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**UNIVERSIDAD NACIONAL AUTÓNOMA
DE MÉXICO**

FACULTAD DE INGENIERÍA

**“ANÁLISIS Y DISEÑO DE UNA TORRE
AUTOSOPORTADA DE COMUNICACIÓN”**

T E S I S :
QUE PARA OBTENER EL TÍTULO DE :

INGENIERO CIVIL

P R E S E N T A :

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DIRECTOR DE TESIS :

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Señor
OSCAR GARCIA SEGURA
Presente

En atención a su solicitud me es grato hacer de su conocimiento el tema que propuso el profesor M.I. CARLOS MONTOYA BELTRAN, que aprobó esta Dirección, para que lo desarrolle usted como tesis de su examen profesional de INGENIERO CIVIL.

"ANÁLISIS Y DISEÑO DE UNA TORRE AUTOSOPORTADA DE COMUNICACIÓN"

- I. INTRODUCCION
- II. BASES DE DISEÑO
- III. CARGAS O ACCIONES PERMANENTES, VARIABLES Y ACCIDENTALES
- IV. ANÁLISIS DE LA ESTRUCTURA ANTE LAS ACCIONES PERMANENTES Y EVENTUALES
- V. DISEÑO
- VI. INSTALACIÓN DE LA ESTRUCTURA
- CONCLUSIONES
- BIBLIOGRAFIA

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Asimismo le recuerdo que la Ley de Profesiones estipula que deberá prestar servicio social durante un tiempo mínimo de seis meses como requisito para sustentar Examen Profesional.

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ING. CARLOS A. MONTOYA BELTRÁN
Presente.

El señor OSCAR GARCIA SEGURA de la carrera de INGENIERO CIVIL, me ha solicitado designar al profesor que le señale Tema de Tesis para su Examen Profesional.

En atención a esa solicitud ruego a usted se sirva formular el Tema solicitado y enviarlo a esta Dirección para comunicarlo oficialmente al interesado.

Doy a usted de antemano las más cumplidas gracias por su atención y le reitero las seguridades de mi consideración más distinguida.

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Dedicatoria.

Este trabajo representa la culminación del logro más valioso y trascendente de mi vida, es por esto que deseo dedicarlo:

A mi Padre:

José de Jesús García Sandoval.

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***“ANÁLISIS Y DISEÑO
DE UNA
TORRE AUTOSOPORTADA
DE
COMUNICACIÓN”.***

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Introducción.

a) Objetivos.

En este trabajo se presentarán las bases y criterios de diseño estructural para una torre de comunicación de tipo autoportada, el material de dicha estructura será acero, y su sección en planta será triangular. Esto se llevará a cabo, sin perder de vista que la estructura sea segura y también que resulte lo más económica posible, logrando su óptima funcionalidad.

b) Antecedentes.

Las torres de comunicación son estructuras diseñadas para soportar antenas emisoras y/o receptoras de señal, que sirven para la comunicación celular, telecomunicación, radiocomunicación, etc. Normalmente se localizan en zonas montañosas, aunque también es común encontrarlas dentro de las ciudades.

En cuanto a torres existen dos tipos:

- a) Torres arriostradas. (Fig. a. 1.).
- b) Torres autoportadas. (Fig. b. 1.).

a) Torres arriostradas.

Las torres arriostradas son aquellas que como su nombre lo indica, se encuentran arriostradas en toda su longitud o altura, por cables de acero tensados y anclados a la superficie del terreno o estructuras existentes, esto es con el fin de que la torre tenga la rigidez necesaria que evite grandes deflexiones, que podrían ocasionar que esta se colapse, ya que son estructuras muy esbeltas comparadas con las torres autoportadas.

Por otro lado, este tipo de estructuras son más económicas que las autoportadas, aunque para su instalación se requiere de terrenos de mayor dimensión, esto provoca en ocasiones la necesidad de valorar el costo del terreno contra el costo de una torre autoportada.

En cuanto a su mantenimiento las torres arriostradas son menos seguras que las torres autoportadas, ya que cuando se realiza el cambio de algún elemento estructural, se deben tener los cuidados necesarios, o de lo contrario la torre podría venirse abajo. Además se requiere un mayor mantenimiento preventivo en el retensado de cables y sustitución de los mismos, así como la revisión periódica en cuanto a su verticalidad

Los tamaños comúnmente diseñados son de (12m), (24m), (30m), (36m), (42m) y (48m). Así mismo en cuanto a los diseños extraordinarios podemos encontrar alturas desde (72m) hasta en algunos casos de (300m). Estas alturas extraordinarias se pueden alcanzar debido a que este tipo de estructuras resultan ser de mayor economía.

