C	-0.03	-8.63	0.00
1825	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.088	43
		32.75	T	0.00	-2.68	1.50
1826	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.310	40
		206.15	T	0.07	-13.68	1.45
1827	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.222	40
		139.56	C	-0.05	-9.59	1.45
1828	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.051	44
		5.40	C	0.00	1.90	1.50
1829	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.309	40
		204.71	T	-0.07	-13.68	1.45
1830	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.218	40
		128.72	C	0.04	-9.59	1.45
1831	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.058	44
		5.25	C	0.00	2.21	1.50
1832	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.091	47
		32.29	C	-0.37	-4.01	0.00
1833	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.098	42

78.09 C            0.05            -3.84            0.00

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ALL UNITS ARE - KN    METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1834	ST	W6X16	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.110	43
		6.48 C	0.00	4.31	0.00
1835	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.084	44
		27.08 T	-0.11	-4.13	0.00
1836	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.069	44
		12.62 T	-0.11	-3.54	0.00
1837	ST	W6X16	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.108	43
		6.07 C	0.00	4.25	0.00
1838	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.248	43
		140.85 T	-0.12	-11.33	1.45
1839	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.167	43
		59.38 T	-0.07	-8.31	1.45
1840	ST	W6X16	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.123	41
		6.65 C	0.00	-4.90	0.00
1841	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.242	43
		137.35 T	-0.04	-11.20	1.45
1842	ST	W12X35	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.171	43
		79.10	T	-0.12	-8.11	1.45
1843	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.127	41
		7.16	C	0.00	-5.02	0.00
1844	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.216	41
		152.07	T	-0.08	-9.34	1.45
1845	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.176	44
		176.14	C	-0.05	-6.12	1.33
1846	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.043	43
		6.18	C	0.00	1.56	0.00
1847	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.220	41
		156.19	T	0.08	-9.50	1.45
1848	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.169	44
		153.99	C	-0.10	-6.19	1.33

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1849	ST	W6X16		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.044	43
		6.22	C	0.00	1.59	0.00
1851	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.350	41
		131.45	C	0.00	0.00	0.00

1852	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.136	44
			82.45 T	0.00	0.00	0.00
1853	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.149	43
			55.73 C	0.00	0.00	0.00
1855	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.291	43
			109.22 C	0.00	0.00	0.00
1856	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.101	44
			61.38 T	0.00	0.00	0.00
1857	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.096	44
			36.02 C	0.00	0.00	2.09
1858	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.239	41
			145.38 T	0.00	0.00	0.00
1859	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.334	41
			125.41 C	0.00	0.00	0.00
1860	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.119	44
			72.45 T	0.00	0.00	0.00
1861	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.125	42
			46.86 C	0.00	0.00	0.00
1862	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.095	44
			35.65 C	0.00	0.00	2.09
1863	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.100	43
			60.75 T	0.00	0.00	2.09
1864	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.293	44
			110.00 C	0.00	0.00	2.09
1865	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.208	43

126.39 T                    0.00                    0.00                    2.09

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1869	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.108	44
		120.36 C	-0.13	-3.38	1.33
1870	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.093	43
		84.94 T	-0.10	-3.54	1.45
1872	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.254	41
		154.29 T	0.00	0.00	0.00
1873	LD	L30306	(AISC SECTIONS)		
		PASS	TENSION	0.207	44
		126.06 T	0.00	0.00	0.00
1879	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.801	42
		27.32 T	-39.62	-37.11	1.00
1880	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.185	44
		111.72 C	-10.08	-5.62	0.92
1883	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.549	43
		451.81 C	8.77	291.22	0.00
1884	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.532	44
		188.08 C	-13.12	-291.00	0.00
1885	ST	W18X86	(AISC SECTIONS)		



			PASS	LRFD-H1-1B-C	0.278	44
		313.22 C		16.59	90.78	2.50
1886	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.350	44
		290.45 C		18.55	134.42	2.50
1887	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.366	44
		454.19 C		32.85	71.76	2.50
1888	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.574	44
		449.53 C		49.07	150.32	2.50
1889	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.359	44
		327.31 C		33.41	78.44	2.50
1890	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.440	44
		406.29 C		29.22	141.49	2.50
1891	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.371	44
		404.46 C		-24.47	-113.28	2.29
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1892	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.460	42
		255.20 C		-34.92	-148.78	2.50
1893	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.408	44
		453.32 C		34.68	93.06	2.50

1894	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.502	44	
			416.87 C	43.34	126.94	2.50	
1895	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.386	43	
			213.39 C	-37.69	-92.20	2.50	
1896	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.416	44	
			530.50 C	-41.46	-63.75	2.29	
1897	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.415	43	
			217.61 C	-48.56	-68.37	2.50	
1898	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.423	44	
			375.94 C	-49.83	-52.56	2.29	
1899	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.235	44	
			248.86 C	-15.82	-71.47	2.29	
1900	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.237	44	
			265.26 C	-21.79	-47.27	2.29	
1901	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.371	44	
			305.78 C	-21.06	-137.11	2.29	
1902	ST	W18X86		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.331	44	
			117.39 C	-9.84	-174.40	0.00	
1903	ST	W14X68		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.986	44	
			0.82 T	-17.28	-284.62	1.50	
1904	ST	W12X65		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.565	43	
			2.48 T	14.93	142.29	0.00	
1905	ST	W12X65		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.648	44	
			11.31 C	-12.29	-171.71	2.00	
1906	ST	W12X65		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.659	44	

6.09 C            -14.37            -171.61            2.00

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1907	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.569	42
		11.26 C	0.01	171.27	3.00
1908	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.735	42
		3.54 C	0.00	221.97	3.00
1909	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.434	44
		9.68 C	12.73	106.56	3.00
1910	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.358	44
		23.94 C	12.06	83.88	3.00
1911	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.365	44
		1.78 T	-11.16	-88.97	1.50
1912	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.688	44
		0.53 T	-15.10	-179.26	1.75
1913	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.411	44
		5.95 T	-12.26	-100.81	0.46
1914	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.411	44
		2.54 T	-13.25	-99.05	0.46
1915	ST	W14X68	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.990	44
		313.39	T	-17.15	-268.40	0.00
1916	ST	W12X65	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.583	44
		125.65	T	-14.66	-141.46	0.00
1917	ST	W12X65	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.652	44
		189.48	T	-11.56	-164.63	0.00
1918	ST	W12X65	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.671	44
		194.81	T	-13.79	-165.87	0.00
1919	ST	W12X65	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.477	42
		175.03	T	0.14	134.20	0.00
1920	ST	W12X65	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.782	42
		260.58	T	0.00	221.97	0.00
1921	ST	W12X65	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.434	43
		6.03	T	12.73	106.56	0.00

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1922	ST	W12X65	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.356	43
		13.99	C	12.06	83.88	0.00
1923	ST	W12X65	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.344	44
		108.26	T	-11.03	-77.26	0.00

1924	ST	W12X65		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.692	44	
		199.17	T	-14.59	-170.67	0.00	
1926	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.643	43	
		451.23	C	0.00	0.00	0.00	
1927	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.644	43	
		451.91	C	0.00	0.00	0.00	
1928	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.379	43	
		265.59	C	0.00	0.00	0.00	
1929	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.258	43	
		180.66	C	0.00	0.00	0.00	
1930	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.431	43	
		302.10	C	0.00	0.00	0.00	
1931	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.425	43	
		297.86	C	0.00	0.00	0.00	
1932	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.442	43	
		309.74	C	0.00	0.00	0.00	
1933	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.403	43	
		282.54	C	0.00	0.00	0.00	
1934	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.553	44	
		219.71	C	0.17	30.31	3.54	
1935	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.650	42	
		250.70	C	0.18	37.07	3.54	
1936	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.353	43	
		247.31	C	0.00	0.00	0.00	
1937	ST	W10X30		(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.537	42	

373.63 C                    0.00                    -0.56                    1.47

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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1938	ST	W10X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.104	42
		138.75 C	0.00	-0.58	1.77
1939	ST	W10X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.287	43
		200.91 C	0.00	0.00	0.00
1940	ST	W10X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.053	43
		74.60 C	0.00	0.00	0.00
1941	ST	W10X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.238	43
		166.86 C	0.00	0.00	0.00
1942	ST	W10X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.223	43
		156.45 C	0.00	0.00	0.00
1943	ST	W10X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.224	43
		156.82 C	0.00	0.00	0.00
1944	ST	W10X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.429	43
		301.13 C	0.00	0.00	0.00
1945	ST	W10X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.413	43
		289.46 C	0.00	0.00	0.00
1981	ST	W12X50	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.641	42
		81.10	T	18.57	-79.60	1.50
1982	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.355	42
		44.13	T	0.43	71.13	0.00
1983	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.223	42
		14.89	T	0.54	-148.21	0.00
1984	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.190	44
		54.10	C	-7.18	-95.15	0.00
1985	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.304	42
		210.15	C	0.00	-0.56	1.47
1986	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.228	43
		160.03	C	0.00	0.00	0.00
1987	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.290	44
		35.45	T	-2.33	-52.86	0.00

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1988	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.623	42
		4.53	C	18.57	-79.60	0.00
1989	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.071	43
		99.39	C	0.00	0.00	0.00

1990	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.057	43
		144.42	T	0.00	0.00	3.54
1995	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.707	44
		3.31	T	-23.99	-95.98	0.00
1996	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.702	44
		3.44	T	-24.05	-94.41	0.00
2005	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.061	41
		0.01	C	-0.06	-14.69	0.00
2006	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.056	40
		1.48	C	-0.03	-13.54	0.00
2007	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.061	44
		18.13	C	-0.02	-1.68	4.06
2008	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.058	47
		12.95	C	0.01	-1.68	2.06
2009	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.110	44
		36.82	C	-0.03	-1.77	4.53
2010	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.101	43
		43.18	C	0.01	-0.68	0.00
2011	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.068	44
		18.82	C	0.00	-2.07	4.06
2012	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.056	43
		19.99	C	0.00	-0.80	0.00
2013	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.100	44
		35.76	C	0.00	-1.43	4.53
2014	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.091	43



42.37 C                    0.01                    -0.25                    0.00

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2015	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.269	44
		24.05 C	-0.01	-11.85	4.45
2016	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.311	44
		40.02 C	-0.01	-12.39	4.45
2017	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.253	43
		15.51 T	-0.01	-13.06	4.85
2018	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.294	44
		26.32 C	-0.02	-12.91	4.45
2019	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.370	44
		3.03 T	0.04	-89.97	0.00
2020	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.386	44
		2.98 T	-0.05	-94.06	0.00
2021	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.248	44
		10.35 T	-0.02	-12.92	0.00
2022	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.265	42
		21.14 C	0.01	-11.94	0.00
2023	ST	W8X21	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.265	44
		23.62	T	-0.02	-13.41	0.00
2024	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.257	44
		11.62	T	-0.01	-13.37	0.00
2029	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.722	44
		4.95	T	-24.22	-98.71	0.00
2030	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.578	44
		3.98	T	-23.81	-64.94	0.00
2037	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.261	44
		19.51	C	-0.01	-5.07	4.45
2038	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.255	43
		30.00	T	0.00	-5.42	4.85
2039	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.293	44
		20.46	C	-0.01	-5.75	4.45

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2040	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.293	43
		30.95	T	0.00	-6.28	4.85
2041	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.234	44
		11.75	T	0.00	-5.17	0.00

2042	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.255	44
			29.70 T	-0.01	-5.42	0.00
2043	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.265	44
			8.98 T	-0.01	-5.91	0.00
2044	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.294	44
			31.52 T	-0.01	-6.28	0.00
2045	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.193	44
			35.52 C	-0.01	-2.80	4.45
2046	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.295	44
			56.81 C	0.02	-1.83	4.85
2047	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.198	44
			34.68 C	-0.02	-2.95	4.45
2048	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.348	44
			55.70 C	-0.02	-3.28	4.45
2049	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.159	43
			46.62 C	0.01	-1.53	0.00
2050	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.308	43
			59.98 C	0.02	-1.83	0.00
2051	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.160	44
			56.96 T	-0.04	-2.86	0.00
2052	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.326	43
			67.15 C	0.02	-1.59	0.00
2053	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.309	44
			10.79 T	-17.05	-59.35	0.00
2054	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.175	42

9.44 T                    1.56                    -25.15                    0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2055	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.068	42
		1.16 C	0.00	-4.92	0.90
2059	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.232	44
		137.91 C	-28.40	-32.40	0.87
2061	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.261	43
		50.26 C	-35.22	-34.15	0.95
2062	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.254	43
		126.60 C	-36.11	-18.37	0.95
2063	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.166	42
		30.75 T	2.28	-17.53	2.20
2064	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.285	43
		79.59 C	-38.36	-35.14	0.95
2065	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.172	43
		6.48 T	-4.64	-10.00	2.20
2066	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.400	43
		41.35 C	0.64	100.89	0.00
2067	ST	W12X50	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.624	44
		41.40 C		17.41	102.30	1.30
2069	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.100	42
		1.36 T		0.00	-6.72	1.10
2070	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.262	44
		21.20 T		-7.48	-42.26	0.00
2072	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.102	42
		0.40 C		0.00	-6.85	1.10
2073	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.344	44
		31.66 C		-8.74	-58.83	0.98
2075	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.103	42
		0.03 T		0.00	-6.97	1.10
2076	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.280	43
		13.48 T		-10.39	-37.33	1.35

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2078	ST	W8X21	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.103	42
		0.01 C		0.00	-6.97	1.10
2079	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.268	43
		39.41 C		6.22	46.75	0.00

2081	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.103	42
			0.05 C	0.00	-6.97	1.10
2082	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.160	42
			1.71 C	0.00	-10.78	1.30
2083	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.284	44
			0.19 C	-5.16	-2.10	1.52
2084	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.206	42
			0.25 T	-1.80	-30.42	0.00
2085	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.161	42
			0.25 T	0.38	-28.34	0.00
2086	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.372	42
			45.40 T	-1.87	-58.22	0.00
2087	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.307	42
			45.38 T	0.41	-52.84	0.00
2089	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.223	42
			0.05 T	-1.54	-30.42	3.50
2090	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.260	44
			47.08 C	-35.22	-34.13	0.00
2091	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.391	44
			23.68 C	-5.08	-29.39	1.33
2092	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.381	44
			48.54 C	1.34	120.57	1.45
2093	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.415	44
			12.53 T	-21.73	-82.76	0.00
2094	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.393	44

193.66 C                    30.90                    98.44                    1.45

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2095	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.243	42
		27.09 T	-0.81	-40.21	1.80
2096	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.644	44
		130.04 C	4.25	85.59	2.60
2097	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.367	42
		50.96 T	-1.62	-58.22	1.70
2098	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.458	42
		0.48 C	18.79	-0.02	1.45
2099	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.313	44
		18.74 T	-8.62	-51.98	0.00
2100	ST W10X30		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.322	43
		8.64 C	4.80	20.85	0.00
2101	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.288	44
		59.80 C	0.10	69.87	2.88
2103	ST W10X30		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.021	47
		0.12 C	-0.54	-0.48	1.35
2106	ST W12X50		(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.332	43
		43.16 C		0.33	83.18	0.00
2107	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.516	44
		44.65 C		-25.73	-104.37	0.00
2108	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.311	44
		28.93 C		0.19	35.01	2.88
2109	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.295	44
		20.26 C		-2.48	-25.89	0.00
2110	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.155	40
		0.01 T		0.00	-10.30	1.20
2111	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.172	43
		11.34 T		-0.10	-21.96	1.81
2113	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.173	44
		14.31 T		-0.10	-21.95	0.00

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2114	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.091	48
		0.40 C		0.54	-8.99	1.20
2115	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.579	44
		13.38 C		-10.60	-82.76	0.00



2116	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.328	44
			8.31 C	0.33	38.08	2.88
2117	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.288	44
			8.88 C	-0.60	-33.17	0.00
2118	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.155	40
			0.00 C	0.00	-10.30	1.20
2119	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.614	43
			3.44 T	-8.23	-32.85	2.50
2120	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.615	44
			6.66 T	-8.23	-32.85	0.00
2121	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.162	43
			53.46 C	17.55	35.37	0.00
2124	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.419	43
			40.67 C	11.92	-15.40	0.00
2125	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.077	41
			2.92 T	0.00	-12.81	1.35
2126	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.843	42
			20.08 T	6.74	-198.92	0.00
2127	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.543	44
			75.22 C	39.94	0.01	2.50
2129	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.347	43
			10.34 T	-0.09	-90.76	2.56
2131	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.574	44
			3.13 T	-0.72	-146.34	0.00
2132	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.566	43

11.54 T            -0.72            -146.31            2.56

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ALL UNITS ARE - KN    METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2133	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.228	42
		10.22 C	0.50	-57.70	2.56
2134	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.163	47
		0.16 C	-0.60	-8.83	1.20
2135	ST	W12X65	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.051	44
		9.65 T	-4.18	-7.02	0.96
2229	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.063	42
		28.10 T	0.00	-1.08	2.43
2230	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.063	41
		13.92 T	0.00	-1.26	2.43
2231	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.069	45
		11.27 C	0.00	-1.08	2.43
2232	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.071	43
		28.94 C	0.00	-0.33	0.40
2233	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.063	42
		28.09 T	0.00	-1.08	2.43
2234	ST	W8X21	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.063	40
		13.87 T		0.00	-1.26	2.43
2235	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.069	45
		11.25 C		0.00	-1.08	2.43
2236	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.076	43
		38.61 C		0.00	0.00	0.00
2237	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.124	44
		31.88 C		-0.02	-3.06	4.53
2238	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.108	43
		34.73 C		0.03	-1.88	0.00
2239	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.131	44
		33.54 C		-0.02	-3.26	4.53
2240	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.109	43
		35.91 C		0.03	-1.85	0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2241	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.142	42
		42.07 C		0.01	-2.95	4.12
2242	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.140	42
		42.05 C		0.01	-2.83	0.82

2243	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.146	42
		46.05	C	0.01	-2.72	3.71
2244	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.143	42
		45.95	C	0.01	-2.62	1.24
2245	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.132	43
		17.01	T	-0.01	-6.69	4.66
2246	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.132	44
		16.65	T	-0.01	-6.69	0.00
2247	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.145	43
		20.65	T	-0.01	-7.05	4.94
2248	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.146	44
		20.94	T	-0.01	-7.05	0.00
2249	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.144	44
		13.00	C	-0.01	-6.58	4.28
2250	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.129	44
		7.23	T	-0.01	-6.84	0.00
2251	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.134	44
		5.59	C	-0.02	-6.43	4.53
2252	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.132	44
		10.97	T	-0.01	-6.64	0.00
2253	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.293	43
		48.40	T	-0.01	-14.04	4.94
2254	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.293	44
		45.10	T	-0.03	-14.08	0.00
2255	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.290	43

41.42 T                    0.00                    -14.11                    4.94

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2256	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.294	44
		44.05 T	-0.04	-14.13	0.00
2257	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.301	44
		23.81 C	-0.03	-13.29	4.53
2258	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.291	44
		43.60 T	0.01	-14.09	0.00
2259	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.291	43
		41.31 T	-0.03	-14.08	4.94
2260	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.288	44
		38.27 T	0.01	-14.07	0.00
2269	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.130	42
		48.61 C	-0.01	-2.11	3.54
2270	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.118	44
		31.96 C	-0.01	-3.17	4.32
2271	ST	W8X21	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.104	44
		28.92 C	-0.02	-2.69	4.32
2272	ST	W8X21	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.134	42
		47.31 C		-0.02	-2.48	3.54
2273	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.131	42
		48.80 C		0.00	-2.19	1.18
2274	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.101	43
		36.33 C		0.02	-1.74	0.00
2275	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.089	43
		33.14 C		0.02	-1.46	0.00
2276	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.135	42
		47.45 C		0.00	-2.53	1.18
2277	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.034	48
		26.92 T		0.00	-1.02	2.36
2278	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.070	43
		37.66 C		0.00	0.00	0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2279	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.083	43
		44.62 C		0.00	0.00	0.00
2280	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.034	43
		18.61 C		0.00	0.00	0.00

2281	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.033	48
			25.11 T	0.00	-1.02	2.36
2282	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.071	43
			38.18 C	0.00	0.00	0.00
2283	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.086	43
			46.12 C	0.00	0.00	0.00
2284	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.035	43
			18.96 C	0.00	0.00	0.00
2285	LD	L30306		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.207	43
			30.04 C	0.00	0.00	0.00
2286	LD	L30306		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.325	43
			47.23 C	0.00	0.00	0.00
2287	LD	L30306		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.573	43
			74.43 C	0.00	0.00	0.00
2288	LD	L30306		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.070	42
			37.12 T	0.00	-0.36	1.70
2289	LD	L30306		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.219	43
			40.72 C	0.00	0.00	0.00
2290	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.516	44
			389.77 C	13.15	46.74	1.70
2291	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.451	44
			14.32 C	-13.75	-69.00	0.00
2292	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.328	44
			90.12 C	7.46	54.22	2.50
2293	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.264	44

48.89 C                    9.00                    35.36                    1.55

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2295	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.386	44
		164.40 C	-16.28	-31.93	2.75
2296	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.331	43
		93.17 C	19.23	13.98	0.00
2297	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.209	43
		26.93 C	-11.78	-12.82	1.55
2298	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1A-C	0.810	43
		329.30 C	23.75	80.25	0.00
2301	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.424	44
		232.06 C	23.00	15.14	2.50
2302	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.448	43
		238.96 C	28.52	1.80	0.00
2303	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.199	43
		24.43 C	13.61	4.02	0.00
2304	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.171	43
		7.05 C	-12.27	2.31	0.50
2305	ST	W12X50	(AISC SECTIONS)		



			PASS	LRFD-H1-1B-C	0.215	44
		25.40 C		8.28	18.99	1.45
2310	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.435	44
		23.38 T		-26.81	-21.00	0.00
2311	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.146	44
		25.54 C		5.14	18.38	2.85
2313	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.459	44
		41.18 T		-7.70	-92.38	0.00
2314	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.442	44
		2.73 T		-1.45	-109.81	0.00
2315	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.708	42
		4.26 C		52.51	-5.45	0.00
2317	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.368	44
		35.04 T		-6.10	-72.70	0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2318	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.348	44
		9.74 T		-1.23	-87.40	0.00
2319	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.273	44
		8.62 T		-7.32	-45.70	0.00

2321	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.390	44
			49.83 C	2.43	89.30	2.85
2322	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.351	44
			21.59 T	-0.09	-91.50	0.00
2326	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.160	44
			6.26 C	-2.82	-25.43	4.35
2327	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.233	43
			12.01 C	4.32	36.53	0.00
2328	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.127	44
			4.10 C	-3.25	-17.33	3.75
2329	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.159	42
			3.86 C	4.74	-20.17	1.50
2330	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.141	43
			0.12 C	-6.58	-11.49	1.80
2331	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.192	44
			35.57 C	6.13	26.36	2.85
2332	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.183	42
			1.21 C	-5.60	-23.06	3.50
2333	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.311	44
			9.24 T	-18.16	-19.01	0.00
2334	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.141	44
			0.33 C	-6.58	-11.49	0.00
2335	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.180	45
			12.20 T	1.71	-4.35	0.41
2336	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.116	44

4.91 C                    1.42                    20.28                    1.50

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2337	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.184	42
		2.72 T	-5.60	-23.06	0.00
2338	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.121	42
		3.50 C	0.03	-7.26	1.43
2339	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.173	43
		5.16 T	-2.19	-2.37	0.41
2340	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.282	42
		2.31 T	-0.51	-15.55	0.00
2342	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.122	42
		5.64 T	-0.02	-7.26	0.00
2343	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.286	42
		7.84 T	-0.51	-15.55	0.10
2344	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.193	42
		3.32 C	0.21	-11.07	0.10
2345	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.259	42
		7.84 T	0.08	-15.50	0.91
2346	ST	W8X18	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.191	42
		3.32 C		-0.36	-10.39	0.91
2347	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.045	42
		3.77 T		0.00	-1.91	1.12
2348	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.048	42
		6.70 T		0.00	-1.92	1.12
2349	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.498	42
		71.67 T		1.91	-49.39	0.00
2350	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.171	44
		1.45 C		4.80	22.61	5.00
2351	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.771	42
		1.99 C		8.13	-17.68	0.41
2352	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.590	42
		5.41 T		-2.67	-26.55	0.10

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2353	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.568	42
		2.88 C		-2.13	-27.29	0.10
2354	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.590	42
		5.41 T		-2.67	-26.55	0.00

2355	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.772	42
			2.46 C	8.13	-17.68	0.00
2356	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.207	42
			5.41 T	-0.78	-9.74	0.00
2357	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.222	42
			3.35 C	0.88	-10.41	0.10
2358	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.222	42
			3.35 C	0.88	-10.41	0.00
2359	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.134	44
			5.77 T	-1.29	-3.29	0.00
2360	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.104	44
			43.62 C	1.62	18.99	1.42
2361	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.243	44
			10.60 C	1.52	58.38	1.42
2362	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.120	44
			3.03 C	1.76	25.48	1.42
2363	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.086	42
			9.04 T	0.00	-4.88	0.90
2364	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.123	42
			1.03 C	-0.03	-7.53	1.13
2365	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.123	42
			1.45 C	0.01	-7.53	0.38
2366	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.093	42
			48.98 C	0.00	-3.39	0.75
2367	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.083	42

9.43 C                    0.00                    -2.56                    1.27

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2368	ST	C10X15	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.060	42
		0.21 T	0.00	-2.55	1.27
2369	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.546	42
		28.97 C	-1.88	-56.58	0.00
2370	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.193	44
		19.17 C	1.24	35.97	1.50
2371	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.316	43
		4.88 T	-2.54	-10.02	0.41
2372	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.375	42
		4.69 C	0.57	-20.99	0.00
2373	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.479	44
		18.12 T	-2.47	-19.92	0.00
2374	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.627	43
		3.39 T	-6.99	-12.87	0.91
2376	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.627	44
		4.54 T	-6.99	-12.87	0.00
2377	ST	C10X15	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.348	44
		7.33 C		1.96	-1.82	1.03
2378	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.352	44
		9.55 C		2.00	-1.55	1.03
2381	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.473	43
		8.39 T		-2.47	-19.92	0.91
2383	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.156	44
		7.73 T		-7.14	-12.83	0.00
2385	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.225	44
		12.25 C		3.61	0.04	0.10
2387	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.586	43
		10.17 T		-7.62	-7.76	0.91
2388	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.587	44
		10.70 T		-7.62	-7.76	0.00

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2390	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.737	43
		20.13 T		-3.93	-5.61	1.25
2391	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.749	43
		27.37 T		-3.97	-5.66	1.25

2392	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.582	44	
		100.51	T	-6.47	-45.17	0.00	
2393	ST	W8X18		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.343	43	
		13.34	C	5.58	0.00	0.00	
2394	ST	W8X18		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.298	44	
		11.08	T	-4.58	1.03	0.00	
2397	ST	C10X15		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.384	44	
		8.04	T	-2.02	-3.46	0.00	
2398	ST	C10X15		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.513	43	
		18.54	C	2.91	2.28	0.00	
2399	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.428	43	
		22.98	T	-0.78	-57.96	4.75	
2400	ST	W12X35		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.642	44	
		35.68	C	-7.76	-50.80	1.60	
2401	ST	W12X50		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.229	44	
		9.50	C	5.31	32.64	5.00	
2402	ST	W8X18		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.632	43	
		13.96	T	-8.21	-8.23	0.41	
2403	ST	W8X18		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.255	42	
		2.80	T	0.36	-14.42	0.00	
2404	ST	W8X18		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-C	0.373	44	
		16.09	C	4.88	4.32	0.10	
2405	ST	W8X18		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.238	42	
		2.80	T	0.33	-13.43	0.00	
2406	ST	W8X18		(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.076	42	



2.80 T                    0.10                    -4.25                    0.00

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2407	ST W8X18		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.633	44
		15.26 T	-8.21	-8.23	0.00
2408	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.374	44
		31.37 C	4.29	81.99	1.42
2409	ST W8X18		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.009	43
		13.53 T	0.00	0.00	1.00
2410	ST W8X18		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.203	44
		8.46 T	-3.19	-0.44	0.00
2411	ST W18X86		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.428	44
		299.06 C	-39.64	-104.07	0.00
2412	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.489	44
		303.76 C	7.88	79.29	2.50
2413	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.495	44
		11.31 T	-0.57	-128.48	0.00
2414	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.333	44
		14.79 T	-0.78	-57.96	0.00
2415	ST W12X50		(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.266	44
		3.46 C		1.25	52.18	1.75
2416	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.608	44
		0.96 C		8.20	7.29	0.41
2417	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.670	43
		2.60 T		-2.39	-32.72	0.10
2418	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.678	44
		4.32 T		-2.53	-32.62	0.00
2419	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.745	43
		2.02 T		-7.90	-16.94	0.91
2420	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.746	44
		2.76 T		-7.90	-16.94	0.00
2421	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.677	43
		2.48 T		-2.53	-32.62	0.91

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2422	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.395	44
		39.72 C		2.09	-1.48	1.03
2423	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.371	44
		9.10 C		2.14	-1.46	1.03

2424	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.165	43
			1.86 T	-0.68	-7.67	0.41
2425	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.566	44
			5.36 T	-2.36	-26.22	0.00
2426	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.564	43
			1.85 T	-2.36	-26.22	0.10
2427	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.531	44
			2.63 T	-1.41	-27.67	0.00
2428	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.483	44
			0.97 T	-4.62	-12.82	0.00
2429	ST	W8X18		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.525	43
			1.86 T	-2.20	-24.40	0.91
2430	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.592	44
			11.82 T	-0.39	-42.31	0.00
2431	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.316	43
			7.52 C	0.38	21.81	0.00
2445	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.348	43
			35.35 C	3.65	10.76	0.00
2446	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.416	43
			13.32 C	3.89	15.98	0.00
2447	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.554	43
			31.16 T	-3.01	-3.19	1.03
2448	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.548	43
			19.22 T	-3.05	-3.04	1.03
2449	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.343	44

15.31 C                    1.84                    83.47                    1.42

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2450	ST	W8X18	(AISC SECTIONS)		
		PASS	SHEAR-Y	0.056	42
		0.15 T	0.00	0.00	0.00
2451	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.327	44
		29.56 C	0.68	82.25	1.42
2452	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.449	44
		20.29 C	0.31	116.51	1.42
2453	ST	W8X18	(AISC SECTIONS)		
		PASS	SHEAR-Y	0.056	42
		1.47 C	0.00	0.00	0.00
2454	ST	W8X18	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.076	42
		0.99 C	0.00	-4.62	0.87
2455	ST	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.266	43
		28.68 C	0.00	0.00	0.00
2456	ST	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.219	43
		23.61 C	0.00	0.00	0.00
2457	ST	L30306	(AISC SECTIONS)		
		PASS	COMPRESSION	0.198	43
		17.82 C	0.00	0.00	0.00
2458	ST	L30306	(AISC SECTIONS)		

			PASS	COMPRESSION	0.128	43
		11.55	C	0.00	0.00	0.00
2459	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.228	43
		24.59	C	0.00	0.00	0.00
2460	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.243	45
		26.16	C	0.00	0.00	0.00
2461	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.146	47
		15.77	C	0.00	0.00	0.00
2462	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.068	43
		7.29	C	0.00	0.00	0.00
2463	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.186	43
		20.02	C	0.00	0.00	0.00
2464	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.366	43
		31.77	C	0.00	0.00	0.00

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2465	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.179	43
		15.52	C	0.00	0.00	0.00
2466	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.107	43
		11.57	C	0.00	0.00	0.00

2467	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.100	43
			10.76 C	0.00	0.00	0.00
2468	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.114	43
			12.27 C	0.00	0.00	0.00
2469	ST	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.136	43
			14.70 C	0.00	0.00	0.00
2473	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.284	44
			36.23 T	-1.07	-4.99	0.00
2474	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.275	44
			15.54 T	-1.15	-4.71	0.00
2475	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.358	42
			15.81 T	1.00	-10.32	0.00
2476	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.377	42
			14.01 C	1.06	-10.64	0.00
2477	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.218	44
			19.75 T	-0.82	-4.22	0.00
2478	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.221	44
			14.26 T	-0.88	-4.06	0.00
2479	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.246	43
			43.08 C	1.05	-2.12	0.00
2480	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.233	43
			21.02 C	1.11	-2.11	0.00
2485	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.374	43
			21.58 T	-1.38	-7.66	1.25
2486	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.391	43

32.37 T            -1.44            -7.62            1.25

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ALL UNITS ARE - KN    METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2487	ST	C10X15	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.569	43
		6.03 C	3.46	-1.02	0.00
2488	ST	C10X15	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.601	43
		27.07 C	3.49	-1.16	0.00
2489	ST	C10X15	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.210	42
		36.67 T	-0.25	-8.04	1.02
2490	ST	C10X15	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.165	44
		16.37 T	-0.60	-3.25	0.00
2491	ST	C10X15	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.185	44
		33.93 C	-0.49	-4.15	0.94
2492	ST	C10X15	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.167	44
		13.95 C	0.74	-2.04	1.02
2493	ST	C10X15	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.290	44
		38.03 C	-1.14	-4.16	0.94
2494	ST	C10X15	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.291	43
		25.47 T	-1.26	-4.09	1.02
2495	ST	C10X15	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.320	44
		34.00	T	-1.31	-4.95	0.00
2496	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.302	44
		19.83	C	1.54	2.28	1.02
2497	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.438	44
		16.64	T	-2.35	-3.17	0.00
2498	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.439	44
		13.74	T	-2.39	-3.03	0.00
2499	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.482	43
		40.97	C	2.62	-1.61	0.00
2500	ST	C10X15		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.466	43
		20.15	C	2.66	-1.57	0.00
2509	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.235	44
		14.72	C	5.32	33.80	2.50

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2510	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.227	43
		155.29	C	15.08	73.49	0.00
2511	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.148	44
		71.19	C	-5.84	-69.68	2.75



2512	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.262	44
			23.18 C	1.87	48.48	2.50
2513	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.239	43
			32.40 C	5.32	33.80	0.00
2514	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.258	43
			10.32 C	1.87	48.48	0.00
2515	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.142	44
			1.71 C	-5.53	-74.68	0.00
2516	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.151	44
			28.56 C	-7.84	-69.04	0.00
2517	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.074	43
			103.45 C	0.00	0.00	0.00
2518	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.080	43
			112.30 C	0.00	0.00	0.00
2519	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.199	44
			37.45 T	-1.99	-34.46	0.00
2520	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.232	44
			25.53 T	-3.44	-38.08	0.00
2521	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.035	43
			87.95 T	0.00	0.00	3.54
2522	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.037	43
			94.44 T	0.00	0.00	3.54
2526	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.240	43
			8.86 T	-0.06	-37.63	5.00
2527	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.176	43

2.11 T            -0.09            -27.61            5.00

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ALL UNITS ARE - KN    METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2528	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.240	44
		7.53 T	-0.06	-37.63	0.00
2529	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.176	44
		2.48 T	-0.09	-27.61	0.00
2530	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.028	42
		10.09 C	0.00	-4.65	3.25
2531	ST W12X50		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.025	40
		0.01 T	0.00	-5.24	3.25
2532	ST W6X16		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.112	42
		31.77 C	0.00	-2.65	0.00
2533	ST W6X16		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.064	43
		33.57 T	-0.35	-0.39	2.50
2534	ST W6X16		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.114	43
		29.62 C	0.25	2.01	0.00
2535	LD L30306		(AISC SECTIONS)		
		PASS	TENSION	0.218	42
		132.45 T	0.00	0.00	0.00
2536	LD L30306		(AISC SECTIONS)		

			PASS	COMPRESSION	0.456	42
		78.65	C	0.00	0.00	0.00
2537	LD	L30306		(AISC SECTIONS)		
			PASS	COMPRESSION	0.344	44
		59.36	C	0.00	0.00	3.54
2538	LD	L30306		(AISC SECTIONS)		
			PASS	TENSION	0.169	43
		102.62	T	0.00	0.00	3.54
2539	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.475	43
		96.98	C	-4.85	-101.76	2.50
2540	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.446	41
		89.70	C	-1.52	-53.81	0.00
2541	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.521	42
		60.50	T	-33.05	-77.98	0.00
2542	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.097	44
		25.87	T	-4.87	-6.10	0.00
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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2543	ST	W12X50		(AISC SECTIONS)	
			PASS	LRFD-H1-1B-C	0.145
		149.29	C	6.07	-5.66
					0.00
2544	ST	W8X21		(AISC SECTIONS)	
			PASS	COMPRESSION	0.045
		15.82	C	0.00	0.00
					0.00

2545	ST	W8X21		(AISC SECTIONS)		
			PASS	COMPRESSION	0.036	43
			12.64 C	0.00	0.00	0.00
2546	ST	W14X68		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.802	42
			120.31 T	53.28	159.46	2.60
2547	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.090	47
			32.43 C	-1.84	-2.11	2.62
2549	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.134	48
			2.17 C	2.74	-7.39	2.71
2550	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.241	44
			34.10 C	-36.11	-18.20	0.00
2551	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.223	44
			4.25 C	-30.14	-32.88	0.00
2554	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.430	43
			15.15 C	0.01	80.19	0.00
2555	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.475	44
			9.12 C	0.01	89.57	2.88
2556	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.016	48
			2.30 T	0.49	-0.56	1.35
2557	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.174	44
			11.16 T	-4.64	-10.00	0.00
2559	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.184	43
			9.64 T	-6.02	-27.36	1.35
2561	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.166	42
			31.63 T	2.28	-17.53	0.00
2562	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.185	42

5.60 T            -1.15            -44.71            0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2564	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.280	42
		5.60 T	-0.52	-72.06	0.00
2566	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.326	44
		3.98 T	-10.36	-50.13	0.00
2568	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.326	43
		5.35 T	-10.36	-50.13	1.35
2569	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.297	43
		7.47 T	-9.76	-44.52	1.35
2570	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.318	43
		1.41 C	-9.53	-36.64	2.70
2571	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.465	44
		38.03 C	-8.90	-64.03	1.80
2572	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.101	48
		1.92 C	4.78	-6.99	4.00
2573	ST	W12X50	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.611	44
		9.37 C	17.89	71.05	2.65
2574	ST	W12X50	(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.895	44
		45.56 C		17.64	123.51	2.65
2575	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.308	47
		1.82 C		-4.15	-47.76	3.33
2576	ST	W8X21		(AISC SECTIONS)		
			PASS	COMPRESSION	0.045	43
		15.95 C		0.00	0.00	0.00
2577	ST	W8X21		(AISC SECTIONS)		
			PASS	COMPRESSION	0.036	43
		12.74 C		0.00	0.00	0.00
2578	ST	W8X21		(AISC SECTIONS)		
			PASS	COMPRESSION	0.026	43
		9.28 C		0.00	0.00	0.00
2579	ST	W8X21		(AISC SECTIONS)		
			PASS	COMPRESSION	0.025	43
		8.88 C		0.00	0.00	0.00
2580	ST	W8X21		(AISC SECTIONS)		
			PASS	COMPRESSION	0.036	43
		12.67 C		0.00	0.00	0.00

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2581	ST	W8X21		(AISC SECTIONS)		
			PASS	COMPRESSION	0.034	43
		12.20 C		0.00	0.00	0.00
2582	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.181	44
		45.56 C		0.01	27.16	3.00

2583	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.229	44
			9.26 C	0.00	27.18	2.88
2584	ST	W12X65		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.316	44
			25.39 T	-1.97	-90.35	0.00
2585	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.370	43
			50.99 C	0.08	40.51	0.00
2588	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.359	44
			32.79 C	0.17	65.74	2.88

\*\*\*\*\* END OF TABULATED RESULT OF DESIGN \*\*\*\*\*

2228. CHECK CODE MEMB 2723 TO 2726

STAAD SPACE

-- PAGE NO. 264

STAAD.Pro CODE CHECKING - (LRFD 3RD EDITION)

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2723	ST	W12X50		(AISC SECTIONS)		
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			PASS	LRFD-H1-1B-T	0.383	43
		3.26 T		-0.16	-92.87	4.00
2724	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.384	43
		3.40 T		0.08	-93.41	4.00
2725	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.383	44
		3.26 T		-0.18	-92.85	0.00
2726	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.384	44
		3.39 T		0.07	-93.35	0.00

\*\*\*\*\* END OF TABULATED RESULT OF DESIGN \*\*\*\*\*

2229. CHECK CODE MEMB 2685 TO 2718

STAAD SPACE

-- PAGE NO. 265

STAAD.Pro CODE CHECKING - (LRFD 3RD EDITION)

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2685	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.240	44



			117.41 T	-3.73	-14.61	0.00
2686	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.232	44
			92.95 T	-3.73	-14.66	0.00
2687	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.101	40
			0.00 T	0.00	-19.15	4.00
2688	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.261	44
			85.23 T	-4.98	-14.80	0.00
2689	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.252	44
			58.98 T	-4.95	-14.76	0.00
2690	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.101	40
			0.01 T	0.00	-19.15	4.00
2691	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.133	43
			1.75 T	-0.04	-24.96	4.00
2692	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.124	43
			1.79 T	0.00	-23.48	4.00
2693	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.276	44
			201.97 T	-4.40	-14.16	0.00
2694	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.266	44
			175.16 T	-4.39	-14.13	0.00
2695	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.101	40
			0.00 T	0.00	-19.15	4.00
2696	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.256	44
			170.12 T	-3.82	-14.01	0.00
2697	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.243	44
			140.29 T	-3.78	-13.83	0.00
2698	ST	W12X50		(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.101	40
		0.01 T		0.00	-19.15	4.00
2699	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.125	43
		1.13 T		-0.14	-23.24	4.00
2700	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.126	43
		1.09 T		0.06	-23.64	4.00
STAAD SPACE						-- PAGE NO. 266

ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2701	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.101	41
		0.08 T		0.00	-19.15	4.00
2702	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.101	41
		0.11 C		0.00	-19.15	4.00
2703	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.119	40
		0.09 T		0.05	-22.36	4.00
2704	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.118	41
		0.20 T		0.01	-22.26	4.00
2705	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.683	44
		103.73 T		-7.62	-53.52	0.00
2706	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.101	41
		0.09 C		0.00	-19.15	4.00

2707	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.581	42
			49.22 T	-0.12	-65.01	3.25
2708	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.581	42
			49.22 T	-0.12	-65.01	0.00
2709	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.324	43
			54.55 T	-6.11	-18.18	3.25
2710	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.324	44
			53.27 T	-6.11	-18.18	0.00
2711	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.317	44
			34.68 T	-6.08	-18.21	0.00
2712	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.101	40
			0.01 T	0.00	-19.15	4.00
2713	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.124	44
			1.78 T	-0.01	-23.48	0.00
2714	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.133	44
			1.74 T	-0.03	-24.98	0.00
2715	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.124	44
			1.15 T	-0.13	-23.21	0.00

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2716	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.126	44
			1.11 T	0.07	-23.62	0.00
2717	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.119	41
			0.10 C	0.06	-22.43	0.00
2718	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.118	41
			0.00 C	0.01	-22.42	0.00

\*\*\*\*\* END OF TABULATED RESULT OF DESIGN \*\*\*\*\*

2230. CHECK CODE MEMB 2663 TO 2684 2719 TO 2722

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-- PAGE NO. 268

STAAD.Pro CODE CHECKING - (LRFD 3RD EDITION)

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2663	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.216	44
			44.93 C	-3.73	-17.12	2.98

2664	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.282	43
		145.16	C	5.47	-11.62	0.00
2665	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.285	43
		148.40	C	5.54	-11.65	0.00
2666	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.235	43
		85.37	C	4.39	-11.94	0.00
2667	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.226	43
		70.74	C	4.33	-11.75	0.00
2668	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.101	40
		0.03	T	0.00	-19.14	4.00
2669	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.101	40
		0.03	T	0.00	-19.14	4.00
2670	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.215	43
		49.88	C	4.16	-14.71	0.00
2671	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.640	44
		12.99	T	-7.79	-51.61	0.00
2672	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.101	40
		0.02	C	0.00	-19.14	4.00
2673	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.101	40
		0.06	T	0.00	-19.14	4.00
2674	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.101	40
		0.07	T	0.00	-19.14	4.00
2675	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.396	43
		237.15	C	4.30	-12.54	0.00
2676	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.249	43

			134.21 C	3.82	-12.45	0.00
2677	ST	W12X35	(AISC SECTIONS)			
			PASS	LRFD-H1-1A-C	0.378	43
			218.46 C	4.26	-12.59	0.00
2678	ST	W12X35	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.250	44
			135.35 T	-3.82	-14.78	0.00

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-- PAGE NO. 269

ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2679	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.101	40
			0.02 T	0.00	-19.14	4.00
2680	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.101	40
			0.02 T	0.00	-19.14	4.00
2681	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.128	43
			2.01 T	-0.11	-30.69	4.00
2682	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.132	43
			1.98 T	-0.14	-31.58	4.00
2683	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.123	43
			0.90 T	-0.01	-30.03	4.00
2684	ST	W12X50	(AISC SECTIONS)			
			PASS	LRFD-H1-1B-T	0.121	43
			0.92 T	-0.10	-29.21	4.00
2719	ST	W12X50	(AISC SECTIONS)			

			PASS	LRFD-H1-1B-T	0.132	44
		2.03	T	-0.14	-31.58	0.00
2720	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.128	44
		2.03	T	-0.11	-30.69	0.00
2721	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.121	44
		1.03	T	-0.10	-29.21	0.00
2722	ST	W12X50		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.123	44
		1.01	T	-0.01	-30.03	0.00

\*\*\*\*\* END OF TABULATED RESULT OF DESIGN \*\*\*\*\*

2231. CHECK CODE MEMB 2664 TO 2667 2671 2675 TO 2678

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STAAD.Pro CODE CHECKING - (LRFD 3RD EDITION)

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2664	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.282	43

			145.16 C	5.47	-11.62	0.00
2665	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.285	43
			148.40 C	5.54	-11.65	0.00
2666	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.235	43
			85.37 C	4.39	-11.94	0.00
2667	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.226	43
			70.74 C	4.33	-11.75	0.00
2671	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.640	44
			12.99 T	-7.79	-51.61	0.00
2675	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.396	43
			237.15 C	4.30	-12.54	0.00
2676	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.249	43
			134.21 C	3.82	-12.45	0.00
2677	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1A-C	0.378	43
			218.46 C	4.26	-12.59	0.00
2678	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.250	44
			135.35 T	-3.82	-14.78	0.00

\*\*\*\*\* END OF TABULATED RESULT OF DESIGN \*\*\*\*\*

2232. CHECK CODE MEMB 2604 TO 2631 2633 TO 2662

STAAD SPACE

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2604	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.284	43
		31.55 C	10.94	0.06	0.00
2605	ST	W18X86	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.154	44
		16.49 T	-5.05	-83.05	0.00
2606	ST	W14X68	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.428	42
		116.00 C	-6.04	-120.06	0.00
2607	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.254	44
		12.78 T	0.31	-39.66	0.00
2608	ST	W12X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.354	43
		20.60 T	7.32	8.68	0.92
2609	ST	W12X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.088	43
		1.29 T	-2.66	-1.57	1.18
2610	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.920	43
		5.99 C	32.69	22.60	0.00
2611	ST	W12X35	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.281	43
		22.23 C	4.81	24.75	0.00
2612	ST	W12X30	(AISC SECTIONS)		
		PASS	LRFD-H1-1B-C	0.233	43

			12.12 C	0.51	14.06	0.00
2613	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.087	43
			5.05 T	-2.60	-1.49	1.18
2614	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.228	43
			7.41 T	-7.32	-1.70	1.00
2615	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.259	44
			6.52 T	-8.80	-6.84	0.00
2616	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.228	43
			24.07 T	-7.09	-1.84	1.00
2617	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.120	44
			38.58 C	-0.32	-14.88	3.21
2618	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.327	44
			10.29 C	9.70	2.57	1.00
2619	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.045	44
			2.74 C	-1.67	-23.74	0.00

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2620	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.090	44
			13.60 C	-0.21	13.98	2.65
2621	ST	W12X30		(AISC SECTIONS)		

			PASS	LRFD-H1-1B-C	0.533	43
		7.13 C		3.56	28.02	0.00
2622	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.086	43
		1.88 C		0.22	-14.06	2.65
2623	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.078	44
		28.79 C		-0.05	-11.33	2.43
2624	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.107	44
		10.72 T		-2.48	-4.45	0.00
2625	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.063	43
		2.83 T		-0.04	-10.75	2.65
2626	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.425	43
		6.36 T		-2.52	-23.04	3.85
2627	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.072	44
		24.13 C		0.18	9.93	2.65
2628	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.092	44
		8.59 T		-2.49	-2.21	0.00
2629	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.144	44
		30.05 C		3.67	2.01	3.85
2630	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.119	44
		7.12 C		3.59	1.11	3.85
2631	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.431	43
		1.05 T		4.58	19.60	0.00
2633	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.634	43
		13.96 C		23.44	10.74	0.00
2634	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.815	44
		13.66 C		25.07	27.95	1.40

2635	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.655	42
			23.33 T	11.57	-51.32	1.25
STAAD SPACE						-- PAGE NO. 273

ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2636	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.148	43
			0.53 T	-4.49	-6.07	3.85
2637	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.708	43
			7.66 T	-7.05	-74.95	1.25
2638	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.143	44
			2.70 T	-4.49	-6.07	0.00
2639	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.242	44
			9.26 C	7.90	-1.09	1.85
2640	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.234	44
			2.14 C	7.56	-1.90	1.85
2641	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.240	43
			4.63 C	7.90	-1.08	0.00
2642	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.238	43
			11.98 C	7.56	-1.90	0.00
2643	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.349	43

			0.89 T	7.89	7.77	0.00
2644	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.325	43
			5.59 C	9.70	2.57	0.00
2645	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.342	44
			8.05 T	-8.00	-6.93	0.00
2646	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.386	44
			7.43 T	-9.73	-6.50	0.00
2647	ST	W12X40		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.012	41
			11.49 T	0.00	-1.80	0.71
2648	ST	W12X40		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.012	41
			11.53 C	0.00	-1.80	0.71
2649	ST	W12X40		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.012	41
			12.18 C	0.00	-1.80	0.71
2650	ST	W12X40		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.012	41
			12.19 T	0.00	-1.80	0.71

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2651	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.090	44
			6.16 T	-2.66	-1.57	0.00
2652	ST	W12X30		(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.086	44
		1.29	T	-2.60	-1.49	0.00
2653	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.055	43
		15.86	T	-1.41	1.18	1.18
2654	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.053	43
		4.55	T	-1.41	1.50	1.18
2655	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.211	43
		0.31	C	-4.27	-5.71	1.49
2656	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.358	43
		1.09	C	-9.14	-5.97	1.49
2657	ST	W12X40		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.007	41
		7.00	T	0.00	-1.12	0.83
2658	ST	W12X40		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.007	41
		5.67	C	0.00	-1.12	0.83
2659	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.215	44
		8.62	T	-4.27	-5.70	0.00
2660	ST	W12X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.385	44
		9.68	C	-9.68	-6.46	0.17
2661	ST	W12X40		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.008	41
		7.01	C	0.00	-1.12	0.83
2662	ST	W12X40		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.007	41
		5.63	T	0.00	-1.12	0.83

\*\*\*\*\* END OF TABULATED RESULT OF DESIGN \*\*\*\*\*

STAAD.Pro CODE CHECKING - (LRFD 3RD EDITION)

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
=====					
2594	ST W10X30		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.536	43
		13.42 T	-7.45	-35.76	2.90
2595	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.401	43
		3.78 T	-10.92	-23.12	2.90
2596	ST W10X30		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.529	43
		6.66 T	-7.48	-35.16	2.90
2597	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.404	43
		4.53 T	-10.95	-23.39	2.90
2598	ST W10X30		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.534	44
		10.17 T	-7.45	-35.76	0.00
2599	ST W12X35		(AISC SECTIONS)		
		PASS	LRFD-H1-1B-T	0.402	44
		4.59 T	-10.92	-23.13	0.00
2601	ST W10X30		(AISC SECTIONS)		

			PASS	LRFD-H1-1B-T	0.528	44
		3.40 T		-7.48	-35.17	0.00
2602	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-T	0.404	44
		6.41 T		-10.95	-23.39	0.00
2632	ST	W12X35		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.970	44
		41.28 C		28.14	50.31	1.35

\*\*\*\*\* END OF TABULATED RESULT OF DESIGN \*\*\*\*\*

2234. CHECK CODE MEMB 2589 2590 2593 2600 2603 2727 TO 2734

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STAAD.Pro CODE CHECKING - (LRFD 3RD EDITION)

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ALL UNITS ARE - KN METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
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2589	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.327	43
		40.73 C		0.07	36.14	0.00
2590	ST	W18X86		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.222	43



			42.46 C	26.07	43.74	0.00
2593	ST	W10X30		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.483	44
			12.07 C	0.07	30.09	2.88
2600	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.155	40
			0.00 C	0.00	-10.30	1.20
2603	ST	W8X21		(AISC SECTIONS)		
			PASS	LRFD-H1-1B-C	0.155	40
			0.00 C	0.00	-10.30	1.20
2727	LD	L40406		(AISC SECTIONS)		
			PASS	COMPRESSION	0.055	43
			14.85 C	0.00	0.00	0.00
2728	LD	L40406		(AISC SECTIONS)		
			PASS	COMPRESSION	0.147	43
			29.03 C	0.00	0.00	0.00
2729	LD	L40406		(AISC SECTIONS)		
			PASS	COMPRESSION	0.037	43
			13.30 C	0.00	0.00	0.00
2730	LD	L40406		(AISC SECTIONS)		
			PASS	COMPRESSION	0.109	43
			26.72 C	0.00	0.00	0.00
2731	LD	L40406		(AISC SECTIONS)		
			PASS	COMPRESSION	0.039	43
			10.48 C	0.00	0.00	0.00
2732	LD	L40406		(AISC SECTIONS)		
			PASS	COMPRESSION	0.112	43
			22.04 C	0.00	0.00	0.00
2733	LD	L40406		(AISC SECTIONS)		
			PASS	COMPRESSION	0.042	43
			14.80 C	0.00	0.00	0.00
2734	LD	L40406		(AISC SECTIONS)		
			PASS	COMPRESSION	0.084	43
			20.62 C	0.00	0.00	0.00