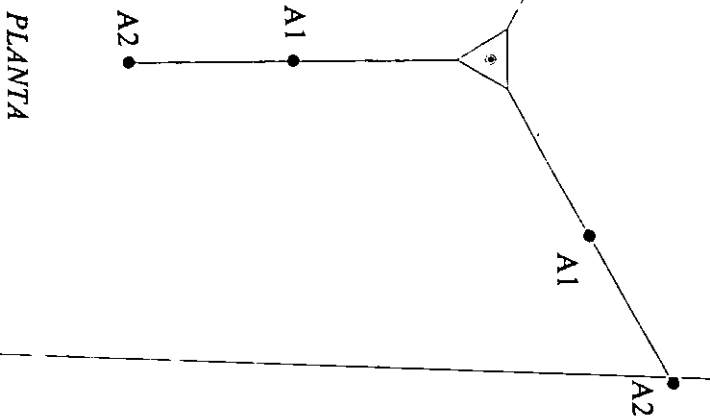
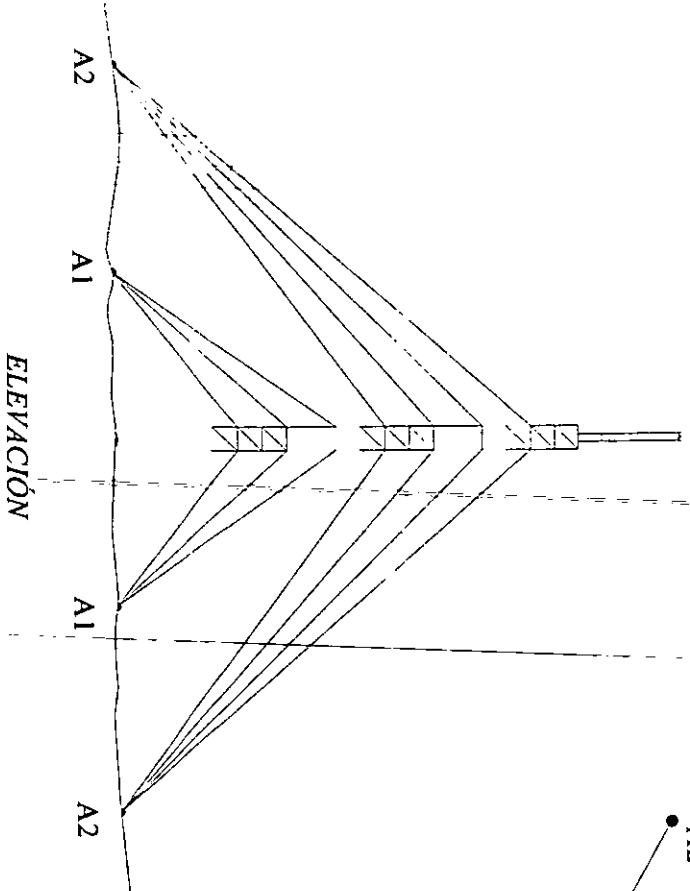


Fig. a. 1. Torre Arriostada.

b) Torres autoportadas.

Las torres autoportadas son aquellas que como su nombre lo menciona, se soportan así mismas. Estas estructuras tienden a ser anchas en su parte inferior, pueden ser de sección variable, siendo delgadas en su parte superior o bien de sección recta.

Las torres autoportadas resultan ser más costosas que las torres arriostradas debido a su conformación física y también a que están en posibilidades de soportar mayor carga.

Los tamaños comúnmente diseñados pueden variar desde (12m) hasta (72m). Así mismo en cuanto a los diseños extraordinarios podemos encontrar alturas desde (80m) hasta (150m).

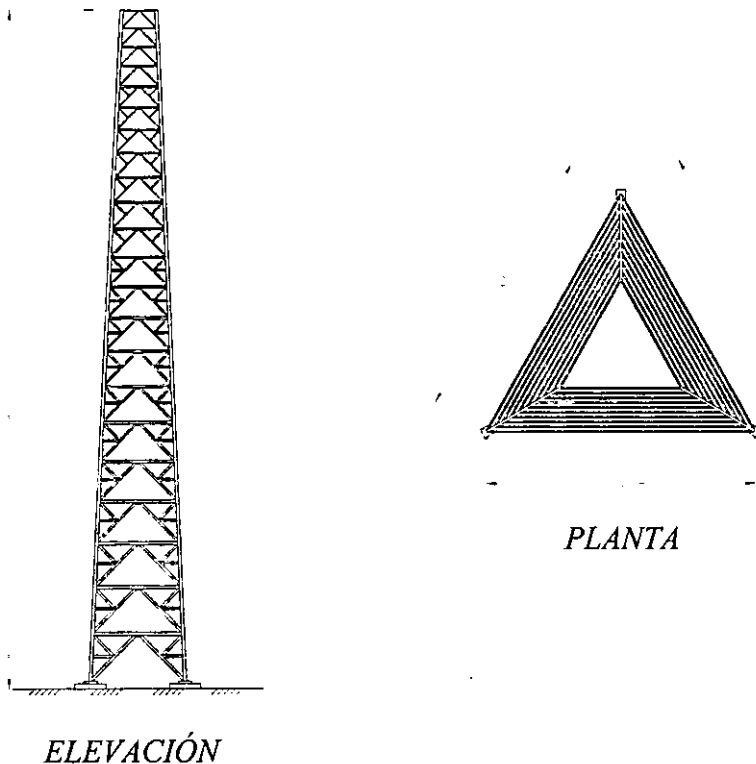


Fig. b. 1. Torre Autoportada.

c) Definiciones.