\*\*\*\*\* END OF TABULATED RESULT OF DESIGN \*\*\*\*\*

2235. STEEL MEMBER TAKE OFF LIST 482 TO 506 509 512 515 518 521 524 527 530 TO 589 -  
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STEEL TAKE-OFF  
 -----

PROFILE	LENGTH (METE)	WEIGHT (KN )
2236. 591 TO 631 633 TO 694 696 TO 703 705 TO 708 710 TO 723 725 TO 738 -		
2237. 740 TO 753 755 TO 762 877 879 881 883 885 887 889 TO 891 1009 1011 1013 1015 -		
2238. 1017 1019 1021 TO 1023 1027 TO 1035 1051 TO 1053 1055 1056 1061 TO 1065 1067 -		
2239. 1070 1073 TO 1081 1083 1085 TO 1087 1089 TO 1107 1109 TO 1114 1165 1174 1183 -		
2240. 1253 1438 TO 1456 1465 TO 1484 1493 TO 1525 1528 1531 1533 1543 1544 1584 -		
2241. 1594 1597 1599 1600 1605 TO 1611 1613 1615 TO 1621 1623 1625 TO 1631 1633 -		
2242. 1639 TO 1646 1651 TO 1669 1685 1686 1689 TO 1727 1729 TO 1772 1777 1778 1781 -		
2243. 1782 TO 1784 1787 1788 1793 1794 1797 TO 1800 1803 TO 1849 1851 TO 1853 1855 -		
2244. 1856 TO 1865 1869 1870 1872 1873 1879 1880 1883 TO 1924 1926 TO 1945 1981 -		
2245. 1982 TO 1990 1995 1996 2005 TO 2024 2029 2030 2037 TO 2055 2059 2061 TO 2067 -		
2246. 2069 2070 2072 2073 2075 2076 2078 2079 2081 TO 2087 2089 TO 2101 2103 2106 -		
2247. 2107 TO 2111 2113 TO 2121 2124 TO 2127 2129 2131 TO 2135 2229 TO 2260 2269 -		
2248. 2270 TO 2293 2295 TO 2298 2301 TO 2305 2310 2311 2313 TO 2315 2317 TO 2319 -		
2249. 2321 2322 2326 TO 2340 2342 TO 2374 2376 TO 2378 2381 2383 2385 2387 2388 -		
2250. 2390 TO 2394 2397 TO 2431 2445 TO 2469 2473 TO 2522 2526 TO 2547 -		
2251. 2549 TO 2551 2554 TO 2557 2559 2561 2562 2564 2566 2568 TO 2585 2588 TO 2590 -		
2252. 2593 TO 2734		
ST W18X86	441.75	553.903
ST W12X50	1210.47	881.874
ST W14X68	99.95	99.072
ST W12X65	163.60	154.865
ST W12X35	482.00	246.049
ST W8X21	490.23	149.664
ST W6X16	127.50	29.952
LD L35358	48.18	15.520
LD L30306	197.02	41.187
ST W12X30	77.96	33.964

ST	W10X30	220.25	96.494
ST	W8X18	38.10	9.932
ST	C10X15	44.63	9.932
ST	L30306	31.86	3.331
ST	W12X40	12.33	7.212
LD	L40406	36.29	10.285
PRISMATIC STEEL		10.94	6.305
-----			
TOTAL =			2349.539

TOTAL VOLUME OF PRISMATIC STEEL SECTIONS = 0.08 CUBIC METE

MEMBER	PROFILE	LENGTH (METE)	WEIGHT (KN )
482	ST W18X86	5.20	6.520
483	ST W18X86	1.50	1.881
484	ST W18X86	1.50	1.881
485	ST W12X50	8.00	5.828
486	ST W12X50	8.00	5.828
487	ST W14X68	8.00	7.930
488	ST W18X86	5.20	6.520
489	ST W18X86	3.00	3.762
490	ST W18X86	2.50	3.135

STAAD SPACE

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491	ST W18X86	4.00	5.016
492	ST W18X86	1.50	1.881
493	ST W12X50	8.00	5.828
494	ST W18X86	5.20	6.520
495	ST W18X86	1.50	1.881
496	ST W18X86	1.50	1.881
497	ST W12X50	8.00	5.828
498	ST W12X50	8.00	5.828
499	ST W12X50	8.00	5.828
500	ST W18X86	5.20	6.520

501	ST	W18X86	3.00	3.762
502	ST	W18X86	2.50	3.135
503	ST	W18X86	4.00	5.016
504	ST	W18X86	1.50	1.881
505	ST	W12X50	8.00	5.828
506	ST	W12X65	8.00	7.573
509	ST	W12X65	8.00	7.573
512	ST	W12X50	8.00	5.828
515	ST	W12X50	8.00	5.828
518	ST	W12X50	8.00	5.828
521	ST	W12X50	8.00	5.828
524	ST	W12X50	2.65	1.931
527	ST	W12X50	2.65	1.931
530	ST	W18X86	4.20	5.266
531	ST	W18X86	4.20	5.266
532	ST	W12X50	2.75	2.003
533	ST	W12X50	2.75	2.003
534	ST	W18X86	3.00	3.762
535	ST	W18X86	2.50	3.135
536	ST	W18X86	2.50	3.135
537	ST	W18X86	3.00	3.762
538	ST	W18X86	2.50	3.135
539	ST	W18X86	2.50	3.135
540	ST	W14X68	8.00	7.930
541	ST	W12X50	8.00	5.828
542	ST	W14X68	2.50	2.478
543	ST	W18X86	2.50	3.135
544	ST	W12X50	2.75	2.003
545	ST	W18X86	2.50	3.135
546	ST	W12X50	2.75	2.003
547	ST	W18X86	1.00	1.254
548	ST	W12X50	2.75	2.003
549	ST	W18X86	1.00	1.254
550	ST	W12X50	2.75	2.003
551	ST	W18X86	1.50	1.881
552	ST	W12X50	2.75	2.003
553	ST	W18X86	1.50	1.881
554	ST	W12X50	2.75	2.003

555	ST	W18X86	0.50	0.627
556	ST	W18X86	0.50	0.627
557	ST	W12X50	4.75	3.461
558	ST	W18X86	4.20	5.266
559	ST	W18X86	4.20	5.266
560	ST	W18X86	3.00	3.762
561	ST	W18X86	2.50	3.135
562	ST	W18X86	3.00	3.762

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563	ST	W18X86	2.50	3.135
564	ST	W14X68	4.00	3.965
565	ST	W12X50	8.00	5.828
566	ST	W12X65	2.50	2.367
567	ST	W18X86	3.00	3.762
568	ST	W18X86	3.00	3.762
569	ST	W12X50	3.85	2.805
570	ST	W12X35	1.25	0.638
571	ST	W12X35	2.50	1.276
572	ST	W12X35	5.25	2.680
573	ST	W12X35	2.63	1.340
574	ST	W12X35	2.63	1.340
575	ST	W12X35	5.25	2.680
576	ST	W12X35	2.63	1.340
577	ST	W12X35	2.63	1.340
578	ST	W18X86	1.70	2.132
579	ST	W18X86	4.20	5.266
580	ST	W18X86	1.50	1.881
581	ST	W18X86	1.25	1.567
582	ST	W18X86	3.00	3.762
583	ST	W18X86	2.50	3.135
584	ST	W14X68	4.00	3.965
585	ST	W12X50	8.00	5.828
586	ST	W12X65	2.50	2.367
587	ST	W18X86	1.50	1.881
588	ST	W18X86	1.55	1.944
589	ST	W14X68	3.50	3.469
591	ST	W18X86	4.20	5.266

592	ST	W18X86	1.50	1.881
593	ST	W18X86	1.25	1.567
594	ST	W18X86	3.00	3.762
595	ST	W18X86	2.50	3.135
596	ST	W12X65	4.00	3.786
597	ST	W12X50	8.00	5.828
598	ST	W12X65	2.50	2.367
599	ST	W18X86	1.50	1.881
600	ST	W18X86	1.55	1.944
601	ST	W12X35	3.50	1.787
602	ST	W12X35	2.62	1.340
603	ST	W12X35	1.50	0.766
604	ST	W12X35	2.62	1.340
605	ST	W12X35	2.62	1.340
606	ST	W12X35	5.25	2.680
607	ST	W12X35	2.62	1.340
608	ST	W12X35	3.25	1.659
609	ST	W12X35	5.25	2.680
610	ST	W12X35	1.50	0.766
611	ST	W12X35	5.25	2.680
612	ST	W12X35	1.80	0.919
613	ST	W12X35	1.40	0.715
614	ST	W18X86	4.20	5.266
615	ST	W18X86	4.20	5.266
616	ST	W18X86	1.50	1.881
617	ST	W18X86	2.50	3.135
618	ST	W18X86	1.50	1.881
619	ST	W18X86	2.50	3.135

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620	ST	W12X65	8.00	7.573
621	ST	W12X50	8.00	5.828
622	ST	W12X65	2.50	2.367
623	ST	W18X86	1.55	1.944
624	ST	W18X86	1.55	1.944
625	ST	W12X65	5.12	4.847
626	ST	W12X35	2.70	1.378
627	ST	W12X35	2.70	1.378

628	ST	W12X35	2.70	1.378
629	ST	W12X35	2.70	1.378
630	ST	W12X35	2.70	1.378
631	ST	W12X35	2.70	1.378
633	ST	W12X35	2.70	1.378
634	ST	W18X86	0.50	0.627
635	ST	W12X35	2.70	1.378
636	ST	W18X86	4.20	5.266
637	ST	W18X86	4.20	5.266
638	ST	W18X86	1.50	1.881
639	ST	W18X86	2.50	3.135
640	ST	W18X86	1.50	1.881
641	ST	W18X86	2.50	3.135
642	ST	W14X68	8.00	7.930
643	ST	W12X50	8.00	5.828
644	ST	W12X65	2.50	2.367
645	ST	W18X86	2.50	3.135
646	ST	W18X86	2.50	3.135
647	ST	W12X50	5.12	3.730
648	ST	W18X86	4.20	5.266
649	ST	W18X86	4.20	5.266
650	ST	W18X86	3.00	3.762
651	ST	W18X86	2.50	3.135
652	ST	W18X86	3.00	3.762
653	ST	W18X86	2.50	3.135
654	ST	W12X50	8.00	5.828
655	ST	W12X50	8.00	5.828
656	ST	W12X65	2.50	2.367
657	ST	W18X86	2.50	3.135
658	ST	W18X86	2.50	3.135
659	ST	W12X50	8.00	5.828
660	ST	W18X86	4.20	5.266
661	ST	W18X86	4.20	5.266
662	ST	W18X86	3.00	3.762
663	ST	W18X86	2.50	3.135
664	ST	W18X86	3.00	3.762
665	ST	W18X86	2.50	3.135
666	ST	W12X50	8.00	5.828

667	ST	W12X50	8.00	5.828
668	ST	W12X65	2.50	2.367
669	ST	W18X86	2.50	3.135
670	ST	W18X86	2.50	3.135
671	ST	W12X50	8.00	5.828
672	ST	W18X86	4.20	5.266
673	ST	W18X86	4.20	5.266
674	ST	W18X86	3.00	3.762
675	ST	W18X86	2.50	3.135
676	ST	W18X86	3.00	3.762

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677	ST	W18X86	2.50	3.135
678	ST	W12X50	8.00	5.828
679	ST	W12X50	8.00	5.828
680	ST	W12X65	2.50	2.367
681	ST	W18X86	2.50	3.135
682	ST	W18X86	2.50	3.135
683	ST	W12X50	8.00	5.828
684	ST	W18X86	4.20	5.266
685	ST	W18X86	4.20	5.266
686	ST	W18X86	1.50	1.881
687	ST	W18X86	1.25	1.567
688	ST	W18X86	3.00	3.762
689	ST	W18X86	2.50	3.135
690	ST	W14X68	4.00	3.965
691	ST	W12X50	8.00	5.828
692	ST	W12X65	2.50	2.367
693	ST	W18X86	1.50	1.881
694	ST	W18X86	2.50	3.135
696	ST	W12X50	2.05	1.494
697	ST	W12X50	2.50	1.821
698	ST	W12X35	2.75	1.404
699	ST	W12X35	2.75	1.404
700	ST	W12X50	2.05	1.494
701	ST	W12X50	2.50	1.821
702	ST	W12X35	2.75	1.404
703	ST	W12X35	2.75	1.404



705	ST	W12X50	2.05	1.494
706	ST	W12X50	2.50	1.821
707	ST	W12X35	2.75	1.404
708	ST	W12X35	2.75	1.404
710	ST	W12X50	2.05	1.494
711	ST	W14X68	2.50	2.478
712	ST	W12X35	5.50	2.808
713	ST	W12X35	5.50	2.808
714	ST	W12X35	1.45	0.740
715	ST	W12X50	2.05	1.494
716	ST	W12X50	2.50	1.821
717	ST	W12X35	2.75	1.404
718	ST	W12X35	2.75	1.404
719	ST	W12X65	1.00	0.947
720	ST	W12X50	2.05	1.494
721	ST	W12X50	2.50	1.821
722	ST	W12X35	5.50	2.808
723	ST	W12X35	5.50	2.808
725	ST	W12X50	2.05	1.494
726	ST	W14X68	2.50	2.478
727	ST	W12X50	2.75	2.003
728	ST	W12X50	2.75	2.003
729	ST	W12X50	1.00	0.729
730	ST	W12X50	2.05	1.494
731	ST	W12X50	2.50	1.821
732	ST	W12X50	2.75	2.003
733	ST	W12X50	2.75	2.003
734	ST	W18X86	1.50	1.881
735	ST	W18X86	1.50	1.881
736	ST	W12X35	1.45	0.740

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737	ST	W18X86	1.50	1.881
738	ST	W18X86	1.50	1.881
740	ST	W18X86	1.50	1.881
741	ST	W18X86	1.50	1.881
742	ST	W12X50	8.00	5.828
743	ST	W18X86	6.70	8.401

744	ST	W18X86	6.70	8.401
745	ST	W18X86	1.50	1.881
746	ST	W18X86	1.25	1.567
747	ST	W18X86	3.00	3.762
748	ST	W18X86	2.50	3.135
749	ST	W12X50	4.00	2.914
750	ST	W12X50	8.00	5.828
751	ST	W12X65	8.00	7.573
752	ST	W18X86	1.50	1.881
753	ST	W18X86	2.50	3.135
755	ST	W18X86	1.50	1.881
756	ST	W18X86	1.50	1.881
757	ST	W12X50	8.00	5.828
758	ST	W12X35	6.50	3.318
759	ST	W12X35	6.50	3.318
760	ST	W12X35	3.25	1.659
761	ST	W12X35	3.25	1.659
762	ST	W12X35	6.50	3.318
877	ST	W18X86	1.00	1.254
879	ST	W18X86	1.00	1.254
881	ST	W18X86	1.25	1.567
883	ST	W18X86	1.25	1.567
885	ST	W18X86	1.50	1.881
887	ST	W18X86	1.50	1.881
889	ST	W12X50	6.50	4.736
890	ST	W12X35	6.50	3.318
891	ST	W12X35	6.50	3.318
1009	ST	W18X86	0.05	0.063
1011	ST	W18X86	1.50	1.881
1013	ST	W18X86	1.25	1.567
1015	ST	W18X86	1.25	1.567
1017	ST	W18X86	1.50	1.881
1019	ST	W18X86	1.50	1.881
1021	ST	W14X68	6.50	6.443
1022	ST	W12X35	6.50	3.318
1023	ST	W12X35	6.50	3.318
1027	ST	W12X50	2.50	1.821
1028	ST	W18X86	4.20	5.266

1029	ST	W18X86	3.00	3.762
1030	ST	W12X50	2.50	1.821
1031	ST	W12X50	2.75	2.003
1032	ST	W12X50	2.75	2.003
1033	ST	W12X50	8.00	5.828
1034	ST	W14X68	8.00	7.930
1035	ST	W12X50	1.30	0.947
1051	ST	W14X68	0.35	0.347
1052	ST	W12X35	1.62	0.827
1053	ST	W12X35	1.30	0.664
1055	ST	W12X35	0.50	0.255
1056	ST	W8X21	3.50	1.069

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1061	ST	W12X35	1.30	0.664
1062	ST	W8X21	3.50	1.069
1063	ST	W12X50	1.30	0.947
1064	ST	W12X35	1.30	0.664
1065	ST	W8X21	3.50	1.069
1067	ST	W12X35	0.80	0.408
1070	ST	W12X35	0.60	0.306
1073	ST	W8X21	3.50	1.069
1074	ST	W14X68	1.00	0.991
1075	ST	W12X35	1.00	0.510
1076	ST	W8X21	1.80	0.550
1077	ST	W14X68	0.37	0.367
1078	ST	W12X35	0.65	0.332
1079	ST	W8X21	1.80	0.550
1080	ST	W14X68	0.65	0.644
1081	ST	W12X35	0.65	0.332
1083	ST	W12X50	0.37	0.270
1085	ST	W12X35	1.40	0.715
1086	ST	W12X50	0.90	0.656
1087	ST	W12X35	1.40	0.715
1089	ST	W12X35	1.30	0.664
1090	ST	W12X35	1.30	0.664
1091	ST	W12X35	1.30	0.664
1092	ST	W8X21	3.85	1.175

1093	ST	W12X35	1.30	0.664
1094	ST	W8X21	0.90	0.275
1095	ST	W12X35	1.25	0.638
1096	ST	W12X35	1.25	0.638
1097	ST	W8X21	3.85	1.175
1098	ST	W12X35	1.25	0.638
1099	ST	W8X21	0.90	0.275
1100	ST	W12X35	1.30	0.664
1101	ST	W12X35	1.30	0.664
1102	ST	W8X21	3.85	1.175
1103	ST	W12X35	1.30	0.664
1104	ST	W8X21	0.90	0.275
1105	ST	W12X50	3.25	2.368
1106	ST	W12X35	1.25	0.638
1107	ST	W12X35	1.35	0.689
1109	ST	W12X35	1.35	0.689
1110	ST	W12X35	3.85	1.965
1111	ST	W12X35	1.40	0.715
1112	ST	W12X35	1.40	0.715
1113	ST	W12X35	3.85	1.965
1114	ST	W12X50	1.30	0.947
1165	ST	W12X35	0.35	0.179
1174	ST	W12X50	1.30	0.947
1183	ST	W12X50	1.02	0.743
1253	ST	W18X86	4.20	5.266
1438	ST	W12X50	2.75	2.003
1439	ST	W12X50	2.75	2.003
1440	ST	W12X50	4.00	2.914
1441	ST	W12X50	2.75	2.003
1442	ST	W6X16	2.50	0.587
1443	ST	W12X50	2.75	2.003
1444	ST	W6X16	2.50	0.587

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1445	ST	W12X50	2.75	2.003
1446	ST	W12X50	2.75	2.003
1447	ST	W12X50	8.00	5.828
1448	ST	W6X16	2.50	0.587

1449	ST	W6X16	2.50	0.587
1450	ST	W12X50	2.75	2.003
1451	ST	W12X50	2.75	2.003
1452	ST	W12X50	4.00	2.914
1453	ST	W12X50	2.75	2.003
1454	ST	W6X16	2.50	0.587
1455	ST	W12X50	2.75	2.003
1456	ST	W6X16	2.50	0.587
1465	ST	W12X50	2.75	2.003
1466	ST	W12X50	2.75	2.003
1467	ST	W6X16	3.00	0.705
1468	ST	W12X50	2.75	2.003
1469	ST	W12X50	2.75	2.003
1470	ST	W6X16	3.00	0.705
1471	ST	W12X50	8.00	5.828
1472	ST	W12X50	2.75	2.003
1473	ST	W12X50	2.75	2.003
1474	ST	W6X16	3.00	0.705
1475	ST	W12X50	2.75	2.003
1476	ST	W12X50	2.75	2.003
1477	ST	W6X16	3.00	0.705
1478	ST	W12X50	2.75	2.003
1479	ST	W12X50	2.75	2.003
1480	ST	W6X16	3.00	0.705
1481	ST	W12X50	2.75	2.003
1482	ST	W12X50	2.75	2.003
1483	ST	W6X16	3.00	0.705
1484	ST	W12X65	8.00	7.573
1493	ST	W12X50	2.90	2.113
1494	ST	W12X50	2.90	2.113
1495	ST	W12X50	4.00	2.914
1496	ST	W12X50	2.90	2.113
1497	ST	W12X50	2.90	2.113
1498	ST	W12X65	5.12	4.847
1499	ST	W12X50	2.90	2.113
1500	ST	W12X50	2.90	2.113
1501	ST	W12X50	4.00	2.914
1502	ST	W14X68	2.50	2.478

1503	ST	W12X50	2.50	1.821
1504	ST	W12X50	4.00	2.914
1505	ST	W14X68	2.50	2.478
1506	ST	W12X50	2.50	1.821
1507	ST	W12X50	8.00	5.828
1508	ST	W14X68	2.50	2.478
1509	ST	W12X50	2.50	1.821
1510	ST	W12X50	4.00	2.914
1511	ST	W12X50	2.05	1.493
1512	ST	W12X50	2.05	1.493
1513	ST	W12X50	4.00	2.914
1514	ST	W12X50	2.05	1.494
1515	ST	W12X50	2.05	1.494
1516	ST	W12X50	8.00	5.828

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1517	ST	W12X50	2.05	1.494
1518	ST	W12X50	2.05	1.494
1519	ST	W12X50	4.00	2.914
1520	ST	W12X50	2.75	2.003
1521	ST	W12X50	2.75	2.003
1522	ST	W12X50	8.00	5.828
1523	ST	W12X50	2.75	2.003
1524	ST	W12X50	2.75	2.003
1525	ST	W12X50	8.00	5.828
1528	ST	W18X86	2.50	3.135
1531	ST	W18X86	2.50	3.135
1533	ST	W18X86	2.50	3.135
1543	ST	W12X35	2.50	1.276
1544	ST	W12X35	1.81	0.924
1584	ST	W12X35	2.70	1.378
1594	ST	W12X35	2.75	1.404
1597	ST	W12X35	1.81	0.924
1599	ST	W12X35	2.88	1.470
1600	ST	W12X35	2.88	1.470
1605	ST	W12X50	2.50	1.821
1606	ST	W12X50	2.50	1.821
1607	ST	W6X16	3.00	0.705

1608	ST	W12X50	2.50	1.821
1609	ST	W12X50	2.50	1.821
1610	ST	W6X16	3.00	0.705
1611	ST	W14X68	2.50	2.478
1613	ST	W12X50	2.50	1.821
1615	ST	W12X50	2.50	1.821
1616	ST	W12X50	2.50	1.821
1617	ST	W6X16	3.00	0.705
1618	ST	W12X50	2.50	1.821
1619	ST	W12X50	2.50	1.821
1620	ST	W6X16	3.00	0.705
1621	ST	W14X68	2.50	2.478
1623	ST	W12X50	2.50	1.821
1625	ST	W12X50	2.50	1.821
1626	ST	W12X50	2.50	1.821
1627	ST	W6X16	3.00	0.705
1628	ST	W12X50	2.50	1.821
1629	ST	W12X50	2.50	1.821
1630	ST	W6X16	3.00	0.705
1631	ST	W14X68	2.50	2.478
1633	ST	W12X50	2.50	1.821
1639	LD	L35358	3.91	1.258
1640	LD	L35358	3.91	1.258
1641	LD	L35358	3.91	1.258
1642	LD	L35358	3.91	1.258
1643	LD	L30306	3.91	0.816
1644	LD	L30306	3.91	0.816
1645	LD	L30306	3.91	0.816
1646	LD	L30306	3.91	0.816
1651	LD	L30306	3.72	0.777
1652	LD	L30306	3.72	0.777
1653	LD	L30306	3.72	0.777
1654	LD	L30306	3.72	0.777
1655	LD	L30306	3.72	0.777

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1656	LD	L30306	3.72	0.777
1657	LD	L30306	3.72	0.777

1658	LD	L30306	3.72	0.777
1659	LD	L35358	4.07	1.311
1660	LD	L35358	4.07	1.311
1661	LD	L35358	4.07	1.311
1662	LD	L35358	4.07	1.311
1663	LD	L35358	4.07	1.311
1664	LD	L35358	4.07	1.311
1665	LD	L35358	4.07	1.311
1666	LD	L35358	4.07	1.311
1667	ST	W12X50	8.00	5.828
1668	ST	W12X50	8.00	5.828
1669	ST	W12X65	2.50	2.367
1685	ST	W14X68	4.00	3.965
1686	ST	W14X68	1.40	1.388
1689	ST	W12X65	4.00	3.786
1690	ST	W12X30	6.60	2.875
1691	ST	W12X30	6.60	2.875
1692	ST	W12X30	7.63	3.325
1693	ST	W12X30	7.63	3.325
1694	ST	W12X50	2.90	2.113
1695	ST	W12X65	2.90	2.745
1696	ST	W12X50	8.00	5.828
1697	ST	W12X50	2.05	1.494
1698	ST	W12X50	2.05	1.494
1699	ST	W12X50	8.00	5.828
1700	ST	W12X35	1.45	0.740
1701	ST	W12X35	1.45	0.740
1702	ST	W12X50	8.00	5.828
1703	ST	W12X50	2.05	1.494
1704	ST	W12X50	2.05	1.494
1705	ST	W12X50	8.00	5.828
1706	ST	W12X50	2.05	1.494
1707	ST	W12X50	2.05	1.494
1708	ST	W12X65	2.50	2.367
1709	ST	W12X35	1.45	0.740
1710	ST	W12X35	1.45	0.740
1711	ST	W12X50	8.00	5.828
1712	ST	W12X50	2.90	2.113



1713	ST	W6X16	2.50	0.587
1714	ST	W12X65	2.90	2.745
1715	ST	W6X16	2.50	0.587
1716	ST	W6X16	2.50	0.587
1717	ST	W6X16	2.50	0.587
1718	ST	W12X50	2.90	2.113
1719	ST	W6X16	2.50	0.587
1720	ST	W12X65	2.90	2.745
1721	ST	W6X16	2.50	0.587
1722	LD	L30306	3.83	0.800
1723	LD	L30306	3.83	0.800
1724	LD	L30306	3.83	0.800
1725	LD	L30306	3.83	0.800
1726	LD	L30306	3.83	0.800
1727	LD	L30306	3.83	0.800
1729	LD	L30306	3.83	0.800

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1730	LD	L30306	3.83	0.800
1731	ST	W12X50	2.05	1.493
1732	ST	W12X50	2.05	1.493
1733	ST	W6X16	3.00	0.705
1734	ST	W12X50	2.05	1.493
1735	ST	W12X50	2.05	1.493
1736	ST	W6X16	3.00	0.705
1737	ST	W12X50	2.05	1.493
1738	ST	W6X16	2.50	0.587
1739	ST	W12X50	2.05	1.493
1740	ST	W6X16	2.50	0.587
1741	ST	W6X16	3.00	0.705
1742	ST	W6X16	3.00	0.705
1743	ST	W6X16	2.50	0.587
1744	ST	W6X16	2.50	0.587
1745	ST	W12X50	2.05	1.494
1746	ST	W12X50	2.05	1.494
1747	ST	W6X16	3.00	0.705
1748	ST	W12X50	2.05	1.494
1749	ST	W12X50	2.05	1.494

1750	ST	W6X16	3.00	0.705
1751	ST	W12X50	2.05	1.494
1752	ST	W6X16	2.50	0.587
1753	ST	W12X50	2.05	1.494
1754	ST	W6X16	2.50	0.587
1755	LD	L30306	3.23	0.676
1756	LD	L30306	3.23	0.676
1757	LD	L30306	3.23	0.676
1758	LD	L30306	3.23	0.676
1759	LD	L30306	3.23	0.676
1760	LD	L30306	3.23	0.676
1761	LD	L30306	3.23	0.676
1762	LD	L30306	3.23	0.676
1763	LD	L30306	3.63	0.760
1764	LD	L30306	3.63	0.760
1765	LD	L30306	3.63	0.760
1766	LD	L30306	3.63	0.760
1767	LD	L30306	3.63	0.760
1768	LD	L30306	3.63	0.760
1769	LD	L30306	3.63	0.760
1770	LD	L30306	3.63	0.760
1771	ST	W10X30	4.14	1.814
1772	ST	W10X30	4.30	1.882
1777	ST	W10X30	4.30	1.882
1778	ST	W10X30	4.30	1.882
1781	ST	W10X30	4.37	1.916
1782	ST	W10X30	4.37	1.916
1783	ST	W10X30	4.37	1.916
1784	ST	W10X30	4.22	1.849
1787	ST	W10X30	4.14	1.814
1788	ST	W10X30	4.30	1.882
1793	ST	W10X30	4.30	1.882
1794	ST	W10X30	4.30	1.882
1797	ST	W10X30	4.37	1.916
1798	ST	W10X30	4.37	1.916
1799	ST	W10X30	4.37	1.916

1800	ST	W10X30	4.22	1.849
1803	ST	W12X35	3.25	1.659
1804	ST	W14X68	4.00	3.965
1805	ST	W12X50	4.00	2.914
1806	ST	W12X35	3.25	1.659
1807	ST	W12X35	3.25	1.659
1808	ST	W12X50	5.15	3.755
1809	ST	W12X50	5.15	3.755
1810	ST	W12X50	5.15	3.755
1811	ST	W12X50	5.15	3.755
1812	ST	W12X35	1.45	0.740
1813	ST	W12X35	1.45	0.740
1814	ST	W6X16	1.50	0.352
1815	ST	W12X35	1.45	0.740
1816	ST	W12X35	1.45	0.740
1817	ST	W6X16	1.50	0.352
1818	ST	W6X16	1.50	0.352
1819	ST	W6X16	1.50	0.352
1820	ST	W12X35	1.45	0.740
1821	ST	W12X35	1.45	0.740
1822	ST	W6X16	1.50	0.352
1823	ST	W12X35	1.45	0.740
1824	ST	W12X35	1.45	0.740
1825	ST	W6X16	1.50	0.352
1826	ST	W12X35	1.45	0.740
1827	ST	W12X35	1.45	0.740
1828	ST	W6X16	1.50	0.352
1829	ST	W12X35	1.45	0.740
1830	ST	W12X35	1.45	0.740
1831	ST	W6X16	1.50	0.352
1832	ST	W12X35	1.45	0.740
1833	ST	W12X35	1.45	0.740
1834	ST	W6X16	1.50	0.352
1835	ST	W12X35	1.45	0.740
1836	ST	W12X35	1.45	0.740
1837	ST	W6X16	1.50	0.352
1838	ST	W12X35	1.45	0.740
1839	ST	W12X35	1.45	0.740

1840	ST	W6X16	1.50	0.352
1841	ST	W12X35	1.45	0.740
1842	ST	W12X35	1.45	0.740
1843	ST	W6X16	1.50	0.352
1844	ST	W12X35	1.45	0.740
1845	ST	W12X35	1.45	0.740
1846	ST	W6X16	1.50	0.352
1847	ST	W12X35	1.45	0.740
1848	ST	W12X35	1.45	0.740
1849	ST	W6X16	1.50	0.352
1851	LD	L30306	2.09	0.436
1852	LD	L30306	2.09	0.436
1853	LD	L30306	2.09	0.436
1855	LD	L30306	2.09	0.436
1856	LD	L30306	2.09	0.436
1857	LD	L30306	2.09	0.436
1858	LD	L30306	2.09	0.436
1859	LD	L30306	2.09	0.436

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1860	LD	L30306	2.09	0.436
1861	LD	L30306	2.09	0.436
1862	LD	L30306	2.09	0.436
1863	LD	L30306	2.09	0.436
1864	LD	L30306	2.09	0.436
1865	LD	L30306	2.09	0.436
1869	ST	W12X35	1.45	0.740
1870	ST	W12X35	1.45	0.740
1872	LD	L30306	2.09	0.436
1873	LD	L30306	2.09	0.436
1879	ST	W12X50	1.00	0.729
1880	ST	W12X50	1.00	0.729
1883	ST	W18X86	2.50	3.135
1884	ST	W18X86	2.50	3.135
1885	ST	W18X86	2.50	3.135
1886	ST	W18X86	2.50	3.135
1887	ST	W18X86	2.50	3.135
1888	ST	W18X86	2.50	3.135

1889	ST	W18X86	2.50	3.135
1890	ST	W18X86	2.50	3.135
1891	ST	W18X86	2.50	3.135
1892	ST	W18X86	2.50	3.135
1893	ST	W18X86	2.50	3.135
1894	ST	W18X86	2.50	3.135
1895	ST	W18X86	2.50	3.135
1896	ST	W18X86	2.50	3.135
1897	ST	W18X86	2.50	3.135
1898	ST	W18X86	2.50	3.135
1899	ST	W18X86	2.50	3.135
1900	ST	W18X86	2.50	3.135
1901	ST	W18X86	2.50	3.135
1902	ST	W18X86	2.50	3.135
1903	ST	W14X68	3.00	2.974
1904	ST	W12X65	3.00	2.840
1905	ST	W12X65	3.00	2.840
1906	ST	W12X65	3.00	2.840
1907	ST	W12X65	3.00	2.840
1908	ST	W12X65	3.00	2.840
1909	ST	W12X65	3.00	2.840
1910	ST	W12X65	3.00	2.840
1911	ST	W12X65	3.00	2.840
1912	ST	W12X65	3.00	2.840
1913	ST	W12X65	5.50	5.206
1914	ST	W12X65	5.50	5.206
1915	ST	W14X68	2.50	2.478
1916	ST	W12X65	2.50	2.367
1917	ST	W12X65	2.50	2.367
1918	ST	W12X65	2.50	2.367
1919	ST	W12X65	2.50	2.367
1920	ST	W12X65	2.50	2.367
1921	ST	W12X65	2.50	2.367
1922	ST	W12X65	2.50	2.367
1923	ST	W12X65	2.50	2.367
1924	ST	W12X65	2.50	2.367
1926	ST	W10X30	3.54	1.549
1927	ST	W10X30	3.54	1.549

1928	ST	W10X30	3.54	1.549
1929	ST	W10X30	3.54	1.549
1930	ST	W10X30	3.54	1.549
1931	ST	W10X30	3.54	1.549
1932	ST	W10X30	3.54	1.549
1933	ST	W10X30	3.54	1.549
1934	ST	W10X30	3.54	1.549
1935	ST	W10X30	3.54	1.549
1936	ST	W10X30	3.54	1.549
1937	ST	W10X30	3.54	1.549
1938	ST	W10X30	3.54	1.549
1939	ST	W10X30	3.54	1.549
1940	ST	W10X30	3.54	1.549
1941	ST	W10X30	3.54	1.549
1942	ST	W10X30	3.54	1.549
1943	ST	W10X30	3.54	1.549
1944	ST	W10X30	3.54	1.549
1945	ST	W10X30	3.54	1.549
1981	ST	W12X50	1.50	1.093
1982	ST	W12X50	1.50	1.093
1983	ST	W18X86	2.50	3.135
1984	ST	W18X86	2.50	3.135
1985	ST	W10X30	3.54	1.549
1986	ST	W10X30	3.54	1.549
1987	ST	W12X50	2.50	1.821
1988	ST	W12X50	2.50	1.821
1989	ST	W10X30	3.54	1.549
1990	ST	W10X30	3.54	1.549
1995	ST	W12X50	4.00	2.914
1996	ST	W12X50	4.00	2.914
2005	ST	W12X50	4.00	2.914
2006	ST	W12X50	4.00	2.914
2007	ST	W8X21	4.43	1.352
2008	ST	W8X21	4.94	1.508
2009	ST	W8X21	4.94	1.508
2010	ST	W8X21	4.94	1.508

2011	ST	W8X21	4.43	1.352
2012	ST	W8X21	4.94	1.508
2013	ST	W8X21	4.94	1.508
2014	ST	W8X21	4.94	1.508
2015	ST	W8X21	4.85	1.482
2016	ST	W8X21	4.85	1.482
2017	ST	W8X21	4.85	1.482
2018	ST	W8X21	4.85	1.482
2019	ST	W12X50	4.00	2.914
2020	ST	W12X50	4.00	2.914
2021	ST	W8X21	4.85	1.482
2022	ST	W8X21	4.85	1.482
2023	ST	W8X21	4.85	1.482
2024	ST	W8X21	4.85	1.482
2029	ST	W12X50	4.00	2.914
2030	ST	W12X50	4.00	2.914
2037	ST	W8X21	4.85	1.482
2038	ST	W8X21	4.85	1.482
2039	ST	W8X21	4.85	1.482
2040	ST	W8X21	4.85	1.482

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2041	ST	W8X21	4.85	1.482
2042	ST	W8X21	4.85	1.482
2043	ST	W8X21	4.85	1.482
2044	ST	W8X21	4.85	1.482
2045	ST	W8X21	4.85	1.482
2046	ST	W8X21	4.85	1.482
2047	ST	W8X21	4.85	1.482
2048	ST	W8X21	4.85	1.482
2049	ST	W8X21	4.85	1.482
2050	ST	W8X21	4.85	1.482
2051	ST	W8X21	4.85	1.482
2052	ST	W8X21	4.85	1.482
2053	ST	W14X68	1.25	1.239
2054	ST	W12X35	1.25	0.638
2055	ST	W8X21	1.80	0.550
2059	ST	W18X86	0.95	1.191

2061	ST	W18X86	0.95	1.191
2062	ST	W18X86	0.95	1.191
2063	ST	W12X35	2.20	1.123
2064	ST	W18X86	0.95	1.191
2065	ST	W12X35	2.20	1.123
2066	ST	W12X50	1.35	0.984
2067	ST	W12X50	1.30	0.947
2069	ST	W8X21	2.20	0.672
2070	ST	W12X50	1.30	0.947
2072	ST	W8X21	2.20	0.672
2073	ST	W12X50	1.07	0.780
2075	ST	W8X21	2.20	0.672
2076	ST	W12X50	1.35	0.984
2078	ST	W8X21	2.20	0.672
2079	ST	W12X50	1.35	0.984
2081	ST	W8X21	2.20	0.672
2082	ST	W8X21	2.60	0.794
2083	ST	W8X21	2.60	0.794
2084	ST	W12X35	1.50	0.766
2085	ST	W12X35	1.50	0.766
2086	ST	W12X35	1.50	0.766
2087	ST	W12X35	1.50	0.766
2089	ST	W12X35	3.50	1.787
2090	ST	W18X86	0.50	0.627
2091	ST	W10X30	1.45	0.635
2092	ST	W14X68	1.45	1.437
2093	ST	W14X68	0.35	0.347
2094	ST	W18X86	1.45	1.818
2095	ST	W12X35	1.80	0.919
2096	ST	W12X35	2.60	1.327
2097	ST	W12X35	1.70	0.868
2098	ST	W12X35	1.45	0.740
2099	ST	W12X50	0.28	0.204
2100	ST	W10X30	2.50	1.095
2101	ST	W12X50	2.88	2.098
2103	ST	W10X30	2.70	1.183
2106	ST	W12X50	2.56	1.865
2107	ST	W14X68	0.28	0.278



2108	ST	W10X30	2.88	1.262
2109	ST	W10X30	2.50	1.095

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2110	ST	W8X21	2.88	0.879
2111	ST	W10X30	1.81	0.793
2113	ST	W10X30	1.81	0.793
2114	ST	W10X30	2.88	1.262
2115	ST	W12X50	2.88	2.098
2116	ST	W10X30	2.88	1.262
2117	ST	W10X30	2.50	1.095
2118	ST	W8X21	2.88	0.879
2119	ST	W10X30	2.50	1.095
2120	ST	W10X30	2.75	1.205
2121	ST	W18X86	2.50	3.135
2124	ST	W12X35	2.75	1.404
2125	ST	W12X35	3.25	1.659
2126	ST	W12X50	0.28	0.204
2127	ST	W12X50	2.50	1.821
2129	ST	W12X50	2.56	1.865
2131	ST	W12X50	2.88	2.098
2132	ST	W12X50	2.56	1.865
2133	ST	W12X50	2.56	1.865
2134	ST	W8X21	2.88	0.879
2135	ST	W12X65	2.88	2.726
2229	ST	W8X21	4.85	1.482
2230	ST	W8X21	4.85	1.482
2231	ST	W8X21	4.85	1.482
2232	ST	W8X21	4.85	1.482
2233	ST	W8X21	4.85	1.482
2234	ST	W8X21	4.85	1.482
2235	ST	W8X21	4.85	1.482
2236	ST	W8X21	4.85	1.482
2237	ST	W8X21	4.94	1.508
2238	ST	W8X21	4.94	1.508
2239	ST	W8X21	4.94	1.508
2240	ST	W8X21	4.94	1.508
2241	ST	W8X21	4.94	1.508

2242	ST	W8X21	4.94	1.508
2243	ST	W8X21	4.94	1.508
2244	ST	W8X21	4.94	1.508
2245	ST	W8X21	4.66	1.424
2246	ST	W8X21	4.66	1.424
2247	ST	W8X21	4.94	1.508
2248	ST	W8X21	4.94	1.508
2249	ST	W8X21	4.66	1.424
2250	ST	W8X21	4.66	1.424
2251	ST	W8X21	4.94	1.508
2252	ST	W8X21	4.94	1.508
2253	ST	W8X21	4.94	1.508
2254	ST	W8X21	4.94	1.508
2255	ST	W8X21	4.94	1.508
2256	ST	W8X21	4.94	1.508
2257	ST	W8X21	4.94	1.508
2258	ST	W8X21	4.94	1.508
2259	ST	W8X21	4.94	1.508
2260	ST	W8X21	4.94	1.508
2269	ST	W8X21	4.72	1.440
2270	ST	W8X21	4.72	1.440
2271	ST	W8X21	4.72	1.440

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2272	ST	W8X21	4.72	1.440
2273	ST	W8X21	4.72	1.440
2274	ST	W8X21	4.72	1.440
2275	ST	W8X21	4.72	1.440
2276	ST	W8X21	4.72	1.440
2277	ST	W8X21	4.72	1.440
2278	ST	W8X21	4.72	1.440
2279	ST	W8X21	4.72	1.440
2280	ST	W8X21	4.72	1.440
2281	ST	W8X21	4.72	1.440
2282	ST	W8X21	4.72	1.440
2283	ST	W8X21	4.72	1.440
2284	ST	W8X21	4.72	1.440
2285	LD	L30306	3.85	0.806

2286	LD	L30306	3.85	0.806
2287	LD	L30306	4.07	0.851
2288	LD	L30306	3.40	0.711
2289	LD	L30306	3.40	0.711
2290	ST	W12X50	1.70	1.239
2291	ST	W12X50	3.00	2.186
2292	ST	W12X50	2.50	1.821
2293	ST	W12X50	1.55	1.129
2295	ST	W12X50	3.00	2.186
2296	ST	W12X50	2.50	1.821
2297	ST	W12X50	1.55	1.129
2298	ST	W12X50	4.20	3.060
2301	ST	W12X50	2.50	1.821
2302	ST	W12X50	2.50	1.821
2303	ST	W12X50	0.95	0.692
2304	ST	W12X50	0.50	0.364
2305	ST	W12X50	1.45	1.056
2310	ST	W12X50	1.43	1.038
2311	ST	W12X50	2.85	2.076
2313	ST	W12X50	1.43	1.038
2314	ST	W12X50	2.85	2.076
2315	ST	W12X50	1.43	1.038
2317	ST	W12X50	2.85	2.076
2318	ST	W12X50	1.43	1.038
2319	ST	W12X50	2.85	2.076
2321	ST	W12X50	2.85	2.076
2322	ST	W12X50	1.43	1.038
2326	ST	W12X50	4.75	3.461
2327	ST	W12X50	1.00	0.729
2328	ST	W12X50	5.00	3.643
2329	ST	W12X50	1.50	1.093
2330	ST	W12X50	1.80	1.311
2331	ST	W12X50	2.85	2.076
2332	ST	W12X50	3.50	2.550
2333	ST	W12X50	1.43	1.038
2334	ST	W12X50	4.70	3.424
2335	ST	W8X18	0.41	0.108
2336	ST	W12X50	1.50	1.093

2337	ST	W12X50	1.50	1.093
2338	ST	W8X18	1.43	0.371
2339	ST	W8X18	0.41	0.108
2340	ST	W8X18	1.42	0.371

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2342	ST	W8X18	1.42	0.371
2343	ST	W8X18	0.10	0.026
2344	ST	W8X18	0.10	0.026
2345	ST	W8X18	0.91	0.238
2346	ST	W8X18	0.91	0.238
2347	ST	C10X15	2.23	0.497
2348	ST	C10X15	2.23	0.497
2349	ST	W12X35	5.00	2.552
2350	ST	W12X50	5.00	3.643
2351	ST	W8X18	0.41	0.108
2352	ST	W8X18	0.10	0.026
2353	ST	W8X18	0.10	0.026
2354	ST	W8X18	0.91	0.238
2355	ST	W8X18	0.91	0.238
2356	ST	W8X18	0.41	0.108
2357	ST	W8X18	0.10	0.026
2358	ST	W8X18	0.91	0.238
2359	ST	W8X18	0.41	0.108
2360	ST	W12X50	1.42	1.038
2361	ST	W12X50	1.42	1.038
2362	ST	W12X50	1.42	1.038
2363	ST	W8X18	1.80	0.469
2364	ST	W8X18	1.50	0.391
2365	ST	W8X18	1.50	0.391
2366	ST	W8X18	1.50	0.391
2367	ST	C10X15	2.53	0.563
2368	ST	C10X15	2.53	0.563
2369	ST	W12X35	1.50	0.766
2370	ST	W12X50	1.50	1.093
2371	ST	W8X18	0.41	0.108
2372	ST	W8X18	0.10	0.026
2373	ST	W8X18	0.10	0.026

2374	ST	W8X18	0.91	0.238
2376	ST	W8X18	0.41	0.108
2377	ST	C10X15	1.03	0.228
2378	ST	C10X15	1.03	0.228
2381	ST	W8X18	0.91	0.238
2383	ST	W12X50	0.50	0.364
2385	ST	W8X18	0.10	0.026
2387	ST	W8X18	0.91	0.238
2388	ST	W8X18	0.41	0.108
2390	ST	C10X15	1.25	0.278
2391	ST	C10X15	1.25	0.278
2392	ST	W12X35	0.25	0.128
2393	ST	W8X18	0.41	0.108
2394	ST	W8X18	0.91	0.238
2397	ST	C10X15	0.25	0.056
2398	ST	C10X15	0.25	0.056
2399	ST	W12X35	4.75	2.425
2400	ST	W12X35	1.75	0.893
2401	ST	W12X50	5.00	3.643
2402	ST	W8X18	0.41	0.108
2403	ST	W8X18	0.10	0.026
2404	ST	W8X18	0.10	0.026
2405	ST	W8X18	0.91	0.238
2406	ST	W8X18	0.41	0.108

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2407	ST	W8X18	0.91	0.238
2408	ST	W12X50	1.42	1.038
2409	ST	W8X18	1.00	0.261
2410	ST	W8X18	0.50	0.130
2411	ST	W18X86	2.50	3.135
2412	ST	W12X50	2.50	1.821
2413	ST	W12X50	1.43	1.038
2414	ST	W12X35	1.75	0.893
2415	ST	W12X50	1.75	1.275
2416	ST	W8X18	0.41	0.108
2417	ST	W8X18	0.10	0.026
2418	ST	W8X18	0.10	0.026

2419	ST	W8X18	0.91	0.238
2420	ST	W8X18	0.41	0.108
2421	ST	W8X18	0.91	0.238
2422	ST	C10X15	1.03	0.228
2423	ST	C10X15	1.03	0.228
2424	ST	W8X18	0.41	0.108
2425	ST	W8X18	0.10	0.026
2426	ST	W8X18	0.10	0.026
2427	ST	W8X18	0.91	0.238
2428	ST	W8X18	0.41	0.108
2429	ST	W8X18	0.91	0.238
2430	ST	W8X21	1.50	0.458
2431	ST	W8X21	1.50	0.458
2445	ST	W8X21	1.70	0.519
2446	ST	W8X21	1.70	0.519
2447	ST	C10X15	1.03	0.228
2448	ST	C10X15	1.03	0.228
2449	ST	W12X50	1.42	1.038
2450	ST	W8X18	1.50	0.391
2451	ST	W12X50	1.42	1.038
2452	ST	W12X50	1.42	1.038
2453	ST	W8X18	1.50	0.391
2454	ST	W8X18	1.75	0.456
2455	ST	L30306	2.07	0.216
2456	ST	L30306	2.07	0.216
2457	ST	L30306	2.26	0.236
2458	ST	L30306	2.26	0.236
2459	ST	L30306	2.07	0.216
2460	ST	L30306	2.07	0.216
2461	ST	L30306	2.07	0.216
2462	ST	L30306	2.07	0.216
2463	ST	L30306	2.07	0.216
2464	ST	L30306	2.30	0.240
2465	ST	L30306	2.30	0.240
2466	ST	L30306	2.07	0.216
2467	ST	L30306	2.07	0.216
2468	ST	L30306	2.07	0.216
2469	ST	L30306	2.07	0.216

2473	ST	C10X15	1.03	0.228
2474	ST	C10X15	1.03	0.228
2475	ST	C10X15	1.25	0.278
2476	ST	C10X15	1.25	0.278
2477	ST	C10X15	1.03	0.228
2478	ST	C10X15	1.03	0.228

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2479	ST	C10X15	1.03	0.228
2480	ST	C10X15	1.03	0.228
2481		PRI SMAT	0.91	0.525
2482		PRI SMAT	0.91	0.525
2483		PRI SMAT	0.91	0.525
2484		PRI SMAT	0.91	0.525
2485	ST	C10X15	1.25	0.278
2486	ST	C10X15	1.25	0.278
2487	ST	C10X15	1.25	0.278
2488	ST	C10X15	1.25	0.278
2489	ST	C10X15	1.02	0.228
2490	ST	C10X15	1.02	0.228
2491	ST	C10X15	1.02	0.228
2492	ST	C10X15	1.02	0.228
2493	ST	C10X15	1.02	0.228
2494	ST	C10X15	1.02	0.228
2495	ST	C10X15	1.02	0.228
2496	ST	C10X15	1.02	0.228
2497	ST	C10X15	1.02	0.228
2498	ST	C10X15	1.02	0.228
2499	ST	C10X15	1.02	0.228
2500	ST	C10X15	1.02	0.228
2501		PRI SMAT	0.91	0.525
2502		PRI SMAT	0.91	0.525
2503		PRI SMAT	0.91	0.525
2504		PRI SMAT	0.91	0.525
2505		PRI SMAT	0.91	0.525
2506		PRI SMAT	0.91	0.525
2507		PRI SMAT	0.91	0.525
2508		PRI SMAT	0.91	0.525

2509	ST	W12X50	2.50	1.821
2510	ST	W18X86	4.20	5.266
2511	ST	W18X86	3.00	3.762
2512	ST	W12X50	2.50	1.821
2513	ST	W12X50	1.50	1.093
2514	ST	W12X50	1.50	1.093
2515	ST	W18X86	2.50	3.135
2516	ST	W18X86	2.50	3.135
2517	ST	W10X30	3.54	1.549
2518	ST	W10X30	3.54	1.549
2519	ST	W12X50	2.50	1.821
2520	ST	W12X50	2.50	1.821
2521	ST	W10X30	3.54	1.549
2522	ST	W10X30	3.54	1.549
2526	ST	W12X50	5.00	3.643
2527	ST	W12X50	5.00	3.643
2528	ST	W12X50	5.00	3.643
2529	ST	W12X50	5.00	3.643
2530	ST	W12X50	6.50	4.736
2531	ST	W12X50	6.50	4.736
2532	ST	W6X16	2.50	0.587
2533	ST	W6X16	2.50	0.587
2534	ST	W6X16	2.50	0.587
2535	LD	L30306	3.54	0.739
2536	LD	L30306	3.54	0.739
2537	LD	L30306	3.54	0.739

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2538	LD	L30306	3.54	0.739
2539	ST	W12X50	2.50	1.821
2540	ST	W12X50	1.90	1.384
2541	ST	W12X65	1.90	1.799
2542	ST	W12X50	1.90	1.384
2543	ST	W12X50	1.90	1.384
2544	ST	W8X21	4.10	1.252
2545	ST	W8X21	4.10	1.252
2546	ST	W14X68	2.60	2.577
2547	ST	W12X35	5.25	2.680



2549	ST	W12X35	6.50	3.318
2550	ST	W18X86	3.00	3.762
2551	ST	W18X86	3.00	3.762
2554	ST	W12X50	5.12	3.730
2555	ST	W12X50	2.88	2.098
2556	ST	W12X35	2.70	1.378
2557	ST	W12X35	0.50	0.255
2559	ST	W12X50	1.35	0.984
2561	ST	W12X35	0.50	0.255
2562	ST	W12X50	1.30	0.947
2564	ST	W12X50	1.30	0.947
2566	ST	W12X50	1.35	0.984
2568	ST	W12X50	1.35	0.984
2569	ST	W12X50	1.35	0.984
2570	ST	W12X50	2.70	1.967
2571	ST	W12X50	2.70	1.967
2572	ST	W12X50	8.00	5.828
2573	ST	W12X50	2.65	1.931
2574	ST	W12X50	2.65	1.931
2575	ST	W12X50	8.00	5.828
2576	ST	W8X21	4.10	1.252
2577	ST	W8X21	4.10	1.252
2578	ST	W8X21	4.10	1.252
2579	ST	W8X21	4.10	1.252
2580	ST	W8X21	4.10	1.252
2581	ST	W8X21	4.10	1.252
2582	ST	W12X35	3.00	1.531
2583	ST	W10X30	2.88	1.262
2584	ST	W12X65	2.88	2.726
2585	ST	W10X30	3.00	1.314
2588	ST	W12X50	2.88	2.098
2589	ST	W10X30	3.00	1.314
2590	ST	W18X86	3.00	3.762
2593	ST	W10X30	2.88	1.262
2594	ST	W10X30	2.90	1.271
2595	ST	W12X35	2.90	1.480
2596	ST	W10X30	2.90	1.271
2597	ST	W12X35	2.90	1.480

2598	ST	W10X30	2.90	1.271
2599	ST	W12X35	2.90	1.480
2600	ST	W8X21	2.88	0.879
2601	ST	W10X30	2.90	1.271
2602	ST	W12X35	2.90	1.480
2603	ST	W8X21	2.88	0.879
2604	ST	W12X35	3.50	1.787
2605	ST	W18X86	3.50	4.389

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2606	ST	W14X68	0.25	0.248
2607	ST	W12X35	3.50	1.787
2608	ST	W12X30	1.00	0.436
2609	ST	W12X30	1.18	0.514
2610	ST	W12X35	0.90	0.459
2611	ST	W12X35	3.50	1.787
2612	ST	W12X30	1.18	0.514
2613	ST	W12X30	1.18	0.514
2614	ST	W12X30	1.00	0.436
2615	ST	W12X35	3.50	1.787
2616	ST	W12X30	1.00	0.436
2617	ST	W12X35	3.50	1.787
2618	ST	W12X30	1.00	0.436
2619	ST	W18X86	2.65	3.323
2620	ST	W12X35	2.65	1.353
2621	ST	W12X30	3.85	1.677
2622	ST	W12X35	2.65	1.353
2623	ST	W12X35	2.65	1.353
2624	ST	W12X30	2.75	1.198
2625	ST	W12X35	2.65	1.353
2626	ST	W12X30	3.85	1.677
2627	ST	W12X35	2.65	1.353
2628	ST	W12X30	2.75	1.198
2629	ST	W12X30	3.85	1.677
2630	ST	W12X30	3.85	1.677
2631	ST	W12X30	3.85	1.677
2632	ST	W12X35	1.35	0.689
2633	ST	W12X35	0.90	0.459

2634	ST	W12X35	1.40	0.715
2635	ST	W12X35	1.25	0.638
2636	ST	W12X35	3.85	1.965
2637	ST	W12X35	1.25	0.638
2638	ST	W12X35	0.90	0.459
2639	ST	W12X30	1.85	0.806
2640	ST	W12X30	1.85	0.806
2641	ST	W12X30	1.00	0.436
2642	ST	W12X30	1.00	0.436
2643	ST	W12X30	1.85	0.806
2644	ST	W12X30	0.18	0.078
2645	ST	W12X30	1.00	0.436
2646	ST	W12X30	1.00	0.436
2647	ST	W12X40	1.41	0.827
2648	ST	W12X40	1.41	0.827
2649	ST	W12X40	1.41	0.827
2650	ST	W12X40	1.41	0.827
2651	ST	W12X30	0.39	0.170
2652	ST	W12X30	0.39	0.170
2653	ST	W12X30	1.18	0.514
2654	ST	W12X30	1.18	0.514
2655	ST	W12X30	1.49	0.649
2656	ST	W12X30	1.49	0.649
2657	ST	W12X40	1.67	0.976
2658	ST	W12X40	1.67	0.976
2659	ST	W12X30	1.18	0.514
2660	ST	W12X30	0.18	0.078
2661	ST	W12X40	1.67	0.976

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2662	ST	W12X40	1.67	0.976
2663	ST	W12X35	3.25	1.659
2664	ST	W12X35	2.63	1.340
2665	ST	W12X35	2.63	1.340
2666	ST	W12X35	2.63	1.340
2667	ST	W12X35	2.63	1.340
2668	ST	W12X50	8.00	5.828
2669	ST	W12X50	8.00	5.828

2670	ST	W12X35	3.25	1.659
2671	ST	W12X35	3.25	1.659
2672	ST	W12X50	8.00	5.828
2673	ST	W12X50	8.00	5.828
2674	ST	W12X50	8.00	5.828
2675	ST	W12X35	2.75	1.404
2676	ST	W12X35	2.75	1.404
2677	ST	W12X35	2.75	1.404
2678	ST	W12X35	2.75	1.404
2679	ST	W12X50	8.00	5.828
2680	ST	W12X50	8.00	5.828
2681	ST	W12X50	4.00	2.914
2682	ST	W12X50	4.00	2.914
2683	ST	W12X50	4.00	2.914
2684	ST	W12X50	4.00	2.914
2685	ST	W12X35	2.75	1.404
2686	ST	W12X35	2.75	1.404
2687	ST	W12X50	8.00	5.828
2688	ST	W12X35	2.75	1.404
2689	ST	W12X35	2.75	1.404
2690	ST	W12X50	8.00	5.828
2691	ST	W12X50	4.00	2.914
2692	ST	W12X50	4.00	2.914
2693	ST	W12X35	2.63	1.340
2694	ST	W12X35	2.63	1.340
2695	ST	W12X50	8.00	5.828
2696	ST	W12X35	2.63	1.340
2697	ST	W12X35	2.63	1.340
2698	ST	W12X50	8.00	5.828
2699	ST	W12X50	4.00	2.914
2700	ST	W12X50	4.00	2.914
2701	ST	W12X50	8.00	5.828
2702	ST	W12X50	8.00	5.828
2703	ST	W12X50	4.00	2.914
2704	ST	W12X50	4.00	2.914
2705	ST	W12X35	1.50	0.766
2706	ST	W12X50	8.00	5.828
2707	ST	W12X35	3.25	1.659

2708	ST	W12X35	3.25	1.659
2709	ST	W12X35	3.25	1.659
2710	ST	W12X35	3.25	1.659
2711	ST	W12X35	3.25	1.659
2712	ST	W12X50	8.00	5.828
2713	ST	W12X50	4.00	2.914
2714	ST	W12X50	4.00	2.914
2715	ST	W12X50	4.00	2.914
2716	ST	W12X50	4.00	2.914
2717	ST	W12X50	4.00	2.914

STAAD SPACE

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2718	ST	W12X50	4.00	2.914
2719	ST	W12X50	4.00	2.914
2720	ST	W12X50	4.00	2.914
2721	ST	W12X50	4.00	2.914
2722	ST	W12X50	4.00	2.914
2723	ST	W12X50	4.00	2.914
2724	ST	W12X50	4.00	2.914
2725	ST	W12X50	4.00	2.914
2726	ST	W12X50	4.00	2.914
2727	LD	L40406	4.45	1.261
2728	LD	L40406	5.20	1.475
2729	LD	L40406	3.82	1.082
2730	LD	L40406	4.67	1.325
2731	LD	L40406	4.45	1.261
2732	LD	L40406	5.20	1.475
2733	LD	L40406	3.82	1.082
2734	LD	L40406	4.67	1.325

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TOTAL = 2349.539

\*\*\*\*\* END OF DATA FROM INTERNAL STORAGE \*\*\*\*\*

2253. STEEL TAKE OFF LIST 482 TO 506 509 512 515 518 521 524 527 530 TO 589 591 -

STAAD SPACE

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STEEL TAKE-OFF

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PROFILE	LENGTH (METE)	WEIGHT (KN )
2254. 592 TO 631 633 TO 694 696 TO 703 705 TO 708 710 TO 723 725 TO 738 740 TO 753 -		
2255. 755 TO 762 877 879 881 883 885 887 889 TO 891 1009 1011 1013 1015 1017 1019 -		
2256. 1021 TO 1023 1027 TO 1035 1051 TO 1053 1055 1056 1061 TO 1065 1067 1070 1073 -		
2257. 1074 TO 1081 1083 1085 TO 1087 1089 TO 1107 1109 TO 1114 1165 1174 1183 1253 -		
2258. 1438 TO 1456 1465 TO 1484 1493 TO 1525 1528 1531 1533 1543 1544 1584 1594 -		
2259. 1597 1599 1600 1605 TO 1611 1613 1615 TO 1621 1623 1625 TO 1631 1633 1639 -		
2260. 1640 TO 1646 1651 TO 1669 1685 1686 1689 TO 1727 1729 TO 1772 1777 1778 1781 -		
2261. 1782 TO 1784 1787 1788 1793 1794 1797 TO 1800 1803 TO 1849 1851 TO 1853 1855 -		
2262. 1856 TO 1865 1869 1870 1872 1873 1879 1880 1883 TO 1924 1926 TO 1945 1981 -		
2263. 1982 TO 1990 1995 1996 2005 TO 2024 2029 2030 2037 TO 2055 2059 2061 TO 2067 -		
2264. 2069 2070 2072 2073 2075 2076 2078 2079 2081 TO 2087 2089 TO 2101 2103 2106 -		
2265. 2107 TO 2111 2113 TO 2121 2124 TO 2127 2129 2131 TO 2135 2229 TO 2260 2269 -		
2266. 2270 TO 2293 2295 TO 2298 2301 TO 2305 2310 2311 2313 TO 2315 2317 TO 2319 -		
2267. 2321 2322 2326 TO 2340 2342 TO 2374 2376 TO 2378 2381 2383 2385 2387 2388 -		
2268. 2390 TO 2394 2397 TO 2431 2445 TO 2469 2473 TO 2522 2526 TO 2547 -		
2269. 2549 TO 2551 2554 TO 2557 2559 2561 2562 2564 2566 2568 TO 2585 2588 TO 2590 -		
2270. 2593 TO 2734		
ST W18X86	441.75	553.903
ST W12X50	1210.47	881.874
ST W14X68	99.95	99.072
ST W12X65	163.60	154.865
ST W12X35	482.00	246.049
ST W8X21	490.23	149.664
ST W6X16	127.50	29.952
LD L35358	48.18	15.520
LD L30306	197.02	41.187
ST W12X30	77.96	33.964
ST W10X30	220.25	96.494
ST W8X18	38.10	9.932
ST C10X15	44.63	9.932
ST L30306	31.86	3.331

ST W12X40	12.33	7.212
LD L40406	36.29	10.285
PRISMATIC STEEL	10.94	6.305
	-----	
	TOTAL =	2349.539

TOTAL VOLUME OF PRISMATIC STEEL SECTIONS = 0.08 CUBIC METE

\*\*\*\*\* END OF DATA FROM INTERNAL STORAGE \*\*\*\*\*

2271. LOAD LIST 20 TO 28  
 2272. PRINT JOINT DISPLACEMENTS LIST 305 306 315 316 333 334 343 344 353 354 369 -  
 2273. 370 TO 374 381 TO 386 388 390 TO 396 398 400 TO 406 415 416 427 TO 432 443 -  
 2274. 444 618 619 628 834 874 876 928 930 946 948 1055 1056 1174 1176 1302 TO 1305 -  
 2275. 1307 1308 1313 1317 TO 1320 1329 1334 TO 1347 1352 TO 1365 1368 1369 1380 -  
 2276. 1381

STAAD SPACE

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JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
305	20	-0.1355	-1.0044	0.1105	-0.0002	0.0002	-0.0015
	21	-0.2457	-1.2287	0.3768	0.0000	0.0003	-0.0012
	22	-0.4947	-1.2230	0.1252	-0.0001	0.0002	-0.0014
	23	2.1277	-0.5736	5.3661	0.0021	0.0033	-0.0011
	24	-2.3067	-1.4638	-5.1105	-0.0024	-0.0029	-0.0026
	25	1.0688	-0.9797	0.1388	-0.0002	-0.0003	-0.0019
	26	-1.3398	-1.0292	0.0823	-0.0002	0.0006	-0.0012
	27	0.9600	-1.1874	2.9205	0.0010	-0.0001	-0.0020
	28	-1.2310	-0.8214	-2.6995	-0.0014	0.0004	-0.0011

306	20	-0.1426	-1.7161	-0.0824	-0.0003	0.0001	0.0007
	21	-0.2567	-2.1016	-0.1051	-0.0002	0.0003	0.0006
	22	-0.5069	-1.8150	-0.9137	-0.0006	0.0002	0.0011
	23	2.1197	-0.9961	9.1369	0.0034	0.0032	0.0015
	24	-2.3139	-2.5303	-9.3261	-0.0041	-0.0029	0.0002
	25	1.0607	-1.7358	-0.0121	-0.0003	-0.0003	0.0010
	26	-1.3459	-1.6964	-0.1527	-0.0004	0.0006	0.0003
	27	0.9537	-1.9786	2.9412	0.0009	0.0004	0.0010
	28	-1.2388	-1.4536	-3.1059	-0.0015	-0.0002	0.0003
315	20	-0.1636	-0.9053	0.1132	0.0002	0.0001	-0.0003
	21	-0.2674	-0.9653	0.3795	0.0004	0.0003	-0.0003
	22	0.1932	-0.8933	0.1279	0.0003	0.0002	-0.0004
	23	2.5432	-0.5091	5.3678	0.0025	0.0028	0.0003
	24	-2.7935	-1.3070	-5.1068	-0.0020	-0.0026	-0.0009
	25	0.9702	-0.8727	0.1417	0.0002	0.0006	-0.0005
	26	-1.2975	-0.9378	0.0848	0.0002	-0.0004	0.0000
	27	0.7058	-0.6880	2.9233	0.0014	-0.0002	-0.0005
	28	-1.0331	-1.1226	-2.6968	-0.0010	0.0004	0.0000
316	20	-0.1682	-1.6207	-0.0705	0.0002	0.0001	-0.0004
	21	-0.2733	-1.9720	-0.0851	0.0000	0.0003	-0.0005
	22	0.1899	-1.6392	-0.9047	0.0002	0.0002	-0.0006
	23	2.5392	-0.8867	9.1485	0.0039	0.0028	0.0003
	24	-2.7986	-2.4377	-9.3123	-0.0036	-0.0025	-0.0013
	25	0.9645	-1.6216	-0.0001	0.0002	0.0006	-0.0001
	26	-1.3008	-1.6197	-0.1410	0.0001	-0.0004	-0.0008
	27	0.7010	-1.3823	2.9535	0.0013	0.0005	-0.0001
	28	-1.0373	-1.8591	-3.0946	-0.0010	-0.0002	-0.0008
333	20	-0.1517	-1.3853	-0.1437	-0.0004	-0.0008	-0.0003
	21	-0.2592	-1.4612	-0.1485	-0.0004	-0.0009	-0.0004
	22	-0.4755	-1.6243	-0.0046	-0.0006	-0.0018	-0.0010
	23	2.1873	-0.5715	8.5847	0.0030	0.0036	0.0008
	24	-2.3843	-2.3344	-8.9271	-0.0041	-0.0051	-0.0017
	25	1.0397	-1.2320	-0.1737	-0.0004	-0.0008	-0.0001
	26	-1.3432	-1.5386	-0.1138	-0.0004	-0.0007	-0.0004
	27	0.9443	-1.5399	2.6214	0.0006	-0.0010	-0.0003
	28	-1.2478	-1.2307	-2.9089	-0.0015	-0.0005	-0.0003
334	20	-0.1753	-1.3979	-0.1341	0.0000	0.0007	-0.0003
	21	-0.2634	-1.4796	-0.1369	-0.0001	0.0008	-0.0006





	22	0.2163	-2.3495	-2.4582	-0.0003	-0.0004	-0.0003
	23	2.6563	-1.0243	4.3363	0.0017	0.0043	0.0001
	24	-2.9134	-2.7646	-6.9237	-0.0021	-0.0029	-0.0005
	25	0.9689	-1.8238	-1.3201	-0.0005	0.0008	0.0002
	26	-1.2903	-1.5696	-1.1390	-0.0003	0.0004	-0.0005
	27	0.7162	-1.4586	1.1551	0.0003	0.0014	0.0003
	28	-1.0375	-1.9347	-3.6142	-0.0011	-0.0001	-0.0005
354	20	-0.1372	-1.2596	-1.2452	-0.0012	-0.0002	0.0000
	21	-0.2358	-1.3407	-1.4348	-0.0013	-0.0002	0.0002
	22	-0.4505	-1.8205	-2.4811	-0.0019	-0.0012	-0.0012
	23	2.2335	-1.3507	4.3221	0.0008	0.0026	0.0001
	24	-2.3940	-1.6740	-6.9470	-0.0035	-0.0031	-0.0013
	25	1.0613	-1.2963	-1.3370	-0.0013	-0.0003	0.0004
	26	-1.3357	-1.2230	-1.1534	-0.0010	-0.0001	-0.0004
	27	0.9680	-1.3393	1.1398	-0.0004	-0.0004	0.0003
	28	-1.2424	-1.1799	-3.6302	-0.0020	-0.0001	-0.0003
369	20	-0.0746	-1.2925	0.1099	0.0001	0.0000	-0.0002
	21	-0.0733	-1.4209	0.0471	0.0001	0.0000	-0.0002
	22	0.3349	-0.9912	3.9886	0.0042	-0.0010	0.0000
	23	2.2900	-0.8471	3.8306	0.0035	0.0010	0.0004
	24	-2.3800	-1.8858	-3.5997	-0.0032	-0.0010	-0.0009
	25	0.8687	-1.2479	0.6948	0.0008	0.0000	-0.0006

STAAD SPACE

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JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	26	-1.0178	-1.3370	-0.4749	-0.0005	0.0000	0.0001
	27	0.6332	-0.9441	2.8606	0.0026	0.0002	-0.0006
	28	-0.7824	-1.6409	-2.6407	-0.0023	-0.0001	0.0001
370	20	-0.2067	-1.3019	0.1347	0.0000	0.0000	0.0003
	21	-0.2643	-1.4322	0.0624	0.0000	0.0000	0.0004

	22	0.1650	-0.9998	5.0617	0.0041	-0.0009	0.0004
	23	2.5484	-0.8561	4.7472	0.0037	0.0011	0.0014
	24	-2.9000	-1.8971	-4.4639	-0.0036	-0.0010	-0.0008
	25	0.9432	-1.2570	0.8858	0.0007	0.0001	-0.0004
	26	-1.3567	-1.3469	-0.6164	-0.0006	0.0000	0.0009
	27	0.6826	-0.9528	3.5188	0.0025	0.0002	-0.0004
	28	-1.0961	-1.6511	-3.2494	-0.0024	-0.0002	0.0009
371	20	-0.0720	-1.3865	0.1112	0.0002	0.0000	-0.0005
	21	-0.0733	-1.4744	0.0482	0.0001	0.0000	-0.0004
	22	-0.4338	-2.0109	3.9923	0.0042	-0.0010	-0.0005
	23	1.8290	-1.0335	3.8329	0.0036	0.0010	0.0002
	24	-1.9175	-1.8969	-3.5994	-0.0032	-0.0010	-0.0011
	25	0.8913	-1.4726	0.6978	0.0008	0.0000	-0.0010
	26	-1.0353	-1.3005	-0.4754	-0.0005	0.0000	0.0001
	27	0.7569	-1.7032	2.8636	0.0026	0.0002	-0.0011
	28	-0.9009	-1.0699	-2.6412	-0.0023	-0.0001	0.0002
372	20	-0.1244	-1.4020	0.1631	0.0002	0.0000	0.0002
	21	-0.1742	-1.4918	0.0909	0.0001	0.0000	0.0003
	22	-0.4958	-2.0304	5.1205	0.0043	-0.0010	0.0004
	23	2.1242	-1.0480	4.7901	0.0038	0.0011	0.0012
	24	-2.3109	-1.9148	-4.4498	-0.0034	-0.0011	-0.0008
	25	1.1176	-1.4881	0.9256	0.0008	0.0000	-0.0006
	26	-1.3664	-1.3159	-0.5993	-0.0005	0.0000	0.0009
	27	1.0077	-1.7193	3.5629	0.0027	0.0002	-0.0006
	28	-1.2564	-1.0847	-3.2367	-0.0023	-0.0002	0.0010
373	20	-0.1414	-1.3088	0.1345	0.0000	0.0001	-0.0002
	21	-0.2007	-1.4401	0.0515	0.0000	0.0001	-0.0002
	22	0.2241	-1.0050	6.0029	0.0034	-0.0009	-0.0004
	23	2.6346	-0.8622	5.6161	0.0033	0.0013	0.0001
	24	-2.8649	-1.9054	-5.3305	-0.0033	-0.0011	-0.0005
	25	1.0027	-1.2634	1.0323	0.0005	0.0001	0.0002
	26	-1.2855	-1.3542	-0.7634	-0.0005	0.0000	-0.0006
	27	0.7575	-0.9587	4.1066	0.0022	0.0003	0.0002
	28	-1.0402	-1.6588	-3.8376	-0.0022	-0.0002	-0.0006
374	20	-0.0484	-1.4131	0.1970	0.0001	0.0000	-0.0003
	21	-0.0984	-1.5039	0.1141	0.0000	0.0000	-0.0003
	22	-0.4307	-2.0441	6.0881	0.0034	-0.0010	-0.0005
	23	2.2192	-1.0580	5.6898	0.0034	0.0013	0.0000

24	-2.2720	-1.9279	-5.2790	-0.0032	-0.0013	-0.0008
25	1.1820	-1.4993	1.1031	0.0006	0.0000	0.0001
26	-1.2788	-1.3270	-0.7090	-0.0005	0.0000	-0.0007
27	1.0817	-1.7309	4.1803	0.0022	0.0002	0.0001
28	-1.1785	-1.0953	-3.7862	-0.0021	-0.0002	-0.0006

STAAD SPACE

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JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
381	20	-0.1165	-1.6443	0.0973	-0.0001	0.0000	0.0003
	21	-0.1111	-1.7919	0.0362	-0.0002	0.0000	0.0004
	22	0.3146	-1.1843	5.2086	0.0046	-0.0010	0.0005
	23	2.2306	-1.2525	3.7060	0.0032	0.0010	0.0010
	24	-2.4067	-2.0951	-3.4957	-0.0033	-0.0010	-0.0004
	25	0.8154	-1.5557	0.6525	0.0004	0.0000	-0.0001
	26	-1.0484	-1.7329	-0.4579	-0.0006	0.0000	0.0006
	27	0.5899	-1.3496	2.6862	0.0022	0.0001	0.0000
	28	-0.8228	-1.9390	-2.4916	-0.0023	-0.0001	0.0005
382	20	-0.1964	-1.6544	0.0742	0.0001	0.0001	0.0002
	21	-0.2520	-1.8049	-0.0027	0.0002	0.0001	0.0003
	22	0.1734	-1.1970	6.3162	0.0037	0.0008	0.0002
	23	2.5652	-1.2626	4.4976	0.0030	0.0013	0.0014
	24	-2.8979	-2.1062	-4.3306	-0.0027	-0.0011	-0.0011
	25	0.9306	-1.5662	0.7471	0.0005	0.0002	-0.0004
	26	-1.3234	-1.7425	-0.5988	-0.0003	0.0000	0.0007
	27	0.6647	-1.3598	3.2086	0.0020	0.0005	-0.0004
	28	-1.0576	-1.9490	-3.0603	-0.0017	-0.0002	0.0007
383	20	-0.1227	-1.7337	0.1015	0.0000	0.0000	0.0001
	21	-0.1233	-1.8249	0.0440	0.0000	0.0000	0.0002
	22	-0.5115	-2.2750	5.2139	0.0047	-0.0011	0.0000
	23	1.7527	-1.3186	3.7104	0.0033	0.0010	0.0008

	24	-1.9442	-2.2182	-3.4916	-0.0032	-0.0010	-0.0006
	25	0.8206	-1.7578	0.6575	0.0005	0.0000	-0.0004
	26	-1.0660	-1.7095	-0.4545	-0.0005	0.0000	0.0006
	27	0.6882	-2.0254	2.6908	0.0023	0.0001	-0.0005
	28	-0.9336	-1.4419	-2.4878	-0.0022	-0.0001	0.0007
384	20	-0.1047	-1.7466	0.0763	-0.0004	0.0001	-0.0001
	21	-0.1509	-1.8399	-0.0009	-0.0006	0.0001	0.0000
	22	-0.4324	-2.2876	6.3197	0.0031	-0.0030	-0.0001
	23	2.1476	-1.3314	4.5004	0.0025	0.0013	0.0009
	24	-2.3012	-2.2324	-4.3288	-0.0032	-0.0011	-0.0010
	25	1.1312	-1.7711	0.7501	0.0000	0.0002	-0.0009
	26	-1.3405	-1.7221	-0.5974	-0.0007	0.0000	0.0007
	27	1.0278	-2.0391	3.2119	0.0015	0.0005	-0.0010
	28	-1.2372	-1.4542	-3.0593	-0.0022	-0.0002	0.0008
385	20	-0.1711	-1.6608	0.0431	0.0000	0.0001	-0.0002
	21	-0.2283	-1.8125	-0.0512	-0.0001	0.0001	-0.0003
	22	0.2338	-1.2075	7.0401	0.0026	-0.0008	-0.0005
	23	2.5937	-1.2705	5.1691	0.0025	0.0015	0.0004
	24	-2.8836	-2.1122	-5.0605	-0.0025	-0.0014	-0.0008
	25	0.9668	-1.5731	0.8001	0.0003	0.0002	0.0000
	26	-1.3091	-1.7484	-0.7139	-0.0003	0.0000	-0.0004
	27	0.7341	-1.3671	3.6269	0.0016	0.0004	-0.0002
	28	-1.0764	-1.9544	-3.5408	-0.0016	-0.0002	-0.0002
386	20	-0.0996	-1.7540	0.0359	-0.0003	0.0001	0.0001
	21	-0.1513	-1.8475	-0.0614	-0.0003	0.0001	0.0001
	22	-0.4880	-2.2925	7.0281	0.0022	-0.0010	0.0002

STAAD SPACE

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JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	23	2.1605	-1.3403	5.1617	0.0022	0.0019	0.0005

	24	-2.3084	-2.2394	-5.0679	-0.0028	-0.0017	-0.0005
	25	1.1309	-1.7786	0.7925	0.0000	0.0002	0.0005
	26	-1.3302	-1.7294	-0.7207	-0.0006	0.0000	-0.0004
	27	1.0222	-2.0461	3.6184	0.0012	0.0004	0.0005
	28	-1.2214	-1.4619	-3.5466	-0.0018	-0.0002	-0.0004
388	20	-0.0769	-1.5993	0.5932	0.0005	0.0000	0.0000
	21	-0.0704	-1.6668	0.5938	0.0006	-0.0001	0.0000
	22	-0.3629	-1.7170	-0.6047	-0.0006	-0.0009	0.0005
	23	1.7389	-1.3844	1.7384	0.0015	0.0021	0.0010
	24	-1.8455	-1.9008	-0.5049	-0.0004	-0.0022	-0.0009
	25	0.8387	-1.6045	0.6109	0.0006	-0.0001	-0.0004
	26	-0.9926	-1.5941	0.5755	0.0005	0.0000	0.0005
	27	0.7333	-1.7868	1.3582	0.0012	0.0001	-0.0002
	28	-0.8872	-1.4119	-0.1717	-0.0001	-0.0002	0.0003
390	20	-0.0587	-1.5598	0.5889	0.0011	0.0000	0.0000
	21	-0.0435	-1.6959	0.5889	0.0011	0.0000	0.0000
	22	0.2825	-1.4267	-0.6118	0.0001	-0.0007	-0.0004
	23	2.2346	-1.4546	1.7339	0.0019	0.0014	0.0012
	24	-2.2969	-1.7208	-0.5091	0.0003	-0.0013	-0.0013
	25	0.8355	-1.5349	0.6057	0.0011	0.0002	-0.0005
	26	-0.9529	-1.5847	0.5721	0.0011	-0.0002	0.0005
	27	0.5731	-1.5054	1.3519	0.0016	0.0003	-0.0004
	28	-0.6906	-1.6142	-0.1741	0.0006	-0.0003	0.0004
391	20	-0.1195	-1.5895	0.8834	0.0008	0.0000	0.0003
	21	-0.1135	-1.7296	0.9055	0.0008	0.0000	0.0005
	22	0.3091	-1.4501	-0.7418	-0.0005	-0.0012	0.0003
	23	2.2229	-1.4802	2.3308	0.0018	0.0011	0.0011
	24	-2.4060	-1.7560	-0.4966	0.0000	-0.0011	-0.0005
	25	0.8066	-1.5633	0.9019	0.0008	0.0000	-0.0001
	26	-1.0456	-1.6156	0.8650	0.0008	0.0000	0.0007
	27	0.5761	-1.5320	1.8211	0.0013	0.0001	-0.0001
	28	-0.8151	-1.6469	-0.0542	0.0003	-0.0002	0.0007
392	20	-0.1811	-1.6046	1.0681	0.0005	0.0001	0.0002
	21	-0.2312	-1.7465	1.0791	0.0005	0.0000	0.0003
	22	0.1995	-1.4670	-0.8704	-0.0005	-0.0049	0.0002
	23	2.5616	-1.4948	2.7394	0.0015	0.0053	0.0014
	24	-2.8656	-1.7729	-0.5207	-0.0004	-0.0052	-0.0010
	25	0.9306	-1.5784	1.0842	0.0005	0.0001	-0.0004

	26	-1.2928	-1.6307	1.0521	0.0005	0.0000	0.0007
	27	0.6656	-1.5458	2.1161	0.0010	0.0003	-0.0003
	28	-1.0277	-1.6633	0.0202	0.0001	-0.0002	0.0007
393	20	-0.1238	-1.6202	0.8765	0.0009	0.0000	0.0000
	21	-0.1235	-1.6887	0.8966	0.0009	0.0000	0.0002
	22	-0.5090	-1.7474	-0.7402	-0.0004	-0.0009	-0.0001
	23	1.7355	-1.4034	2.3213	0.0018	0.0009	0.0009
	24	-1.9312	-1.9254	-0.5009	0.0000	-0.0010	-0.0009
	25	0.8130	-1.6252	0.8947	0.0009	0.0000	-0.0007

STAAD SPACE

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JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	26	-1.0605	-1.6153	0.8583	0.0009	0.0000	0.0007
	27	0.6873	-1.8093	1.8129	0.0013	0.0001	-0.0007
	28	-0.9348	-1.4312	-0.0598	0.0004	-0.0002	0.0008
394	20	-0.0930	-1.6298	1.0702	0.0006	0.0001	-0.0001
	21	-0.1391	-1.6993	1.0818	0.0005	0.0001	0.0001
	22	-0.3993	-1.7621	-0.8575	-0.0003	0.0012	-0.0002
	23	2.1522	-1.4122	2.7471	0.0016	0.0016	0.0011
	24	-2.2860	-1.9374	-0.5220	-0.0003	-0.0014	-0.0012
	25	1.1368	-1.6349	1.0866	0.0006	0.0003	-0.0009
	26	-1.3228	-1.6247	1.0538	0.0006	0.0000	0.0007
	27	1.0366	-1.8199	2.1223	0.0011	0.0005	-0.0009
	28	-1.2225	-1.4396	0.0181	0.0001	-0.0003	0.0008
395	20	-0.2063	-1.6159	1.1958	0.0006	0.0001	0.0001
	21	-0.2677	-1.7592	1.1895	0.0006	0.0001	0.0001
	22	0.2196	-1.4778	-0.9249	0.0002	-0.0010	-0.0002
	23	2.5730	-1.5056	3.1180	0.0017	0.0014	0.0006
	24	-2.9326	-1.7861	-0.6300	-0.0003	-0.0012	-0.0005
	25	0.9328	-1.5900	1.2134	0.0007	0.0002	0.0002

	26	-1.3453	-1.6419	1.1782	0.0006	0.0000	0.0000
	27	0.7068	-1.5564	2.3720	0.0011	0.0004	-0.0001
	28	-1.1194	-1.6755	0.0196	0.0001	-0.0002	0.0002
396	20	-0.1223	-1.6383	1.1906	0.0002	0.0001	0.0002
	21	-0.1710	-1.7089	1.1845	0.0001	0.0001	0.0002
	22	-0.5025	-1.7763	-0.9314	-0.0005	-0.0008	0.0007
	23	2.1427	-1.4202	3.1109	0.0011	0.0012	0.0008
	24	-2.3330	-1.9481	-0.6356	-0.0008	-0.0010	-0.0004
	25	1.1109	-1.6435	1.2083	0.0002	0.0002	0.0006
	26	-1.3555	-1.6330	1.1730	0.0002	0.0000	-0.0002
	27	0.9959	-1.8294	2.3668	0.0007	0.0004	0.0007
	28	-1.2404	-1.4472	0.0145	-0.0003	-0.0002	-0.0003
398	20	-0.0660	-1.4078	0.6312	0.0005	-0.0001	0.0001
	21	-0.0591	-1.4730	0.6480	0.0005	-0.0001	0.0001
	22	-0.3700	-1.3682	0.2236	0.0001	-0.0010	0.0008
	23	1.6955	-1.2177	1.6640	0.0014	0.0020	0.0018
	24	-1.7817	-1.6883	-0.3467	-0.0004	-0.0022	-0.0016
	25	0.8183	-1.4294	0.6309	0.0005	-0.0002	-0.0006
	26	-0.9503	-1.3861	0.6314	0.0005	0.0000	0.0009
	27	0.7148	-1.5647	1.2087	0.0011	0.0001	-0.0004
	28	-0.8468	-1.2508	0.0536	-0.0001	-0.0003	0.0007
400	20	-0.0100	-1.1773	0.6275	0.0009	0.0000	0.0004
	21	0.0180	-1.2456	0.6445	0.0010	0.0000	0.0004
	22	0.3322	-1.2321	0.2206	0.0006	-0.0007	-0.0002
	23	2.2302	-1.1168	1.6591	0.0017	0.0013	0.0024
	24	-2.1972	-1.2946	-0.3492	0.0002	-0.0013	-0.0016
	25	0.8638	-1.2043	0.6255	0.0009	0.0002	-0.0004
	26	-0.8837	-1.1503	0.6294	0.0009	-0.0002	0.0012
	27	0.6128	-1.2349	1.2035	0.0014	0.0003	-0.0004
	28	-0.6327	-1.1197	0.0514	0.0004	-0.0003	0.0012

STAAD SPACE

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JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
401	20	-0.1545	-1.1957	0.8695	0.0008	0.0000	0.0003
	21	-0.1573	-1.2660	0.8932	0.0008	0.0000	0.0005
	22	0.2670	-1.2534	0.3276	0.0005	-0.0010	0.0003
	23	2.2088	-1.1329	2.1884	0.0017	0.0010	0.0010
	24	-2.4619	-1.3170	-0.3755	0.0000	-0.0010	-0.0004
	25	0.7769	-1.2255	0.8724	0.0008	0.0001	0.0000
	26	-1.0858	-1.1659	0.8665	0.0008	0.0000	0.0007
	27	0.5363	-1.2566	1.6351	0.0013	0.0002	0.0000
402	28	-0.8452	-1.1348	0.1039	0.0003	-0.0001	0.0007
	20	-0.1858	-1.2027	1.0671	0.0007	0.0001	0.0001
	21	-0.2341	-1.2737	1.0967	0.0008	0.0001	0.0002
	22	0.2156	-1.2620	0.5031	0.0007	-0.0019	0.0001
	23	2.5390	-1.1408	2.5876	0.0016	0.0017	0.0012
	24	-2.8529	-1.3244	-0.3636	0.0000	-0.0016	-0.0011
	25	0.9192	-1.2328	1.0748	0.0008	0.0001	-0.0005
	26	-1.2908	-1.1727	1.0594	0.0007	0.0000	0.0006
403	27	0.6596	-1.2634	1.9403	0.0012	0.0003	-0.0005
	28	-1.0312	-1.1421	0.1939	0.0003	-0.0002	0.0006
	20	-0.1311	-1.4270	0.8724	0.0008	0.0000	0.0000
	21	-0.1300	-1.4938	0.8984	0.0009	0.0000	0.0001
	22	-0.4989	-1.3871	0.3309	0.0006	-0.0009	-0.0003
	23	1.7384	-1.2361	2.1890	0.0017	0.0011	0.0008
	24	-1.9501	-1.7103	-0.3701	0.0000	-0.0010	-0.0008
	25	0.8170	-1.4510	0.8750	0.0008	0.0001	-0.0007
404	26	-1.0793	-1.4030	0.8699	0.0008	0.0000	0.0006
	27	0.6981	-1.5866	1.6364	0.0013	0.0002	-0.0007
	28	-0.9604	-1.2674	0.1084	0.0003	-0.0001	0.0007
	20	-0.0829	-1.4379	1.0669	0.0004	0.0002	0.0000
	21	-0.1246	-1.5065	1.0954	0.0003	0.0002	0.0001
	22	-0.3722	-1.3992	0.5040	0.0003	0.0017	-0.0001
	23	2.1527	-1.2468	2.5943	0.0013	0.0013	0.0011
	24	-2.2643	-1.7230	-0.3687	-0.0003	-0.0009	-0.0012
	25	1.1439	-1.4621	1.0741	0.0004	0.0003	-0.0008
	26	-1.3097	-1.4136	1.0596	0.0004	0.0000	0.0007
	27	1.0399	-1.5980	1.9433	0.0008	0.0005	-0.0008

	28	-1.2057	-1.2777	0.1904	0.0001	-0.0002	0.0007
405	20	-0.2095	-1.2078	1.2116	0.0007	0.0001	0.0001
	21	-0.2710	-1.2787	1.2369	0.0007	0.0001	0.0001
	22	0.2144	-1.2683	0.6263	0.0007	-0.0010	0.0000
	23	2.5800	-1.1463	2.9526	0.0016	0.0013	0.0005
	24	-2.9464	-1.3302	-0.4250	-0.0001	-0.0011	-0.0002
	25	0.9299	-1.2378	1.2217	0.0007	0.0002	0.0002
	26	-1.3489	-1.1777	1.2015	0.0007	0.0000	0.0001
	27	0.7035	-1.2683	2.1901	0.0011	0.0003	0.0000
	28	-1.1224	-1.1472	0.2331	0.0003	-0.0002	0.0002
406	20	-0.1466	-1.4437	1.2022	0.0004	0.0001	0.0004
	21	-0.1973	-1.5127	1.2257	0.0004	0.0001	0.0004
	22	-0.5058	-1.4054	0.6138	0.0002	-0.0008	0.0009

STAAD SPACE

-- PAGE NO. 309

JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	23	2.1248	-1.2529	2.9415	0.0011	0.0012	0.0008
	24	-2.3678	-1.7301	-0.4350	-0.0005	-0.0011	0.0000
	25	1.0913	-1.4681	1.2121	0.0004	0.0002	0.0007
	26	-1.3846	-1.4193	1.1923	0.0003	0.0000	0.0001
	27	0.9768	-1.6041	2.1804	0.0007	0.0003	0.0008
	28	-1.2700	-1.2833	0.2240	0.0000	-0.0002	0.0000
415	20	-0.2119	-1.1019	-0.1178	0.0001	0.0001	0.0001
	21	-0.2734	-1.1609	-0.1904	0.0001	0.0001	0.0001
	22	0.2106	-1.0690	0.4816	0.0004	-0.0009	0.0001
	23	2.5803	-0.4779	3.5188	0.0021	0.0013	0.0004
	24	-2.9519	-1.7575	-3.7362	-0.0018	-0.0011	-0.0002
	25	0.9269	-0.8774	-0.0253	0.0002	0.0002	0.0001
	26	-1.3508	-1.3263	-0.2104	0.0001	0.0000	0.0001
	27	0.7003	-0.7352	1.9146	0.0010	0.0003	0.0000

	28	-1.1242	-1.4685	-2.1502	-0.0008	-0.0002	0.0003
416	20	-0.1471	-1.0482	-0.1257	-0.0003	0.0001	0.0004
	21	-0.1980	-1.0777	-0.1980	-0.0003	0.0001	0.0004
	22	-0.5021	-1.2572	0.4678	-0.0001	-0.0009	0.0010
	23	2.1285	-0.3741	3.5097	0.0016	0.0013	0.0006
	24	-2.3725	-1.7663	-3.7452	-0.0023	-0.0011	0.0001
	25	1.0930	-0.8318	-0.0334	-0.0002	0.0002	0.0006
	26	-1.3871	-1.2646	-0.2179	-0.0003	0.0000	0.0001
	27	0.9785	-1.0745	1.9064	0.0007	0.0003	0.0007
	28	-1.2727	-1.0218	-2.1577	-0.0012	-0.0002	0.0000
427	20	-0.1687	-1.3733	0.0867	0.0001	0.0000	-0.0001
	21	-0.1655	-1.4599	0.0310	0.0001	0.0000	0.0001
	22	-0.5270	-1.9943	3.3356	0.0041	-0.0008	0.0002
	23	1.7256	-1.0214	3.2884	0.0035	0.0011	0.0003
	24	-1.9973	-1.8817	-3.1056	-0.0033	-0.0010	-0.0004
	25	0.7825	-1.4594	0.5769	0.0007	0.0000	-0.0002
	26	-1.1200	-1.2872	-0.4035	-0.0005	0.0000	0.0001
	27	0.6399	-1.6895	2.4595	0.0026	0.0002	-0.0003
	28	-0.9774	-1.0571	-2.2862	-0.0023	-0.0001	0.0001
428	20	-0.0996	-1.7204	0.0900	0.0000	0.0000	0.0001
	21	-0.0989	-1.8108	0.0390	0.0000	0.0000	0.0001
	22	-0.4600	-2.2626	4.4618	0.0049	-0.0009	0.0006
	23	1.7982	-1.3057	3.1863	0.0034	0.0015	0.0005
	24	-1.9335	-2.2039	-2.9923	-0.0033	-0.0015	-0.0003
	25	0.8548	-1.7448	0.5643	0.0006	0.0000	0.0003
	26	-1.0539	-1.6961	-0.3843	-0.0005	0.0000	0.0000
	27	0.7166	-2.0121	2.3195	0.0024	0.0002	0.0004
	28	-0.9157	-1.4288	-2.1396	-0.0023	-0.0002	-0.0001
429	20	-0.1623	-1.2827	0.0928	0.0001	0.0000	-0.0001
	21	-0.1555	-1.4099	0.0363	0.0001	0.0000	0.0001
	22	0.2735	-0.9830	3.3431	0.0041	-0.0008	-0.0002
	23	2.1991	-0.8381	3.2948	0.0035	0.0009	0.0003
	24	-2.4552	-1.8744	-3.1001	-0.0033	-0.0009	-0.0004
	25	0.7750	-1.2387	0.5843	0.0007	0.0000	-0.0002

JOINT DISPLACEMENT (CM      RADIANS)      STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	26	-1.0996	-1.3268	-0.3988	-0.0005	0.0000	0.0001
	27	0.5353	-0.9353	2.4674	0.0026	0.0002	-0.0002
	28	-0.8599	-1.6302	-2.2819	-0.0023	-0.0001	0.0001
430	20	-0.0951	-1.6321	0.1090	0.0000	0.0000	0.0001
	21	-0.0912	-1.7780	0.0582	0.0000	0.0000	0.0001
	22	0.3382	-1.1704	4.4805	0.0049	-0.0009	-0.0003
	23	2.2746	-1.2402	3.2050	0.0034	0.0009	0.0006
	24	-2.3972	-2.0824	-2.9733	-0.0033	-0.0009	-0.0004
	25	0.8439	-1.5436	0.5825	0.0005	0.0000	0.0001
	26	-1.0341	-1.7206	-0.3645	-0.0005	0.0000	0.0000
	27	0.6015	-1.3373	2.3381	0.0024	0.0002	-0.0001
	28	-0.7917	-1.9269	-2.1201	-0.0023	-0.0002	0.0003
431	20	-0.2915	-1.1853	-0.4692	-0.0001	0.0001	0.0001
	21	-0.3942	-1.3022	-0.6407	-0.0003	0.0001	0.0004
	22	0.1016	-1.3235	-0.6565	0.0000	-0.0008	0.0003
	23	2.4995	-0.3274	6.5320	0.0030	0.0014	0.0005
	24	-3.0383	-2.1004	-7.4665	-0.0031	-0.0012	-0.0002
	25	0.8302	-1.4264	-0.4365	-0.0001	0.0003	0.0003
	26	-1.4133	-0.9442	-0.5019	-0.0001	-0.0001	0.0000
	27	0.6230	-1.1414	2.2419	0.0009	0.0000	0.0002
	28	-1.2061	-1.2292	-3.1802	-0.0012	0.0002	0.0001
432	20	-0.2352	-1.0531	-0.4768	-0.0004	0.0001	0.0007
	21	-0.2959	-1.1121	-0.6503	-0.0004	0.0001	0.0007
	22	-0.6004	-1.0097	-0.6646	-0.0006	-0.0010	0.0006
	23	2.0402	-0.4525	6.5219	0.0026	0.0014	0.0010
	24	-2.4692	-1.7191	-7.4732	-0.0036	-0.0012	0.0005
	25	0.9869	-1.3011	-0.4443	-0.0004	0.0002	0.0008
	26	-1.4573	-0.8051	-0.5093	-0.0004	0.0000	0.0005
	27	0.8676	-1.4994	2.2339	0.0007	0.0000	0.0009
	28	-1.3379	-0.6068	-3.1875	-0.0015	0.0001	0.0005
443	20	-0.2995	-0.8606	-0.5634	-0.0003	0.0001	0.0007

	21	-0.4045	-0.9949	-0.7547	-0.0005	0.0002	0.0006
	22	0.0923	-0.9880	-0.1853	-0.0003	-0.0009	0.0007
	23	2.4906	-0.3005	5.7201	0.0019	0.0014	0.0010
	24	-3.0455	-1.4321	-6.8370	-0.0026	-0.0012	0.0005
	25	0.8216	-0.9821	-0.6584	-0.0004	0.0002	0.0008
	26	-1.4207	-0.7391	-0.4684	-0.0003	0.0000	0.0006
	27	0.6143	-0.7912	1.8279	0.0005	0.0008	0.0007
	28	-1.2133	-0.9300	-2.9547	-0.0012	-0.0006	0.0008
444	20	-0.2361	-0.5399	-0.5642	-0.0004	0.0001	0.0007
	21	-0.2969	-0.5507	-0.7556	-0.0005	0.0001	0.0007
	22	-0.5997	-0.5613	-0.1861	-0.0004	-0.0007	0.0009
	23	2.0392	-0.1186	5.7177	0.0019	0.0014	0.0010
	24	-2.4700	-0.9727	-6.8362	-0.0027	-0.0012	0.0004
	25	0.9864	-0.6309	-0.6592	-0.0004	0.0003	0.0009
	26	-1.4586	-0.4490	-0.4692	-0.0003	-0.0001	0.0004
	27	0.8673	-0.7894	1.8270	0.0004	0.0008	0.0010
	28	-1.3394	-0.2904	-2.9555	-0.0012	-0.0006	0.0004

STAAD SPACE

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JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
618	20	-0.1663	-1.6214	-1.1631	0.0005	0.0003	-0.0014
	21	-0.2476	-1.7597	-1.3360	0.0004	0.0004	-0.0016
	22	0.2101	-2.1939	-2.8320	0.0012	-0.0007	-0.0022
	23	2.6408	-0.9364	4.9182	0.0029	0.0017	-0.0003
	24	-2.9075	-2.6609	-7.3939	-0.0010	-0.0010	-0.0033
	25	0.9610	-1.7999	-1.1882	0.0004	0.0005	-0.0010
	26	-1.2937	-1.4430	-1.1381	0.0006	0.0001	-0.0019
	27	0.7074	-1.4554	1.4872	0.0012	0.0007	-0.0010
	28	-1.0401	-1.7875	-3.8135	-0.0002	-0.0001	-0.0019
619	20	0.0230	-1.8481	-1.1629	0.0005	0.0003	-0.0001

	21	-0.0459	-1.9460	-1.3358	0.0004	0.0003	-0.0002
	22	0.0497	-2.6541	-2.8328	0.0012	-0.0009	-0.0007
	23	3.4309	-1.7307	4.9166	0.0028	0.0016	0.0008
	24	-3.2993	-2.6627	-7.3918	-0.0011	-0.0010	-0.0014
	25	1.2090	-1.9963	-1.1878	0.0004	0.0005	0.0002
	26	-1.1630	-1.7000	-1.1380	0.0006	0.0001	-0.0004
	27	1.1203	-1.8670	1.4865	0.0012	0.0006	0.0003
	28	-1.0743	-1.8293	-3.8123	-0.0002	0.0000	-0.0006
628	20	-0.1702	-1.5718	-1.0517	0.0004	0.0007	0.0011
	21	-0.2543	-1.6772	-1.2076	0.0002	0.0009	0.0011
	22	0.2061	-2.0994	-2.6887	0.0005	0.0021	0.0015
	23	2.6283	-0.8875	5.7132	0.0023	0.0056	0.0033
	24	-2.9014	-2.5944	-7.9289	-0.0010	-0.0040	-0.0004
	25	0.9551	-1.7781	-1.0467	0.0003	0.0008	0.0005
	26	-1.2955	-1.3655	-1.0566	0.0005	0.0007	0.0016
	27	0.7010	-1.4522	1.7407	0.0011	0.0015	0.0007
	28	-1.0413	-1.6913	-3.8440	-0.0004	0.0000	0.0014
834	20	-0.0715	-1.8004	0.0978	-0.0016	0.0001	-0.0002
	21	-0.1225	-1.9147	0.0069	-0.0017	0.0001	-0.0002
	22	-0.4541	-2.4006	6.5281	-0.0003	-0.0008	-0.0001
	23	2.1865	-1.3857	5.3305	0.0012	0.0013	-0.0001
	24	-2.2830	-2.3232	-5.1145	-0.0045	-0.0011	-0.0003
	25	1.1583	-1.8559	0.9229	-0.0015	0.0002	-0.0001
	26	-1.3012	-1.7449	-0.7273	-0.0018	0.0000	-0.0003
	27	1.0559	-2.1148	3.8584	-0.0006	0.0004	-0.0002
	28	-1.1988	-1.4860	-3.6627	-0.0027	-0.0002	-0.0002
874	20	-0.1283	-1.5987	0.8821	0.0010	0.0000	0.0001
	21	-0.1246	-1.7432	0.9057	0.0010	0.0000	0.0000
	22	0.2974	-1.5131	-0.4649	-0.0002	-0.0011	-0.0002
	23	2.2177	-1.5078	2.2236	0.0020	0.0011	0.0003
	24	-2.4186	-1.7466	-0.3906	0.0000	-0.0010	-0.0001
	25	0.7981	-1.5776	0.8971	0.0010	0.0000	0.0001
	26	-1.0548	-1.6198	0.8671	0.0009	0.0000	0.0000
	27	0.5643	-1.5699	1.7814	0.0016	0.0002	0.0000
	28	-0.8210	-1.6275	-0.0171	0.0003	-0.0002	0.0001
876	20	-0.1256	-1.6396	0.8821	0.0010	0.0000	0.0000
	21	-0.1249	-1.7027	0.9057	0.0010	0.0000	0.0000
	22	-0.5064	-1.7144	-0.4649	-0.0002	-0.0011	0.0002

JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	23	1.7326	-1.4419	2.2236	0.0020	0.0010	0.0002
	24	-1.9325	-1.9270	-0.3906	0.0000	-0.0011	-0.0002
	25	0.8118	-1.6396	0.8971	0.0010	0.0000	0.0000
	26	-1.0630	-1.6397	0.8671	0.0009	0.0000	0.0000
	27	0.6882	-1.8090	1.7814	0.0016	0.0002	0.0001
	28	-0.9395	-1.4703	-0.0171	0.0004	-0.0002	-0.0001
928	20	-0.1158	-1.6781	0.2943	0.0002	-0.0010	-0.0001
	21	-0.1100	-1.8204	0.2550	0.0002	-0.0011	-0.0001
	22	0.3158	-1.2994	3.7222	0.0035	0.0073	-0.0005
	23	2.2285	-1.3665	3.4680	0.0058	0.0038	0.0003
	24	-2.4036	-2.0483	-2.8509	-0.0053	-0.0057	-0.0005
	25	0.8141	-1.6067	0.7154	0.0006	-0.0003	-0.0002
	26	-1.0458	-1.7494	-0.1268	-0.0002	-0.0016	0.0000
	27	0.5885	-1.4465	2.8599	0.0021	-0.0004	-0.0004
	28	-0.8202	-1.9096	-2.2714	-0.0017	-0.0015	0.0002
930	20	-0.1229	-1.7506	0.2943	0.0003	-0.0009	0.0000
	21	-0.1233	-1.8324	0.2550	0.0002	-0.0010	0.0000
	22	-0.5128	-2.1859	3.7222	0.0035	0.0073	0.0005
	23	1.7469	-1.3974	3.4680	0.0058	0.0038	0.0003
	24	-1.9391	-2.1785	-2.8508	-0.0052	-0.0057	-0.0004
	25	0.8175	-1.7712	0.7154	0.0007	-0.0003	0.0000
	26	-1.0633	-1.7301	-0.1268	-0.0001	-0.0016	0.0000
	27	0.6865	-2.0158	2.8599	0.0022	-0.0004	0.0001
	28	-0.9322	-1.4855	-2.2713	-0.0016	-0.0015	-0.0001
946	20	-0.0717	-1.6838	0.1061	0.0001	0.0000	-0.0009
	21	-0.0716	-1.7689	0.0435	0.0000	0.0000	-0.0009
	22	-0.4401	-2.3056	4.2846	0.0044	-0.0010	-0.0008

	23	1.8218	-1.3019	3.7626	0.0035	0.0010	-0.0007
	24	-1.9105	-2.2014	-3.5379	-0.0033	-0.0010	-0.0009
	25	0.8840	-1.7738	0.6827	0.0007	0.0000	-0.0008
	26	-1.0274	-1.5938	-0.4704	-0.0005	0.0000	-0.0009
	27	0.7489	-2.0160	2.8156	0.0025	0.0002	-0.0009
	28	-0.8923	-1.3517	-2.6033	-0.0024	-0.0002	-0.0008
948	20	-0.0714	-1.5824	0.1061	0.0001	0.0000	-0.0008
	21	-0.0676	-1.7091	0.0435	0.0000	0.0000	-0.0008
	22	0.3430	-1.2243	4.2846	0.0044	-0.0010	-0.0007
	23	2.2847	-1.1404	3.7625	0.0035	0.0010	-0.0007
	24	-2.3693	-2.1517	-3.5379	-0.0033	-0.0010	-0.0010
	25	0.8673	-1.5384	0.6827	0.0007	0.0000	-0.0008
	26	-1.0100	-1.6264	-0.4704	-0.0005	0.0000	-0.0009
	27	0.6354	-1.2494	2.8156	0.0025	0.0002	-0.0009
	28	-0.7781	-1.9155	-2.6033	-0.0024	-0.0002	-0.0008
1055	20	-0.0941	-1.0854	-1.1859	-0.0010	0.0001	-0.0004
	21	-0.1532	-1.1056	-1.3605	-0.0011	0.0000	-0.0005
	22	-0.4953	-1.5499	-1.7315	-0.0015	-0.0005	-0.0006
	23	2.1847	-1.1431	3.6870	0.0011	0.0025	-0.0003
	24	-2.3371	-1.4448	-6.1945	-0.0033	-0.0023	-0.0006
	25	1.1681	-1.1327	-1.2992	-0.0010	-0.0003	-0.0002

STAAD SPACE

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JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	26	-1.3564	-1.0381	-1.0725	-0.0010	0.0005	-0.0007
	27	1.0638	-1.1926	0.8955	0.0000	-0.0002	-0.0002
	28	-1.2521	-0.9782	-3.2672	-0.0019	0.0004	-0.0006
1056	20	-0.0923	-1.4090	0.1879	0.0001	0.0000	-0.0005
	21	-0.1505	-1.4994	0.1092	0.0001	0.0000	-0.0005
	22	-0.4909	-2.0393	5.7453	0.0038	-0.0010	-0.0005



	23	2.1848	-1.0545	5.3623	0.0036	0.0012	-0.0002
	24	-2.3320	-1.9232	-4.9708	-0.0033	-0.0012	-0.0006
	25	1.1709	-1.4952	1.0436	0.0007	0.0000	-0.0002
	26	-1.3556	-1.3229	-0.6677	-0.0004	0.0000	-0.0007
	27	1.0666	-1.7266	3.9598	0.0024	0.0002	-0.0002
	28	-1.2513	-1.0914	-3.5839	-0.0022	-0.0002	-0.0007
1174	20	-0.0611	-0.7657	-1.1854	-0.0010	0.0001	0.0003
	21	-0.1242	-0.7546	-1.3601	-0.0011	0.0001	0.0004
	22	-0.5914	-1.0978	-1.7313	-0.0014	-0.0004	0.0007
	23	1.9600	-0.2077	3.6869	0.0012	0.0017	0.0009
	24	-2.0167	-1.6862	-6.1936	-0.0034	-0.0014	0.0002
	25	1.0567	-0.8187	-1.2985	-0.0010	-0.0004	0.0001
	26	-1.1788	-0.7126	-1.0723	-0.0010	0.0006	0.0005
	27	0.9917	-1.1450	0.8959	0.0000	-0.0002	0.0001
	28	-1.1138	-0.3863	-3.2667	-0.0019	0.0005	0.0004
1176	20	-0.0708	-0.9071	-1.2467	-0.0011	0.0002	0.0000
	21	-0.1471	-0.9417	-1.4368	-0.0012	0.0003	0.0001
	22	-0.6147	-1.2661	-2.4828	-0.0018	-0.0006	-0.0002
	23	1.9712	-0.3190	4.3240	0.0012	0.0018	0.0002
	24	-2.0463	-1.9326	-6.9516	-0.0036	-0.0015	-0.0004
	25	1.0801	-0.9000	-1.3389	-0.0012	0.0001	-0.0001
	26	-1.2217	-0.9142	-1.1545	-0.0010	0.0004	0.0001
	27	1.0061	-1.2463	1.1391	-0.0002	0.0001	-0.0001
	28	-1.1477	-0.5679	-3.6325	-0.0020	0.0004	0.0001
1302	20	-0.2094	-1.4329	0.1792	0.0076	-0.0002	-0.0009
	21	-0.2675	-1.5946	0.1127	0.0105	-0.0002	-0.0011
	22	0.1653	-1.1128	5.4997	0.0096	-0.0025	-0.0007
	23	2.5498	-0.9855	4.8800	0.0088	0.0009	-0.0007
	24	-2.9061	-2.0225	-4.5067	0.0064	-0.0012	-0.0010
	25	0.9390	-1.3872	1.0003	0.0080	-0.0004	-0.0009
	26	-1.3578	-1.4786	-0.6420	0.0071	0.0001	-0.0009
	27	0.6784	-1.0881	3.6626	0.0087	-0.0005	-0.0009
	28	-1.0971	-1.7777	-3.3042	0.0065	0.0002	-0.0009
1303	20	-0.1269	-1.5822	0.1789	-0.0081	-0.0001	-0.0012
	21	-0.1769	-1.7029	0.1124	-0.0113	-0.0001	-0.0014
	22	-0.4946	-2.2279	5.5049	-0.0081	-0.0029	-0.0013
	23	2.1241	-1.2132	4.8795	-0.0069	0.0010	-0.0010
	24	-2.3160	-2.1008	-4.5067	-0.0093	-0.0011	-0.0013

25	1.1149	-1.6649	1.0025	-0.0082	-0.0004	-0.0012
26	-1.3686	-1.4996	-0.6447	-0.0080	0.0003	-0.0013
27	1.0051	-1.9016	3.6643	-0.0076	-0.0005	-0.0012
28	-1.2588	-1.2629	-3.3065	-0.0087	0.0004	-0.0012

STAAD SPACE

-- PAGE NO. 314

JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
1304	20	-0.1435	-1.3876	0.1467	0.0006	0.0000	-0.0007
	21	-0.2025	-1.5276	0.0606	0.0006	0.0000	-0.0008
	22	0.2240	-1.0606	6.1239	0.0018	-0.0010	-0.0005
	23	2.6302	-0.9408	5.5310	0.0018	0.0014	-0.0005
	24	-2.8651	-1.9771	-5.2203	-0.0006	-0.0014	-0.0008
	25	0.9996	-1.3393	1.0268	0.0008	0.0001	-0.0007
	26	-1.2866	-1.4360	-0.7333	0.0004	-0.0002	-0.0008
	27	0.7551	-1.0400	4.0529	0.0014	0.0004	-0.0007
28	-1.0421	-1.7353	-3.7594	-0.0002	-0.0005	-0.0007	
1305	20	-0.0524	-1.5154	0.1489	-0.0003	0.0003	-0.0009
	21	-0.1026	-1.6146	0.0627	-0.0003	0.0003	-0.0010
	22	-0.4348	-2.1441	6.1259	0.0009	-0.0006	-0.0009
	23	2.2135	-1.1477	5.5331	0.0009	0.0018	-0.0008
	24	-2.2738	-2.0332	-5.2181	-0.0015	-0.0012	-0.0010
	25	1.1779	-1.5967	1.0293	-0.0001	0.0005	-0.0009
	26	-1.2827	-1.4341	-0.7315	-0.0005	0.0001	-0.0010
	27	1.0773	-1.8336	4.0553	0.0005	0.0008	-0.0009
28	-1.1821	-1.1972	-3.7576	-0.0011	-0.0002	-0.0009	
1307	20	-0.1364	-1.4167	0.1567	-0.0004	0.0000	0.0006
	21	-0.2206	-1.5075	0.0599	-0.0005	0.0000	0.0008
	22	-0.4664	-2.0484	6.9391	0.0021	-0.0010	0.0004
	23	2.5891	-1.0607	6.5480	0.0022	0.0015	0.0026
	24	-2.7345	-1.9323	-6.2147	-0.0031	-0.0015	-0.0018

	25	0.9454	-1.5029	1.1959	0.0000	0.0000	0.0011
	26	-1.2183	-1.3304	-0.8825	-0.0008	0.0000	0.0000
	27	0.8811	-1.7350	4.7238	0.0013	0.0002	0.0010
	28	-1.1540	-1.0984	-4.4104	-0.0021	-0.0002	0.0002
1308	20	-0.1845	-1.3100	0.1593	0.0003	0.0001	0.0003
	21	-0.2796	-1.4414	0.0625	0.0002	0.0001	0.0005
	22	0.2634	-1.0054	6.9416	0.0029	-0.0008	0.0000
	23	2.9961	-0.8627	6.5495	0.0030	0.0015	0.0021
	24	-3.2653	-1.9074	-6.2109	-0.0024	-0.0012	-0.0017
	25	0.8923	-1.2645	1.1985	0.0007	0.0001	0.0004
	26	-1.2614	-1.3555	-0.8798	-0.0001	0.0001	0.0002
	27	0.6330	-0.9595	4.7263	0.0020	0.0004	0.0005
	28	-1.0020	-1.6605	-4.4077	-0.0015	-0.0002	0.0001
1313	20	-0.0927	-1.3548	-0.0865	-0.0001	-0.0054	-0.0011
	21	-0.1510	-1.4322	-0.1829	-0.0001	-0.0057	-0.0014
	22	-0.4918	-1.9758	4.3294	0.0028	-0.0282	-0.0015
	23	2.1847	-1.0953	4.7057	0.0029	0.0120	0.0008
	24	-2.3329	-1.8431	-4.8913	-0.0032	-0.0233	-0.0018
	25	1.1704	-1.4337	0.5854	0.0004	-0.0090	-0.0013
	26	-1.3558	-1.2758	-0.7585	-0.0005	-0.0017	-0.0010
	27	1.0661	-1.6334	3.3748	0.0020	-0.0116	-0.0019
	28	-1.2515	-1.0761	-3.5478	-0.0021	0.0008	-0.0003
1317	20	-0.1518	-1.0793	0.1123	0.0007	0.0000	-0.0007
	21	-0.2486	-1.1921	0.3786	0.0009	-0.0001	-0.0006
	22	0.0257	-1.1347	0.1270	0.0010	-0.0012	-0.0007

STAAD SPACE

-- PAGE NO. 315

JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	23	2.7363	-0.9175	5.3674	0.0013	0.0021	-0.0002
	24	-2.9566	-1.2539	-5.1083	0.0001	-0.0021	-0.0014

	25	1.1599	-1.0496	0.1407	0.0007	0.0005	-0.0010
	26	-1.4634	-1.1089	0.0840	0.0007	-0.0006	-0.0004
	27	0.7757	-1.0346	2.9226	0.0010	0.0004	-0.0010
	28	-1.0792	-1.1239	-2.6980	0.0004	-0.0004	-0.0004
1318	20	-0.1518	-1.9456	-0.0745	0.0012	0.0000	-0.0001
	21	-0.2486	-2.2841	-0.0918	0.0012	-0.0001	-0.0001
	22	0.0257	-2.0875	-0.9077	0.0018	-0.0012	-0.0001
	23	2.7369	-1.7087	9.1457	0.0027	0.0021	0.0007
	24	-2.9572	-2.3637	-9.3179	0.0001	-0.0021	-0.0007
	25	1.1598	-1.9510	-0.0041	0.0012	0.0006	0.0003
	26	-1.4634	-1.9403	-0.1449	0.0012	-0.0006	-0.0004
	27	0.7763	-1.9034	2.9497	0.0017	0.0003	0.0003
	28	-1.0799	-1.9879	-3.0986	0.0007	-0.0003	-0.0004
1319	20	-0.1547	-1.1577	0.1114	-0.0003	0.0000	-0.0011
	21	-0.2759	-1.3290	0.3777	-0.0001	-0.0001	-0.0009
	22	-0.3364	-1.3088	0.1261	0.0000	-0.0012	-0.0010
	23	2.5902	-0.9844	5.3670	0.0003	0.0025	-0.0007
	24	-2.8147	-1.3521	-5.1096	-0.0009	-0.0025	-0.0020
	25	1.1891	-1.1300	0.1398	-0.0003	-0.0003	-0.0014
	26	-1.4985	-1.1854	0.0831	-0.0003	0.0003	-0.0008
	27	0.8828	-1.1679	2.9217	0.0000	0.0004	-0.0015
	28	-1.1922	-1.1474	-2.6988	-0.0006	-0.0004	-0.0008
1320	20	-0.1547	-2.0136	-0.0785	-0.0008	0.0000	0.0003
	21	-0.2759	-2.3724	-0.0985	-0.0007	-0.0001	0.0003
	22	-0.3364	-2.2183	-0.9107	-0.0011	-0.0012	0.0005
	23	2.5906	-1.7852	9.1419	0.0002	0.0025	0.0011
	24	-2.8152	-2.4424	-9.3227	-0.0023	-0.0025	-0.0002
	25	1.1891	-2.0244	-0.0081	-0.0008	-0.0003	0.0007
	26	-1.4985	-2.0027	-0.1488	-0.0009	0.0003	-0.0001
	27	0.8822	-2.0757	2.9455	-0.0004	0.0003	0.0006
	28	-1.1916	-1.9515	-3.1024	-0.0013	-0.0003	-0.0001
1329	20	-0.1360	-1.7560	-0.0791	-0.0005	0.0000	0.0002
	21	-0.2199	-1.8495	-0.1889	-0.0005	0.0000	0.0003
	22	-0.4676	-2.2951	7.5537	0.0015	-0.0010	-0.0002
	23	2.5872	-1.3416	5.7263	0.0016	0.0015	0.0022
	24	-2.7326	-2.2422	-5.8588	-0.0026	-0.0014	-0.0022
	25	0.9461	-1.7807	0.7496	-0.0003	0.0000	0.0007
	26	-1.2182	-1.7314	-0.9078	-0.0007	0.0000	-0.0004

	27	0.8813	-2.0485	3.9064	0.0007	0.0003	0.0005
	28	-1.1534	-1.4636	-4.0646	-0.0017	-0.0002	-0.0002
1334	20	-0.0607	-1.5773	-0.7480	0.0007	0.0002	-0.0002
	21	-0.0714	-1.6832	-0.9771	0.0006	0.0003	-0.0003
	22	0.4308	-2.1049	-2.1988	0.0012	-0.0005	-0.0004
	23	2.6884	-0.8936	7.0618	0.0027	0.0021	0.0001
	24	-2.6913	-2.5987	-8.1969	-0.0004	-0.0015	-0.0006
	25	0.9925	-1.7842	-0.7939	0.0006	0.0004	-0.0001

STAAD SPACE

-- PAGE NO. 316

JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	26	-1.1139	-1.3704	-0.7021	0.0008	0.0001	-0.0003
	27	0.6975	-1.4583	2.4475	0.0013	0.0007	0.0000
	28	-0.8189	-1.6963	-3.9435	0.0001	-0.0002	-0.0004
1335	20	-0.0613	-1.6985	-1.1756	0.0003	0.0004	-0.0001
	21	-0.0721	-1.8807	-1.4431	0.0002	0.0004	-0.0002
	22	0.4302	-2.3516	-2.1700	0.0009	-0.0003	-0.0003
	23	2.6881	-1.0238	5.5538	0.0022	0.0024	0.0002
	24	-2.6924	-2.7690	-7.6313	-0.0008	-0.0014	-0.0006
	25	0.9919	-1.8258	-1.3249	0.0002	0.0006	0.0000
	26	-1.1146	-1.5712	-1.0263	0.0004	0.0003	-0.0002
	27	0.6968	-1.4597	1.5886	0.0008	0.0010	0.0001
	28	-0.8194	-1.9373	-3.9398	-0.0002	-0.0001	-0.0003
1336	20	0.0255	-1.7942	-1.2383	-0.0003	0.0001	0.0003
	21	-0.0430	-1.9250	-1.4264	-0.0004	0.0002	0.0002
	22	0.0557	-2.7161	-2.4713	-0.0003	-0.0004	0.0002
	23	3.4389	-1.6492	4.3274	0.0008	0.0017	0.0010
	24	-3.3024	-2.6869	-6.9363	-0.0011	-0.0014	-0.0004
	25	1.2123	-1.8868	-1.3294	-0.0004	0.0002	0.0004
	26	-1.1612	-1.7016	-1.1472	-0.0001	0.0001	0.0002

	27	1.1242	-1.6808	1.1463	0.0000	0.0005	0.0006
	28	-1.0731	-1.9077	-3.6230	-0.0005	-0.0002	0.0000
1337	20	-0.0453	-1.7990	-1.1753	0.0002	0.0004	0.0002
	21	-0.0599	-1.9300	-1.4428	0.0001	0.0004	0.0002
	22	0.1658	-2.7196	-2.1691	0.0008	-0.0003	0.0000
	23	3.0881	-1.6580	5.5544	0.0019	0.0023	0.0010
	24	-3.0189	-2.6863	-7.6307	-0.0006	-0.0014	-0.0006
	25	1.0627	-1.8920	-1.3246	0.0001	0.0006	0.0004
	26	-1.1533	-1.7061	-1.0260	0.0003	0.0003	0.0001
	27	0.8613	-1.6883	1.5888	0.0007	0.0010	0.0006
	28	-0.9519	-1.9098	-3.9395	-0.0002	-0.0001	-0.0001
1338	20	0.0214	-1.8826	-1.0513	0.0007	0.0007	0.0005
	21	-0.0476	-1.9568	-1.2072	0.0006	0.0008	0.0004
	22	0.0454	-2.6083	-2.6874	0.0007	0.0019	0.0005
	23	3.4258	-1.6814	5.7131	0.0024	0.0049	0.0012
	24	-3.2971	-2.7509	-7.9278	-0.0007	-0.0033	-0.0001
	25	1.2070	-2.0602	-1.0463	0.0005	0.0008	0.0002
	26	-1.1641	-1.7050	-1.0563	0.0008	0.0007	0.0007
	27	1.1177	-1.9815	1.7406	0.0013	0.0013	0.0004
	28	-1.0748	-1.7837	-3.8432	0.0000	0.0001	0.0005
1339	20	-0.0456	-1.8833	-0.7483	0.0006	0.0002	0.0001
	21	-0.0603	-1.9573	-0.9775	0.0005	0.0003	0.0000
	22	0.1655	-2.6083	-2.1991	0.0011	-0.0005	-0.0002
	23	3.0880	-1.6785	7.0615	0.0026	0.0021	0.0008
	24	-3.0196	-2.7549	-8.1971	-0.0005	-0.0015	-0.0007
	25	1.0623	-2.0614	-0.7942	0.0006	0.0004	0.0002
	26	-1.1536	-1.7052	-0.7024	0.0007	0.0001	0.0000
	27	0.8608	-1.9827	2.4472	0.0012	0.0007	0.0004
	28	-0.9520	-1.7839	-3.9438	0.0001	-0.0002	-0.0002

STAAD SPACE

-- PAGE NO. 317

JOINT DISPLACEMENT (CM      RADIANS)      STRUCTURE TYPE = SPACE

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JOINT    LOAD    X-TRANS    Y-TRANS    Z-TRANS    X-ROTAN    Y-ROTAN    Z-ROTAN

1340	20	-0.0604	-1.6282	-0.8444	0.0006	0.0006	-0.0002
	21	-0.0711	-1.7667	-1.0811	0.0005	0.0006	-0.0003
	22	0.4312	-2.2045	-2.1027	0.0013	-0.0001	-0.0004
	23	2.6882	-0.9490	6.5890	0.0026	0.0025	0.0002
	24	-2.6906	-2.6658	-7.9420	-0.0005	-0.0013	-0.0007
	25	0.9928	-1.8044	-0.9302	0.0006	0.0007	-0.0001
	26	-1.1136	-1.4521	-0.7586	0.0007	0.0004	-0.0003
	27	0.6980	-1.4606	2.2085	0.0012	0.0012	0.0000
	28	-0.8187	-1.7959	-3.8973	0.0001	-0.0001	-0.0004
1341	20	-0.0460	-1.8520	-0.8448	0.0005	0.0006	0.0001
	21	-0.0607	-1.9499	-1.0815	0.0004	0.0006	0.0000
	22	0.1651	-2.6592	-2.1030	0.0011	-0.0001	-0.0002
	23	3.0878	-1.7378	6.5887	0.0024	0.0025	0.0008
	24	-3.0201	-2.6646	-7.9424	-0.0006	-0.0013	-0.0007
	25	1.0620	-1.9972	-0.9306	0.0004	0.0007	0.0002
	26	-1.1539	-1.7067	-0.7589	0.0006	0.0004	0.0000
	27	0.8603	-1.8675	2.2081	0.0010	0.0012	0.0004
	28	-0.9523	-1.8364	-3.8977	0.0000	-0.0001	-0.0002
1342	20	-0.1054	-1.6979	-1.2468	0.0002	0.0005	-0.0002
	21	-0.1465	-1.8802	-1.4820	0.0001	0.0005	-0.0003
	22	0.3358	-2.3509	-2.3808	0.0006	-0.0003	-0.0004
	23	2.6714	-1.0236	4.9974	0.0021	0.0024	0.0002
	24	-2.7874	-2.7680	-7.4236	-0.0010	-0.0013	-0.0007
	25	0.9796	-1.8252	-1.3724	0.0001	0.0006	-0.0001
	26	-1.1904	-1.5707	-1.1212	0.0003	0.0003	-0.0003
	27	0.7022	-1.4593	1.3696	0.0008	0.0011	0.0000
	28	-0.9130	-1.9366	-3.8632	-0.0004	-0.0002	-0.0003
1343	20	-0.0148	-1.7978	-1.2462	0.0002	0.0005	0.0001
	21	-0.0528	-1.9287	-1.4813	0.0001	0.0005	0.0001
	22	0.1174	-2.7187	-2.3809	0.0008	-0.0003	-0.0002
	23	3.2342	-1.6562	4.9959	0.0022	0.0024	0.0009
	24	-3.1363	-2.6860	-7.4216	-0.0010	-0.0013	-0.0008
	25	1.1275	-1.8908	-1.3718	0.0001	0.0006	0.0003
	26	-1.1571	-1.7049	-1.1206	0.0003	0.0003	0.0000
	27	0.9748	-1.6867	1.3698	0.0008	0.0011	0.0005
	28	-1.0045	-1.9090	-3.8621	-0.0004	-0.0002	-0.0002

1344	20	-0.1059	-1.5761	-0.8967	0.0006	0.0003	-0.0003
	21	-0.1470	-1.6821	-1.0959	0.0005	0.0003	-0.0004
	22	0.3365	-2.1037	-2.4619	0.0010	-0.0004	-0.0005
	23	2.6663	-0.8923	6.4383	0.0026	0.0022	0.0001
	24	-2.7820	-2.5978	-8.0979	-0.0005	-0.0016	-0.0007
	25	0.9788	-1.7830	-0.9227	0.0005	0.0004	-0.0001
	26	-1.1905	-1.3692	-0.8707	0.0007	0.0001	-0.0004
	27	0.7020	-1.4571	2.1448	0.0012	0.0007	-0.0001
	28	-0.9137	-1.6952	-3.9382	0.0000	-0.0002	-0.0004
1345	20	-0.1044	-1.6263	-0.9891	0.0005	0.0004	-0.0001
	21	-0.1452	-1.7650	-1.1984	0.0004	0.0005	-0.0003
	22	0.3377	-2.2018	-2.4161	0.0012	-0.0002	-0.0003

STAAD SPACE

-- PAGE NO. 318

JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	23	2.6676	-0.9457	5.8993	0.0027	0.0023	0.0002
	24	-2.7807	-2.6644	-7.7576	-0.0008	-0.0014	-0.0006
	25	0.9806	-1.8031	-1.0503	0.0004	0.0006	0.0000
	26	-1.1893	-1.4495	-0.9278	0.0006	0.0003	-0.0003
	27	0.7040	-1.4592	1.9149	0.0011	0.0010	0.0000
	28	-0.9127	-1.7934	-3.8930	-0.0001	-0.0002	-0.0003
1346	20	-0.0170	-1.8829	-0.8960	0.0006	0.0003	0.0001
	21	-0.0551	-1.9569	-1.0951	0.0004	0.0003	0.0000
	22	0.1149	-2.6080	-2.4613	0.0010	-0.0004	-0.0002
	23	3.2298	-1.6786	6.4390	0.0026	0.0022	0.0008
	24	-3.1357	-2.7541	-8.0977	-0.0006	-0.0016	-0.0007
	25	1.1243	-2.0610	-0.9220	0.0005	0.0004	0.0002
	26	-1.1584	-1.7048	-0.8700	0.0006	0.0001	0.0000
	27	0.9709	-1.9823	2.1452	0.0012	0.0007	0.0004
	28	-1.0050	-1.7835	-3.9372	-0.0001	-0.0002	-0.0002



1347	20	-0.0172	-1.8504	-0.9887	0.0005	0.0004	0.0001
	21	-0.0553	-1.9483	-1.1980	0.0004	0.0005	0.0000
	22	0.1148	-2.6577	-2.4157	0.0012	-0.0002	-0.0002
	23	3.2310	-1.7357	5.8996	0.0027	0.0023	0.0008
	24	-3.1374	-2.6633	-7.7573	-0.0008	-0.0014	-0.0007
	25	1.1245	-1.9962	-1.0499	0.0005	0.0006	0.0002
	26	-1.1588	-1.7045	-0.9275	0.0006	0.0003	0.0000
	27	0.9709	-1.8667	1.9152	0.0011	0.0010	0.0004
	28	-1.0053	-1.8341	-3.8926	0.0000	-0.0002	-0.0002
1352	20	-0.0611	-1.6540	-0.9012	0.0006	0.0009	-0.0002
	21	-0.0719	-1.8041	-1.1430	0.0004	0.0010	-0.0004
	22	0.4304	-2.2497	-2.0904	0.0012	0.0002	-0.0004
	23	2.6882	-0.9775	6.3781	0.0024	0.0031	0.0001
	24	-2.6921	-2.6981	-7.8559	-0.0004	-0.0011	-0.0007
	25	0.9921	-1.8180	-1.0023	0.0005	0.0011	-0.0001
	26	-1.1143	-1.4899	-0.8002	0.0006	0.0007	-0.0004
	27	0.6970	-1.4683	2.0901	0.0010	0.0017	-0.0001
	28	-0.8191	-1.8396	-3.8926	0.0001	0.0001	-0.0004
1353	20	-0.0453	-1.8475	-0.9013	0.0004	0.0009	0.0001
	21	-0.0599	-1.9541	-1.1431	0.0003	0.0010	0.0000
	22	0.1657	-2.6845	-2.0904	0.0010	0.0002	-0.0002
	23	3.0880	-1.7453	6.3780	0.0022	0.0031	0.0008
	24	-3.0189	-2.6605	-7.8559	-0.0005	-0.0011	-0.0008
	25	1.0627	-1.9797	-1.0023	0.0003	0.0011	0.0002
	26	-1.1533	-1.7154	-0.8003	0.0005	0.0007	-0.0001
	27	0.8613	-1.8310	2.0900	0.0009	0.0017	0.0004
	28	-0.9519	-1.8640	-3.8926	0.0000	0.0001	-0.0003
1354	20	-0.0615	-1.6891	-1.1328	0.0004	0.0008	-0.0001
	21	-0.0724	-1.8603	-1.3971	0.0003	0.0009	-0.0002
	22	0.4300	-2.3222	-2.1982	0.0010	0.0001	-0.0003
	23	2.6882	-1.0162	5.7361	0.0022	0.0029	0.0002
	24	-2.6930	-2.7481	-7.7137	-0.0005	-0.0012	-0.0006
	25	0.9917	-1.8295	-1.2666	0.0003	0.0010	0.0000

JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	26	-1.1148	-1.5486	-0.9989	0.0005	0.0006	-0.0003
	27	0.6964	-1.4693	1.6874	0.0009	0.0015	0.0001
	28	-0.8195	-1.9088	-3.9529	-0.0001	0.0001	-0.0003
1355	20	-0.0450	-1.8240	-1.1325	0.0003	0.0008	0.0002
	21	-0.0596	-1.9464	-1.3969	0.0002	0.0009	0.0001
	22	0.1661	-2.7153	-2.1976	0.0009	0.0001	-0.0001
	23	3.0881	-1.7080	5.7367	0.0020	0.0029	0.0010
	24	-3.0183	-2.6738	-7.7135	-0.0005	-0.0012	-0.0007
	25	1.0630	-1.9308	-1.2664	0.0002	0.0010	0.0003
	26	-1.1530	-1.7172	-0.9987	0.0004	0.0006	0.0001
	27	0.8618	-1.7465	1.6875	0.0007	0.0015	0.0005
	28	-0.9517	-1.9016	-3.9526	-0.0001	0.0001	-0.0001
1356	20	-0.0203	-1.7321	-1.1751	0.0003	0.0001	0.0000
	21	-0.0281	-1.9014	-1.4425	0.0002	0.0000	-0.0001
	22	0.3999	-2.4499	-2.1692	0.0010	-0.0007	-0.0003
	23	2.7862	-1.2150	5.5550	0.0018	0.0019	0.0004
	24	-2.6945	-2.7325	-7.6312	-0.0003	-0.0017	-0.0006
	25	1.0483	-1.8505	-1.3243	0.0002	0.0002	0.0001
	26	-1.0889	-1.6137	-1.0258	0.0004	-0.0001	-0.0002
	27	0.7929	-1.5344	1.5894	0.0007	0.0004	0.0002
	28	-0.8335	-1.9297	-3.9395	0.0000	-0.0003	-0.0003
1357	20	-0.0056	-1.6983	-0.8447	0.0007	0.0001	-0.0001
	21	-0.0114	-1.8263	-1.0815	0.0006	0.0001	-0.0002
	22	0.4168	-2.3351	-2.1030	0.0013	-0.0006	-0.0003
	23	2.7826	-1.1845	6.5886	0.0025	0.0019	0.0003
	24	-2.6644	-2.6577	-7.9424	-0.0002	-0.0016	-0.0007
	25	1.0628	-1.8662	-0.9306	0.0006	0.0002	0.0000
	26	-1.0739	-1.5304	-0.7589	0.0008	-0.0001	-0.0003
	27	0.8135	-1.5794	2.2080	0.0011	0.0005	0.0001
	28	-0.8246	-1.8172	-3.8975	0.0002	-0.0003	-0.0003
1358	20	-0.0863	-1.7799	-1.1749	0.0002	0.0001	0.0001
	21	-0.1039	-1.9240	-1.4424	0.0001	0.0000	0.0000

	22	0.1959	-2.6329	-2.1688	0.0009	-0.0007	-0.0001
	23	2.9747	-1.5036	5.5553	0.0017	0.0019	0.0008
	24	-3.0016	-2.7132	-7.6309	-0.0003	-0.0016	-0.0006
	25	1.0063	-1.8818	-1.3242	0.0001	0.0002	0.0003
	26	-1.1789	-1.6780	-1.0256	0.0003	-0.0001	0.0000
	27	0.7652	-1.6347	1.5895	0.0005	0.0004	0.0004
	28	-0.9379	-1.9251	-3.9393	-0.0001	-0.0003	-0.0001
1359	20	-0.1007	-1.8070	-0.8449	0.0005	0.0001	0.0000
	21	-0.1203	-1.9154	-1.0817	0.0004	0.0001	-0.0001
	22	0.1795	-2.5537	-2.1032	0.0011	-0.0006	-0.0003
	23	2.9711	-1.5624	6.5885	0.0023	0.0019	0.0006
	24	-3.0238	-2.6629	-7.9426	-0.0004	-0.0016	-0.0007
	25	0.9921	-1.9604	-0.9308	0.0004	0.0002	0.0001
	26	-1.1935	-1.6535	-0.7591	0.0006	-0.0001	-0.0001
	27	0.7449	-1.7738	2.2079	0.0010	0.0005	0.0003
	28	-0.9463	-1.8401	-3.8977	0.0000	-0.0003	-0.0003

STAAD SPACE

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JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
1360	20	-0.0608	-1.6018	-0.7740	0.0007	0.0003	-0.0002
	21	-0.0716	-1.7218	-1.0047	0.0006	0.0004	-0.0003
	22	0.4306	-2.1498	-2.1414	0.0012	-0.0003	-0.0004
	23	2.6884	-0.9202	6.8885	0.0026	0.0022	0.0002
	24	-2.6916	-2.6296	-8.0851	-0.0004	-0.0014	-0.0006
	25	0.9924	-1.7958	-0.8366	0.0006	0.0005	-0.0001
	26	-1.1140	-1.4078	-0.7114	0.0008	0.0002	-0.0003
	27	0.6973	-1.4621	2.3672	0.0012	0.0009	0.0000
	28	-0.8190	-1.7416	-3.9152	0.0002	-0.0002	-0.0004
1361	20	-0.0455	-1.8728	-0.7742	0.0006	0.0003	0.0001
	21	-0.0601	-1.9571	-1.0049	0.0005	0.0004	0.0000

	22	0.1656	-2.6331	-2.1416	0.0011	-0.0003	-0.0002
	23	3.0880	-1.7153	6.8883	0.0025	0.0022	0.0008
	24	-3.0193	-2.7105	-8.0853	-0.0005	-0.0014	-0.0007
	25	1.0625	-2.0371	-0.8368	0.0005	0.0005	0.0002
	26	-1.1534	-1.7085	-0.7116	0.0007	0.0002	0.0000
	27	0.8610	-1.9367	2.3671	0.0011	0.0009	0.0004
	28	-0.9520	-1.8089	-3.9154	0.0001	-0.0002	-0.0002
1362	20	-0.0608	-1.6087	-0.7902	0.0007	0.0004	-0.0002
	21	-0.0716	-1.7332	-1.0223	0.0006	0.0005	-0.0003
	22	0.4307	-2.1634	-2.1306	0.0013	-0.0003	-0.0003
	23	2.6883	-0.9275	6.8160	0.0026	0.0023	0.0002
	24	-2.6915	-2.6388	-8.0481	-0.0004	-0.0014	-0.0006
	25	0.9924	-1.7983	-0.8584	0.0006	0.0006	-0.0001
	26	-1.1140	-1.4190	-0.7219	0.0008	0.0003	-0.0003
	27	0.6974	-1.4620	2.3298	0.0012	0.0010	0.0000
	28	-0.8189	-1.7553	-3.9101	0.0002	-0.0001	-0.0003
1363	20	-0.0455	-1.8679	-0.7904	0.0006	0.0004	0.0001
	21	-0.0602	-1.9556	-1.0225	0.0005	0.0005	0.0000
	22	0.1656	-2.6399	-2.1309	0.0011	-0.0003	-0.0002
	23	3.0880	-1.7235	6.8158	0.0025	0.0023	0.0008
	24	-3.0194	-2.6970	-8.0483	-0.0005	-0.0014	-0.0007
	25	1.0624	-2.0277	-0.8586	0.0005	0.0006	0.0002
	26	-1.1535	-1.7082	-0.7221	0.0007	0.0003	0.0000
	27	0.8609	-1.9200	2.3296	0.0011	0.0010	0.0004
	28	-0.9520	-1.8159	-3.9103	0.0001	-0.0001	-0.0002
1364	20	-0.0360	-1.6739	-0.7484	0.0009	0.0000	-0.0001
	21	-0.0453	-1.7695	-0.9776	0.0008	-0.0001	-0.0002
	22	0.3720	-2.2612	-2.1993	0.0014	-0.0008	-0.0003
	23	2.7975	-1.1725	7.0611	0.0029	0.0018	0.0003
	24	-2.7413	-2.6145	-8.1969	-0.0003	-0.0018	-0.0006
	25	1.0339	-1.8720	-0.7943	0.0008	0.0001	0.0000
	26	-1.1058	-1.4757	-0.7025	0.0009	-0.0002	-0.0002
	27	0.7760	-1.6220	2.4470	0.0014	0.0003	0.0001
	28	-0.8480	-1.7257	-3.9438	0.0003	-0.0004	-0.0003
1365	20	-0.0068	-1.7105	-0.8447	0.0007	-0.0002	-0.0001
	21	-0.0130	-1.8366	-1.0815	0.0006	-0.0002	-0.0002
	22	0.4024	-2.3581	-2.1030	0.0013	-0.0010	-0.0003

JOINT DISPLACEMENT (CM RADIANS) STRUCTURE TYPE = SPACE

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JOINT	LOAD	X-TRANS	Y-TRANS	Z-TRANS	X-ROTAN	Y-ROTAN	Z-ROTAN
	23	2.8003	-1.2245	6.5887	0.0024	0.0017	0.0003
	24	-2.6828	-2.6581	-7.9424	-0.0002	-0.0020	-0.0007
	25	1.0641	-1.8770	-0.9306	0.0006	-0.0001	0.0000
	26	-1.0776	-1.5440	-0.7589	0.0007	-0.0003	-0.0002
	27	0.8183	-1.6000	2.2081	0.0011	0.0001	0.0001
	28	-0.8319	-1.8210	-3.8975	0.0002	-0.0005	-0.0003
1368	20	-0.0704	-1.7997	-0.7485	0.0008	0.0000	0.0000
	21	-0.0865	-1.8838	-0.9777	0.0007	-0.0001	-0.0001
	22	0.2242	-2.4654	-2.1994	0.0013	-0.0008	-0.0003
	23	2.9531	-1.4986	7.0610	0.0028	0.0018	0.0006
	24	-2.9441	-2.6670	-8.1970	-0.0004	-0.0018	-0.0007
	25	1.0209	-1.9867	-0.7944	0.0007	0.0001	0.0001
	26	-1.1617	-1.6127	-0.7026	0.0009	-0.0002	-0.0001
	27	0.7822	-1.8326	2.4469	0.0013	0.0003	0.0002
	28	-0.9231	-1.7667	-3.9439	0.0002	-0.0004	-0.0003
1369	20	-0.0995	-1.7981	-0.8449	0.0005	-0.0002	0.0000
	21	-0.1186	-1.9084	-1.0816	0.0004	-0.0002	-0.0001
	22	0.1940	-2.5341	-2.1031	0.0011	-0.0010	-0.0003
	23	2.9517	-1.5287	6.5886	0.0023	0.0017	0.0006
	24	-3.0036	-2.6629	-7.9426	-0.0004	-0.0020	-0.0007
	25	0.9908	-1.9530	-0.9307	0.0004	-0.0001	0.0001
	26	-1.1898	-1.6432	-0.7590	0.0006	-0.0003	-0.0001
	27	0.7400	-1.7566	2.2079	0.0010	0.0001	0.0003
	28	-0.9390	-1.8397	-3.8977	0.0000	-0.0005	-0.0003
1380	20	-0.1506	-1.2761	0.3976	0.0003	0.0017	0.0002
	21	-0.1539	-1.3408	0.3868	0.0003	0.0018	0.0002
	22	0.2700	-1.2871	0.3696	0.0004	-0.0001	0.0004
	23	2.2259	-0.9798	2.0703	0.0016	0.0067	0.0014

	24	-2.4711	-1.6166	-1.2301	-0.0009	-0.0031	-0.0010
	25	0.7865	-1.1787	0.4309	0.0004	0.0016	0.0006
	26	-1.0878	-1.3735	0.3642	0.0003	0.0018	-0.0003
	27	0.5445	-1.1235	1.7818	0.0012	0.0004	0.0010
	28	-0.8457	-1.4287	-0.9866	-0.0005	0.0030	-0.0006
1381	20	-0.1276	-1.3650	0.3976	0.0004	0.0017	0.0007
	21	-0.1273	-1.4131	0.3868	0.0004	0.0019	0.0008
	22	-0.4895	-1.4465	0.3696	0.0004	-0.0001	0.0003
	23	1.7515	-1.0129	2.0703	0.0017	0.0067	0.0020
	24	-1.9561	-1.7841	-1.2301	-0.0009	-0.0031	-0.0005
	25	0.8283	-1.2687	0.4309	0.0004	0.0016	0.0011
	26	-1.0835	-1.4613	0.3642	0.0003	0.0018	0.0003
	27	0.7103	-1.4576	1.7817	0.0012	0.0004	0.0009
	28	-0.9655	-1.2724	-0.9866	-0.0005	0.0031	0.0005

STAAD SPACE

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\*\*\*\*\* END OF LATEST ANALYSIS RESULT \*\*\*\*\*

2277. LOAD LIST 20 TO 28

2278. PRINT SUPPORT REACTION LIST 1095 TO 1121 1181 1182 1249 1252 1298

STAAD SPACE

-- PAGE NO. 323

SUPPORT REACTIONS -UNIT KN    METE    STRUCTURE TYPE = SPACE

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JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
1095	20	3.40	400.61	-1.84	0.00	0.00	0.00
	21	3.52	488.83	-1.55	0.00	0.00	0.00
	22	3.89	422.72	8.54	0.00	0.01	0.00
	23	14.47	594.22	50.68	0.00	0.01	0.00

	24	-7.60	228.06	-54.18	0.00	-0.02	0.00
	25	-3.87	405.67	-1.92	0.00	0.00	0.00
	26	10.68	395.54	-1.77	0.00	-0.01	0.00
	27	-0.79	463.88	-20.94	0.00	-0.01	0.00
	28	7.59	337.34	17.25	0.00	0.01	0.00
1096	20	5.86	238.45	-3.21	0.00	0.00	0.00
	21	5.78	290.18	-2.75	0.00	0.00	0.00
	22	6.77	288.34	0.18	0.00	0.00	0.00
	23	18.02	349.11	23.17	0.00	0.03	0.00
	24	-6.24	134.22	-29.52	0.00	-0.04	0.00
	25	-2.18	231.92	-3.29	0.00	0.01	0.00
	26	13.90	244.99	-3.13	0.00	-0.02	0.00
	27	1.00	282.29	-19.43	0.00	0.01	0.00
	28	10.72	194.62	13.00	0.00	-0.01	0.00
1097	20	3.45	379.34	4.75	0.00	0.00	0.00
	21	4.54	459.88	5.09	0.00	0.00	0.00
	22	1.93	385.13	15.14	0.00	0.01	0.00
	23	17.43	573.41	57.29	0.00	0.01	0.00
	24	-10.34	204.02	-47.59	0.00	-0.02	0.00
	25	-3.76	380.16	4.64	0.00	-0.01	0.00
	26	10.66	378.52	4.86	0.00	0.00	0.00
	27	-0.57	322.24	-14.42	0.00	-0.01	0.00
	28	7.47	436.44	23.93	0.00	0.01	0.00
1098	20	6.05	216.23	3.84	0.00	0.00	0.00
	21	6.97	230.34	4.31	0.00	0.00	0.00
	22	4.50	213.77	7.24	0.00	0.00	0.00
	23	21.49	313.19	30.23	0.00	0.03	0.00
	24	-9.18	120.60	-22.46	0.00	-0.04	0.00
	25	-1.88	207.87	3.76	0.00	-0.02	0.00
	26	13.99	224.59	3.93	0.00	0.01	0.00
	27	1.45	163.35	-12.45	0.00	0.01	0.00
	28	10.66	269.12	20.14	0.00	-0.02	0.00
1099	20	-7.02	321.49	-9.15	0.00	0.00	0.00
	21	-10.60	337.75	-9.64	0.00	0.00	0.00
	22	-11.39	375.97	-14.73	0.00	0.01	0.00
	23	19.03	551.94	71.43	0.00	0.07	0.00
	24	-35.77	121.21	-90.03	0.00	-0.07	0.00
	25	-22.79	281.35	-9.12	0.00	0.00	0.00

	26	8.76	361.62	-9.18	0.00	0.00	0.00
	27	-20.58	356.95	-35.60	0.00	0.00	0.00
	28	6.55	286.03	17.29	0.00	0.00	0.00
1100	20	2.90	327.80	8.83	0.00	0.00	0.00
	21	0.96	345.77	8.41	0.00	0.00	0.00
	22	6.07	376.58	3.36	0.00	0.01	0.00

STAAD SPACE

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SUPPORT REACTIONS -UNIT KN METE STRUCTURE TYPE = SPACE

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JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
	23	48.63	572.78	89.48	0.00	0.05	0.00
	24	-39.88	119.26	-72.03	0.00	-0.05	0.00
	25	-12.36	286.79	8.91	0.00	0.00	0.00
	26	18.17	368.82	8.75	0.00	0.00	0.00
	27	-7.50	230.98	-17.66	0.00	0.00	0.00
	28	13.31	424.62	35.32	0.00	0.00	0.00
1101	20	-29.83	300.19	0.13	0.00	0.00	0.00
	21	-37.79	321.05	1.61	0.00	0.00	0.00
	22	-7.16	314.62	13.02	0.00	0.01	0.00
	23	85.37	526.97	33.53	0.00	0.05	0.00
	24	-136.36	110.02	-33.34	0.00	-0.05	0.00
	25	-87.94	366.41	0.21	0.00	0.00	0.00
	26	28.28	233.96	0.04	0.00	0.00	0.00
	27	-92.04	423.22	-18.68	0.00	0.00	0.00
	28	32.37	177.15	18.93	0.00	0.00	0.00
1102	20	-0.14	318.76	18.67	0.00	0.00	0.00
	21	-2.67	343.71	20.19	0.00	0.00	0.00
	22	1.81	416.51	31.71	0.00	0.01	0.00
	23	129.55	584.98	52.02	0.00	0.03	0.00
	24	-122.04	98.29	-14.58	0.00	-0.03	0.00
	25	-49.38	383.75	18.81	0.00	0.00	0.00



	26	49.11	253.77	18.53	0.00	0.00	0.00
	27	-36.41	299.23	-0.13	0.00	0.00	0.00
	28	36.14	338.29	37.46	0.00	0.00	0.00
1103	20	-2.54	291.29	-14.86	0.00	0.21	0.00
	21	-3.89	308.97	-15.88	0.00	0.18	0.00
	22	4.27	413.95	-3.29	0.00	0.53	0.00
	23	24.65	385.84	48.57	0.00	1.37	0.00
	24	-25.07	308.48	-78.14	0.00	-0.61	0.00
	25	-15.16	301.27	-18.22	0.00	-0.19	0.00
	26	10.08	281.31	-11.50	0.00	0.61	0.00
	27	-16.50	310.51	-48.74	0.00	0.22	0.00
	28	11.42	272.07	19.02	0.00	0.19	0.00
1104	20	1.04	395.39	13.06	0.00	0.00	0.00
	21	0.67	436.53	12.89	0.00	0.00	0.00
	22	1.50	547.40	24.37	0.00	0.02	0.00
	23	12.22	648.94	58.35	0.00	0.03	0.00
	24	-9.37	231.89	-28.51	0.00	-0.03	0.00
	25	-3.26	426.38	12.27	0.00	0.00	0.00
	26	5.35	364.40	13.84	0.00	0.00	0.00
	27	-2.13	338.81	-9.21	0.00	0.00	0.00
	28	4.22	451.97	35.32	0.00	0.01	0.00
1105	20	0.37	258.52	-11.50	0.00	-0.05	0.00
	21	1.21	263.55	-12.15	0.00	-0.04	0.00
	22	1.62	365.61	-8.70	0.00	-0.10	0.00
	23	7.38	341.10	43.84	0.00	0.14	0.00
	24	-10.75	268.67	-67.26	0.00	-0.30	0.00
	25	-2.54	269.61	-8.27	0.00	0.03	0.00

STAAD SPACE

-- PAGE NO. 325

SUPPORT REACTIONS -UNIT KN METE STRUCTURE TYPE = SPACE

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JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
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	26	3.29	247.43	-14.74	0.00	-0.13	0.00
	27	0.40	284.78	-34.73	0.00	-0.06	0.00
	28	0.35	232.26	11.72	0.00	-0.04	0.00
1106	20	-1.21	356.62	14.84	0.00	0.00	0.00
	21	-1.71	391.10	13.96	0.00	0.00	0.00
	22	-6.10	465.07	23.21	0.00	0.02	0.00
	23	5.93	559.43	62.35	0.00	0.03	0.00
	24	-9.82	242.55	-30.61	0.00	-0.03	0.00
	25	-4.83	360.79	17.08	0.00	0.00	0.00
	26	2.41	352.46	12.61	0.00	0.00	0.00
	27	-3.84	286.15	-3.51	0.00	-0.01	0.00
	28	1.41	427.10	33.20	0.00	0.01	0.00
1107	20	1.54	324.01	-4.75	0.00	-0.01	0.00
	21	1.44	343.85	-4.74	0.00	-0.01	0.00
	22	3.18	472.80	-45.24	0.00	0.01	0.00
	23	6.83	446.93	36.51	0.00	0.02	0.00
	24	-5.97	239.16	-46.07	0.00	-0.03	0.00
	25	-0.59	345.39	-11.04	0.00	-0.01	0.00
	26	3.67	302.64	1.54	0.00	0.00	0.00
	27	1.40	401.31	-36.71	0.00	-0.01	0.00
	28	1.68	246.72	27.20	0.00	0.00	0.00
1108	20	-2.79	306.45	4.03	0.00	0.01	0.00
	21	-3.70	336.57	4.07	0.00	0.02	0.00
	22	-9.67	235.62	-36.11	0.00	0.04	0.00
	23	3.24	450.91	45.04	0.00	0.05	0.00
	24	-10.79	197.77	-36.91	0.00	-0.03	0.00
	25	-5.91	296.41	-2.25	0.00	0.02	0.00
	26	0.34	316.49	10.31	0.00	0.00	0.00
	27	-4.96	221.99	-27.75	0.00	0.01	0.00
	28	-0.62	390.92	35.81	0.00	0.01	0.00
1109	20	1.13	409.91	-7.95	0.00	0.00	0.00
	21	0.97	431.18	-8.36	0.00	0.00	0.00
	22	0.21	544.81	-66.28	0.00	0.02	0.00
	23	8.01	528.28	31.37	0.00	0.04	0.00
	24	-5.72	307.69	-47.30	0.00	-0.04	0.00
	25	-2.26	416.12	-14.07	0.00	0.00	0.00
	26	4.51	403.69	-1.83	0.00	0.00	0.00
	27	-2.42	481.83	-37.24	0.00	0.00	0.00

	28	4.67	337.98	21.33	0.00	0.00	0.00
1110	20	1.10	389.12	7.87	0.00	0.00	0.00
	21	0.91	423.38	7.53	0.00	0.00	0.00
	22	1.66	273.07	-50.45	0.00	0.02	0.00
	23	9.87	499.95	47.27	0.00	0.03	0.00
	24	-7.76	291.99	-31.52	0.00	-0.03	0.00
	25	-1.87	367.21	1.71	0.00	0.00	0.00
	26	4.07	411.02	14.03	0.00	0.00	0.00
	27	-0.04	316.01	-21.61	0.00	0.00	0.00
	28	2.24	462.23	37.35	0.00	0.00	0.00

STAAD SPACE

-- PAGE NO. 326

SUPPORT REACTIONS -UNIT KN METE STRUCTURE TYPE = SPACE

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JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
1111	20	-1.32	385.81	-20.62	0.00	0.00	0.00
	21	-1.36	402.06	-22.25	0.00	0.00	0.00
	22	-1.99	410.63	13.52	0.00	0.02	0.00
	23	5.80	458.88	1.43	0.00	0.04	0.00
	24	-8.53	333.36	-43.02	0.00	-0.04	0.00
	25	-5.05	387.28	-20.10	0.00	0.00	0.00
	26	2.41	384.34	-21.14	0.00	0.00	0.00
	27	-5.03	431.70	-32.42	0.00	0.00	0.00
	28	2.39	339.91	-8.82	0.00	0.00	0.00
1112	20	-1.20	372.50	8.15	0.00	0.00	0.00
	21	-1.25	404.44	8.48	0.00	0.00	0.00
	22	-1.06	342.65	51.77	0.00	0.01	0.00
	23	7.43	409.80	43.16	0.00	0.03	0.00
	24	-10.03	348.38	-26.70	0.00	-0.03	0.00
	25	-4.39	366.98	7.75	0.00	0.00	0.00
	26	2.00	378.01	8.55	0.00	0.00	0.00
	27	-2.80	360.22	-15.45	0.00	-0.01	0.00

	28	0.40	384.78	31.75	0.00	0.01	0.00
1113	20	-1.39	338.82	-16.99	0.00	0.00	0.00
	21	-1.51	354.36	-17.54	0.00	0.00	0.00
	22	0.47	329.36	-10.42	0.00	0.02	0.00
	23	8.76	407.03	2.57	0.00	0.04	0.00
	24	-11.94	292.05	-37.05	0.00	-0.04	0.00
	25	-6.52	342.81	-17.13	0.00	0.00	0.00
	26	3.75	334.82	-16.84	0.00	0.00	0.00
	27	-5.69	376.27	-27.72	0.00	0.00	0.00
	28	2.92	301.36	-6.25	0.00	0.01	0.00
1114	20	-0.58	282.53	3.92	0.00	0.00	0.00
	21	-0.76	298.59	4.25	0.00	0.00	0.00
	22	-1.94	294.74	17.22	0.00	0.01	0.00
	23	11.02	309.67	34.79	0.00	0.02	0.00
	24	-12.52	268.71	-27.05	0.00	-0.03	0.00
	25	-5.48	287.64	4.39	0.00	0.00	0.00
	26	4.33	277.41	3.46	0.00	0.00	0.00
	27	-5.08	294.82	-13.11	0.00	-0.01	0.00
	28	3.93	270.23	20.96	0.00	0.01	0.00
1115	20	4.77	250.83	-7.60	0.00	0.00	0.00
	21	8.11	258.25	-7.61	0.00	0.00	0.00
	22	8.02	296.89	-10.91	0.00	0.02	0.00
	23	83.40	424.24	16.12	0.00	0.04	0.00
	24	-76.67	86.99	-31.33	0.00	-0.04	0.00
	25	-30.81	198.55	-8.37	0.00	0.00	0.00
	26	40.35	303.10	-6.83	0.00	0.00	0.00
	27	-21.36	257.80	-24.71	0.00	0.00	0.00
	28	30.89	243.85	9.51	0.00	0.01	0.00
1116	20	18.56	264.09	7.60	0.00	0.00	0.00
	21	25.00	278.81	7.60	0.00	0.00	0.00
	22	2.35	254.37	4.39	0.00	0.02	0.00

STAAD SPACE

-- PAGE NO. 327

SUPPORT REACTIONS -UNIT KN METE STRUCTURE TYPE = SPACE

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JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
	23	103.57	422.08	31.31	0.00	0.03	0.00
	24	-68.99	112.88	-16.10	0.00	-0.02	0.00
	25	-17.31	209.82	6.75	0.00	0.00	0.00
	26	54.43	318.37	8.45	0.00	0.00	0.00
	27	-16.55	174.86	-9.72	0.00	0.00	0.00
	28	53.66	353.32	24.92	0.00	0.01	0.00
1117	20	-2.74	247.85	-6.87	0.00	0.00	0.00
	21	0.18	260.30	-6.41	0.00	0.00	0.00
	22	-7.50	239.70	-6.75	0.00	0.02	0.00
	23	59.06	408.42	42.99	0.00	0.04	0.00
	24	-67.75	102.18	-56.69	0.00	-0.04	0.00
	25	-34.43	307.77	-7.10	0.00	0.00	0.00
	26	28.96	187.93	-6.65	0.00	0.00	0.00
	27	-30.99	356.18	-28.21	0.00	0.00	0.00
	28	25.52	139.52	14.47	0.00	0.00	0.00
1118	20	5.55	276.46	8.57	0.00	0.00	0.00
	21	11.71	301.69	9.06	0.00	0.00	0.00
	22	-8.24	309.47	8.69	0.00	0.02	0.00
	23	75.35	498.08	58.52	0.00	0.04	0.00
	24	-67.48	68.01	-41.31	0.00	-0.04	0.00
	25	-23.53	334.68	8.42	0.00	0.00	0.00
	26	34.64	218.23	8.71	0.00	0.00	0.00
	27	-13.84	265.09	-12.77	0.00	0.00	0.00
	28	24.94	287.82	29.90	0.00	0.01	0.00
1119	20	-3.10	130.57	-3.54	0.00	0.00	0.00
	21	-3.35	133.20	-4.02	0.00	0.00	0.00
	22	-1.62	135.63	-7.15	0.00	0.01	0.00
	23	5.77	234.78	22.20	0.00	0.01	0.00
	24	-12.46	29.14	-29.34	0.00	-0.01	0.00
	25	-7.90	152.84	-3.36	0.00	0.00	0.00
	26	1.69	108.30	-3.72	0.00	0.00	0.00
	27	-7.66	191.04	-16.15	0.00	-0.01	0.00
	28	1.45	70.09	9.07	0.00	0.01	0.00
1120	20	-1.70	202.38	1.85	0.00	0.00	0.00

	21	-1.74	232.56	1.37	0.00	0.00	0.00
	22	-2.75	231.02	-1.76	0.00	0.02	0.00
	23	5.48	339.55	27.56	0.00	0.02	0.00
	24	-9.21	67.94	-23.93	0.00	-0.02	0.00
	25	-4.53	231.44	2.03	0.00	0.00	0.00
	26	1.12	173.32	1.66	0.00	0.00	0.00
	27	-3.46	185.31	-10.84	0.00	0.00	0.00
	28	0.05	219.45	14.54	0.00	0.00	0.00
1121	20	-0.22	181.16	16.12	0.00	0.00	0.00
	21	-0.23	205.07	17.22	0.00	0.00	0.00
	22	-0.22	257.49	28.53	0.00	0.00	0.00
	23	3.08	260.12	34.54	0.00	0.03	0.00
	24	-3.51	104.41	-1.82	0.00	-0.04	0.00
	25	-0.21	177.98	15.07	0.00	-0.02	0.00

STAAD SPACE

-- PAGE NO. 328

SUPPORT REACTIONS -UNIT KN METE STRUCTURE TYPE = SPACE

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JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
	26	-0.23	184.35	17.17	0.00	0.01	0.00
	27	-0.01	125.97	1.29	0.00	-0.01	0.00
	28	-0.43	236.36	30.95	0.00	0.01	0.00
1181	20	1.46	214.13	-15.62	0.00	0.22	0.00
	21	0.17	221.97	-16.51	0.00	0.20	0.00
	22	10.61	294.71	-14.32	0.00	0.51	0.00
	23	22.36	454.23	9.40	0.00	1.36	0.00
	24	-17.01	73.79	-44.76	0.00	-0.59	0.00
	25	-9.16	212.31	-18.21	0.00	-0.15	0.00
	26	12.08	215.94	-13.03	0.00	0.59	0.00
	27	-8.47	295.06	-31.01	0.00	0.27	0.00
	28	11.39	133.19	-0.22	0.00	0.17	0.00
1182	20	-1.99	179.27	-7.94	0.00	-0.05	0.00

	21	-2.25	176.98	-7.98	0.00	-0.04	0.00
	22	0.00	254.76	-10.43	0.00	-0.10	0.00
	23	5.29	392.82	9.80	0.00	0.13	0.00
	24	-10.84	47.48	-27.14	0.00	-0.29	0.00
	25	-6.20	191.73	-6.66	0.00	0.03	0.00
	26	2.22	166.81	-9.21	0.00	-0.13	0.00
	27	-5.66	268.54	-16.75	0.00	-0.06	0.00
	28	1.68	90.00	0.87	0.00	-0.04	0.00
1298	20	0.13	125.95	9.31	0.00	-0.01	0.00
	21	0.13	134.52	10.04	0.00	-0.01	0.00
	22	-0.24	156.52	18.13	0.00	0.00	0.00
	23	3.81	191.94	24.83	0.00	0.03	0.00
	24	-3.60	61.96	-5.89	0.00	-0.04	0.00
	25	-0.38	128.45	9.92	0.00	-0.02	0.00
	26	0.64	123.45	8.71	0.00	0.01	0.00
	27	-0.45	91.00	1.33	0.00	-0.02	0.00
	28	0.70	160.90	17.30	0.00	0.01	0.00

\*\*\*\*\* END OF LATEST ANALYSIS RESULT \*\*\*\*\*

2279. FINISH

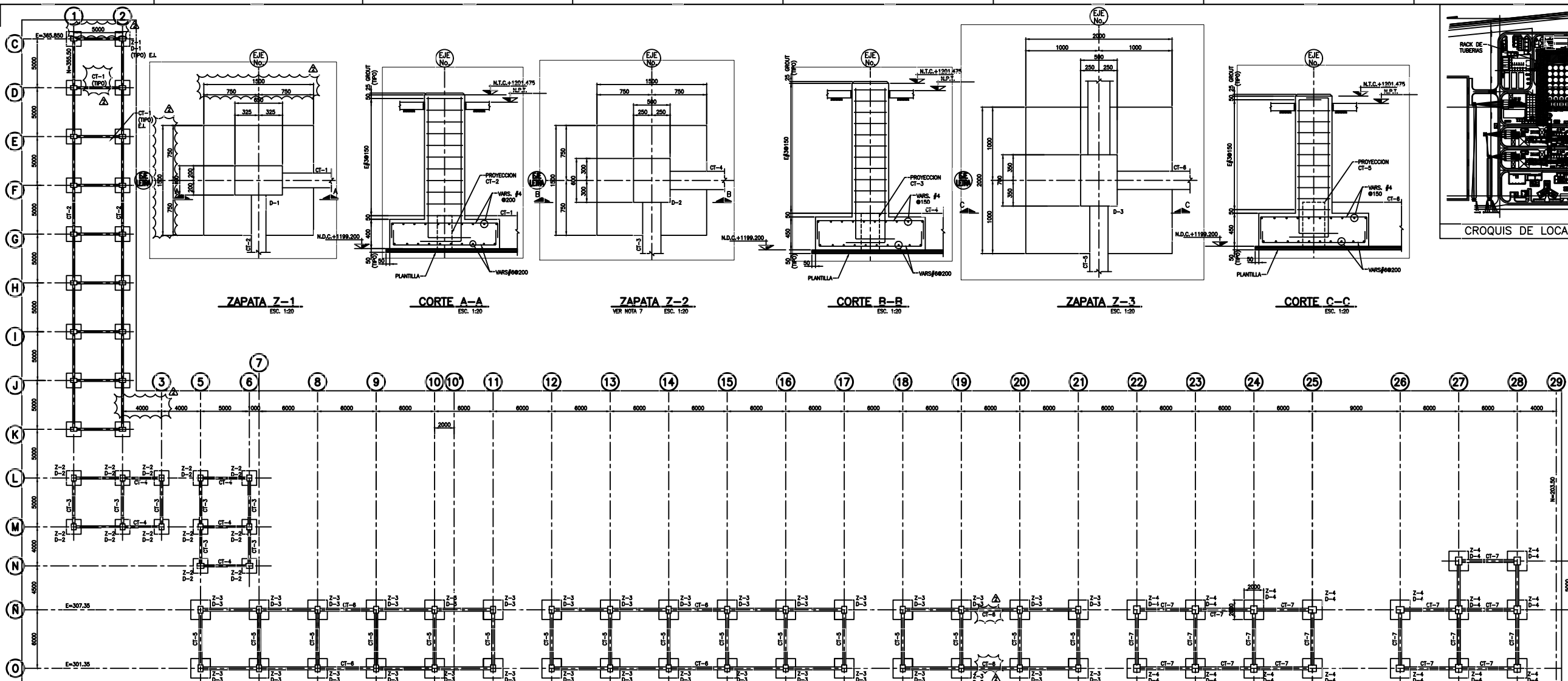
STAAD SPACE

-- PAGE NO. 329

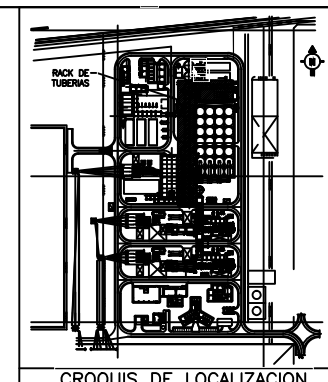
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\*\*\*\* DATE= SEP 5,2018 TIME= 19:57:19 \*\*\*\*

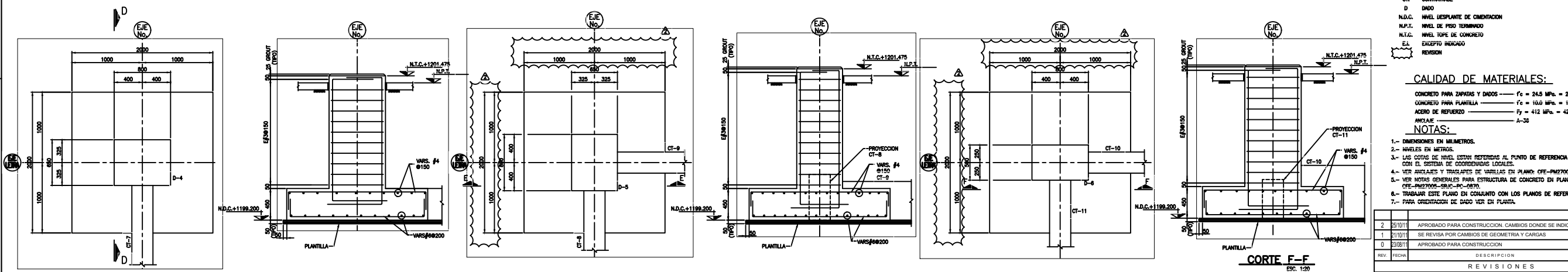
0080-PC



**PLANTA DE CIMENTACION ZONA NORTE**  
ESC. 1:150



CROQUIS DE LOCALIZACION



**ZAPATA Z-4** VER NOTA 7 ESC. 1:20  
**CORTE D-D** ESC. 1:20  
**ZAPATA Z-5** ESC. 1:20  
**CORTE E-E** ESC. 1:20  
**ZAPATA Z-6** ESC. 1:20  
**CORTE F-F** ESC. 1:20

**NOMENCLATURA:**

- Z ZAPATA
- CT. CONTRANTE
- D DADO
- N.D.C. NIVEL DESPLANTE DE CIMENTACION
- N.P.T. NIVEL DE PISO TERMINADO
- N.T.C. NIVEL TOPE DE CONCRETO
- E.L. EXCEPTO INDICADO
- REVISION

**CALIDAD DE MATERIALES:**

- CONCRETO PARA ZAPATAS Y DADOS —  $f_c = 24.5 \text{ MPa} = 250 \text{ kg/cm}^2$
- CONCRETO PARA PLANTILLA —  $f_c = 10.0 \text{ MPa} = 100 \text{ kg/cm}^2$
- ACERO DE REFUERZO —  $f_y = 412 \text{ MPa} = 4200 \text{ kg/cm}^2$
- ANCLAJE — A-36

- NOTAS:**
- 1.- DIMENSIONES EN MILIMETROS.
  - 2.- NIVELES EN METROS.
  - 3.- LAS COTAS DE NIVEL ESTAN REFERIDAS AL PUNTO DE REFERENCIA CON EL SISTEMA DE COORDENADAS LOCALES.
  - 4.- VER ANCLAJES Y TRASLAPES DE VARILLAS EN PLANO CFE-PM27005-SRJC-PC-0885.
  - 5.- VER NOTAS GENERALES PARA ESTRUCTURA DE CONCRETO EN PLANO: CFE-PM27005-SRJC-PC-0870.
  - 6.- TRABAJAR ESTE PLANO EN CONSULTA CON LOS PLANOS DE REFERENCIA.
  - 7.- PARA ORIENTACION DE DADO VER EN PLANTA.

REV.	FECHA	DESCRIPCION	REVISOR	APROB.
2	25/10/11	APROBADO PARA CONSTRUCCION. CAMBIOS DONDE SE INDICA		
1	21/10/11	SE REVISÓ POR CAMBIOS DE GEOMETRIA Y CARGAS		
0	23/08/11	APROBADO PARA CONSTRUCCION		

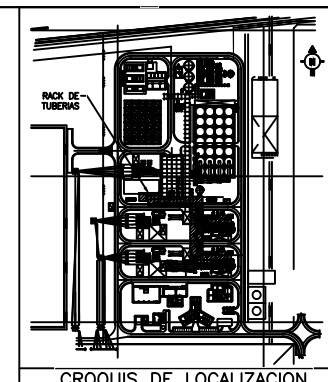
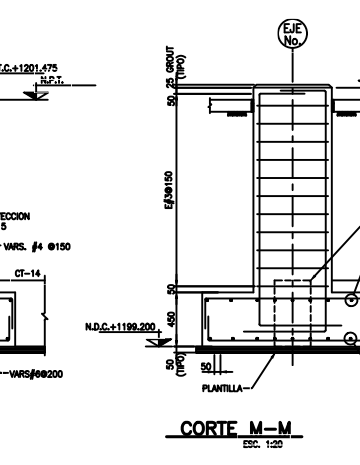
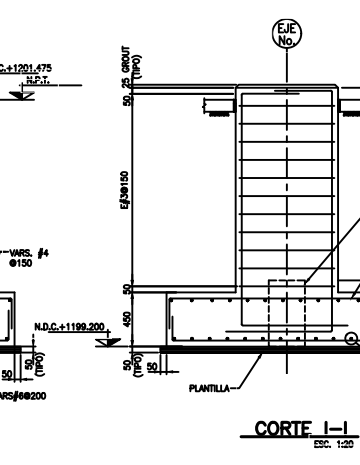
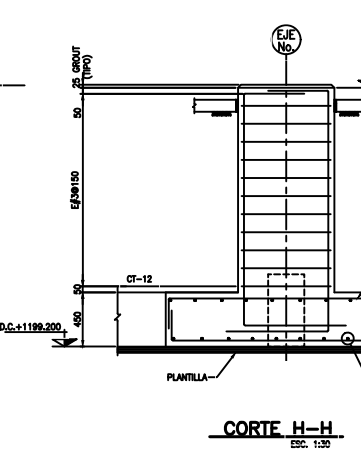
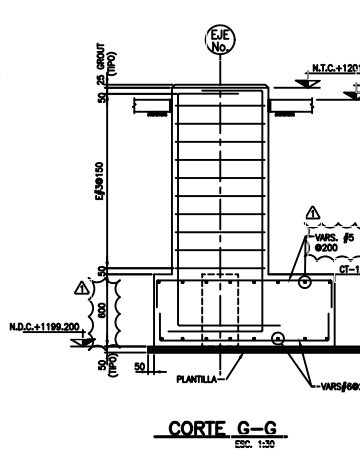
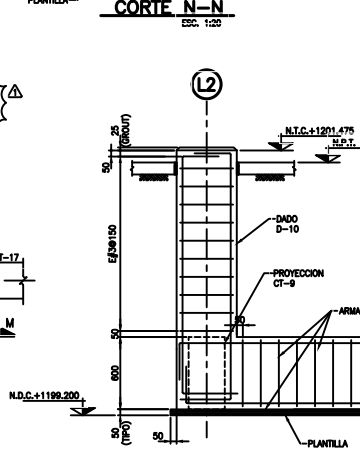
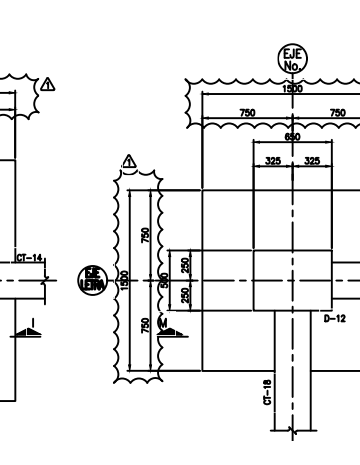
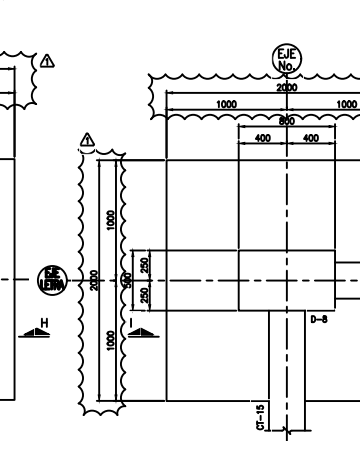
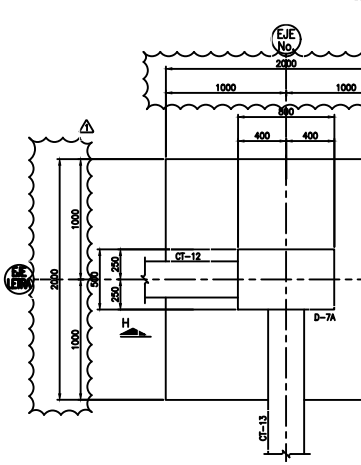
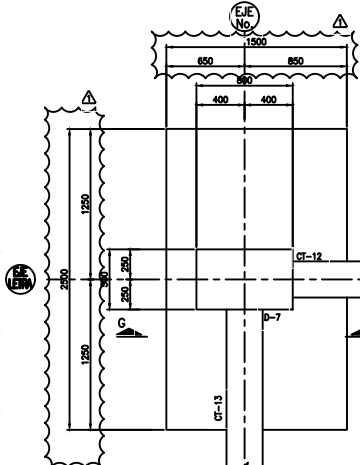
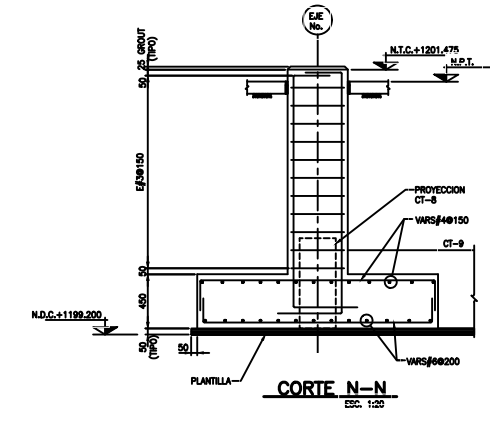
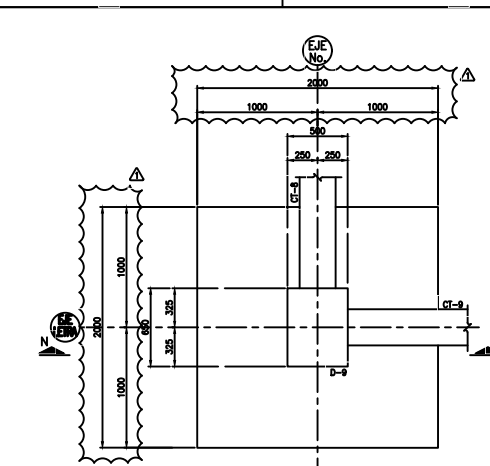
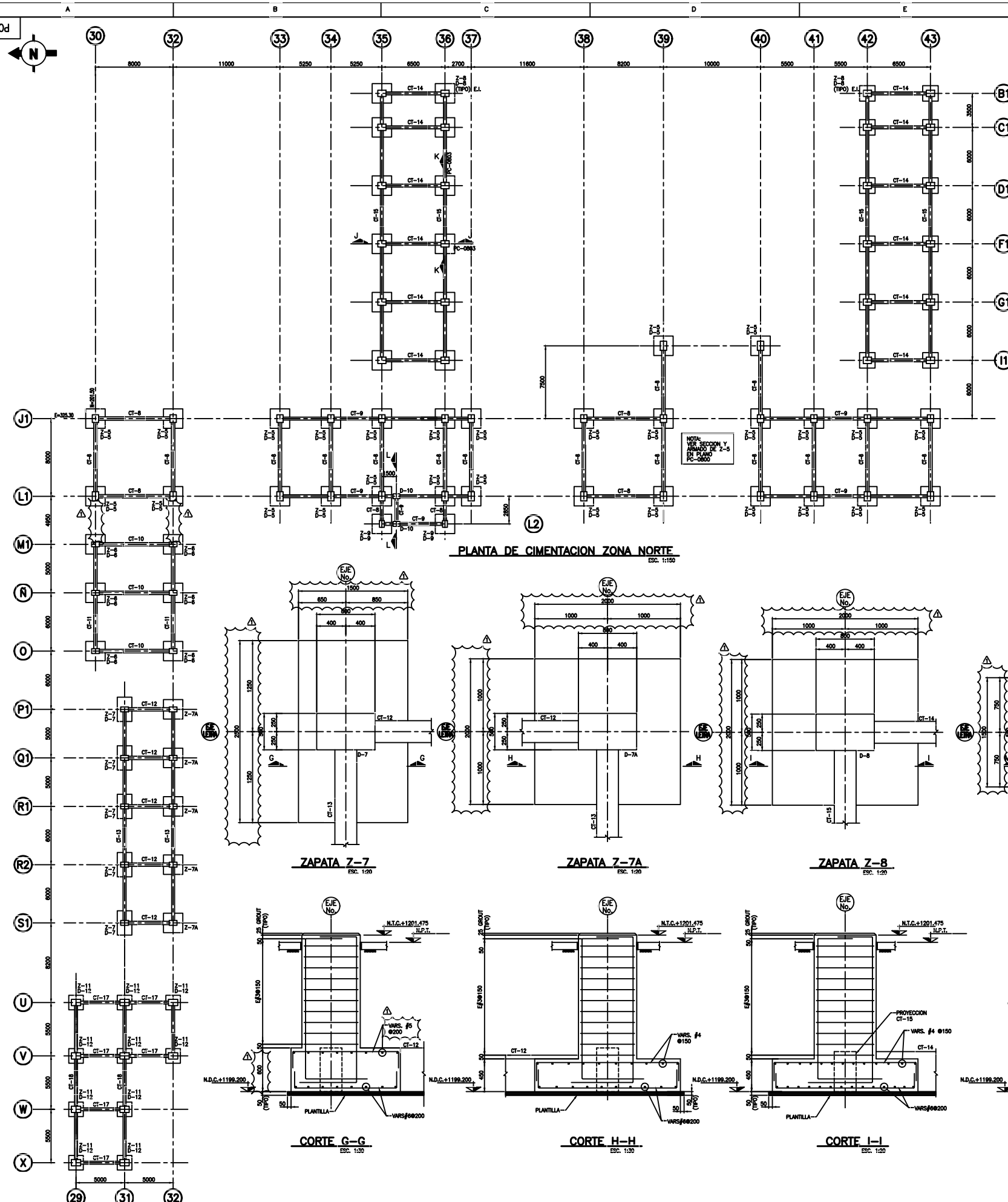
**REVISIONES**

ESTADO DE DOCUMENTO COMO SE ANOTA		PROYECTO	
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2	<input type="checkbox"/> CONFORME SIN COMENTARIOS	TITULO	
3	<input type="checkbox"/> CONFORME CON COMENTARIOS	RACK DE TUBERIAS	
4	<input type="checkbox"/> NO CONFORME	CIMENTACION PLANTA 1 DE 2	
5	<input type="checkbox"/> TAL COMO SE CONSTRUYO	APROBADO	
FECHA DE RECEPCION: Revisado por Ing. Supervisor		REALIZADO	
Fecha:		ESCALA	
IMPORTANTE:		INDICADA	
NO. DE CONTROL:		ORDEN DE COMPRA	
		DOCUMENTO DE PROYECTO No.	
		DOCUMENTO SUBASTADOR No.	
		REVISION	
		APROB.	

**PLANOS REFERENCIA**

- CFE-PM27005-SRJC-PC-0801 RACK DE TUBERIAS CIMENTACION PLANTAS 2 DE 2
- CFE-PM27005-SRJC-PC-0802 RACK DE TUBERIAS CIMENTACION DETALLES
- CFE-PM27005-SRJC-PC-0870 NOTAS GENERALES DE CONCRETO
- CFE-PM27005-SRJC-PC-0885 DETALLES TIPO DE DOBLEZ DE VARILLAS





**NOMENCLATURA:**

- Z ZAPATA
- CT. CONTRAVIRTE
- D DADO
- N.D.C. NIVEL DESPLANTE DE CIMENTACION
- H.P.T. NIVEL DE PISO TERMINADO
- N.T.C. NIVEL TOPE DE CONCRETO
- E.L. EXCEPTO INDICADO
- REVISION

**CALIDAD DE MATERIALES:**

- CONCRETO PARA ZAPATAS Y DADOS  $f_c = 24.5 \text{ MPa} = 250 \text{ kg/cm}^2$
- CONCRETO PARA PLANTILLA  $f_c = 10.0 \text{ MPa} = 100 \text{ kg/cm}^2$
- ACERO DE REFUERZO  $f_y = 412 \text{ MPa} = 4200 \text{ kg/cm}^2$
- ANCLAJE A-35

**NOTAS:**

- 1.- DIMENSIONES EN MILIMETROS.
- 2.- NIVELES EN METROS.
- 3.- LAS COTAS DE REBEL ESTAN REFERIDAS AL PUNTO DE REFERENCIA CON EL SISTEMA DE COORDENADAS LOCALES.
- 4.- VER ANCLAJES Y TRASLAPES DE VARILLAS EN PLANO CFE-PM27005-SRJC-PC-0885.
- 5.- VER NOTAS GENERALES PARA ESTRUCTURA DE CONCRETO EN PLANO: CFE-PM27005-SRJC-PC-0870.
- 6.- TRABAJAR ESTE PLANO EN COORDINACION CON LOS PLANOS DE REFERENCIA.

**PLANOS REFERENCIA**

- PM27005-SRJC-PC-0880 RACK DE TUBERIAS CIMENTACION PLANTAS 1 DE 2
- PM27005-SRJC-PC-0882 RACK DE TUBERIAS CIMENTACION DETALLES
- PM27005-SRJC-PC-0883 NOTAS GENERALES DE CONCRETO
- PM27005-SRJC-PC-0885 DETALLES TIPO DE DUREZ DE VARILLAS

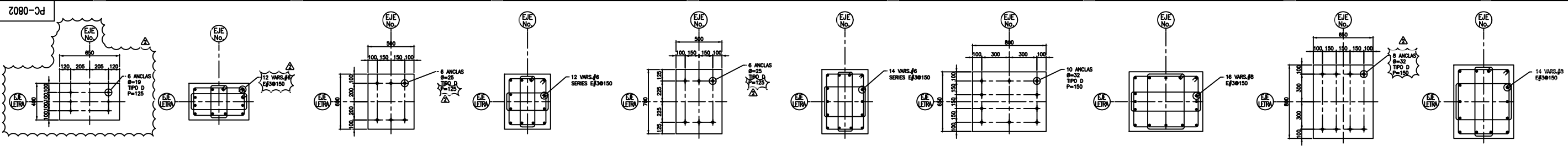
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4	<input type="checkbox"/>	NO CONFORME
5	<input type="checkbox"/>	TAL COMO SE CONSTRUYO

FECHA DE RECEPCION: Revisado por Ing. Supervisor  
Fecha: \_\_\_\_\_

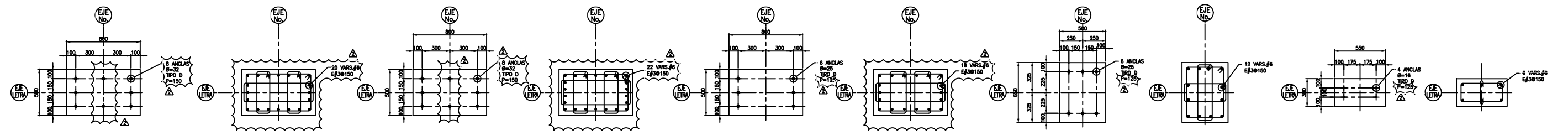
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0	22/08/11	APROBADO PARA CONSTRUCCION		
REV.	FECHA	DESCRIPCION	REVISOR	APROB.
REVISIONES				
PROYECTO: PLANTA DE CICLO COMBINADO				
TITULO: RACK DE TUBERIAS CIMENTACION PLANTA 2 DE 2				
APROBADO	SES	TITULO	RACK DE TUBERIAS CIMENTACION PLANTAS 2 DE 2	
COMPROBADO	SES	TITULO	RACK DE TUBERIAS CIMENTACION PLANTAS 2 DE 2	
ESCALA	INDICADA	ORDEN DE COMPRA	DOCUMENTO DE PROYECTO No.	REV. No.
			PM27005-SRJC-PC-0801	1
			DOCUMENTO ADMINISTRACION No.	REV. No.



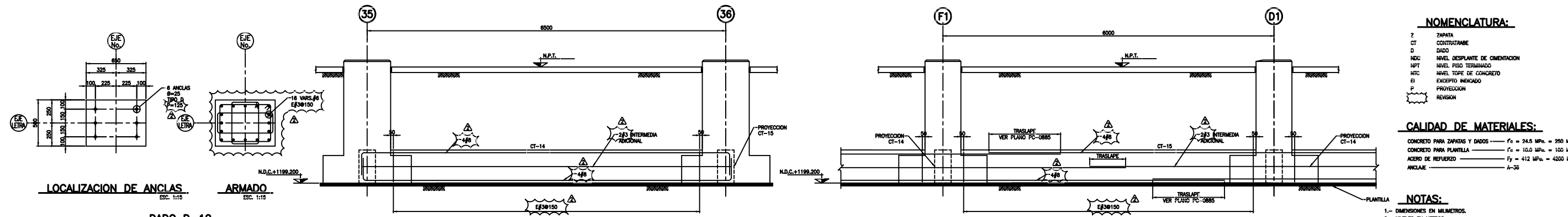
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DADO D-1 ESC. 1:15    DADO D-2 ESC. 1:15    DADO D-3 ESC. 1:15    DADO D-4 ESC. 1:15    DADO D-5 ESC. 1:15



LOCALIZACION DE ANCLAS ESC. 1:15    ARMADO ESC. 1:15    LOCALIZACION DE ANCLAS ESC. 1:15    ARMADO ESC. 1:15    LOCALIZACION DE ANCLAS ESC. 1:15    ARMADO ESC. 1:15    LOCALIZACION DE ANCLAS ESC. 1:15    ARMADO ESC. 1:15    LOCALIZACION DE ANCLAS ESC. 1:15    ARMADO ESC. 1:15

DADO D-6 ESC. 1:15    DADO D-7 y D-7A ESC. 1:15    DADO D-8 ESC. 1:15    DADO D-9 ESC. 1:15    DADO D-10 ESC. 1:15



**NOMENCLATURA:**

7	ZAPATA
CT	CONSTRUYABLE
D	DADO
NDC	NIVEL DESPLANTE DE CIMENTACION
NPT	NIVEL PISO TERMINADO
NTC	NIVEL TOPE DE CONCRETO
Ø	Ø
P	PROTECCION
R	REVISION

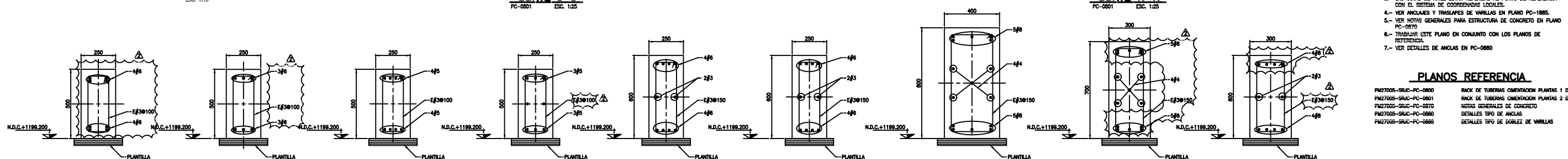
**CALIDAD DE MATERIALES:**

CONCRETO PARA ZAPATAS Y DADOS	$f_c = 24.5 \text{ MPa} = 250 \text{ kg/cm}^2$
CONCRETO PARA PLANTILLA	$f_c = 10.0 \text{ MPa} = 100 \text{ kg/cm}^2$
ACERO DE REFUERZO	$f_y = 412 \text{ MPa} = 4200 \text{ kg/cm}^2$
ANCLAS	A-35

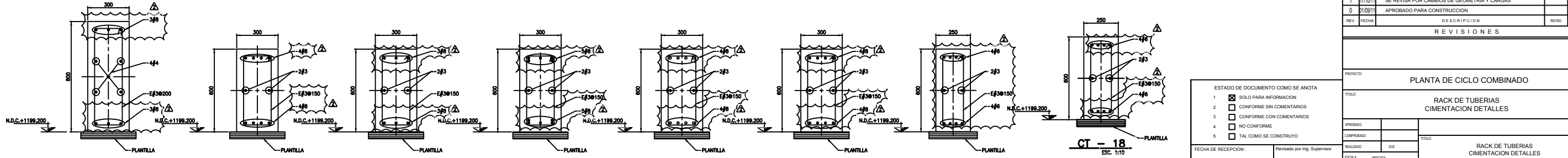
- NOTAS:**
- DIMENSIONES EN MILIMETROS.
  - NIVELES EN METROS.
  - LAS COTAS DE NIVEL ESTAN REFERIDAS AL PUNTO DE REFERENCIA CON EL SISTEMA DE COORDENADAS LOCALES.
  - VER ANCLAS Y TRASLAPES DE VARRILLAS EN PLANO PC-1885.
  - VER NOTAS GENERALES PARA ESTRUCTURA DE CONCRETO EN PLANO PC-0870
  - TRABAJAR ESTE PLANO EN CONJUNTO CON LOS PLANOS DE REFERENCIA.
  - VER DETALLES DE ANCLAS EN PC-0880

**PLANOS REFERENCIA**

PM27005-SRJC-PC-0880	RACK DE TUBERIAS CIMENTACION PLANTAS 1 DE 2
PM27005-SRJC-PC-0881	RACK DE TUBERIAS CIMENTACION PLANTAS 2 DE 2
PM27005-SRJC-PC-0870	NOTAS GENERALES DE CONCRETO
PM27005-SRJC-PC-0885	DETALLES TIPO DE ANCLAS
PM27005-SRJC-PC-0880	DETALLES TIPO DE DOBLEZ DE VARRILLAS



CT - 1 ESC. 1:10    CT - 2 ESC. 1:10    CT - 3 ESC. 1:10    CT - 4 ESC. 1:10    CT - 5 ESC. 1:10    CT - 6 ESC. 1:10    CT - 7 ESC. 1:10    CT - 8 ESC. 1:10    CT - 9 ESC. 1:10



CT - 10 ESC. 1:10    CT - 11 ESC. 1:10    CT - 12 ESC. 1:10    CT - 13 ESC. 1:10    CT - 14 ESC. 1:10    CT - 15 ESC. 1:10    CT - 17 ESC. 1:10

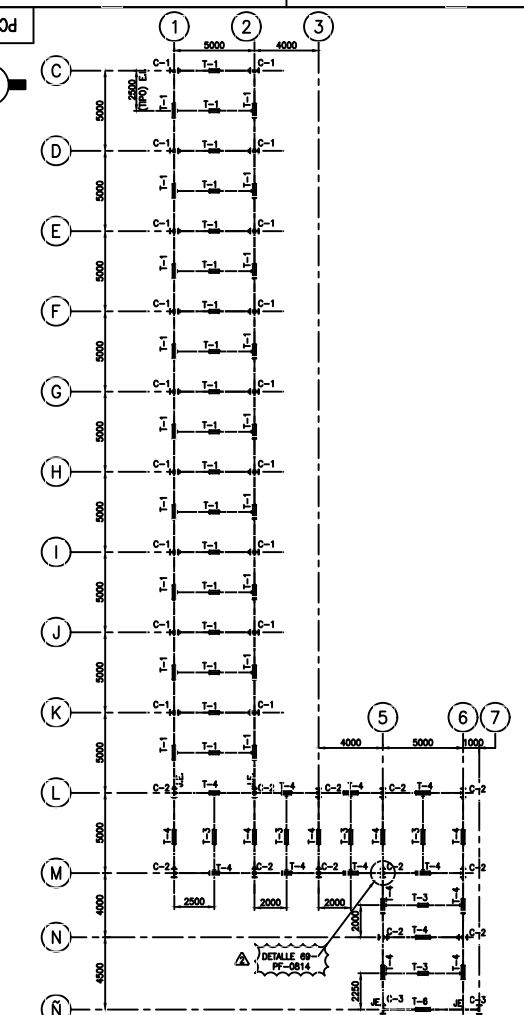
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2	<input type="checkbox"/>	CONFORME SIN COMENTARIOS
3	<input type="checkbox"/>	CONFORME CON COMENTARIOS
4	<input type="checkbox"/>	NO CONFORME
5	<input type="checkbox"/>	TAL COMO SE CONSTRUYO

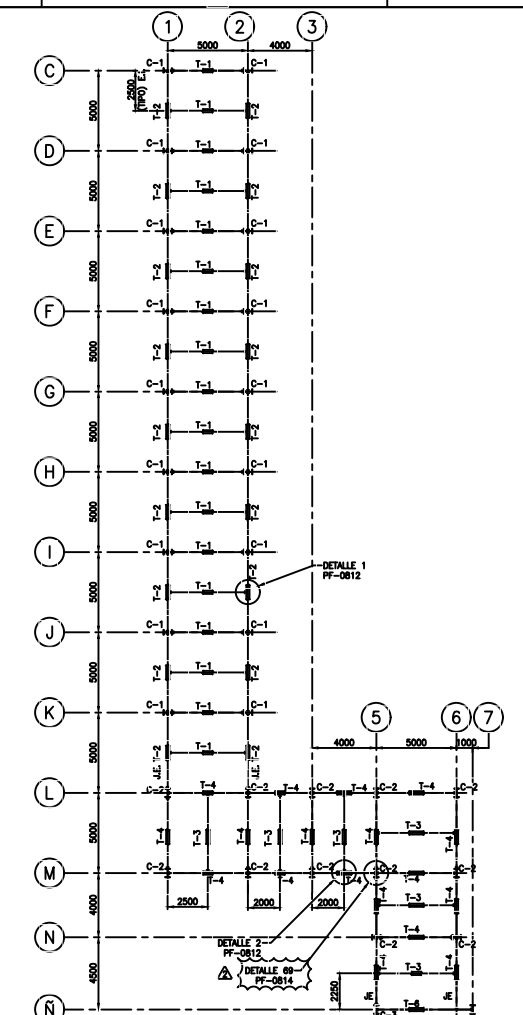
FECHA DE RECEPCION: \_\_\_\_\_ Revisado por Ing. Supervisor \_\_\_\_\_  
 OBRERA DE COMPRA: \_\_\_\_\_ Fecha: \_\_\_\_\_  
 IMPORTANTE: \_\_\_\_\_  
 NO. DE CONTROL: \_\_\_\_\_

2	20/10/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
1	17/10/11	SE REVISIA POR CAMBIOS DE GEOMETRIA Y CARGAS		
0	01/05/11	APROBADO PARA CONSTRUCCION		
REV.	FECHA	DESCRIPCION	REVISO	APROBO
REVISIONES				
PROYECTO: PLANTA DE CICLO COMBINADO				
TITULO: RACK DE TUBERIAS CIMENTACION DETALLES				
APROBADO: _____				
REALIZADO: _____				
ESCALA: INDICADA				
DOCUMENTO DE PROYECTO NO: _____				
DOCUMENTO SUBADMINISTRADOR NO: PM27005-SRJC-PC-0802				
				REV. NO.
				2

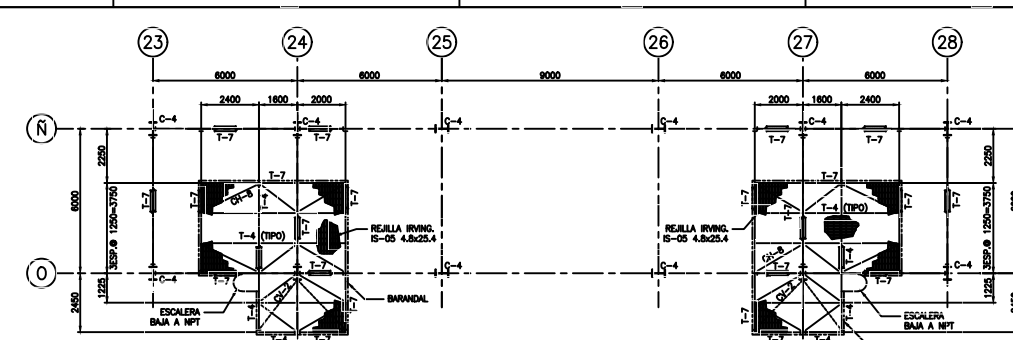
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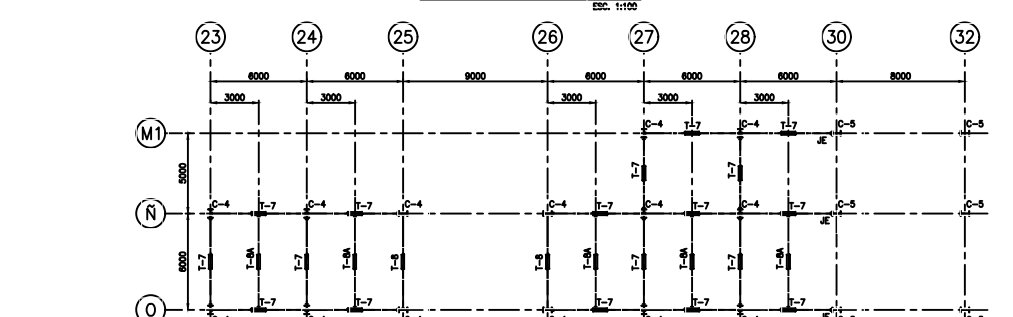
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ESC. 1:150



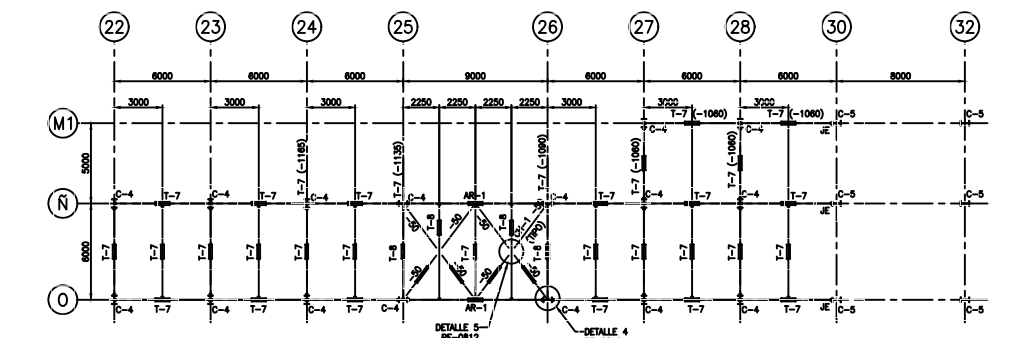
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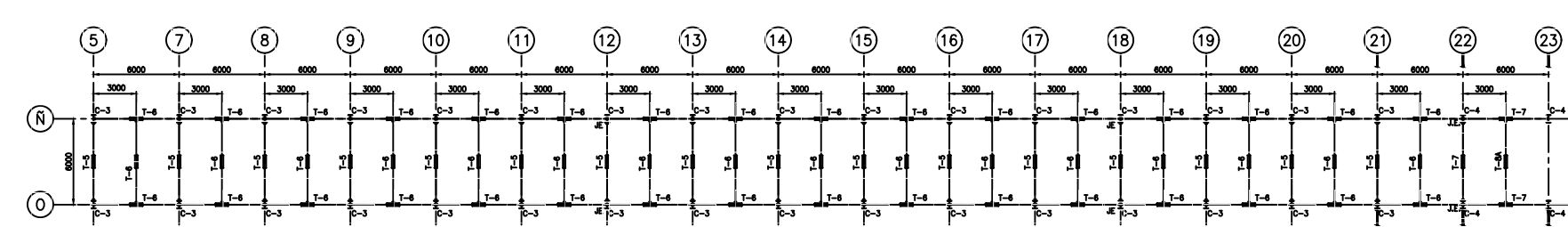
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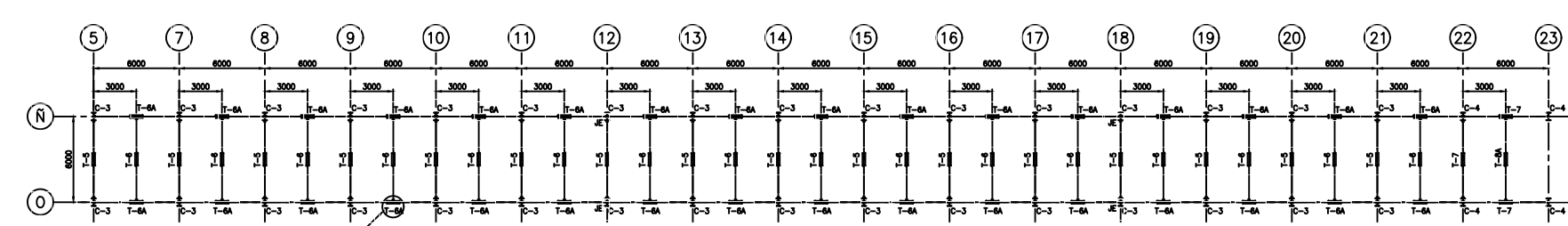
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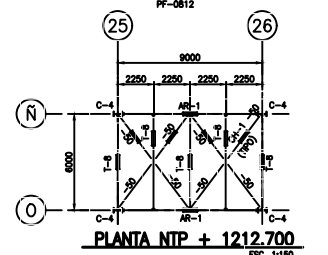
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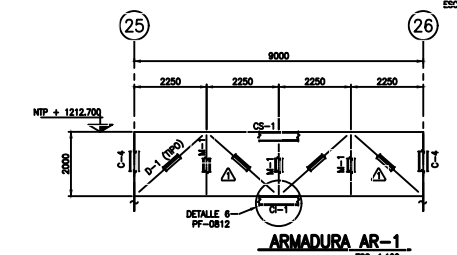
PLANTA NTP + 1205.700  
ESC. 1:150



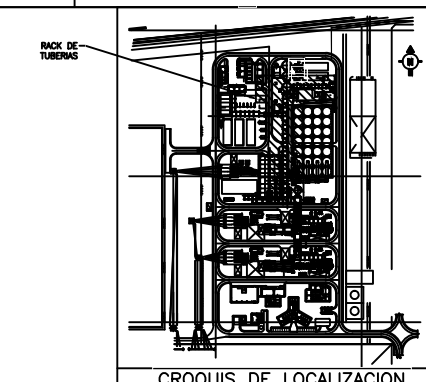
PLANTA NTP + 1207.700  
ESC. 1:150



PLANTA NTP + 1212.700  
ESC. 1:150



ARMADURA AR-1  
ESC. 1:100



CROQUIS DE LOCALIZACION

**TABLA DE PERFILES**

MARCA	PERFIL	PESO L/m	d	bf	hf	tw
C-1	RL-305	55.8	303	203	13.1	7.5
C-2	RL-305	79	306	204	14.8	8.8
C-3	RL-306	110.4	350	256	18.9	11.4
C-4	RL-307	144.3	472	283	22.08	13.5
T-1	RL-254	38.5	282	147	11.2	6.8
T-2	RL-305	52.2	318	167	13.2	7.8
T-3	RL-203	31.2	210	134	10.2	6.4
T-4	RL-254	48.2	247	202	11.0	7.4
T-5	RL-305	74.4	310	205	13.3	8.4
T-6	RL-305	52.2	318	167	13.2	7.8
T-6A	RL-306	63.8	347	205	13.5	7.8
T-7	RL-305	74.4	310	205	13.3	8.4
T-8	RL-305	44.5	313	166	11.2	6.8
T-8A	RL-254	38.5	282	147	11.2	6.8
CS-1	RL-305	44.5	313	166	11.2	6.8
CS-1	RL-305	44.5	313	166	11.2	6.8
CH-1	2U 76x10	21.44	-	-	-	-
CH-1	2U 76x10	21.44	-	-	-	-
M-1	2L 102x10	28.16	180	102	10.3	6.8
CH-1	2L 102x10	28.16	-	-	-	-
CH-2	RL-203	31.2	210	134	10.2	6.4
CH-3	RL-254	44.5	280	148	13.0	7.8
CH-8	L 84x10	6.78	-	-	-	-

**NOMENCLATURA**

C.S.	CUBIERTA SUPERIOR
C.I.F.	CUBIERTA INFERIOR
AR	ARMADURA
PL	PLACA BASE
D	DIAGONAL
DI	DIAGONAL
HT	HERRAJE
HT	HERRAJE PISO TERMINADO
HT	HERRAJE TIPO DE PERFE
HT	HERRAJE REEMPLAZO DE ESTRUCTURA
C	COLUMNA
T	TIRANTE
CH	CONTRAENTE HORIZONTAL
CH	CONTRAENTE VERTICAL
CH	CONEXION Y HERRAJE
CH	USO AJUSTO DEL TIPO DE ESTRUCTURA
CH	REVISION GENERAL

**NOTAS:**

- 1.- PARA NOTAS GENERALES, ABBREVIATURAS Y SIMBOLOGIA VER PLANO CIE-ARMADURA-080-PC-0803.
- 2.- DIMENSIONES EN MILIMETROS.
- 3.- NIVELES Y COORDENADAS EN METROS.
- 4.- TODAS LAS DIMENSIONES SON SIMÉTRICAS CON RESPECTO A LAS LINEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
- 5.- ACCESO ESTRUCTURAL NOTA A-30.
- 6.- TORNILLERÍA ASIM A-305 TIPO 1 GALVANIZADOS A TIPO 3.
- 7.- ESTE PLANO ES DE DISEÑO Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE TALLER QUE CUMPLAN CON LOS REQUISITOS DEL ASC-LRF Y AISI COMO MÍNIMO.
- 8.- LA DESCRIPCIÓN DE LOS PERFILES CORRESPONDE CON LA INDICADA EN EL "MANUAL DE CONSTRUCCIÓN EN ACERO" DEL INSTITUTO MEXICANO DE LA CONSTRUCCIÓN EN ACERO (IMCA).
- 9.- NO TOMAR MEDIDAS A ESCALA. LAS CURVAS SIGEN AL DIBUJO.
- 10.- TRABAJAR ESTE PLANO EN COORDINACIÓN CON LOS PLANOS DE REFERENCIA.
- 11.- VER CONEXIONES A CORTANTE Y A MOMENTO DE LA ESTRUCTURA PRINCIPAL EN PLANO PF-0811.

**PLANOS DE REFERENCIA**

PM27005-SRJC-PF-0804	RACK DE TUBERIAS-ESTRUCTURA ELEVACION
PM27005-SRJC-PC-0805	RACK DE P.T.A. Y RACK DE SERVICIOS
PM27005-SRJC-PC-0806	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PC-0807	RACK AERODINAMICO
PM27005-SRJC-PF-0808	RACK DE TUBERIAS-ESTRUCTURA PLANTAS
PM27005-SRJC-PF-0809	RACK PRINCIPAL Y RACK TREN 1 Y 2
PM27005-SRJC-PF-0810	RACK DE TUBERIAS-ESTRUCTURA ELEVACION
PM27005-SRJC-PF-0811	RACK PRINCIPAL Y RACK TREN 1 Y 2
PM27005-SRJC-PF-0812	RACK DE TUBERIAS-ESTRUCTURA ELEVACIONES
PM27005-SRJC-PF-0813	RACK PRINCIPAL
PM27005-SRJC-PF-0814	RACK DE TUBERIAS-ESTRUCTURA CONEXIONES TIPO Y PLACAS BASE
PM27005-SRJC-PF-0815	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 1
PM27005-SRJC-PF-0816	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 2
PM27005-SRJC-PF-0817	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 3

2	25/10/11	APROBADO PARA CONSTRUCCION REVISADO SEGUN SE INDICA		
1	21/10/11	APROBADO PARA CONSTRUCCION REVISADO SEGUN SE INDICA		
0	23/08/11	APROBADO PARA CONSTRUCCION		
REV.	FECHA	DESCRIPCION	REVISOR	APROBADO

ESTADO DE DOCUMENTO COMO SE ANOTA

1  SOLO PARA INFORMACION

2  CONFORME SIN COMENTARIOS

3  CONFORME CON COMENTARIOS

4  NO CONFORME

5  TAL COMO SE CONSTRUYO

FECHA DE RECEPCION: Revisado por Ing. Supervisor

FECHA:

IMPORTE:

NO. DE CONTROL:

PROYECTO: PLANTA DE CICLO COMBINADO

TITULO: RACK DE TUBERIAS ESTRUCTURA PLANTAS RACK DE P.T.A Y RACK DE SERVICIOS

APROBADO: [ ]

COMPROBADO: [ ]

REALIZADO: [ ]

ESCALA: [ ]

INDICADA: [ ]

ORDEN DE COMPRA: [ ]

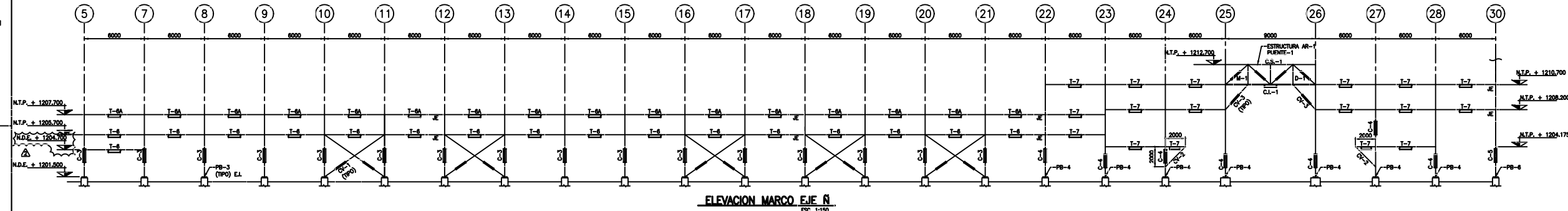
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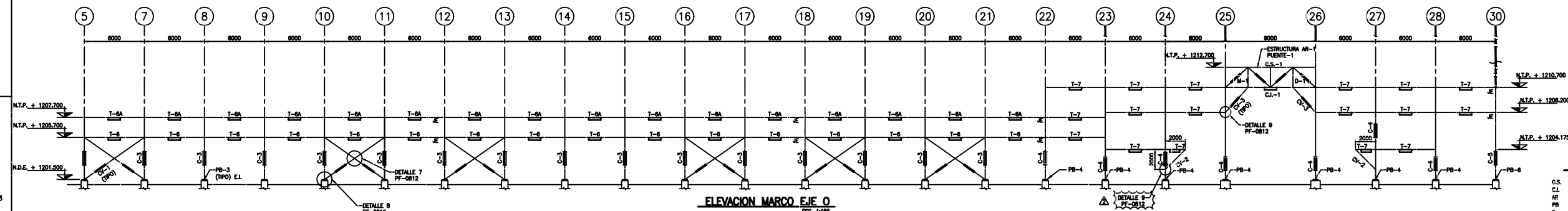
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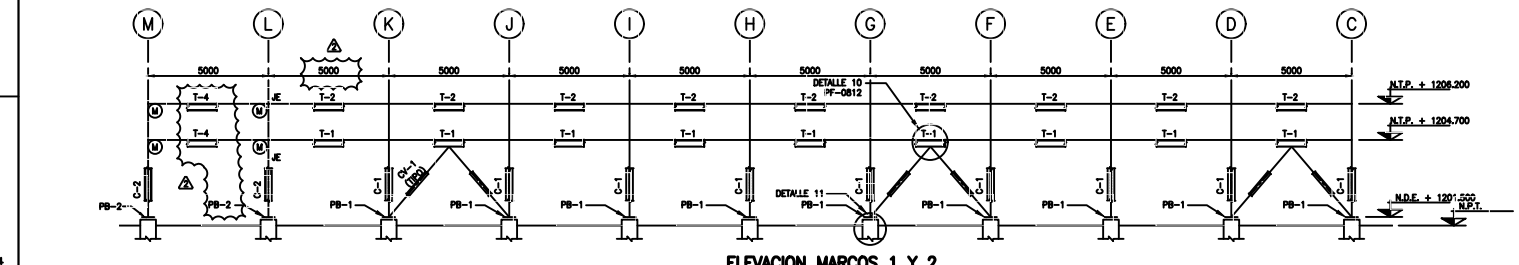
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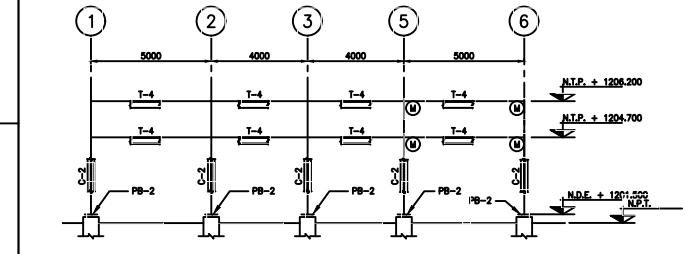
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ESC. 1:150



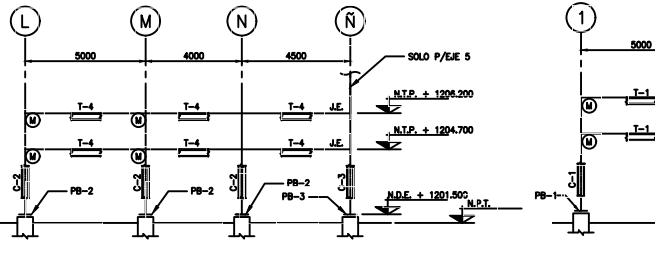
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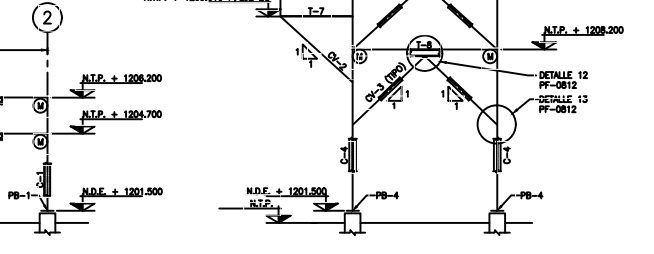
ELEVACION MARCOS 1 Y 2  
ESC. 1:100



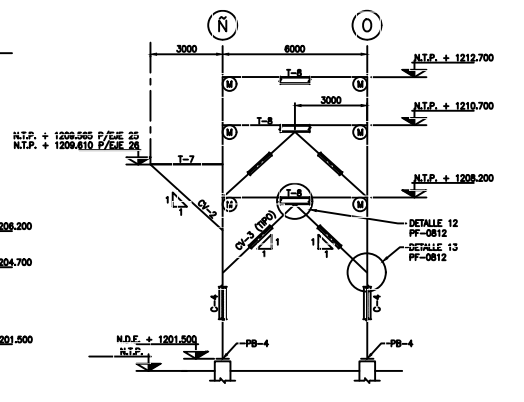
ELEVACION MARCOS EJES L Y M  
ESC. 1:100



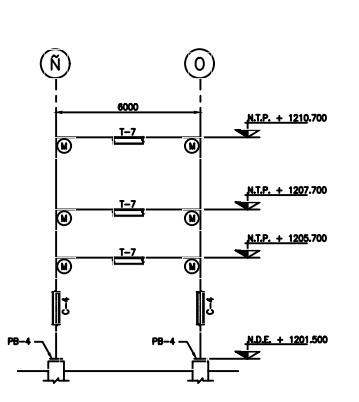
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ESC. 1:100



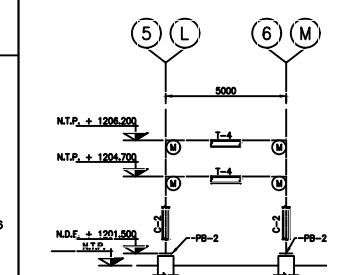
ELEVACION MARCOS EJES DEL C AL K  
ESC. 1:100



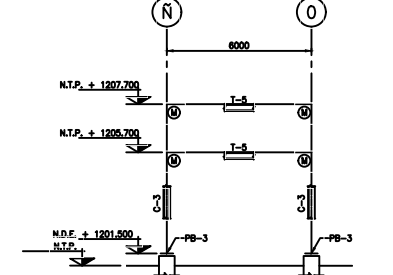
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ESC. 1:100



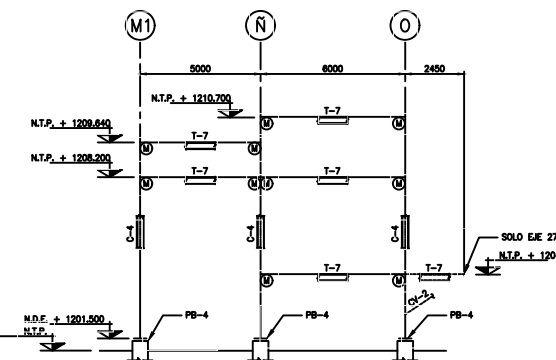
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ESC. 1:100



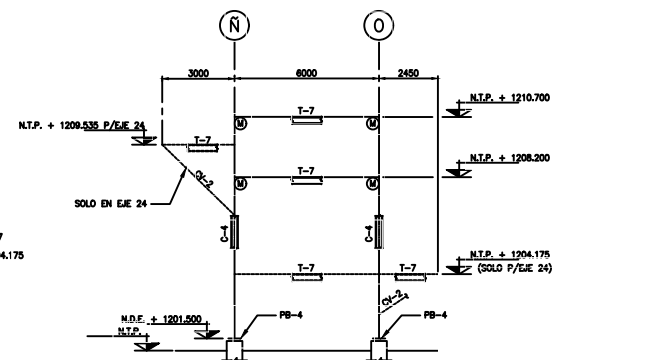
ELEVACION MARCOS EJES N Y 3  
ESC. 1:100



ELEVACION MARCOS EJES DEL 5 AL 21  
ESC. 1:100



ELEVACION MARCOS EJES 27 Y 28  
ESC. 1:100



ELEVACION MARCOS EJES 23 Y 24  
ESC. 1:100

**NOMENCLATURA**

- C.S. CUERDA SUPERIOR
- C.I. CUERDA INFERIOR
- A. ARBOLERA
- P.B. PLACA BASE
- S. SANGRILLO
- M. MONTANTE
- HT. HUEL PISO TERMINADO
- N.T.P. NIVEL TOPE DE PERFILES
- N.D.E. NIVEL DESPLANTE DE ESTRUCTURA
- C. COLUMNA
- T. TRINCHET
- C.H. CONTRALIBRO HORIZONTAL
- C.V. CONTRALIBRO VERTICAL
- C.S. CORDON A SUJETOS
- J.E. JUNTA DE EXPANSION
- R. REVISION

**NOTAS:**

- 1.- PARA NOTAS GENERALES, ABREVIATURAS Y SIMBOLOGIA VER PLANO OFF-PM27005-SRJC-PC-0878.
- 2.- DIMENSIONES EN METROS.
- 3.- NIVELES Y COORDENADAS EN METROS.
- 4.- TODAS LAS DIMENSIONES SON SIMETRICAS CON RESPECTO A LAS LINEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
- 5.- ACERO ESTRUCTURAL ASTA A-36.
- 6.- FUNDACIONES TIPO A-305 TIPO 1, GARANTIZADO A TIPO 3.
- 7.- ESTE PLANO ES DE DISEÑO Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE SALIDA QUE CUMPLAN CON LOS REQUISITOS DEL ASO-LRUF Y AIS COMO ISOMAC.
- 8.- LA DESCRIPCION DE LOS PERFILES CORRESPONDE CON LA INDICADA EN EL "MANUAL DE CONSTRUCCION EN ACERO" DEL INSTITUTO MEXICANO DE LA CONSTRUCCION EN ACERO (IMEA).
- 9.- NO TENER MEDIDAS A ESCALA, LAS CORTES DEBEN AL DIBUJO.
- 10.- TENER EN ESTE PLANO EN CONSULTA CON LOS PLANOS DE REFERENCIA.
- 11.- VER CONEXIONES A CORRIENTE Y A MOMENTO DE LA ESTRUCTURA PRINCIPAL EN PLANO PF-0811.

**PLANOS DE REFERENCIA**

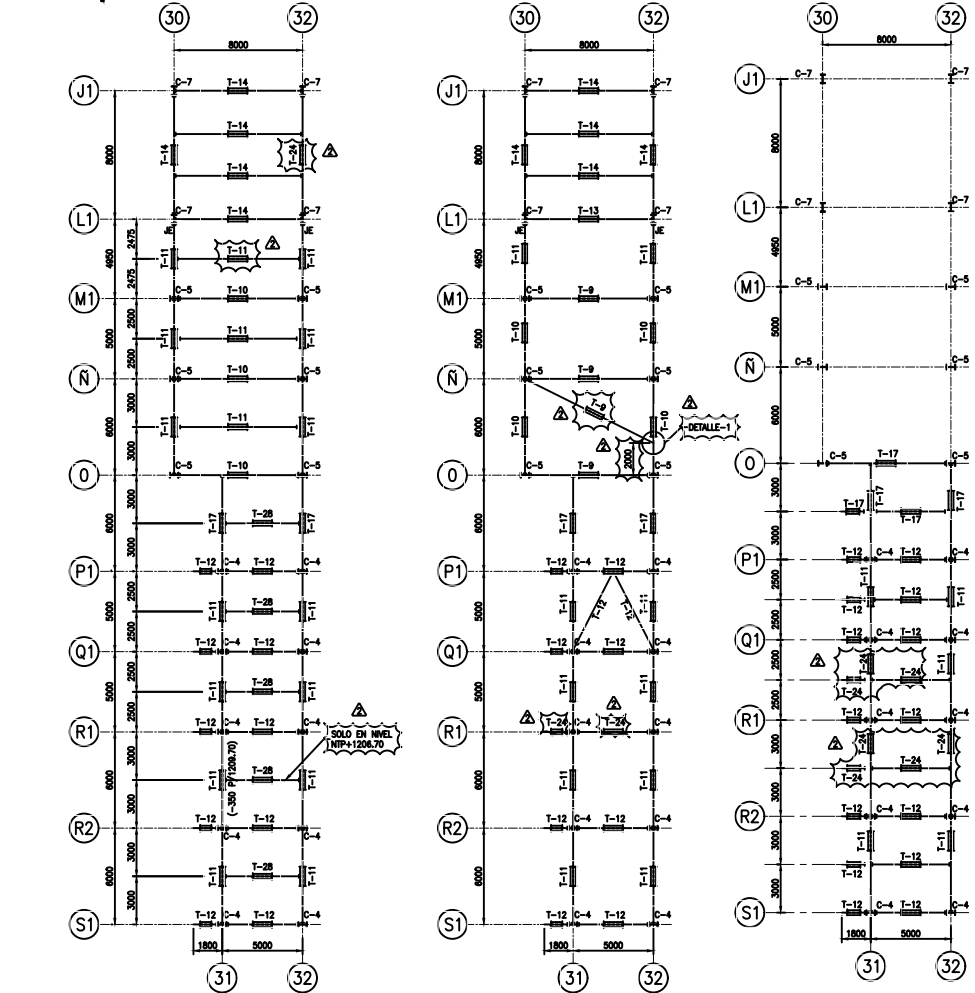
- PM27005-SRJC-PF-0803 RACK DE TUBERIAS-ESTRUCTURA PLANTAS
- PM27005-SRJC-PC-0805 RACK DE P.T.A. Y RACK DE SERVICIOS
- PM27005-SRJC-PC-0806 RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
- PM27005-SRJC-PC-0808 RACK AEROCONDENSADOR
- PM27005-SRJC-PC-0809 RACK DE TUBERIAS-ESTRUCTURA PLANTAS
- PM27005-SRJC-PF-0807 RACK PRINCIPAL Y RACK TREN 1 Y 2
- PM27005-SRJC-PF-0810 RACK DE TUBERIAS-ESTRUCTURA ELEVACION
- PM27005-SRJC-PF-0811 RACK PRINCIPAL Y RACK TREN 1 Y 2
- PM27005-SRJC-PF-0808 RACK DE TUBERIAS-ESTRUCTURA ELEVACIONES
- PM27005-SRJC-PF-0811 RACK PRINCIPAL
- PM27005-SRJC-PF-0811 RACK DE TUBERIAS-ESTRUCTURA CONEXIONES TIPO Y PLANOS SERVO
- PM27005-SRJC-PF-0812 RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 1
- PM27005-SRJC-PF-0813 RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 2
- PM27005-SRJC-PF-0814 RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 3

2	25/10/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
1	24/10/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
0	31/08/11	PARA REVISION INTERNA		
REV.	FECHA	DESCRIPCION	REVISOR	APROB.

**REVISIONES**

PROYECTO:		PLANTA DE CICLO COMBINADO	
TITULO:		RACK DE TUBERIAS ESTRUCTURA ELEVACION RACK P.T.A. Y RACK DE SERVICIOS	
APROBADO:	REALIZADO:	ESCALA:	INDICADA:
COMPROBADO:	FECHA:	DOCUMENTO DE PROYECTO:	REV. NO.
IMPORTANTE:		DOCUMENTO DE PROYECTO:	REV. NO.
NO. DE CONTROL:		DOCUMENTO DE PROYECTO:	REV. NO.

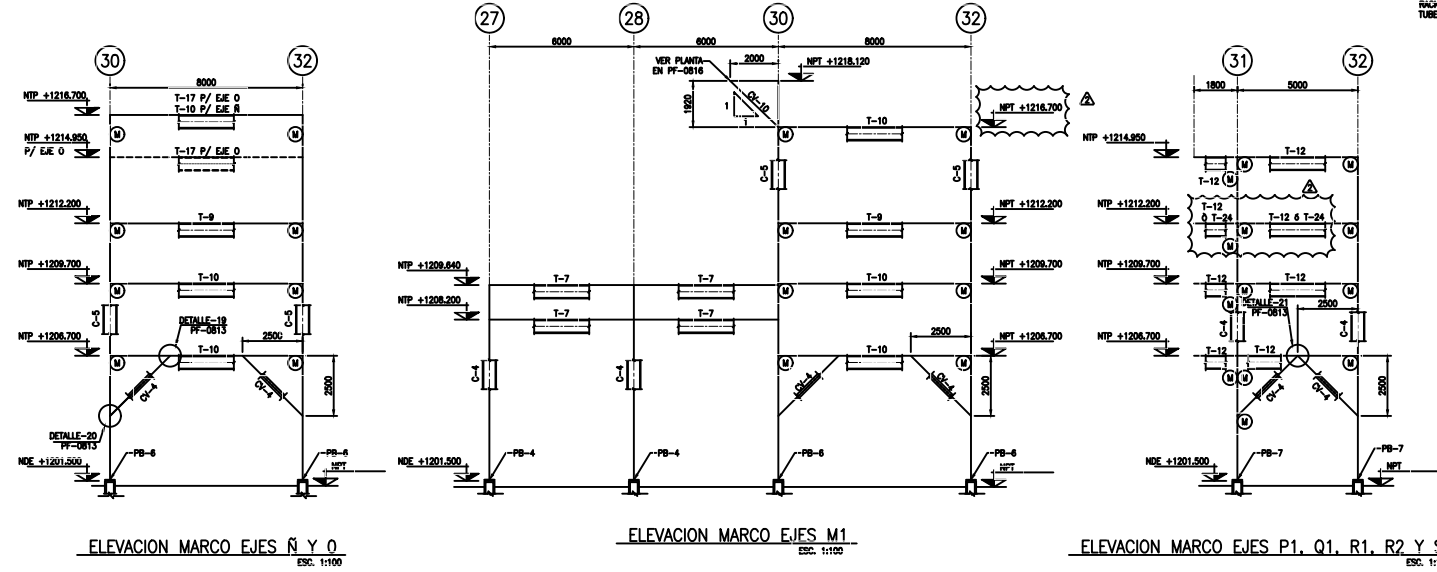
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2	<input type="checkbox"/> CONFORME SIN COMENTARIOS
3	<input type="checkbox"/> CONFORME CON COMENTARIOS
4	<input type="checkbox"/> NO CONFORME
5	<input type="checkbox"/> TAL COMO SE CONSTRUYO
FECHA DE RECEPCION: Revisado por Ing. Supervisor	
Fecha:	
NO. DE CONTROL:	



PLANTA NIVELES NTP +1206.70 Y N.T.P. 1209.70 ESC. 1:100

PLANTA NIVEL NTP +1212.20 ESC. 1:100

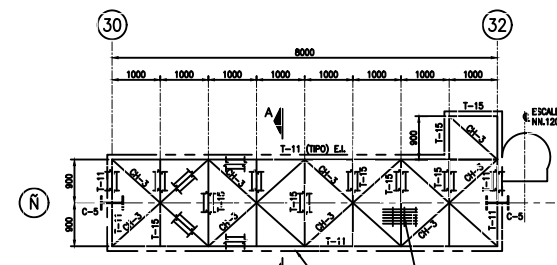
PLANTA NIVEL NTP +1214.950 ESC. 1:100



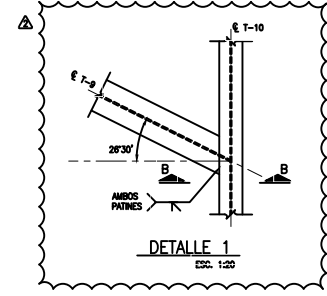
ELEVACION MARCO EJES N Y O ESC. 1:100

ELEVACION MARCO EJES M1 ESC. 1:100

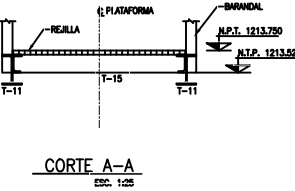
ELEVACION MARCO EJES P1, Q1, R1, R2 Y S1 ESC. 1:100



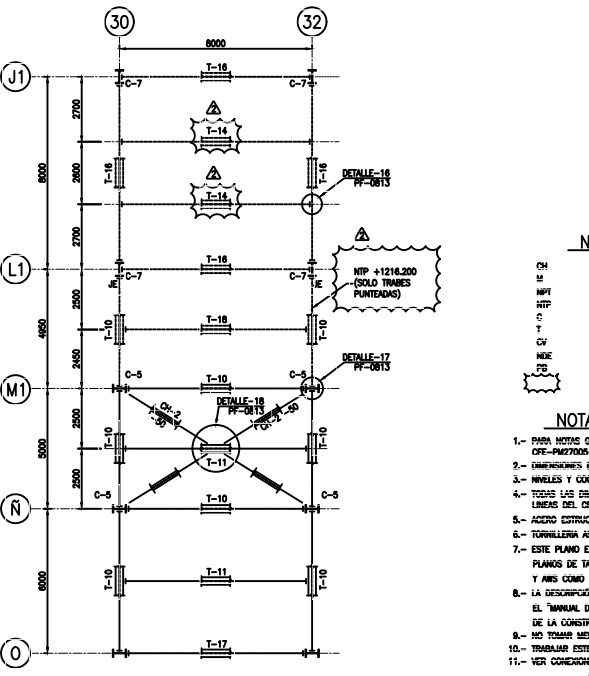
PLANTA PLATAFORMA N.P.T. 1213.750 ESC. 1:50



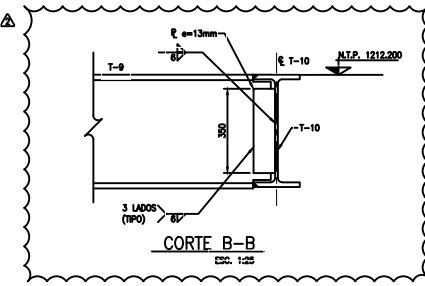
DETALLE 1 ESC. 1:25



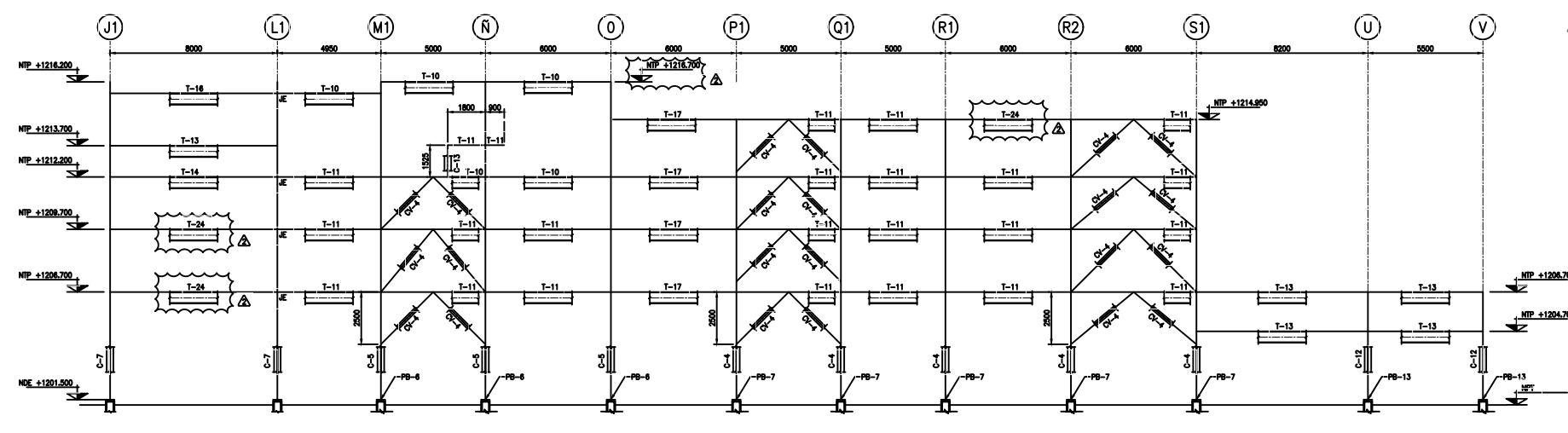
CORTE A-A ESC. 1:25



PLANTA NIVEL NTP +1216.700 ESC. 1:100



CORTE B-B ESC. 1:25



ELEVACION MARCO 32 ESC. 1:100

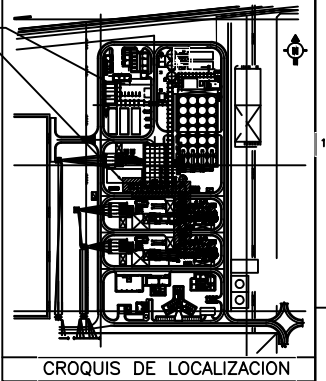


Table with 4 columns: MARCA, P.F.R.F.L., PESO S/m, and other technical specifications for various structural profiles.

NOMENCLATURA

- List of symbols and their corresponding descriptions for structural elements like horizontal/vertical connections, levels, and plates.

NOTAS:

- Series of numbered notes providing general instructions, dimensions, and references for the construction of the structure.

PLANOS DE REFERENCIA

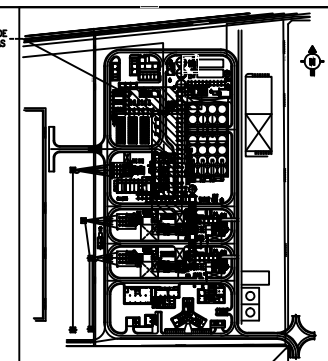
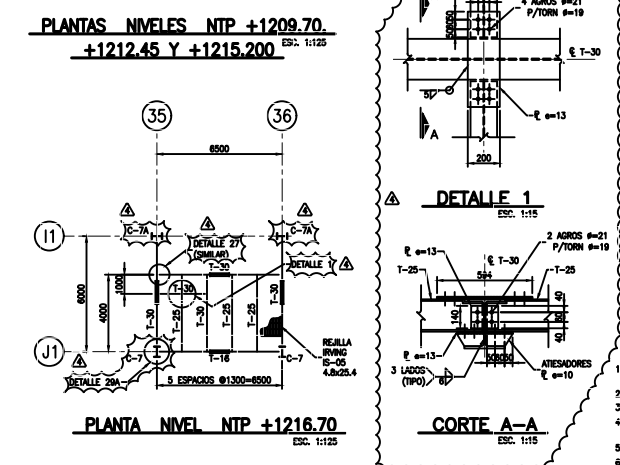
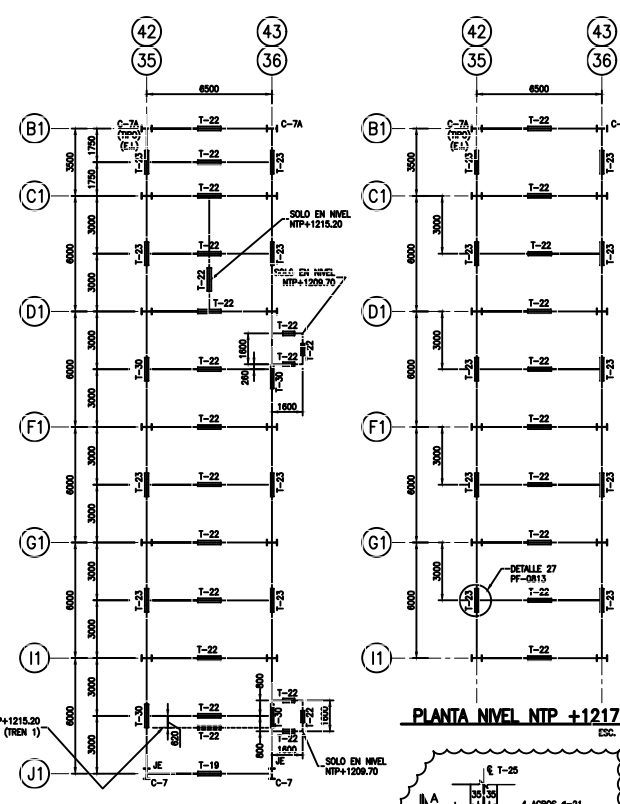
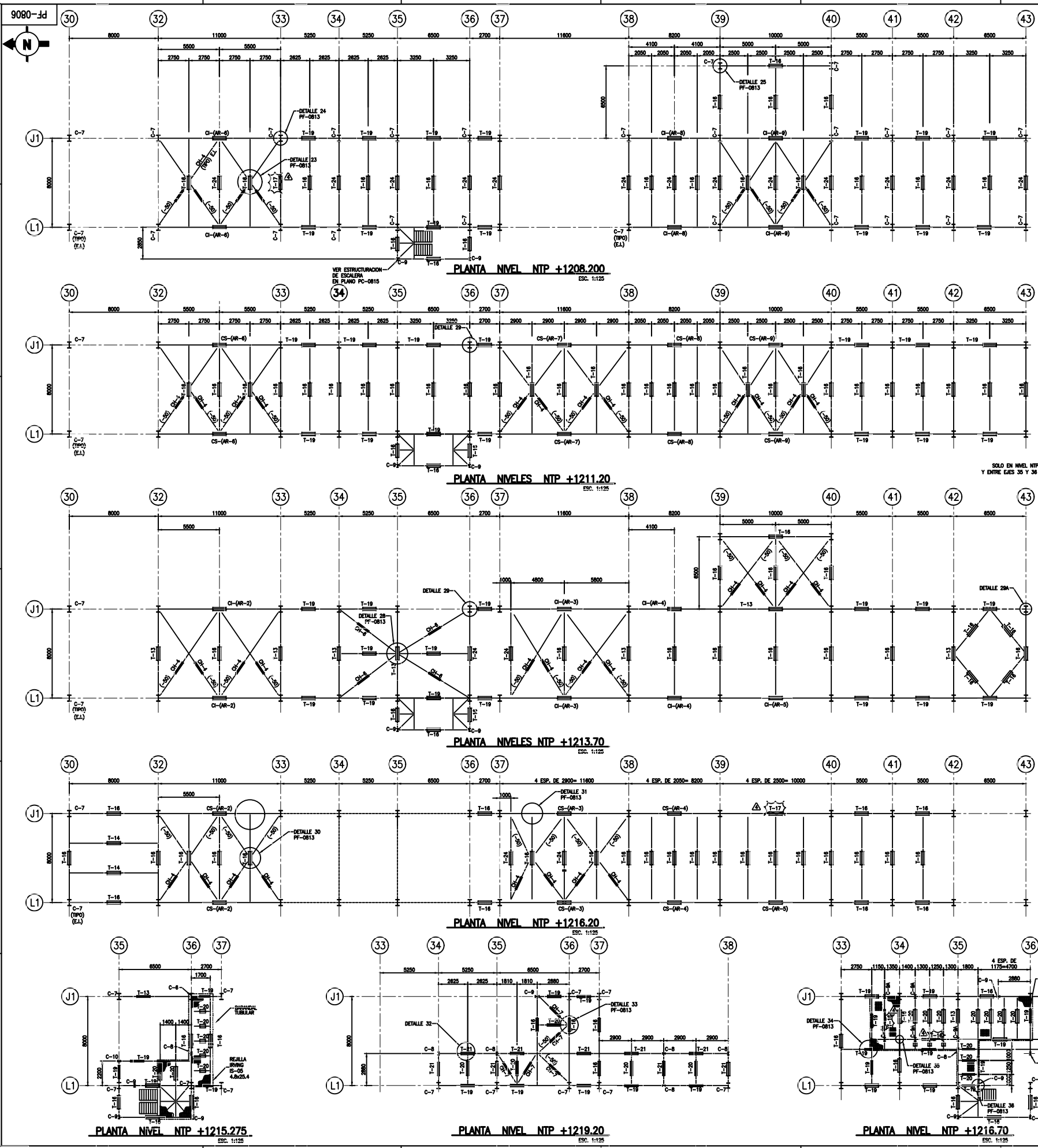
- List of reference drawings (e.g., PM27005-SRUC-PF-0803) and their descriptions, including rack and elevation drawings.

Revision table with columns for revision number, date, and description of changes.

Form for 'ESTADO DE DOCUMENTO COMO SE ANOTA' with checkboxes for 'SOLO PARA INFORMACION', 'CONFORME SIN COMENTARIOS', etc.

Form for 'FECHA DE RECEPCION' and 'NO. DE CONTROL' with fields for date, supervisor, and control number.

Project information form including 'PROYECTO', 'TITULO', 'RACK DE TUBERIAS - ESTRUCTURA PLANTAS Y ELEVACIONES RACK AEROCONDENSADOR', and 'PM27005-SRJC-PF-0805'.



**TABLA DE PERFILES**

MARKA	PERFIL	PESO K/m	d	M	W	W <sub>y</sub>
C-7	RL-357	128.70	487	201	18.26	12.16
C-7A	RL-357	124.20	472	203	22.26	12.59
C-7B	RL-305	98.81	303	203	13.1	7.5
C-8	RL-254	44.8	286	148	13.0	7.8
C-9	RL-305	74.40	310	205	18.3	9.4
C-9A	RL-305	92.2	318	187	13.2	7.8
C-10	RL-203	31.2	210	134	10.2	6.4
T-18	RL-305	74.40	310	205	18.3	9.4
T-20	RL-203	52.8	303	204	14.8	8.5
T-13	RL-304	101.3	357	255	18.3	10.5
T-32	RL-305	74.40	310	205	18.3	9.5
T-19	RL-305	92.2	318	187	13.2	7.8
T-20	RL-203	31.2	210	134	10.2	6.4
T-21	RL-254	44.8	286	148	13.0	7.8
T-22	RL-305	98.8	303	203	13.1	7.5
T-23	RL-305	92.8	303	204	14.8	8.5
T-24	RL-305	98.7	308	205	15.4	8.2
T-25	RL-203	35.9	201	185	10.2	6.2
CH-4	RL-203	31.2	210	134	10.2	6.4
CH-5	RL-254	44.8	286	148	13.0	7.8
CH-8	RL-203	31.2	210	134	10.2	6.4
CH-9	RL-305	44.5	313	188	11.8	8.1
CH-7	RL-78x10	21.44				
CH-7	RL-78x10	21.44				

**ARMADURAS AR-2, AL AR-6, AR-8, y AR-9**

MARKA	PERFIL	PESO K/m	d	M	W	W <sub>y</sub>
C.L.	RL-305	74.4	310	205	18.3	9.4
C.L.	RL-305	92.2	318	205	18.3	9.4
D	LL-84x10	11.16				
M	RL-152	24	100	102	10.3	6.8

**ARMADURAS AR-7**

MARKA	PERFIL	PESO K/m	d	M	W	W <sub>y</sub>
C.L.	RL-305	74.4	310	205	18.3	9.4
C.L.	RL-305	92.2	318	187	13.2	7.8
D	LL-84x10	11.16				
M	RL-152	24	100	102	10.3	6.8

- NOMENCLATURA**
- C.S. CUBIERTA SUPERIOR
  - C.L. CUBIERTA INFERIOR
  - AR ARMADURA
  - PF PLACA BASE
  - D DIBUJO
  - SI SOSTITUTO
  - SPF NIVEL PISO TERMINADO
  - NTP NIVEL TOPE DE PISO
  - SEC NIVEL SECANTE DE ESTRUCTURA
  - C COLUMNA
  - T TRINCHERO
  - CH COMENTARIO HORIZONTAL
  - CV COMENTARIO VERTICAL
  - CONEXION A MOEDRO CONEXION A MOEDRO
  - EXCEPTO INDICADO
  - REVISION
  - (-50) -50mm ABAJO DEL NTP

- NOTAS:**
- 1.- PARA NOTAS GENERALES, ABREVIATURAS Y SIMBOLOGIA VER PLANO CPE-PM27005-SRJC-PF-0815.
  - 2.- DIMENSIONES EN MILIMETROS.
  - 3.- NIVELES Y CONCORDANCIAS EN METROS.
  - 4.- TODAS LAS DIMENSIONES SON QUATRO CON RESPECTO A LAS LINEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
  - 5.- ACERO ESTRUCTURAL ASTM A-36.
  - 6.- TORNERILERIA ASTM A-325 TIPO 1 GALVANIZADO 8 TIPO 3.
  - 7.- ESTE PLANO ES DE DISEÑO Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE TALLER QUE CUMPLAN CON LOS REQUISITOS DEL ASO-URDF Y AIS COMO MINIMO.
  - 8.- LA DESCRIPCION DE LOS PERFILES CORRESPONDE CON LA INDICADA EN EL "MANUAL DE CONSTRUCCION EN ACERO" DEL INSTITUTO MEXICANO DE LA CONSTRUCCION EN ACERO (IMCA).
  - 9.- NO TOMAR MEDIDAS A ESCALA. LAS COTAS SIEMPRE AL DIBUJO.
  - 10.- TRABAJAR ESTE PLANO EN CONJUNTO CON LOS PLANOS DE REFERENCIA.
  - 11.- VER CONDICIONES A GENERAL Y A MONEDRO DE LA ESTRUCTURA PRINCIPAL EN PLANO PF-0811.
  - 12.- LOS CONTRANTES HORIZONTALES SE PONDRAN 50mm ABAJO DEL NTP.

**PLANOS DE REFERENCIA**

PM27005-SRJC-PF-0803	RACK DE TUBERIAS-ESTRUCTURA PLANTAS
PM27005-SRJC-PF-0804	RACK DE P.T.A. Y RACK DE SERVICIOS
PM27005-SRJC-PC-0805	RACK DE TUBERIAS-ESTRUCTURA PLANTAS
PM27005-SRJC-PF-0807	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELECCIONES
PM27005-SRJC-PF-0808	RACK HERMOSEADOR
PM27005-SRJC-PF-0811	RACK PRINCIPAL Y RACK TREN 1 Y 2
PM27005-SRJC-PF-0812	RACK DE TUBERIAS-ESTRUCTURA ELECCIONES
PM27005-SRJC-PF-0813	RACK DE TUBERIAS-ESTRUCTURA CONEXIONES TIPO Y PLACAS BASE
PM27005-SRJC-PF-0814	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 1
PM27005-SRJC-PF-0815	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 2
PM27005-SRJC-PF-0816	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 3

**ESTADO DE DOCUMENTO COMO SE ANOTA**

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3	<input type="checkbox"/>	CONFORME CON COMENTARIOS
4	<input type="checkbox"/>	NO CONFORME
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FECHA DE RECEPCION: Revisado por Ing. Supervisor  
Fecha: \_\_\_\_\_

NO. DE CONTROL: \_\_\_\_\_

PROYECTO: PLANTA DE CICLO COMBINADO

TITULO: RACK DE TUBERIAS ESTRUCTURA PLANTAS RACK PRINCIPAL Y RACK TREN 1 Y 2

APROBADO: \_\_\_\_\_

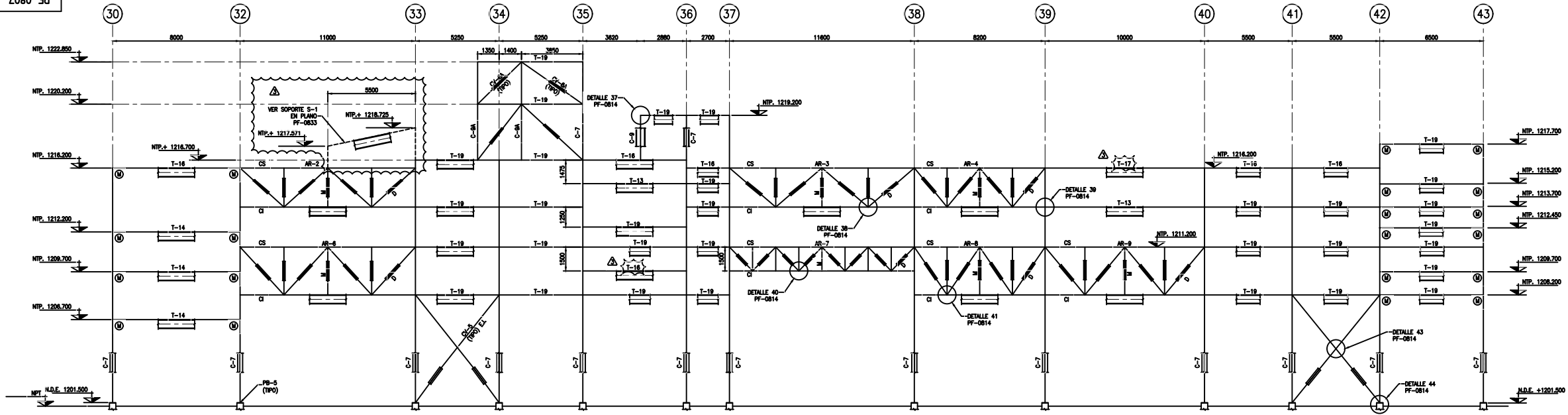
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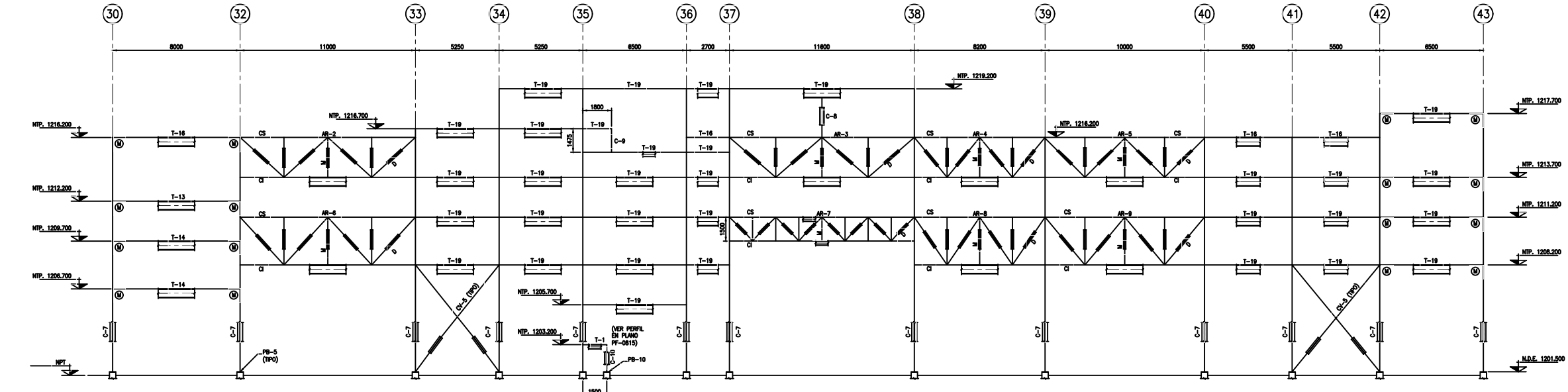
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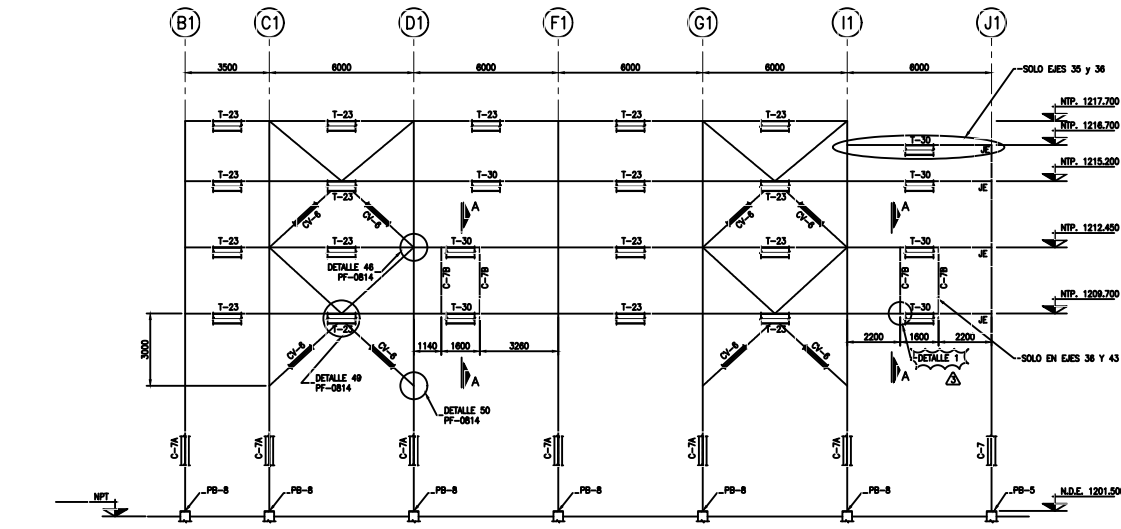
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2	25/12/11	APROBADO PARA CONSTRUCCION SE INCORPORAN COMENTARIOS DE CPE		
1	14/10/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
0	24/08/11	APROBADO PARA CONSTRUCCION		



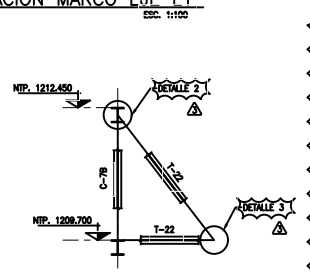
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ESC. 1:100



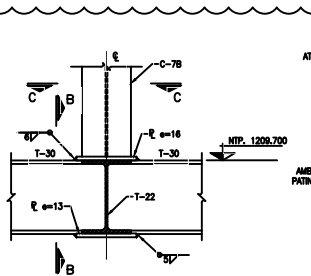
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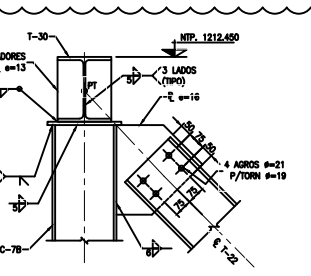
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ESC. 1:100



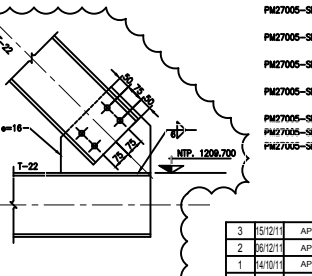
CORTE A-A  
ESC. 1:50



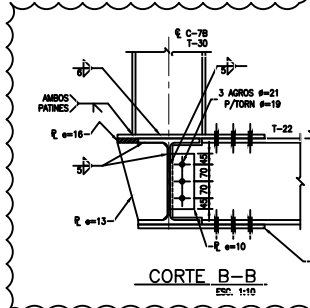
DETALLE 1  
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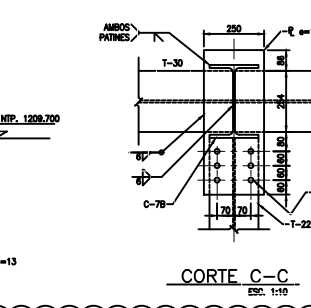
DETALLE 2  
ESC. 1:10



DETALLE 3  
ESC. 1:10



CORTE B-B  
ESC. 1:10



CORTE C-C  
ESC. 1:10

**NOMENCLATURA**

NDC	NIVEL DESPLANTE DE ESTRUCTURA
HPT	NIVEL PISO TERMINADO
NTP	NIVEL TOPE DE PERFIL
C	COLUMNA
T	TRUSS
CV	CONTELEVAMENTO
E.I.	EXCEPTO INDICADO
PT	PUNTO DE TRABAJO
JE	UNTA EXPANSION
⊗	CONEXION A MOMENTO
REVISION	REVISION

- NOTAS:**
- 1.- PARA NOTAS GENERALES, ABREVIATURAS Y SIMBOLOGIA VER PLANO OFE-PM27005-SRJC-PF-0807.
  - 2.- DIMENSIONES EN MILIMETROS.
  - 3.- NIVELES Y COORDENADAS EN METROS.
  - 4.- TODAS LAS DIMENSIONES SON SIMETRICAS CON RESPECTO A LAS LINEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
  - 5.- ACERO ESTRUCTURAL ASTM A-36.
  - 6.- TORNILLERIA ASTM A-325 TIPO 1 GALVANIZADO O TIPO 3.
  - 7.- ESTE PLANO ES DE DISEÑO Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE TALLER QUE CUMPLAN CON LOS REQUISITOS DEL ASISC-LRDF Y AISI COMO MINIMO.
  - 8.- LA DESCRIPCION DE LOS PERFILES CORRESPONDE CON LA INDICADA EN EL "MANUAL DE CONSTRUCCION EN ACERO" DEL INSTITUTO MEXICANO DE LA CONSTRUCCION EN ACERO (IMCA).
  - 9.- NO TOMAR MEDIDAS A ESCALA, LAS COPIAS DEBEN AL DIBUJO.
  - 10.- TRABAJAR ESTE PLANO EN COORDINACION CON LOS PLANOS DE REFERENCIA.

**PLANOS DE REFERENCIA**

PM27005-SRJC-PF-0803	RACK DE TUBERIAS-ESTRUCTURA PLANTAS
PM27005-SRJC-PF-0804	RACK DE P.T.A. Y RACK DE SERVICIOS
PM27005-SRJC-PF-0805	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PF-0806	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PF-0807	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PF-0808	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PF-0809	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PF-0810	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PF-0811	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PF-0812	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PF-0813	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PF-0814	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES

3	15/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
2	25/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
1	14/10	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
0	31/08	APROBADO PARA CONSTRUCCION		
REV.	FECHA	DESCRIPCION	REVISADO	APROBADO

PROYECTO: PLANTA DE CICLO COMBINADO

TITULO: RACK DE TUBERIAS ESTRUCTURA ELEVACIONES RACK PRINCIPAL Y RACK TREN 1 Y 2

APROBADO: [ ]  
COMPROBADO: [ ]

FECHA DE RECEPCION: [ ] Revisado por Ing. Supervisor [ ]  
Fecha: [ ]

IMPORTE: [ ]

NO. DE CONTROL: [ ]

REALIZADO: [ ]  
ESCALA: [ ]

ORDEN DE COMPRA: [ ]  
DOCUMENTO DE PROYECTO: [ ]  
DOCUMENTO ADMINISTRATIVO: [ ]

PM27005-SRJC-PF-0807

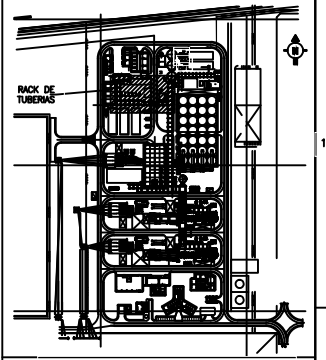
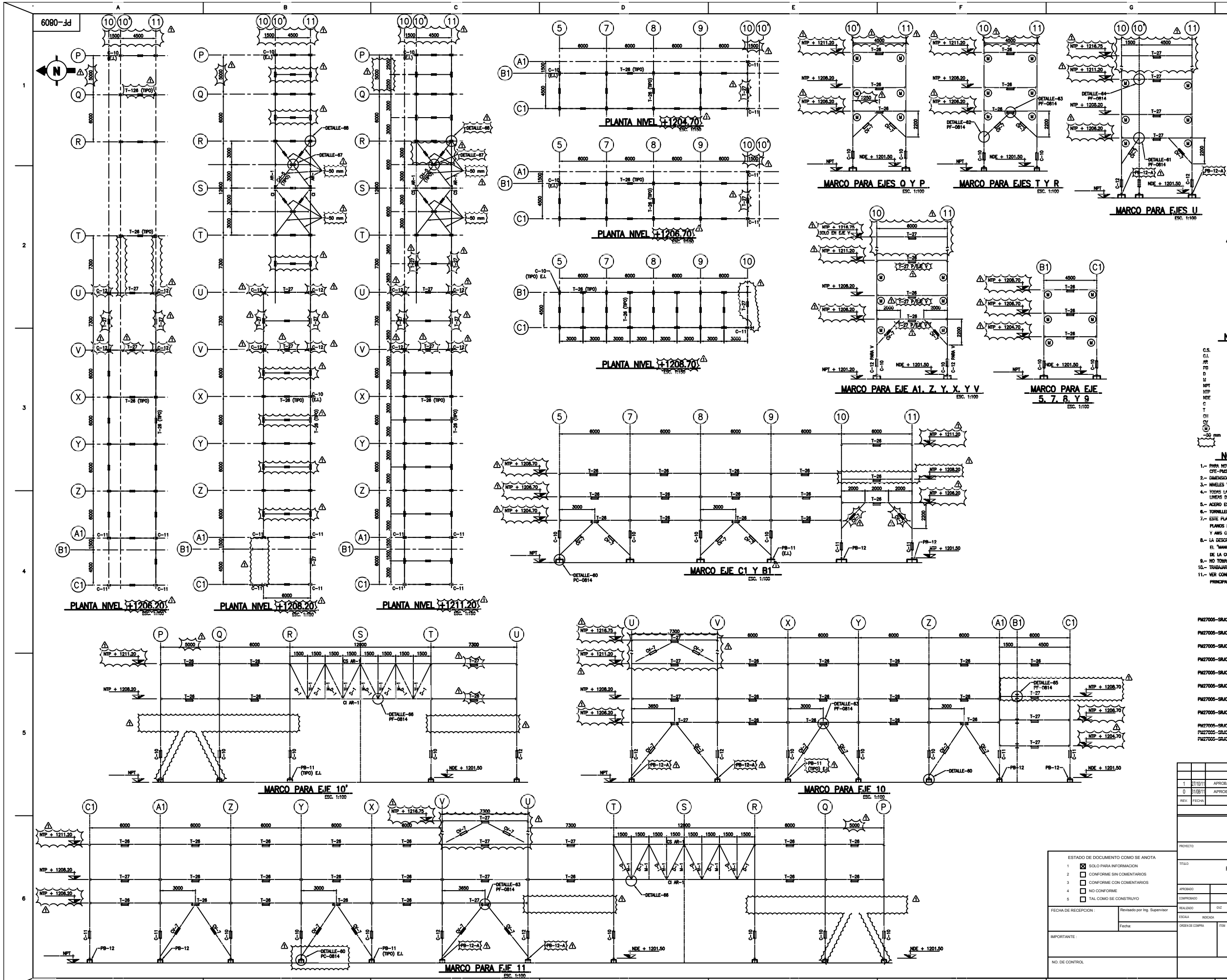
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ESTADO DE DOCUMENTO COMO SE ANOTA

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3	<input type="checkbox"/>	CONFORME CON COMENTARIOS
4	<input type="checkbox"/>	NO CONFORME
5	<input type="checkbox"/>	TAL COMO SE CONSTRUYO







**TABLA DE PERFILES**

MARCA	PERFIL	PESO k/m	d	h	tf	tr
C-10	RL-457	144.3	472	283	32.1	13.6
C-11	RL-457	172.6	482	298	35.1	15.6
C-12	RL-510	265.71	600	339	38.1	22.2
T-26	RL-308	78.0	354	205	16.8	9.4
T-27	RL-457	144.3	472	283	22.1	13.6
CH-7	RL-254	44.8	266	148	13.0	7.6

**AR-1**

MARCA	PERFIL	PESO k/m	d	h	tf	tr
CX-1	RL-308	78.0	354	205	14.8	9.4
CX-1	RL-308	78.0	354	205	14.8	9.4
CH-9	RL-203	28.8	207	133	8.4	5.8
B-1	RL-152	22.4	152	152	6.8	5.8
D-1	ZL-78x10	9.08				

- NOMENCLATURA**
- C.S. CUERDA SUPERIOR
  - C.I. CUERDA INFERIOR
  - AR ARMADURA
  - PL PLACA BASE
  - DI DIAGONAL
  - MO MONTANTE
  - NPT NIVEL TOPE DE PERFIL
  - NDE NIVEL DESPLAZE DE ESTRUCTURA
  - C COLUMNA
  - T TRASE
  - CH CONTRALIBRE HORIZONTAL
  - CHV CONTRALIBRE VERTICAL
  - CA CONEXION A ALCANTARILLO
  - FOR FOR DEBIDO DEL NIVEL TOPE DE ESTRUCTURA
  - REVISOR

- NOTAS:**
- 1.- PARA NOTAS GENERALES, ABRONCANTURAS Y SIMBOLOGIA VER PLANO OFE-PAUTOS-GRAC-PO-007A.
  - 2.- DIMENSIONES EN METROS.
  - 3.- NIVELES Y COORDENADAS EN METROS.
  - 4.- TODAS LAS DIMENSIONES SON SIMÉTRICAS CON RESPECTO A LAS LINEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
  - 5.- ACERO ESTRUCTURAL ASTM A-36.
  - 6.- TORNILLERÍA NOM A-325 TIPO 1 GALVANIZADOS A TIPO 3.
  - 7.- ESTE PLANO ES DE EJECUCIÓN Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE VALLER QUE CUMPLAN CON LOS REQUISITOS DEL ASO-LURF Y AIS COMO MÍNIMO.
  - 8.- LA DESCRIPCIÓN DE LOS PERFILES CORRESPONDE CON LA INDICADA EN EL "MANUAL DE CONSTRUCCIÓN EN ACERO" DEL INSTITUTO MEXICANO DE LA CONSTRUCCIÓN EN ACERO (IMCA).
  - 9.- NO TOMAR MEDIDAS A ESCALA. LAS COTAS ROJEAN AL DIBUJO.
  - 10.- TRABAJAR ESTE PLANO EN CONSULTA CON LOS PLANOS DE REFERENCIA.
  - 11.- VER CONEXIONES A CORTANTE Y A MOMENTO DE LA ESTRUCTURA PRINCIPAL EN PLANO PF-0811.

- PLANOS DE REFERENCIA**
- PM27005-SRJC-PF-0803 RACK DE TUBERIAS - ESTRUCTURA PLANTAS
  - PM27005-SRJC-PF-0804 RACK DE P.T.A Y RACK DE SERVICIOS
  - PM27005-SRJC-PC-0805 RACK DE TUBERIAS-ESTRUCTURA PLANTAS
  - PM27005-SRJC-PC-0806 RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
  - PM27005-SRJC-PC-0807 RACK ALMACENAMIENTO
  - PM27005-SRJC-PC-0808 RACK DE TUBERIAS-ESTRUCTURA PLANTAS
  - PM27005-SRJC-PF-0809 RACK PRINCIPAL Y RACK TREN 1 Y 2
  - PM27005-SRJC-PF-0810 RACK DE TUBERIAS-ESTRUCTURA ELEVACION
  - PM27005-SRJC-PF-0811 RACK PRINCIPAL Y RACK TREN 1 Y 2
  - PM27005-SRJC-PF-0812 RACK DE TUBERIAS-ESTRUCTURA ELEVACIONES
  - PM27005-SRJC-PF-0813 RACK PRINCIPAL
  - PM27005-SRJC-PF-0814 RACK DE TUBERIAS-ORIENTACION RACK DEL AEROREFRIGERADOR AUXILIAR
  - PM27005-SRJC-PF-0815 RACK DE TUBERIAS-ESTRUCTURA CONEXIONES TIPO Y PLACAS BASE
  - PM27005-SRJC-PF-0816 RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 1
  - PM27005-SRJC-PF-0817 RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 2
  - PM27005-SRJC-PF-0818 RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 3

1	27/10/11	APROBADO PARA CONSTRUCCION REVISADO SEGUN DONDE SE INDICA
0	31/08/11	APROBADO PARA CONSTRUCCION
REV.	FECHA	DESCRIPCION
REVISIONES		

ESTADO DE DOCUMENTO COMO SE ANOTA

- 1  SOLO PARA INFORMACION
- 2  CONFORME SIN COMENTARIOS
- 3  CONFORME CON COMENTARIOS
- 4  NO CONFORME
- 5  TAL COMO SE CONSTRUYO

FECHA DE RECEPCION: Revisado por Ing. Supervisor

IMPORTEANTE:

NO. DE CONTROL:

PROYECTO: PLANTA DE CICLO COMBINADO

TITULO: RACK DE TUBERIAS ESTRUCTURA RACK DEL AEROREFRIGERADOR AUXILIAR

REALIZADO: [ ]

ESCALA: [ ]

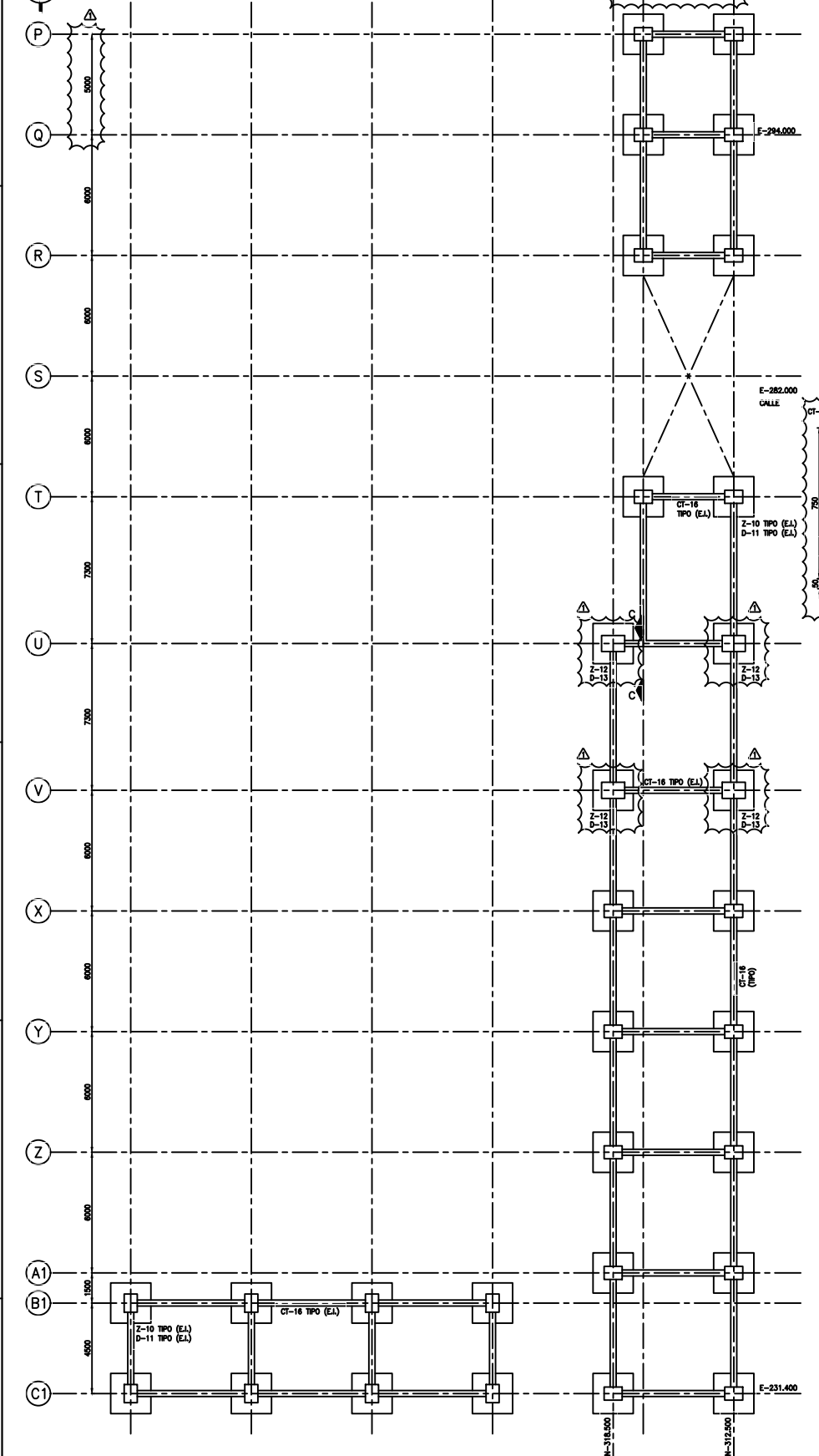
ORDEN DE COMPRA: [ ]

DOCUMENTO DE PROYECTO: [ ]

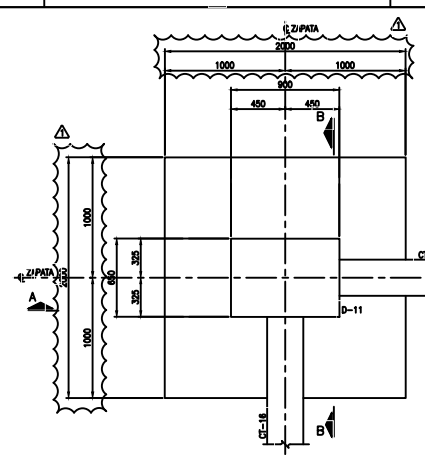
DOCUMENTO SUBADMINISTRACION: [ ]

REV. NO. 1

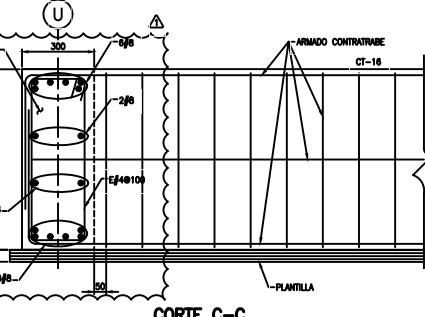
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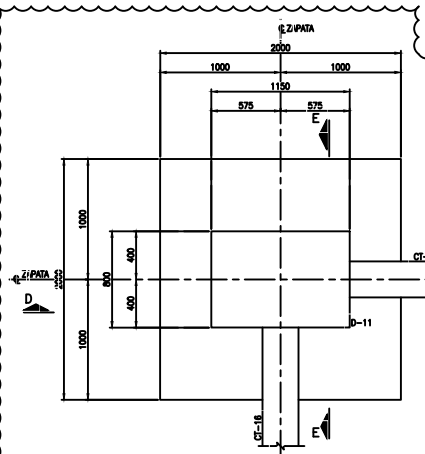
**PLANTA DE CIMENTACION**  
ESC. 1:100



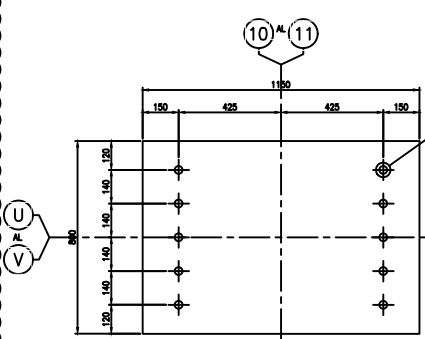
**ZAPATA Z-10**  
(VER NOTA 7) ESC. 1:20



**CORTE C-C**  
ESC. 1:10

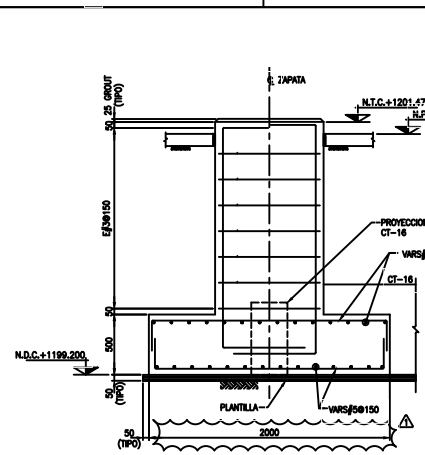


**ZAPATA Z-12**  
(VER NOTA 7) ESC. 1:20

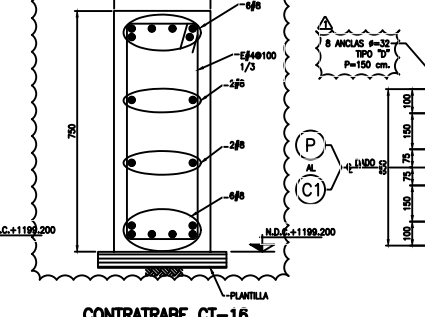


**LOCALIZACION DE ANCLAS**  
ESC. 1:10

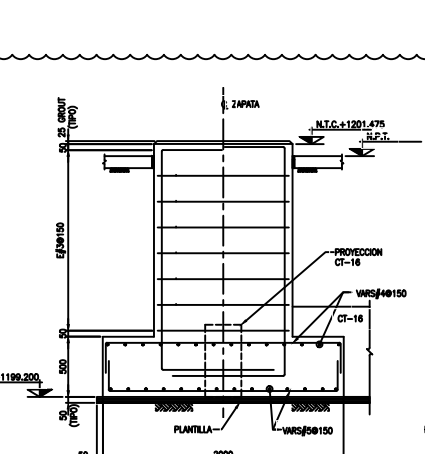
**DADO D-13**  
ESC. 1:10



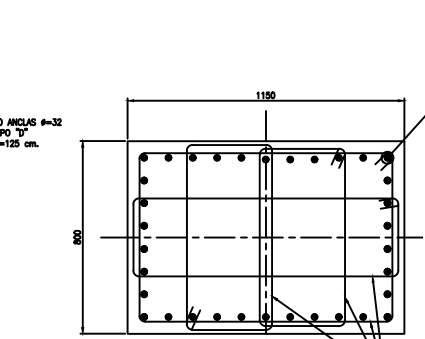
**CORTE A-A**  
ESC. 1:20



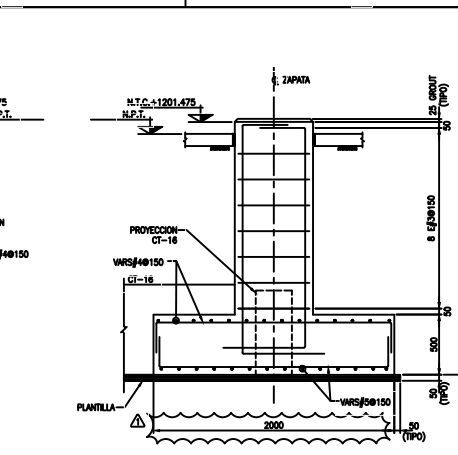
**CORTE B-B**  
ESC. 1:20



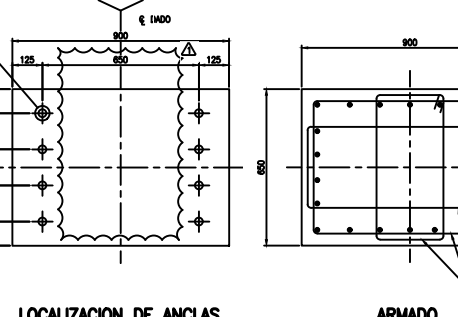
**CORTE D-D**  
ESC. 1:20



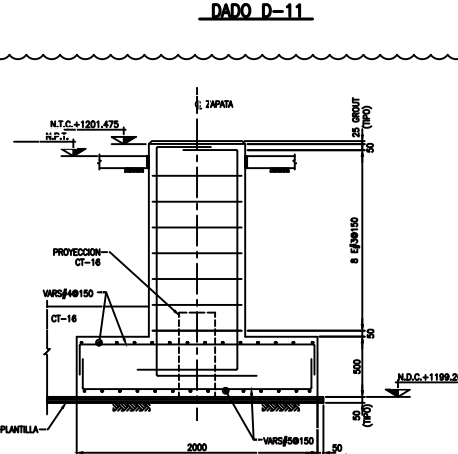
**ARMADO DADO D-13**  
ESC. 1:10



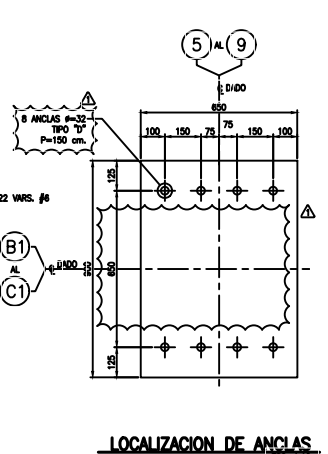
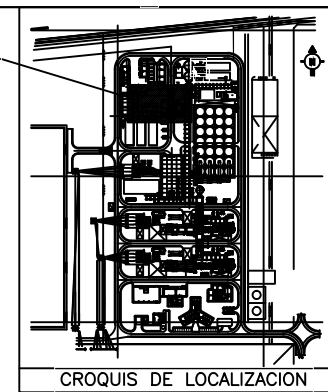
**CORTE E-E**  
ESC. 1:20



**ARMADO**  
ESC. 1:10



**LOCALIZACION DE ANCLAS**  
ESC. 1:10



**LOCALIZACION DE ANCLAS**  
ESC. 1:10

**NOMENCLATURA:**

- Z ZAPATA
- CT. CONTRATRABE
- D DADO
- N.D.C. NIVEL DESPLANTE DE CIMENTACION
- N.P.T. NIVEL DE PISO TERMINADO
- N.T.G. NIVEL TOPE DE CONCRETO
- E.I. EXCEPTO INDICADO

**CALIDAD DE MATERIALES:**

- CONCRETO PARA ZAPATAS Y DADOS  $f_c = 24.5 \text{ MPa} = 250 \text{ kg/cm}^2$
- CONCRETO PARA PLANTILLA  $f_c = 9.8 \text{ MPa} = 100 \text{ kg/cm}^2$
- ACERO DE REFUERZO  $f_y = 412 \text{ MPa} = 4200 \text{ kg/cm}^2$
- ANCLAS A-30

**NOTAS:**

- 1.- DIMENSIONES EN MILIMETROS.
- 2.- NIVELES EN METROS.
- 3.- LAS COTAS DE NIVEL ESTAN REFERIDAS AL PUNTO DE REFERENCIA CON EL SISTEMA DE COORDENADAS LOCALES.
- 4.- VER ANCLAS Y TRAVESAPES DE VARILLAS EN PLANO OPE-PM27005-SRJC-PC-0885.
- 5.- VER NOTAS GENERALES PARA ESTRUCTURA DE CONCRETO EN PLANO: OPE-PM27005-SRJC-PC-0870.
- 6.- TRAZAR ESTE PLANO EN COORDINADO CON LOS PLANOS DE REFERENCIA.
- 7.- LA ORIENTACION DEL DADO SE REGULARA DE ACUERDO A LA ORIENTACION DE LA COLUMNA Y PLACA BASE VER PLANO PC-0811.

**PLANOS REFERENCIA:**

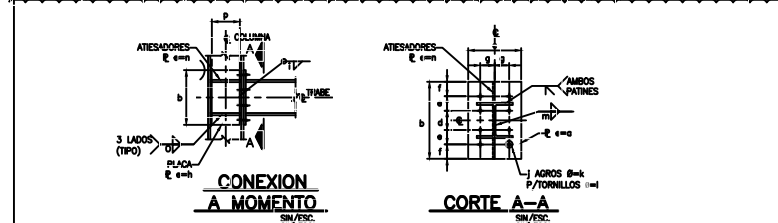
- PM27005-SRJC-PC-0880 RACK DE TUBERIAS CIMENTACION PLANTAS 1 DE 2
- PM27005-SRJC-PC-0881 RACK DE TUBERIAS CIMENTACION PLANTAS 2 DE 2
- PM27005-SRJC-PC-0870 NOTAS GENERALES DE CONCRETO
- PM27005-SRJC-PC-0885 DETALLES TIPO DE ANCLAS
- PM27005-SRJC-PC-0886 DETALLES TIPO DE DOBLEZ DE VARILLAS

ESTADO DE DOCUMENTO COMO SE ANOTA	
1	<input checked="" type="checkbox"/> SOLO PARA INFORMACION
2	<input type="checkbox"/> CONFORME SIN COMENTARIOS
3	<input type="checkbox"/> CONFORME CON COMENTARIOS
4	<input type="checkbox"/> NO CONFORME
5	<input type="checkbox"/> TAL COMO SE CONSTRUYO
FECHA DE RECEPCION: Revisado por Ing. Supervisor	
IMPORTEANTE:	Fecha:
NO. DE CONTROL:	

1	27/10/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA	REVISOR	APROBADO
0	23/08/11	APROBADO PARA CONSTRUCCION	REVISOR	APROBADO
REVISIONES				
PROYECTO: PLANTA DE CICLO COMBINADO				
TITULO: RACK DE TUBERIAS CIMENTACION RACK DEL AEROFREIADOR AUXILIAR				
APROBADO	REALIZADO	ESCALA	INDICADA	TITULO
COMPROBADO	ESCALA	INDICADA	INDICADA	RACK DE TUBERIAS CIMENTACION RACK DEL AEROFREIADOR AUXILIAR
ORDEN DE COMPRA	ITEM	DOCUMENTO DE PROYECTO No.	REV. No.	
		PM27005-SRJC-PF-0810	1	
		DOCUMENTO SUBASTADOR No.	REV. No.	

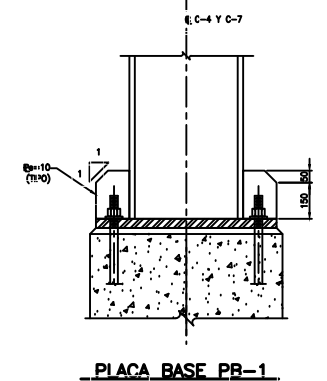
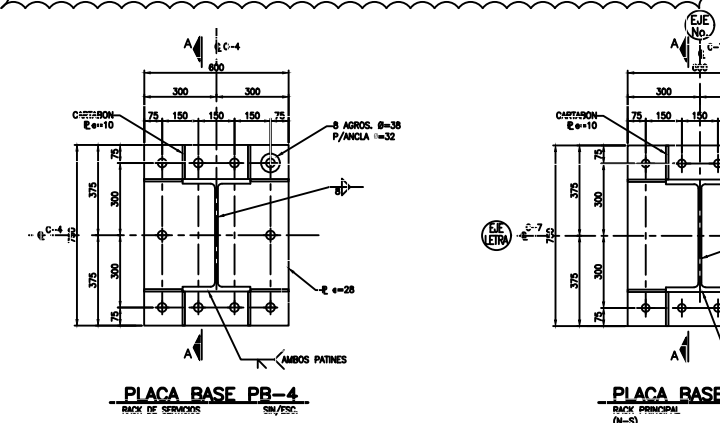
### CONEXIONES A MOMENTO EN MARCOS PRINCIPALES

COLUMNA	TIRANTE	a	b	c	d	e	f	g	h	i	j	k	l	m	n	o	p	OBSERVACIONES
IR 305x50.20	IR 254x38.5	19	200	203	150	70	20	45										EJES DEL C AL K
IR 305x79	IR 254x38.5	22	400	254	140	90	40	70	25	5	8	23	22	5	18	5	213	EJES DEL L AL O Y EAE N
IR 305x110.4	IR 305x74.4	28	500	256	200	100	50	70										EJES DEL S AL 22
IR 457x144.30	IR 335x94.5	29	550	280	210	100	70	70										EJES DEL 23 AL 28
IR 457x128.10	IR 305x74.4	29	500	280	210	100	70	70										EJES DEL P1 AL S1 Y L65 DEL 23 AL 28
IR 457x144.30	IR 457x144.30	29	700	280	330	115	70	70										EJES J1 Y L1
IR 457x105.3	IR 305x74.4	25	500	200	185	100	52.5	70										EJES DEL M1 AL O
IR 457x128.10	IR 305x74.4	25	500	205	185	100	52.5	70										EJES DEL 32 AL 43
IR 457x144.30	IR 305x59.8	25	500	280	180	100	70	70										EJES DEL 32 AL 43
IR 457x128.10	IR 305x101.3	32	800	281	320	120	70	70	18	8	8	38	32	6	16	6		EJES DEL M1 AL J1
IR 457x144.30	IR 335x79.0	32	800	270	210	120	70	70										EJES DEL 33 AL 38
IR 457x144.30	IR 457x144.30	32	750	280	320	120	70	70										EJES DEL 33 AL 38
IR 610x285	IR 457x144.30	32	800	280	450	140	70	70										EJES DEL 33 AL 38
IR 457x128.10	IR 305x67.7	25	500	310	210	130	70	70	18	8	8	30	28	6	18	6		EJES DEL 33 AL 38
IR 457x128.10	IR 305x62.2	25	500	280	210	100	70	70										EJES DEL 33 AL 38

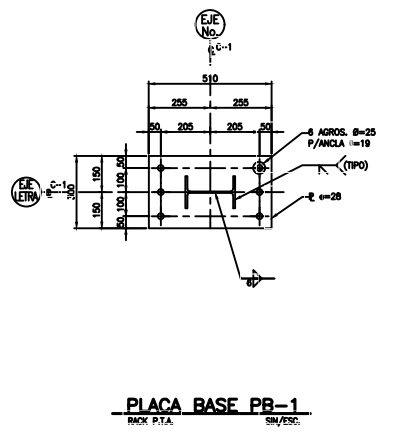


### CONEXIONES A CORTANTE EN MARCOS PRINCIPALES

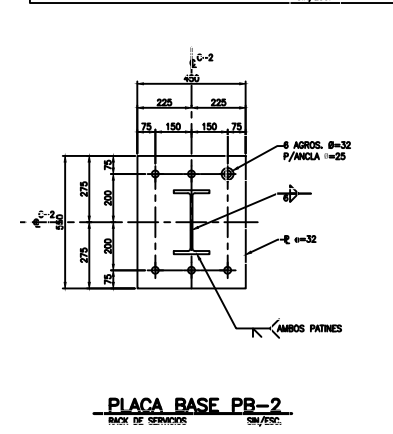
COLUMNA	TIRANTE	a	b	c	d	e	f	g	h	i	k	l	Observaciones
IR 305x59.8	IR 254x38.5	13	200	50	50	3	18	16	6	50	50		EJES 1 Y 2 (INV. 1204.700)
IR 305x79	IR 254x38.5	13	190	35	60	3	24	22	6	50	50		EJES 1 Y 2
IR 305x110.40	IR 305x52.20	13	230	45	70	3	18	16	6	50	50		EJES R, C, DEL 5 AL 22 (INV. 1205.700)
IR 457x144.30	IR 305x44.5	13	230	45	70	3	24	22	6	50	50		EJES R Y O DEL 5 AL 22 (INV. 1)
IR 457x144.30	IR 305x74.4	13	230	45	70	3	21	19	6	50	50		EJES 35, 36, 42 Y 43 TIENEN 1 Y 2 DEL 22 AL 30
IR 457x144.30	IR 305x59.8	13	240	40	75	3	22	20	6	50	50		EJES 36, 38, 42 Y 43 TIENEN 1 Y 2 DEL 22 AL 30
IR 457x128.10	IR 305x74.4	13	230	45	70	3	24	22	6	50	50		EJES RACK PRINCIPAL ESTE-OESTE
IR 457x144.30	IR 457x144.30	13	250	50	75	4	22	20	6	50	50		RACK AERODINAMICO
IR 457x112.9	IR 305x74.4	13	230	45	70	3	24	22	6	50	50		RACK PRINCIPAL ESTE-OESTE DE 0 S1
IR 457x144.30	IR 305x79.0	13	250	45	75	6	27	25	6	50	75		AERODINAMICO
IR 305x59.80	IR 305x52.20	13	230	45	70	3	18	16	6	50	50		EJES 1 Y 2 (INV. 1208.200)
IR 305x110.40	IR 305x62.20	13	250	50	75	3	21	19	6	50	50		EJES R Y O DE 5 AL 22 (INV. 1007.700)
IR 457x112.90	IR 335x110.40	13	250	50	75	3	21	19	6	50	50		EJES 31 Y 32 CORTE O Y P1
IR 457x128.10	IR 457x128.10	13	370	65	80	4	22	20	6	50	75		RACK PRINCIPAL ESTE-OESTE M1 A O (30 Y 32)
IR 305x65.10	IR 305x101.30	13	280	60	80	3	27	25	6	50	75		RACK ELECTRICOS, EJES 28, 31 Y 32
IR 457x128.10	IR 305x52.20	13	230	45	70	3	24	22	6	50	50		RACK PRINCIPAL N-S EJES J1 Y L1
IR 457x128.10	IR 305x74.40	13	230	40	75	3	27	25	6	50	75		RACK PRINCIPAL N-S
IR 254x44.80	IR 254x44.8	13	200	50	50	3	18	16	6	50	50		RACK PRINCIPAL N-S
IR 457x144.30	IR 305x79.0	13	240	40	75	3	27	25	6	50	50		RACK TIENEN 1 Y 2



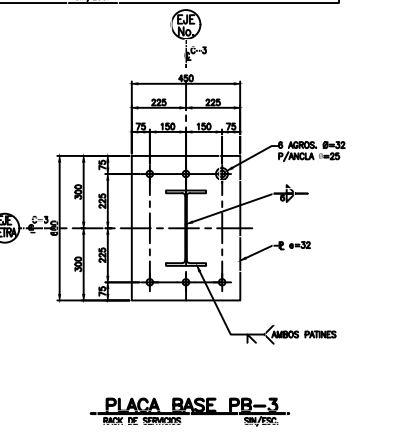
PLACA BASE PR-1  
RACK P.T.A. SM/ESC.



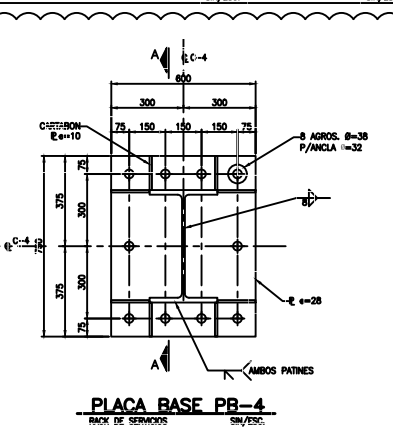
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RACK P.T.A. SM/ESC.



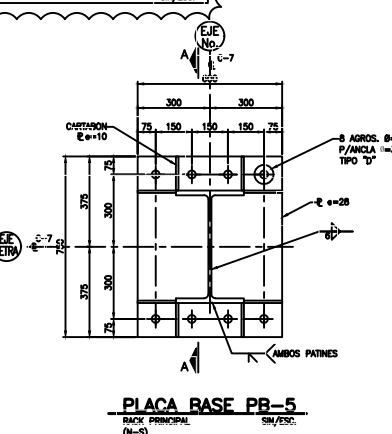
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RACK DE SERVICIOS SM/ESC.



PLACA BASE PR-3  
RACK DE SERVICIOS SM/ESC.



PLACA BASE PR-4  
RACK DE SERVICIOS SM/ESC.



PLACA BASE PR-5  
RACK PRINCIPAL (N-S) SM/ESC.

### NOMENCLATURA

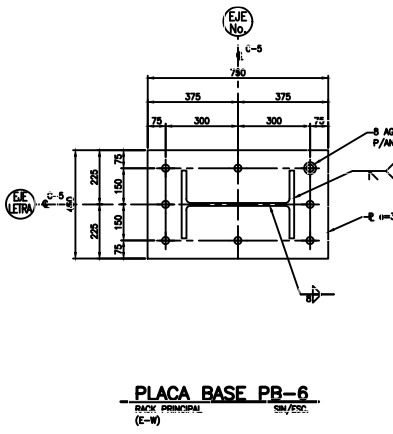
- C.S. CUERNA SUPERIOR
- C.I. CUERNA INFERIOR
- AR. ARMADURA
- PR. PLACA BASE
- D. DIAGONAL
- M. MONTANTE
- NPT. NIVEL PISO TERMINADO
- HTP. NIVEL TOPE DE PERFE.
- NDE. NIVEL DESPLANTE DE ESTRUCTURA
- C. COLUMNA
- T. TIRANTE
- CH. CONTRAVIENTO HORIZONTAL
- CV. CONTRAVIENTO VERTICAL
- CONEXION A BLOQUEO JUNTA DE DIVISION
- REVISION
- PENDIENTE

### NOTAS:

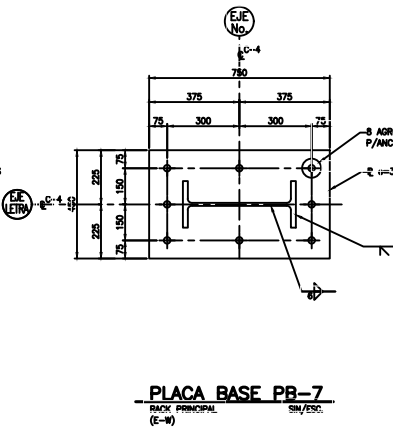
- 1.- PARA NOTAS GENERALES, ABREVIATURAS Y SIMBOLOGIA VER PLANO CFE-PM27005-SRJC-PF-0875.
- 2.- DIMENSIONES EN MILIMETROS.
- 3.- NIVELES Y COORDENADAS EN METROS.
- 4.- TODAS LAS DIMENSIONES SON SIMÉTRICAS CON RESPECTO A LAS LINEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
- 5.- ACERO ESTRUCTURAL ASTM A-36.
- 6.- TORNILLERIA ASTM A-325 TIPO 1 GALVANIZADOS O TIPO 3.
- 7.- ESTE PLANO ES DE USO Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE BARRER QUE CUMPLAN CON LOS REQUISITOS DEL ABC-1007 Y ASES CUANTO MENOS.
- 8.- LA DESCRIPCION DE LOS PERFILES CORRESPONDE CON LA INDICADA EN EL "MANUAL DE CONSTRUCCION EN ACERO" DEL INSTITUTO MEXICANO DE LA CONSTRUCCION EN ACERO (IMCA).
- 9.- NO TOMAR MEDIDAS A ESCALA, LAS COTAS DEBEN AL DIBUJO.
- 10.- TENER PRESENTE ESTE PLANO EN CONJUNTO CON LOS PLANOS DE REFERENCIA.
- 11.- ORDENACION DE LA PLACA DE ALICATA A ORDENACION DE DADO Y COLUMNA VER EN PLANOS CORRESPONDIENTES.

### PLANOS DE REFERENCIA

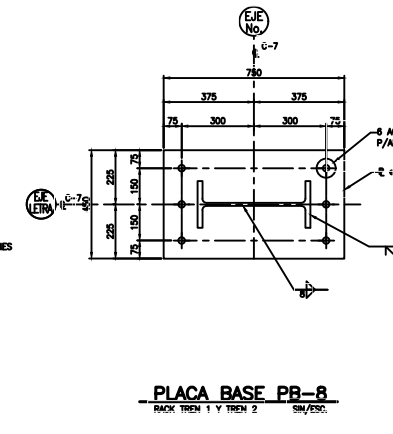
- PM27005-SRJC-PF-0803 RACK DE TUBERIAS-ESTRUCTURA PLANTAS
- PM27005-SRJC-PF-0805 RACK DE P.T.A. Y RACK DE SERVICIOS
- PM27005-SRJC-PF-0806 RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
- PM27005-SRJC-PF-0807 RACK AERODINAMICO
- PM27005-SRJC-PF-0808 RACK DE TUBERIAS-ESTRUCTURA PLANTAS
- PM27005-SRJC-PF-0809 RACK PRINCIPAL Y RACK TIENEN 1 Y 2
- PM27005-SRJC-PF-0810 RACK DE TUBERIAS-ESTRUCTURA ELEVACION
- PM27005-SRJC-PF-0811 RACK PRINCIPAL Y RACK TIENEN 1 Y 2
- PM27005-SRJC-PF-0812 RACK DE TUBERIAS-ESTRUCTURA ELEVACIONES
- PM27005-SRJC-PF-0813 RACK PRINCIPAL
- PM27005-SRJC-PF-0814 RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 1
- PM27005-SRJC-PF-0815 RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 2
- PM27005-SRJC-PF-0816 RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 3



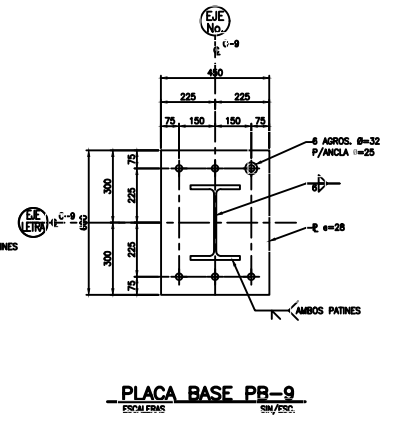
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RACK PRINCIPAL (E-W) SM/ESC.



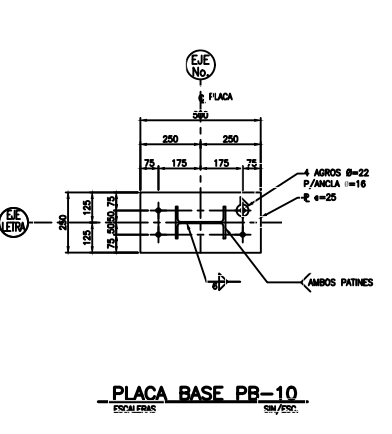
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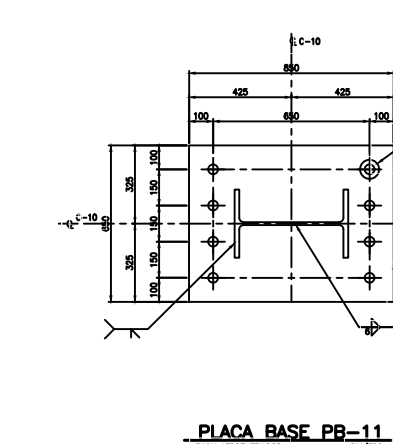
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RACK TIENEN 1 Y TIENEN 2 SM/ESC.



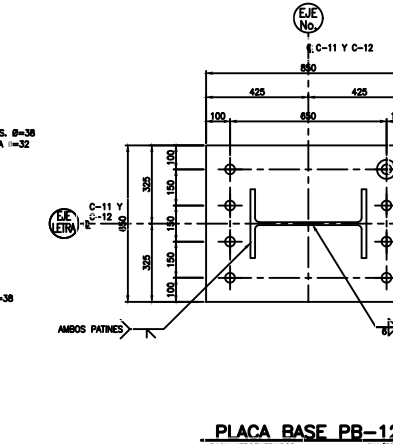
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ESCALERAS SM/ESC.



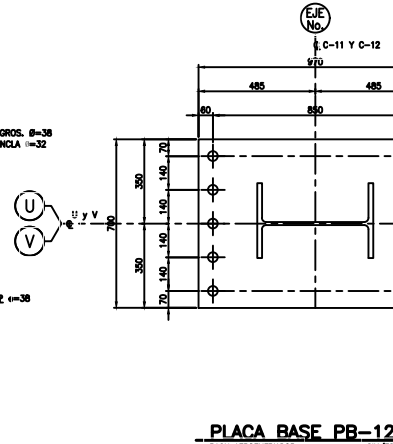
PLACA BASE PR-10  
ESCALERAS SM/ESC.



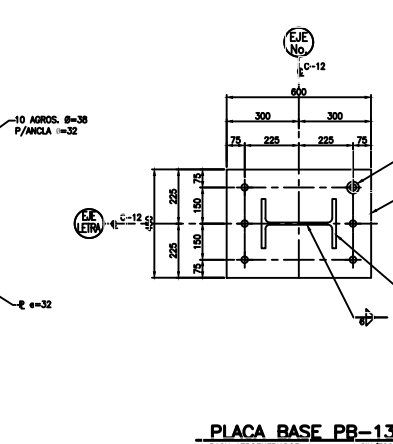
PLACA BASE PR-11  
RACK AERODINAMICO SM/ESC.



PLACA BASE PR-12  
RACK AERODINAMICO SM/ESC.



PLACA BASE PR-12A  
RACK AERODINAMICO SM/ESC.



PLACA BASE PR-13  
RACK AERODINAMICO (E-W) SM/ESC.

ESTADO DE DOCUMENTO COMO SE ANOTA

- SOLO PARA INFORMACION
- CONFORME SIN COMENTARIOS
- CONFORME CON COMENTARIOS
- NO CONFORME
- TAL COMO SE CONSTRUYO

FECHA DE RECEPCION: Revisado por Ing. Supervisor

FECHA: \_\_\_\_\_

IMPORTANTE: La revision de CFE no releva al PROVEEDOR de su responsabilidad de acuerdo a lo establecido en el contrato.

NO DEL CONTROL DE CFE:

REV.	FECHA	DESCRIPCION	REVISADO	APROBADO
2	12/12/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
1	25/10/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
0	01/09/11	APROBADO PARA CONSTRUCCION		

PROYECTO: PLANTA DE CICLO COMBINADO

TITULO: RACK DE TUBERIAS CONEXIONES TIPO Y PLACAS BASE

APROBADO: J.L. RUC

COMPROBADO: J.M.P.

FECHA DE RECEPCION: \_\_\_\_\_

FECHA: \_\_\_\_\_

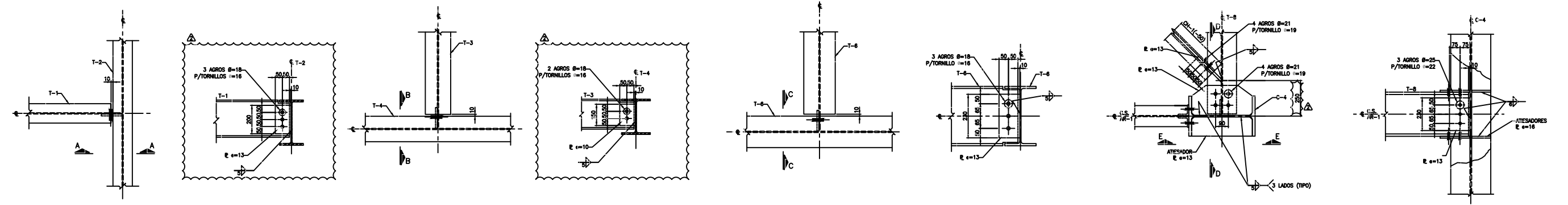
ESCALA: INDICADA

ORDEN DE COMPRA: \_\_\_\_\_

DOCUMENTO DE PROYECTO: PM27005-SRJC-PF-0811

DOCUMENTO SUBASTADOR: \_\_\_\_\_

REV. 01: 2



**DETALLE 1**  
PF-0803 SIM/ESC.

**CORTE A-A**  
SIM/ESC.

**DETALLE 2**  
PF-0803 SIM/ESC.

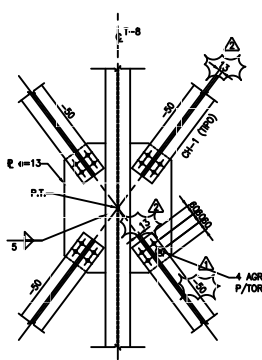
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SIM/ESC.

**DETALLE 3**  
PF-0803 SIM/ESC.

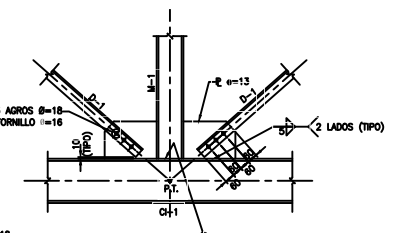
**CORTE C-C**  
SIM/ESC.

**DETALLE 4**  
PF-0803 SIM/ESC.

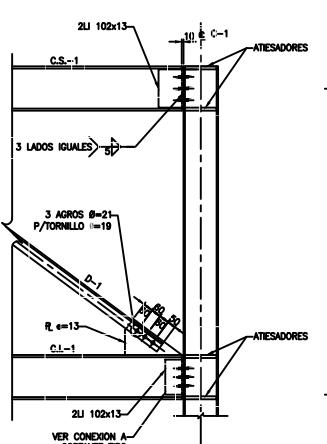
**CORTE D-D**  
SIM/ESC.



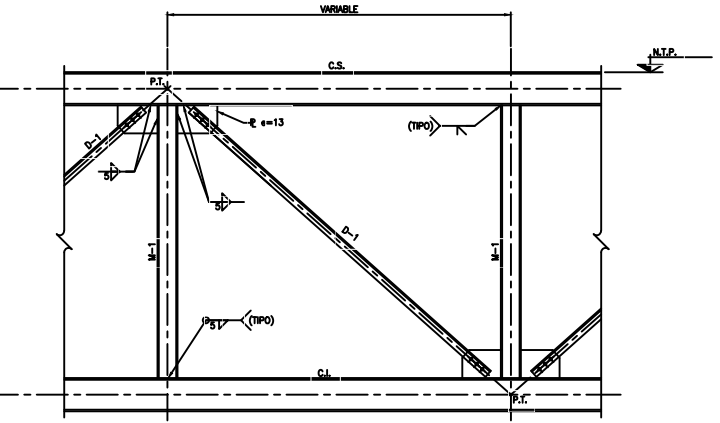
**DETALLE 5**  
PF-0804 SIM/ESC.



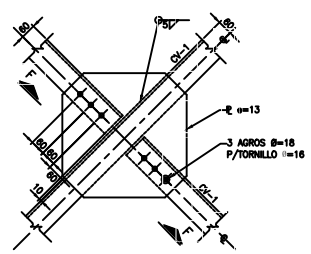
**DETALLE 6**  
PF-0804 SIM/ESC.



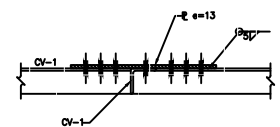
**CORTE F-F**  
SIM/ESC.



**ARMADO TIPO**  
PF-0804 SIM/ESC.



**DETALLE 7**  
PF-0804 SIM/ESC.



**CORTE F-F**  
SIM/ESC.

**NOMENCLATURA**

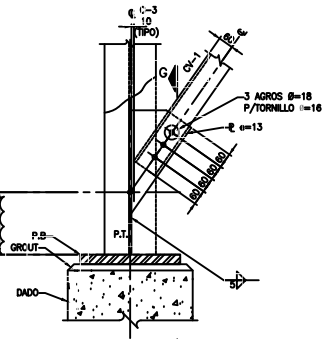
- C.S. CUERDA SUPERIOR
- C.I. CUERDA INFERIOR
- AR. ARMADURA
- P.B. PLACA BASE
- D. DIAGONAL
- M. MONTANTE
- N.P. NIVEL PISO TERMINADO
- H.T.P. NIVEL TOPE DE PERFIL
- H.D.E. NIVEL DESPLANTE DE ESTRUCTURA
- C. COLUMNA
- T. TRABE
- CH. CONTRAVIENTO HORIZONTAL
- CV. CONTRAVIENTO VERTICAL
- CM. CONEXION A MOMENTO
- JE. JUNTA DE EXPANSION
- AS. ASADO DEL NIVEL TOPE DE ESTRUCTURA
- REVISION

**NOTAS:**

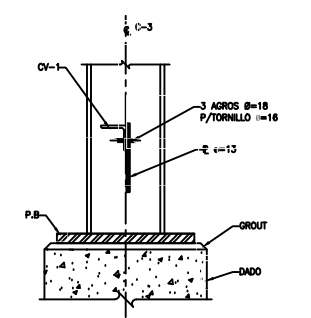
- 1.- PARA NOTAS GENERALES, ABREVIATURAS Y SIMBOLOGIA VER PLANO CFE-PM27005-SRUC-PC-0875.
- 2.- DIMENSIONES EN MILIMETROS.
- 3.- NIVELES Y COORDENADAS EN METROS.
- 4.- TODAS LAS DIMENSIONES SON SIMÉTRICAS CON RESPECTO A LAS LINEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
- 5.- ACERO ESTRUCTURAL ASTM A-36.
- 6.- TORNERILLERIA ASTM A-325 TIPO 1 GALVANIZADO O TIPO 3.
- 7.- ESTE PLANO ES DE DISEÑO Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE TALLER QUE CUMPLAN CON LOS REQUISITOS DEL AISC-LRFD Y AWS COMO MÍNIMO.
- 8.- LA DESCRIPCIÓN DE LOS PERFILES CORRESPONDE CON LA INDICADA EN EL "MANUAL DE CONSTRUCCIÓN EN ACERO" DEL INSTITUTO MEXICANO DE LA CONSTRUCCIÓN EN ACERO (IMCA).
- 9.- NO TOMAR MEDIDAS A ESCALA, LAS COTAS SON AL DIBUJO.
- 10.- TRABAJAR ESTE PLANO EN CONJUNTO CON LOS PLANOS DE REFERENCIA.

**PLANOS DE REFERENCIA**

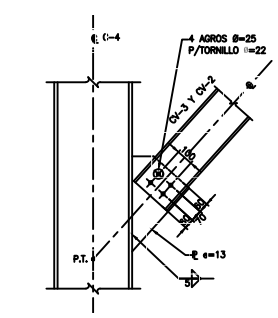
- PM27005-SRUC-PF-0803 RACK DE TUBERÍAS-ESTRUCTURA PLANTAS
- PM27005-SRUC-PC-0805 RACK DE P.Z.A. Y RACK DE SERVICIOS
- PM27005-SRUC-PC-0805 RACK DE TUBERÍAS-ESTRUCTURA PLANTAS Y ELEVACIONES
- PM27005-SRUC-PC-0808 RACK AEROCOENSDENSADOR
- PM27005-SRUC-PF-0808 RACK DE TUBERÍAS-ESTRUCTURA PLANTAS
- PM27005-SRUC-PF-0807 RACK PRINCIPAL Y RACK TRER 1 Y 2
- PM27005-SRUC-PF-0807 RACK DE TUBERÍAS-ESTRUCTURA ELEVACION
- PM27005-SRUC-PF-0808 RACK PRINCIPAL Y RACK TRER 1 Y 2
- PM27005-SRUC-PF-0808 RACK DE TUBERÍAS-ESTRUCTURA ELEVACIONES
- PM27005-SRUC-PF-0811 RACK PRINCIPAL
- PM27005-SRUC-PF-0811 RACK DE TUBERÍAS-ESTRUCTURA CONEXIONES TIPO Y PLACAS BASE
- PM27005-SRUC-PF-0813 RACK DE TUBERÍAS-ESTRUCTURA CORTES Y DETALLES 2
- PM27005-SRUC-PF-0814 RACK DE TUBERÍAS-ESTRUCTURA CORTES Y DETALLES 3



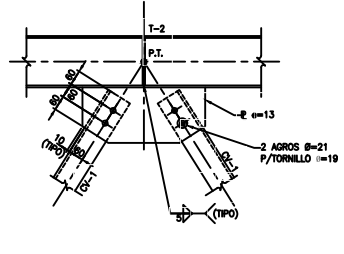
**DETALLE 8**  
PF-0804 SIM/ESC.



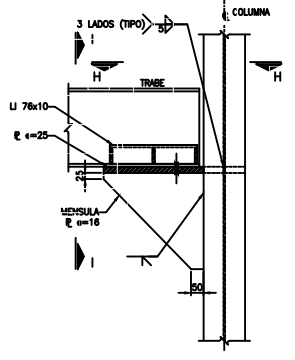
**CORTE G-G**  
SIM/ESC.



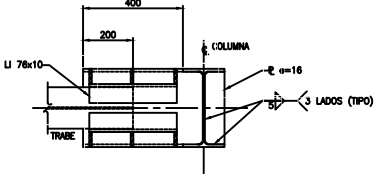
**DETALLE 9**  
PF-0804 SIM/ESC.



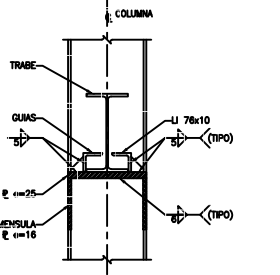
**DETALLE 10**  
PF-0804 SIM/ESC.



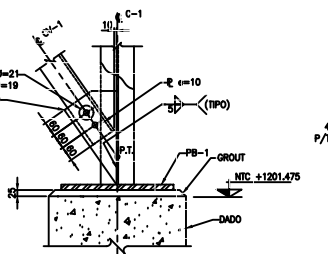
**JUNTA DE EXPANSION**  
PF-0804 SIM/ESC.



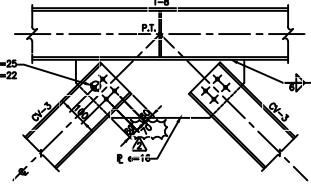
**CORTE H-H**  
SIM/ESC.



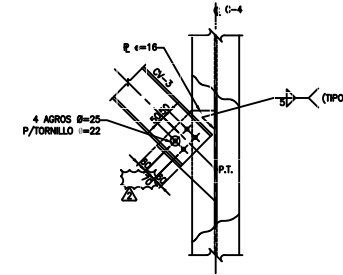
**CORTE I-I**  
SIM/ESC.



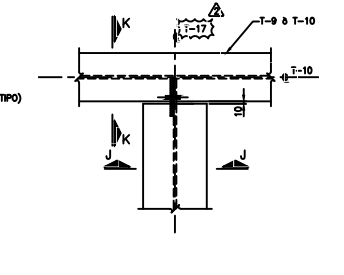
**DETALLE 11**  
PF-0804 SIM/ESC.



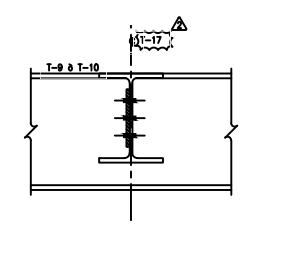
**DETALLE 12**  
PF-0804 SIM/ESC.



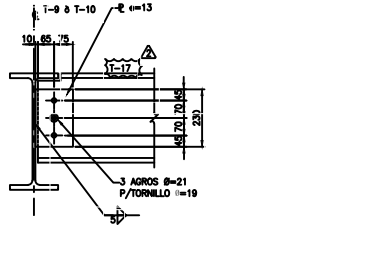
**DETALLE 13**  
PF-0804 SIM/ESC.



**DETALLE 14**  
PF-0805 SIM/ESC.



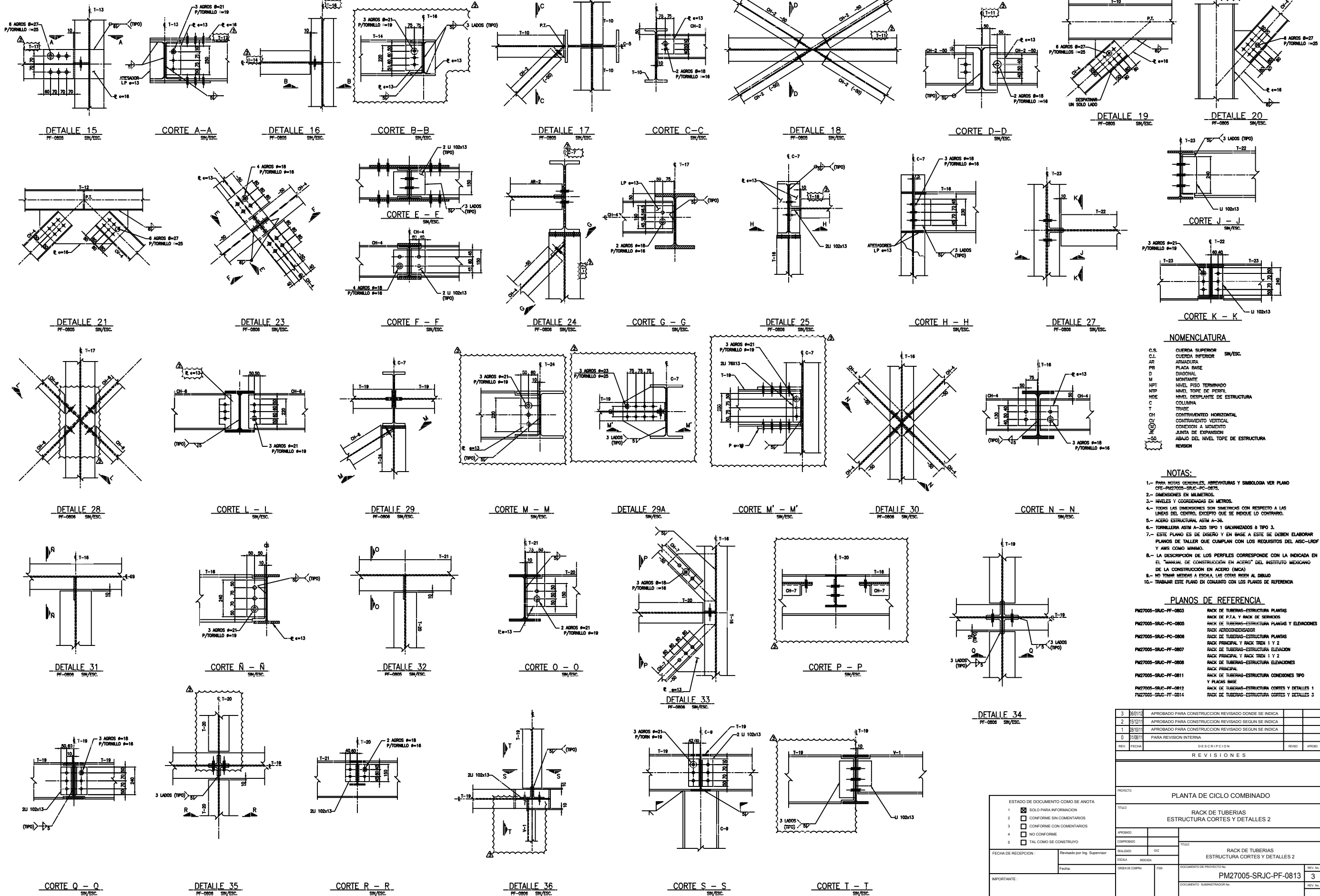
**CORTE J-J**  
SIM/ESC.



**CORTE K-K**  
SIM/ESC.

ESTADO DE DOCUMENTO COMO SE ANOTA	
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3	<input type="checkbox"/> CONFORME CON COMENTARIOS
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FECHA DE RECEPCION:	Revisado por Ing. Supervisor
IMPORTE:	Fecha:
NO. DE CONTROL:	

2	15/01/12	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
1	28/10/11	APROBADO PARA CONSTRUCCION REVISADO SEGUN SE INDICA		
0	31/08/11	APROBADO PARA CONSTRUCCION		
REV.	FECHA	DESCRIPCION	REVISO	APROBO
REVISIONES				
PROYECTO: PLANTA DE CICLO COMBINADO				
TITULO: RACK DE TUBERIAS ESTRUCTURA CORTES Y DETALLES 1				
APROBADO:	ELABORADO:	REVISADO:	FECHA:	NO. DE CONTROL:
COMPROBADO:	ESCALA:	INDICADA:	ORDEN DE COPIA:	ESSE
FECHA DE RECEPCION:	REALIZADO:	DISEÑO:	DOCUMENTO DE PROYECTO NO.:	REV. NO.:
IMPORTE:	Fecha:	ESSE	DOCUMENTO DE PROYECTO NO.:	REV. NO.:
			PM27005-SRUC-PF-0812	2
			DOCUMENTO SUBMINISTRADO NO.:	REV. NO.:



**NOMENCLATURA**

C.S.	CUERDA SUPERIOR	SM/ESC.
C.I.	CUERDA INFERIOR	SM/ESC.
AR	ARRASTRE	
PR	PLACA BASE	
D	DIAGONAL	
M	MONTANTE	
HPT	HUEL PISO TERMINADO	
HTP	NIVEL TOPE DE PERFILE	
MDE	NIVEL DESPLANTE DE ESTRUCTURA	
T	TRAMPE	
CH	CONTRAVENTO HORIZONTAL	
CV	CONTRAVENTO VERTICAL	
CA	CONEXION A ANCLAJE	
JE	JUNTA DE EXPANSION	
ES	ABAJO DEL NIVEL TOPE DE ESTRUCTURA	
REVISION		

- NOTAS:**
- 1.- PARA NOTAS GENERALES, ABREVIATURAS Y SIMBOLOGIA VER PLANO CDE-PISTONES-SRJC-PC-007E.
  - 2.- DIMENSIONES EN MILIMETROS.
  - 3.- NIVELES Y COORDENADAS EN METROS.
  - 4.- TODAS LAS DIMENSIONES SON SIMETRICAS CON RESPECTO A LAS LINEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
  - 5.- ACERO ESTRUCTURAL ASTM A-36.
  - 6.- TORNILLERIA ASTM A-325 TIPO 1 GALVANIZADO O TIPO 3.
  - 7.- ESTE PLANO ES DE USO Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE TALLER QUE CUMPLAN CON LOS REQUISITOS DEL ASO-URF Y AIS COMO MINIMO.
  - 8.- LA DESCRIPCION DE LOS PERFILES CORRESPONDE CON LA INDICADA EN EL "MANUAL DE CONSTRUCCION EN ACERO" DEL INSTITUTO MEXICANO DE LA CONSTRUCCION EN ACERO (IMCA).
  - 9.- NO TRABAJAR MEDIDA A ESCALA. LAS CORTES DEBEN AL DIBUJO.
  - 10.- TRABAJAR ESTE PLANO EN CONJUNTO CON LOS PLANOS DE REFERENCIA.

**PLANOS DE REFERENCIA**

PM27005-SRJC-PF-0803	RACK DE TUBERIAS-ESTRUCTURA PLANTAS
PM27005-SRJC-PC-0805	RACK DE P.I.T.A. Y RACK DE SERVIDORES
PM27005-SRJC-PC-0806	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PC-0808	RACK DE TUBERIAS-ESTRUCTURA PLANTAS
PM27005-SRJC-PF-0807	RACK PRINCIPAL Y RACK TREN 1 Y 2
PM27005-SRJC-PF-0808	RACK DE TUBERIAS-ESTRUCTURA ELEVACION
PM27005-SRJC-PF-0808	RACK PRINCIPAL Y RACK TREN 1 Y 2
PM27005-SRJC-PF-0808	RACK DE TUBERIAS-ESTRUCTURA ELEVACIONES
PM27005-SRJC-PF-0811	RACK PRINCIPAL
PM27005-SRJC-PF-0812	RACK DE TUBERIAS-ESTRUCTURA CONDICIONES TIPO Y PLACAS BASE
PM27005-SRJC-PF-0814	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 1
PM27005-SRJC-PF-0814	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 3

3	20/01/12	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
2	15/12/11	APROBADO PARA CONSTRUCCION REVISADO SEGUN SE INDICA		
1	25/10/11	APROBADO PARA CONSTRUCCION REVISADO SEGUN SE INDICA		
0	31/08/11	PARA REVISION INTERNA		
REV.	FECHA	DESCRIPCION	REVISADO	APROBADO

ESTADO DE DOCUMENTO COMO SE ANOTA

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2	<input type="checkbox"/>	CONFORME SIN COMENTARIOS
3	<input type="checkbox"/>	CONFORME CON COMENTARIOS
4	<input type="checkbox"/>	NO CONFORME
5	<input type="checkbox"/>	TAL COMO SE CONSTRUYO

FECHA DE RECEPCION: \_\_\_\_\_ Revisado por Ing. Supervisor \_\_\_\_\_

Fecha: \_\_\_\_\_

IMPORTE: \_\_\_\_\_

NO. DE CONTROL: \_\_\_\_\_

PROYECTO: PLANTA DE CICLO COMBINADO

TITULO: RACK DE TUBERIAS ESTRUCTURA CORTES Y DETALLES 2

ARREGLADO: \_\_\_\_\_

COMPROBADO: \_\_\_\_\_

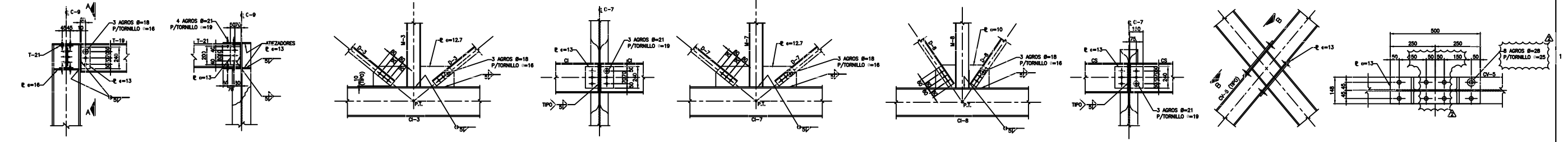
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ESCALA: \_\_\_\_\_ INDICADA

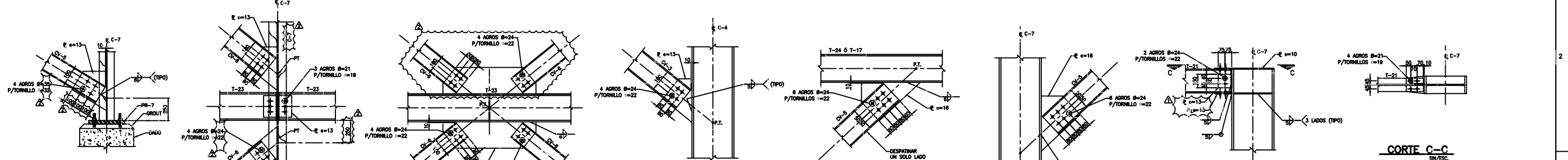
DOCUMENTO DE PROYECTO: PM27005-SRJC-PF-0813

DOCUMENTO SUBASTADOR: \_\_\_\_\_

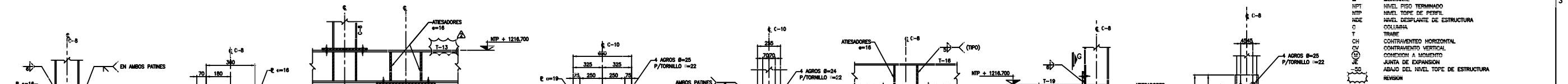
REV. NO. 3



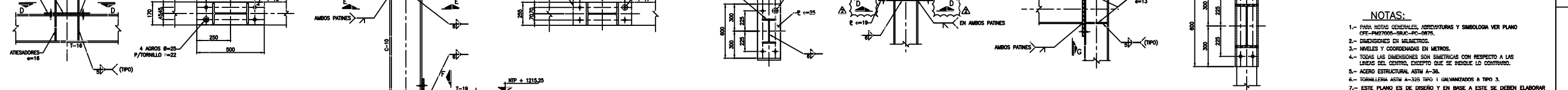
**DETALLE 37** FF-0807 SM/ESC. **CORTE A-A** FF-0807 SM/ESC. **DETALLE 38** FF-0807 SM/ESC. **DETALLE 39** FF-0807 SM/ESC. **DETALLE 40** FF-0807 SM/ESC. **DETALLE 41** FF-0807 SM/ESC. **DETALLE 42** FF-0807 SM/ESC. **DETALLE 43** FF-0807 SM/ESC. **CORTE B-B** SM/ESC.



**DETALLE 44** FF-0807 SM/ESC. **DETALLE 46** FF-0807 SM/ESC. **DETALLE 49** FF-0807 SM/ESC. **DETALLE 50** FF-0807 SM/ESC. **DETALLE 51** FF-0808 SM/ESC. **DETALLE 52** FF-0808 SM/ESC. **DETALLE 53** FF-0808 SM/ESC. **CORTE C-C** SM/ESC.



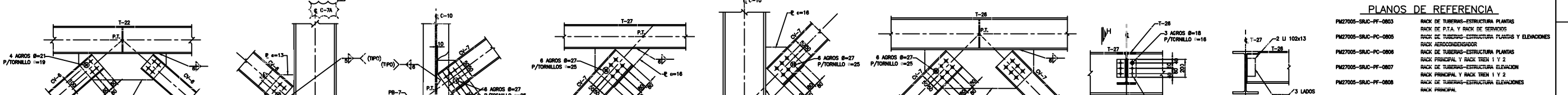
**DETALLE 54** FF-0808 SM/ESC. **CORTE D-D** SM/ESC. **DETALLE 55** FF-0808 SM/ESC. **CORTE E-E** SM/ESC. **CORTE F-F** SM/ESC. **DETALLE 56** FF-0808 SM/ESC. **DETALLE 57** FF-0808 SM/ESC. **CORTE G-G** SM/ESC.



**DETALLE 58** FF-0808 SM/ESC. **DETALLE 59** FF-0808 SM/ESC. **DETALLE 60** FF-0808 SM/ESC. **DETALLE 61** FF-0808 SM/ESC. **DETALLE 62** FF-0808 SM/ESC. **DETALLE 63** FF-0808 SM/ESC. **DETALLE 64** FF-0808 SM/ESC. **CORTE H-H** FF-0808 SM/ESC.



**DETALLE 65** FF-0809 SM/ESC. **CORTE I-I** FF-0809 SM/ESC. **DETALLE 66** FF-0809 SM/ESC. **DETALLE 67** FF-0809 SM/ESC. **CORTE K-K** FF-0809 SM/ESC. **DETALLE 68** FF-0809 SM/ESC. **CORTE L-L** FF-0809 SM/ESC.



PLANOS DE REFERENCIA	
PM27005-SRJC-PF-0803	RACK DE TUBERIAS-ESTRUCTURA PLANTAS
PM27005-SRJC-PC-0805	RACK DE P.I.A. Y RACK DE SERVIDORES
PM27005-SRJC-PC-0806	RACK DE TUBERIAS-ESTRUCTURA PLANTAS Y ELEVACIONES
PM27005-SRJC-PC-0808	RACK AEROCONDICIONADOR
PM27005-SRJC-PF-0807	RACK PRINCIPAL Y RACK TREN 1 Y 2
PM27005-SRJC-PF-0807	RACK DE TUBERIAS-ESTRUCTURA ELEVACION
PM27005-SRJC-PF-0808	RACK PRINCIPAL Y RACK TREN 1 Y 2
PM27005-SRJC-PF-0808	RACK DE TUBERIAS-ESTRUCTURA ELEVACIONES
PM27005-SRJC-PF-0811	RACK PRINCIPAL
PM27005-SRJC-PF-0811	RACK DE TUBERIAS-ESTRUCTURA CONEXIONES TIPO Y PLANCHAS BENS
PM27005-SRJC-PF-0812	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 1
PM27005-SRJC-PF-0813	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 2

REVISIONES	
2	20/01/12 APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA
1	25/01/11 APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA
0	31/08/11 APROBADO PARA CONSTRUCCION

ESTADO DE DOCUMENTO COMO SE ANOTA	
1	SOLO PARA INFORMACION
2	CONFORME SIN COMENTARIOS
3	CONFORME CON COMENTARIOS
4	NO CONFORME
5	TAL COMO SE CONSTRUYO

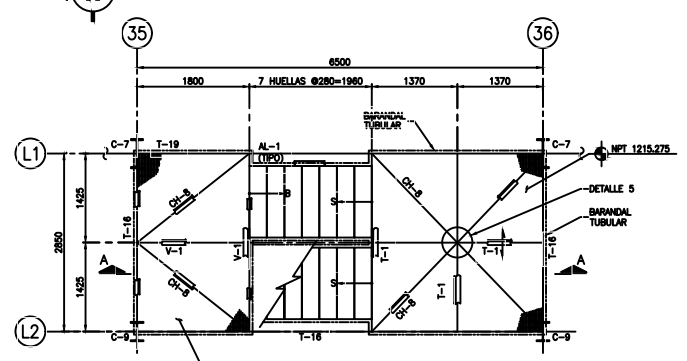
FECHA DE RECEPCION:		Revisado por: Ing. Supervisor	
Fecha:		Fecha:	

PROYECTO:		TITULO:	
PLANTA DE CICLO COMBINADO		RACK DE TUBERIAS ESTRUCTURA CORTES Y DETALLES 3	

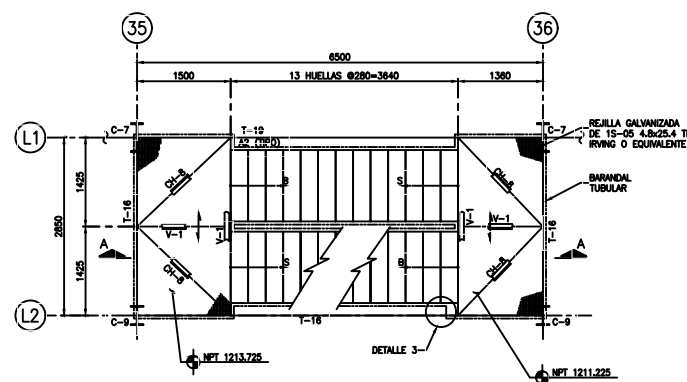
APROBADO:		TITULO:	
REALIZADO:	ESC:	REALIZADO:	ESC:
COMPROBADO:	INDICADA:	COMPROBADO:	INDICADA:
ORDEN DE COMPRA:	ITEM:	ORDEN DE COMPRA:	ITEM:

DOCUMENTO DE PROYECTO:		REV. NO.	
PM27005-SRJC-PF-0814	2	PM27005-SRJC-PF-0814	2

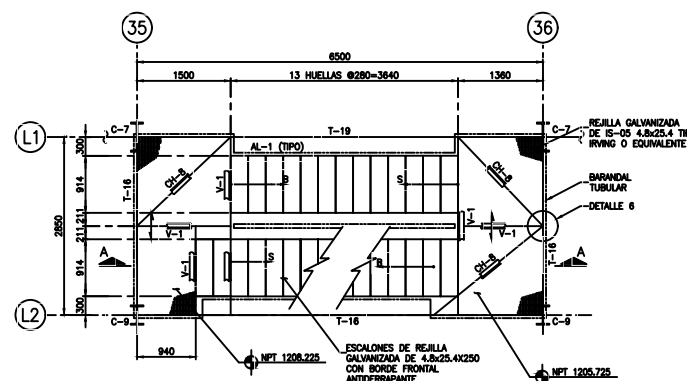
TABLA DE PERFILES						
MARCA	PERFIL	PESO kg/m	d	bf	hf	tw
C-9	W-305	58.8	305	203	15.1	7.5
C-10	W-243	28.6	243	133	8	6.1
T-1	W-203	28.6	203	133	8	6.1
AL-1	CE-254	22.78	254	66	11.1	6.1
CH-8	U-846	18.30	84.6	66	11.1	6.1
V-1	CE-254	22.78	254	66	11.1	6.1



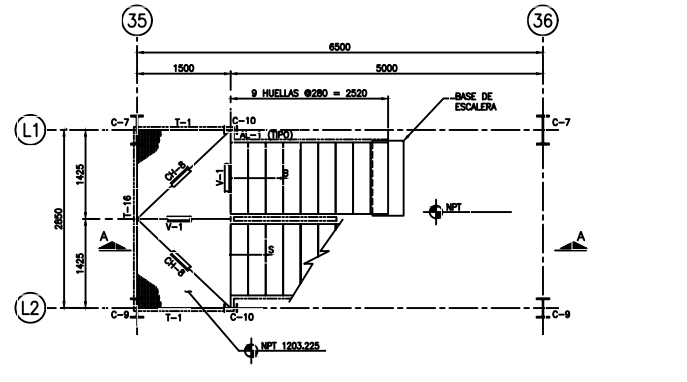
PLANTA NPT 1215.275 A NPT 1216.725 ESC. 1:40



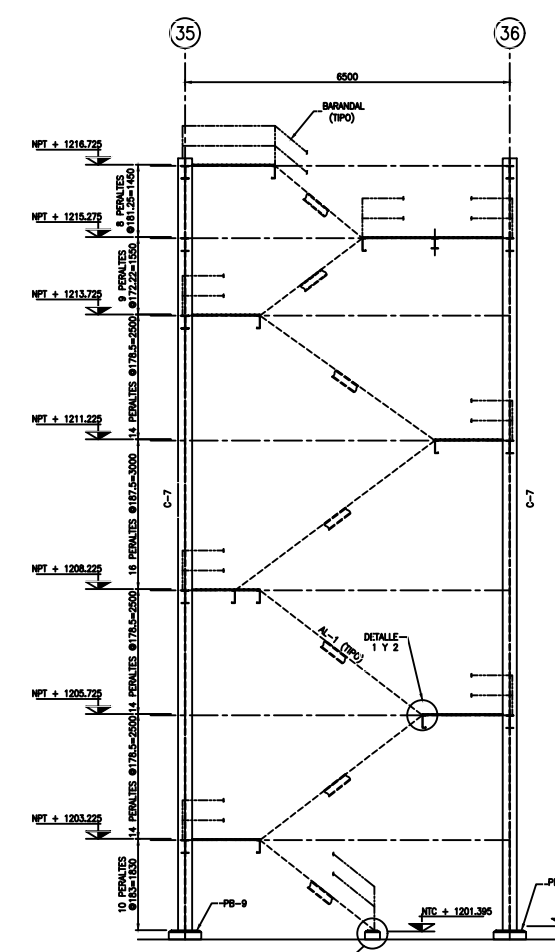
PLANTA NPT 1211.225 A NPT 1213.725 ESC. 1:40



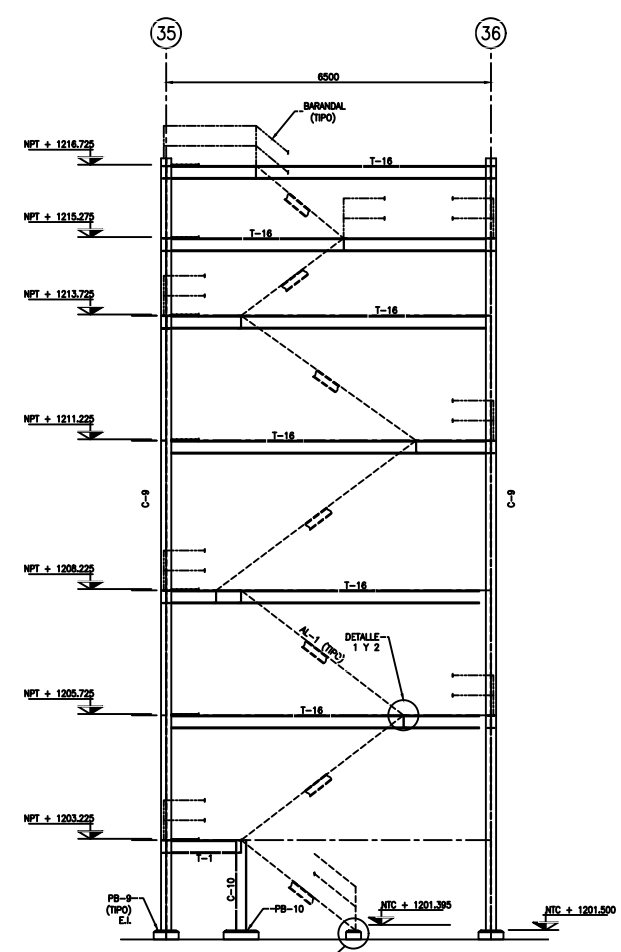
PLANTA NPT 1205.725 A NPT 1208.225 ESC. 1:40



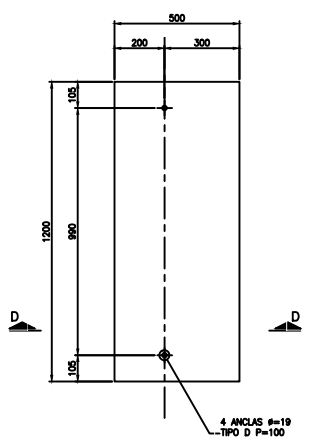
PLANTA NPT 1203.225 ESC. 1:40



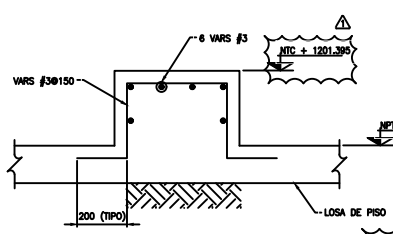
CORTE A-A ESC. 1:50



ELEVACION MARCO EJE 12 ESC. 1:50



BASE DE ARRANQUE DE ESCALERA ESC. 1:10



CORTE D-D ESC. 1:10

**CALIDAD DE MATERIALES:**  
 CONCRETO PARA ZAPATAS Y DADOS  $f_c = 24.5 \text{ MPa} = 250 \text{ kg/cm}^2$   
 ACERO DE REFUERZO  $f_y = 412 \text{ MPa} = 4200 \text{ kg/cm}^2$   
 ANCLAJE A-36

**NOMENCLATURA:**

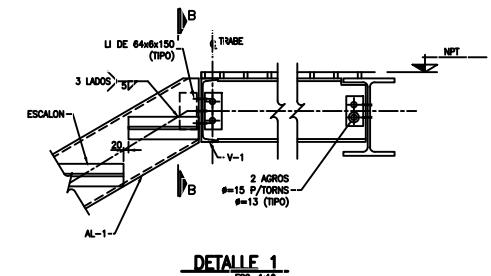
- NPT NIVEL TOPE DE PERFIL
- NPT NIVEL PISO TERMINADO
- C COLUMNA
- T TRABE
- AL ALFARDA
- CH CONTRAFIERTO HORIZONTAL
- CV CONTRAFIERTO VERTICAL
- V VIGA
- REVISION

**NOTAS:**

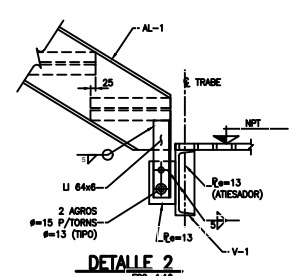
- 1.- DIMENSIONES EN MILIMETROS, NIVELES EN METROS.
- 2.- VER NOTAS GENERALES, ADECUACIONES Y SIMBOLOGIA, EN PLANO DE PROYCCION-SRUC-PF-0815.
- 3.- TODAS LAS DIMENSIONES SON SIMÉTRICAS CON RESPECTO A LAS LINEAS DE CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
- 4.- ACERO ESTRUCTURAL ASTM A-36.
- 5.- TORNELES: ASTM A-105 TIPO 1 Y 2 S GALVANIZADO.
- 6.- NO TOMAR MEDIDAS A ESCALA LAS COTAS DEBEN AL DIBUJO.
- 7.- TRABAJAR ESTE PLANO EN CONTACTO CON LOS PLANOS DE REFERENCIA.
- 8.- ESTE PLANO ES DE OBRERA Y EN BASE A ESTE SE DEBERÁN DESARROLLAR LOS PLANOS DE FABRICACION Y MONTEAR Y RESERVAR CUMPLIR CON LOS REQUERIMIENTOS DEL ADO Y LA ADO COMO MÍNIMO.
- 9.- EL BARRIDO SERÁ DE TIPO #32 CEDAÑA 40 VER PLANO DE REFERENCIA-SRUC-PF-0815.
- 10.- VER PERFILES FALTANTES EN PLANO PF-0806.

**PLANOS REFERENCIA:**

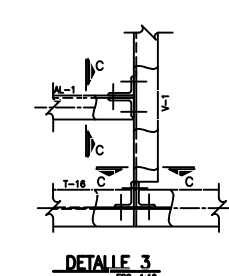
- PM27005-SRUC-PF-0806 RACK DE TUBERIAS-ESTRUCTURA PLANTAS
- PM27005-SRUC-PF-0807 RACK PRINCIPAL 1 PARA TREN 1 Y 2
- PM27005-SRUC-PF-0808 DETALLES TIPO DE BARRILES



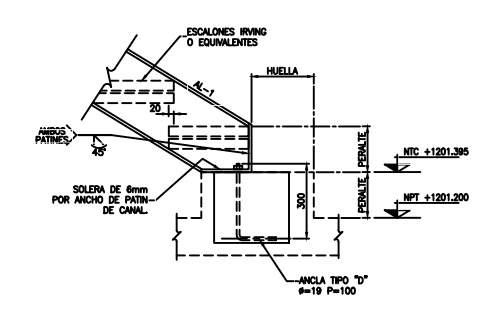
DETALLE 1 ESC. 1:10



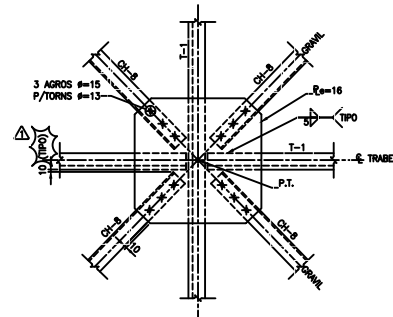
DETALLE 2 ESC. 1:10



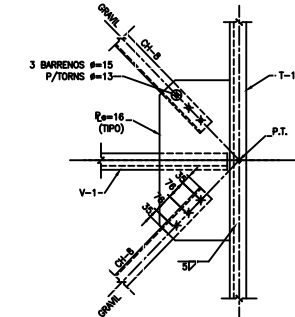
DETALLE 3 ESC. 1:10



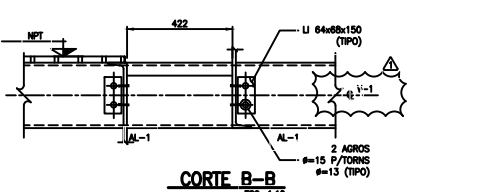
DETALLE 4 ESC. 1:10 VER PLANO PC-380



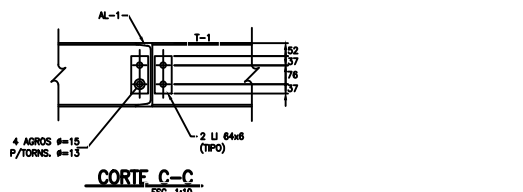
DETALLE 5 ESC. 1:10



DETALLE 6 ESC. 1:10



CORTE B-B ESC. 1:10



CORTE C-C ESC. 1:10

REV.	FECHA	DESCRIPCION	REVISOR	APROBADO
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0	28/08/11	APROBADO PARA CONSTRUCCION		

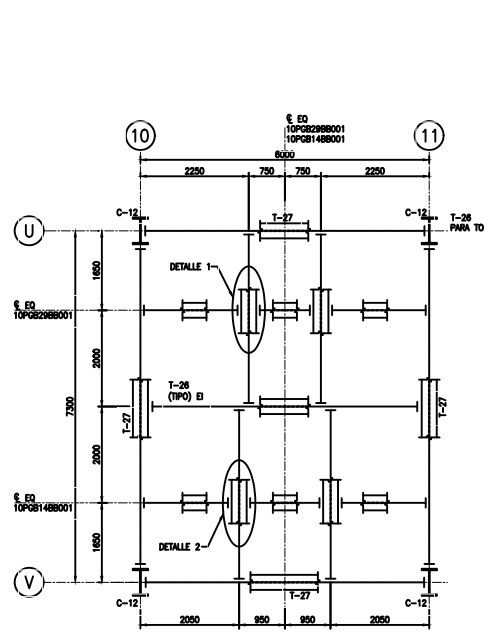
REVISIONES	
PROYECTO:	PLANTA DE CICLO COMBINADO
TITULO:	RACK DE TUBERIAS ESTRUCTURA ESCALERA RACK PRINCIPAL
APROBADO:	
COMPROBADO:	
REALIZADO:	RACK DE TUBERIAS ESTRUCTURA ESCALERA RACK PRINCIPAL
ESCALA:	INDICADA
ORDEN DE COMPRA:	ESM
FECHA DE RECEPCION:	Revisado por Ing. Supervisor
FECHA:	
IMPORTANTE:	
NO. DE CONTROL:	

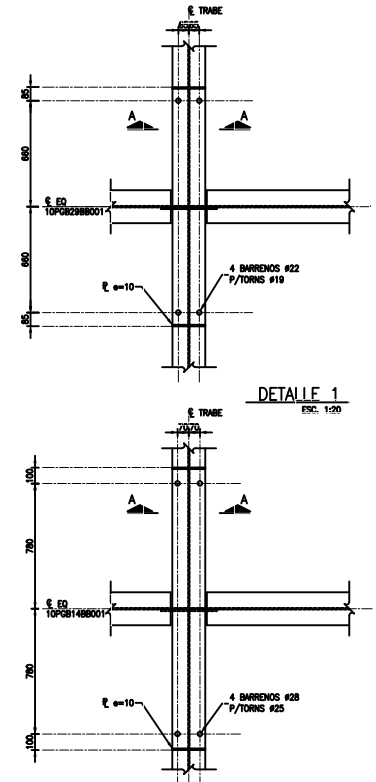
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CONFORME CON COMENTARIOS	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
NO CONFORME	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
TAL COMO SE CONSTRUYO	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>

DOCUMENTO DE PROYECTO NO.	REV. NO.
PM27005-SRJC-PF-0815	1



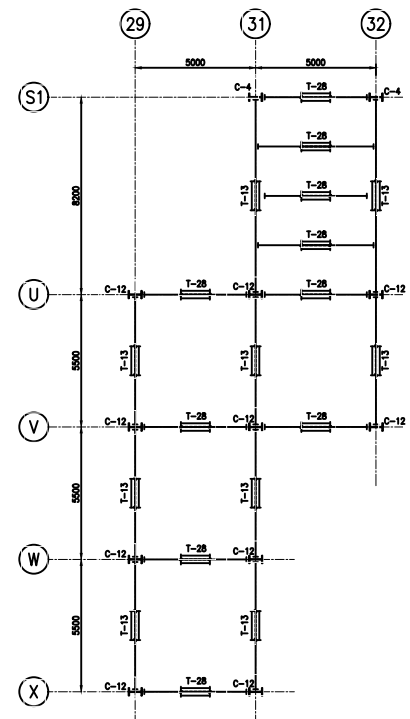
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(RACK DEL AEROCENTRIFUGADOR AUXILIAR) ESC. 1:100  
(PF-0805)



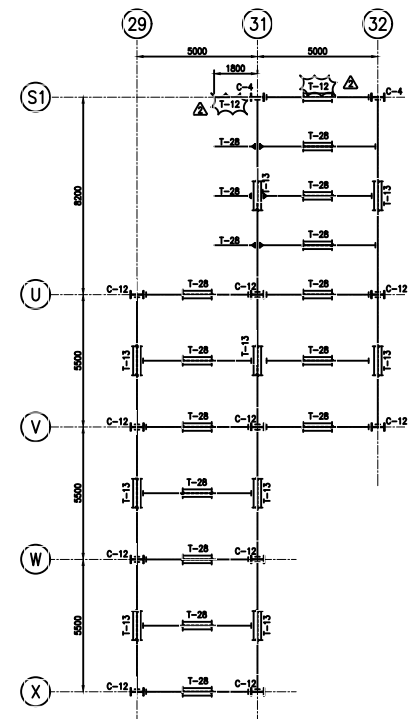
DETALLE 1  
ESC. 1:20

DETALLE 2  
ESC. 1:20

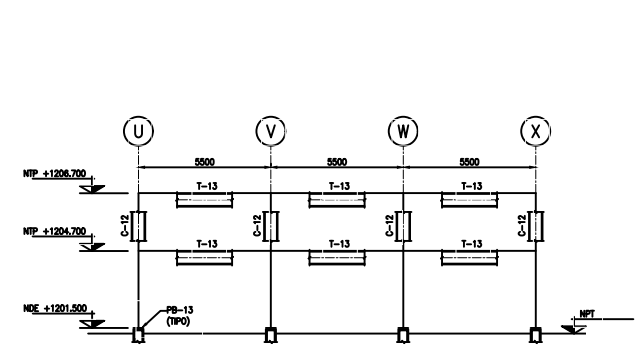
VISTA A-A  
5/8 ESC.



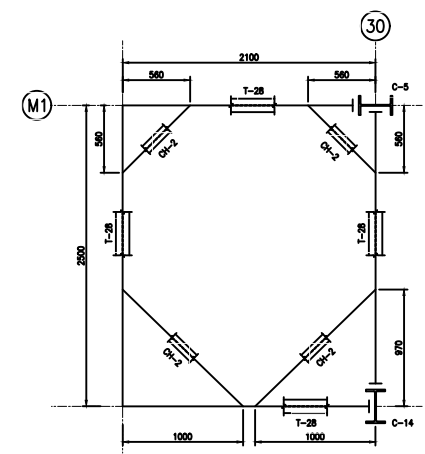
PLANTA NIVEL NTP +1204.700  
ESC. 1:150



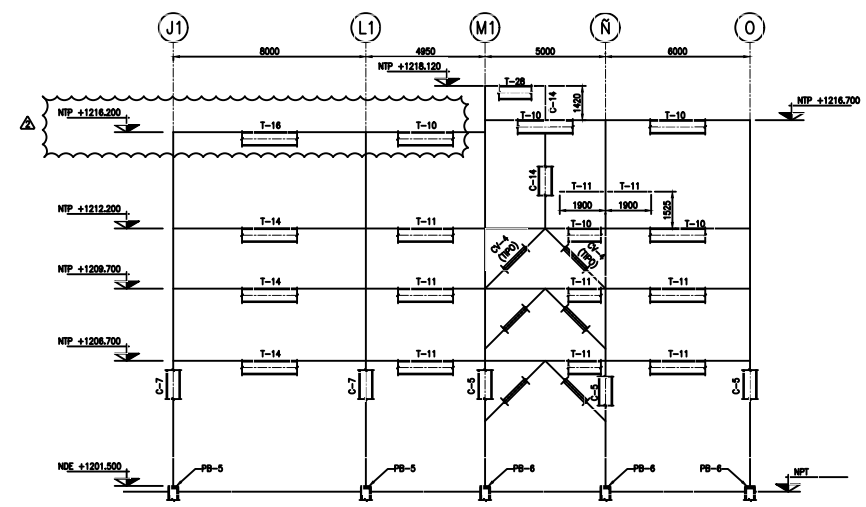
PLANTA NIVEL NTP +1206.700  
ESC. 1:150



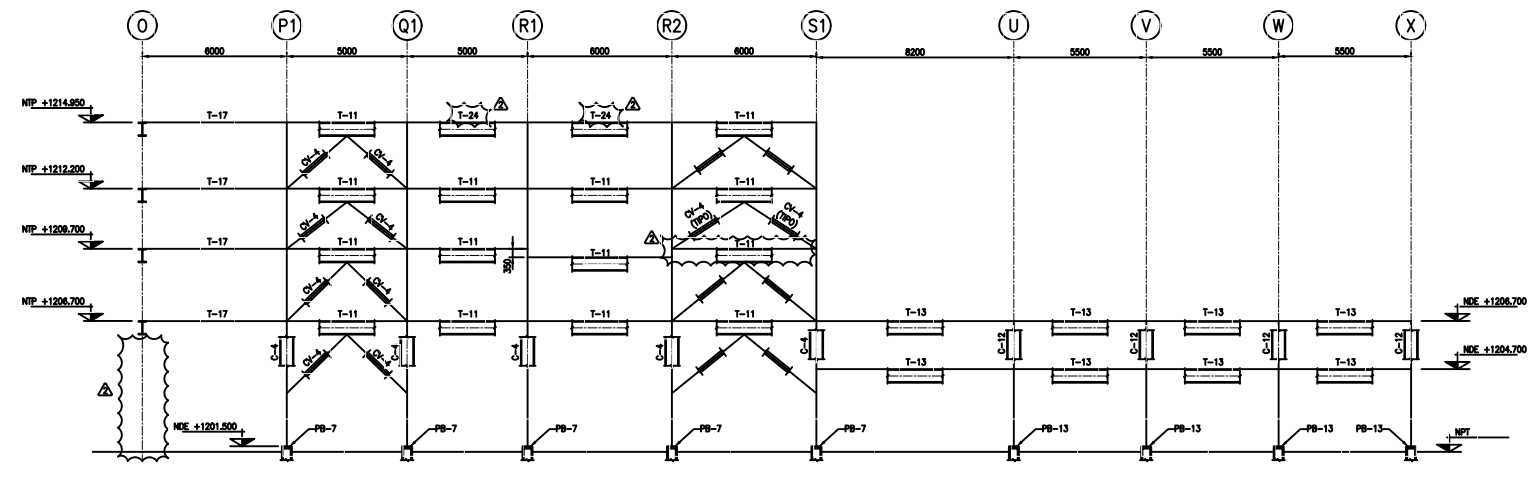
ELEVACION MARCO F.F. 29  
ESC. 1:100



PLANTA NIVEL NTP +1218.120  
ESC. 1:20



ELEVACION MARCO F.F. 30  
VER PLANO PF-0805  
ESC. 1:100



ELEVACION MARCO F.F. 31  
VER PLANO PF-0805  
ESC. 1:100

**NOMENCLATURA**

CH	CONTORNADO HORIZONTAL
SI	CONDICION A MOMENTO
NPT	NIVEL FINO TERMINADO
NTP	NIVEL TOPE DE PERFIL
C	COLUMNA
T	TRABE
CV	CONTORNADO VERTICAL
NDE	NIVEL DE DESPLANTE ESTRUCTURAL
PF	PLANTA BASE
PS	EXCEPTO INDICADO
EI	REVISION

**NOTAS:**

- 1.- PARA NOTAS GENERALES, ABREVIATURAS Y SIMBOLOS VER PLANO OFE-PM27005-SRJC-PC-0816.
- 2.- DIMENSIONES EN MILIMETROS.
- 3.- NIVELES Y COORDENADAS EN METROS.
- 4.- TODAS LAS DIMENSIONES SON SIMÉTRICAS CON RESPECTO A LAS LÍNEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
- 5.- ACERO ESTRUCTURAL ASTM A-36.
- 6.- TORNERILERA ASTM A-305 TIPO 1 GALVANIZADO A TIPO 3.
- 7.- ESTE PLANO ES DE DISEÑO Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE TALLER QUE CUMPLAN CON LOS REQUISITOS DEL ASO-LIOP Y AISI COMO MÍNIMO.
- 8.- LA INFORMACION DE LOS PERFILES CORRESPONDE CON LA MEDIDA EN EL MANUAL DE CONSTRUCCION EN ACERO DEL INSTITUTO MEXICANO DE LA CONSTRUCCION EN ACERO (IMCA).
- 9.- NO TOMAR MEDIDAS A ESCALA, LAS COTAS SIGEN AL DIBUJO.
- 10.- TRABAJAR ESTE PLANO EN COORDINACIÓN CON LOS PLANOS DE REFERENCIA.
- 11.- VER CONEXIONES A CORRIENTE Y A MOMENTO DE LA ESTRUCTURA PRINCIPAL EN PLANO PF-0811.
- 12.- VER TABLA DE PERFILES EN PLANO PF-0805.

**PLANOS DE REFERENCIA**

PM27005-SRJC-PF-0803	RACK DE TUBERIAS-ESTRUCTURAS PLANTAS RACK DE P.T.A. Y RACK DE SERVICIOS
PM27005-SRJC-PF-0804	RACK DE TUBERIAS-ESTRUCTURAS ELEVACION RACK DE P.T.A. Y RACK DE SERVICIOS
PM27005-SRJC-PF-0806	RACK DE TUBERIAS-ESTRUCTURAS PLANTAS RACK PRINCIPAL Y RACK TREN 1 Y 2
PM27005-SRJC-PF-0807	RACK DE TUBERIAS-ESTRUCTURAS ELEVACION RACK PRINCIPAL Y RACK TREN 1 Y 2
PM27005-SRJC-PF-0808	RACK DE TUBERIAS-ESTRUCTURAS ELEVACIONES RACK PRINCIPAL
PM27005-SRJC-PF-0811	RACK DE TUBERIAS-ESTRUCTURA CONEXIONES TIPO Y PLANTAS BASE
PM27005-SRJC-PF-0812	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 1
PM27005-SRJC-PF-0813	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 2
PM27005-SRJC-PF-0814	RACK DE TUBERIAS-ESTRUCTURA CORTES Y DETALLES 3

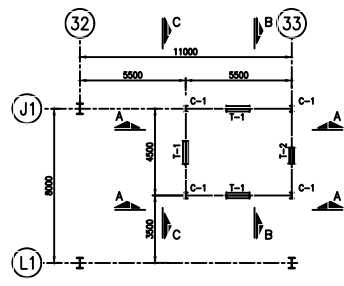
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1	25/10/11	APROBADO PARA CONSTRUCCION REVISADO DONDE SE INDICA		
0	25/08/11	APROBADO PARA CONSTRUCCION		
REV.	FECHA	DESCRIPCION	REVISADO	APROBADO

**PLANTA DE CICLO COMBINADO**

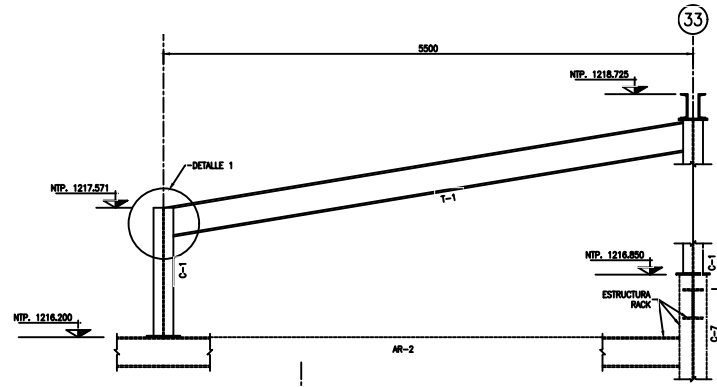
PROYECTO:		PLANTA DE CICLO COMBINADO	
TITULO:		RACK DE TUBERIAS EDIFICIO ELECTRICO PLANTAS Y ELEVACIONES	
APROBADO:		REALIZADO:	SRJ
CONSTRUIDO:		ESCALA:	INDICADA
FECHA DE RECEPCION:		Revisado por Ing. Supervisor	
IMPORTANTE:		Fecha:	
NO. DE CONTROL:		ORDEN DE COMPRA:	ITEM
		DOCUMENTO DE PROYECTO NO.:	PM27005-SRJC-PF-0816
		DOCUMENTO ADMINISTRACION NO.:	
		REV. NO.:	2
		REV. NO.:	

ESTADO DE DOCUMENTO COMO SE ANOTA	
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2	<input type="checkbox"/> CONFORME SIN COMENTARIOS
3	<input type="checkbox"/> CONFORME CON COMENTARIOS
4	<input type="checkbox"/> NO CONFORME
5	<input type="checkbox"/> TAL COMO SE CONSTRUYO

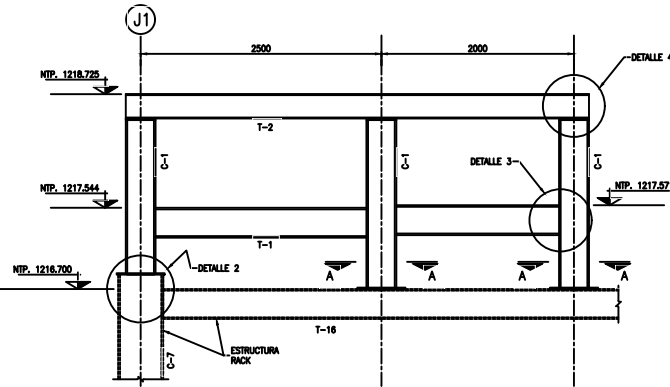




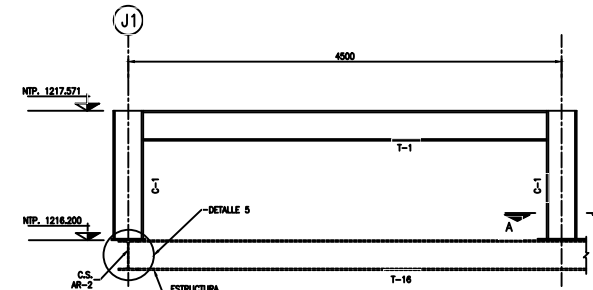
PLANTA SOPORTE S-1  
ESC. 1:25



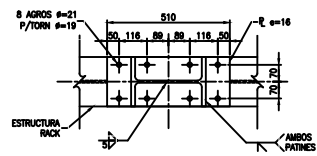
VISTA A-A  
ESC. 1:25



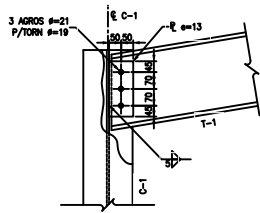
VISTA R-B  
ELEVACION MARCO EJE 33 ESC. 1:25



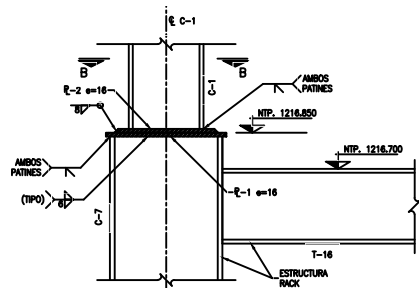
VISTA C-C  
ELEVACION H. 188.000 ESC. 1:25



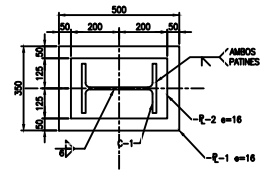
CORTE A-A  
ESC. 1:25



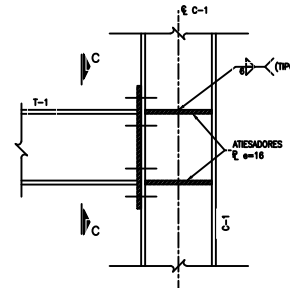
DETALLE 1  
ESC. 1:10



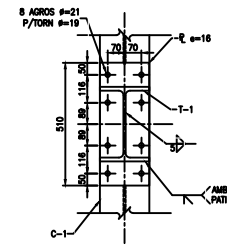
DETALLE 2  
ESC. 1:10



CORTE B-B  
ESC. 1:25



DETALLE 3  
ESC. 1:10



CORTE C-C  
ESC. 1:25

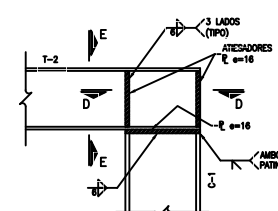
TABLA DE PERFILES						
MARCA	P E R F I L	PERO l/m	d	h	w	h <sub>w</sub>
C-1	RL-300	74.40	310	200	16.3	8.4
T-1	RL-300	74.40	310	200	16.3	8.4
T-2	CE-254	37.20	254	73	11.07	13.26

NOTAS:

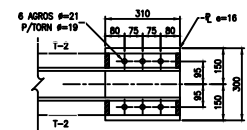
- 1.- PARA NOTAS GENERALES, ABREVIATURAS Y SIMBOLOGIA VER PLANO CST-PANTONES-SRJC-PC-0075.
- 2.- DIMENSIONES EN MILIMETROS.
- 3.- NIVELES Y COORDENADAS EN METROS.
- 4.- TODAS LAS DIMENSIONES SON SIMÉTRICAS CON RESPECTO A LAS LINEAS DEL CENTRO, EXCEPTO QUE SE INDIQUE LO CONTRARIO.
- 5.- ACERO ESTRUCTURAL ASTM A-36.
- 6.- TORILLERA ASTM A-325 TIPO 1 GALVANIZADO A TIPO 3.
- 7.- ESTE PLANO ES DE DISEÑO Y EN BASE A ESTE SE DEBEN ELABORAR PLANOS DE TALLER QUE CUMPLAN CON LOS REQUISITOS DEL AISI-LRFD Y AISI COMO MÍNIMO.
- 8.- LA DESCRIPCIÓN DE LOS PERFILES CORRESPONDE CON LA INDICADA EN EL "MANUAL DE CONSTRUCCIÓN EN ACERO" DEL INSTITUTO MEXICANO DE LA CONSTRUCCIÓN EN ACERO (IMCA).
- 9.- NO TOMAR MEDIDAS A ESCALA LAS CORTES SIEN AL DIBUJO.
- 10.- TRABAJAR ESTE PLANO EN COMPLETO CON LOS PLANOS DE REFERENCIA.

PLANOS DE REFERENCIA

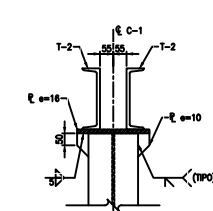
- PM27005-SRJC-PC-0006 RACK DE TUBERIAS-ESTRUCTURA PLANTAS
- PM27005-SRJC-PC-0007 RACK PRINCIPAL Y RACK TREN 1 Y 2
- PM27005-SRJC-PF-0007 RACK DE TUBERIAS-ESTRUCTURA ELEVACION
- PM27005-SRJC-PF-0008 RACK PRINCIPAL Y RACK TREN 1 Y 2
- PM27005-SRJC-PF-0008 RACK DE TUBERIAS-ESTRUCTURA ELEVACIONES
- RACK PRINCIPAL



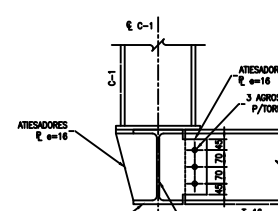
DETALLE 4  
ESC. 1:10



CORTE D-D  
ESC. 1:25



CORTE E-E  
ESC. 1:25



DETALLE 5  
ESC. 1:10

REV.	FECHA	DESCRIPCION	REVISOR	APROB.
A1	15/12/11	PARA REVISION INTERNA		
REVISIONES				

PROYECTO:	PLANTA DE CICLO COMBINADO
TITULO:	SOPORTES SECUNDARIOS ESTRUCTURA ELEVACIONES HOJA 1
ARREGLADO:	
COMPROBADO:	
REALIZADO:	
ESCALA:	INDICADA
ORDEN DE COMPRA:	ITEM
DOCUMENTO DE PROYECTO N°:	PM27005-SRJC-PF-0835
DOCUMENTO SUBASTADOR N°:	A1
NO. DE CONTROL:	

ESTADO DE DOCUMENTO COMO SE ANOTA

1  SOLO PARA INFORMACION

2  CONFORME SIN COMENTARIOS

3  CONFORME CON COMENTARIOS

4  NO CONFORME

5  TAL COMO SE CONSTRUYO

FECHA DE RECEPCION: Revisado por Ing. Supervisor

Fecha: \_\_\_\_\_

IMPORTANTE: \_\_\_\_\_

NO. DE CONTROL: \_\_\_\_\_