A continuación se ilustran los elementos estructurales de una torre autoportada.

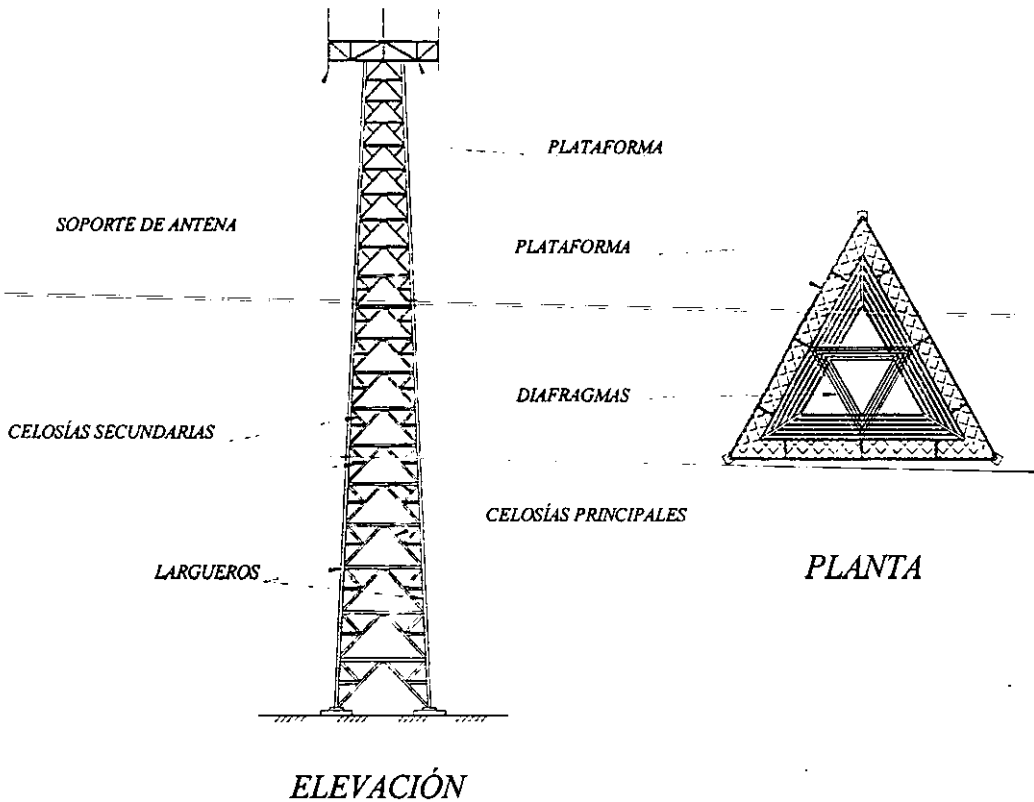
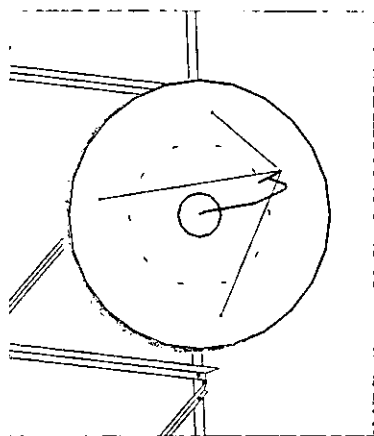


Fig. c. 1. Elementos de una torre autoportada

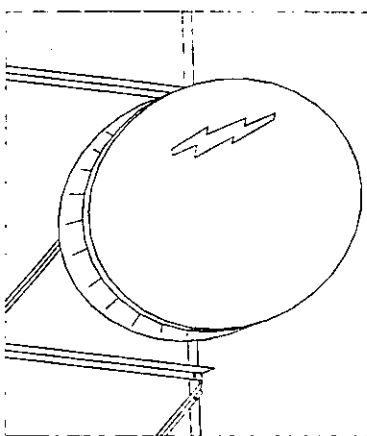
Donde:

- Los largueros son los soportes principales de la estructura (columnas).
- Las celosías principales son los elementos que unen a los largueros y se encargan de dar rigidez a la torre.
- Las celosías secundarias dan refuerzo a los segmentos de largueros y a celosías principales, evitando que estos se pandeen.
- Las plataformas sirven para la instalación de equipo que generalmente es de comunicación celular.
- Los diafragmas contrarrestan los efectos de torsión en la estructura.

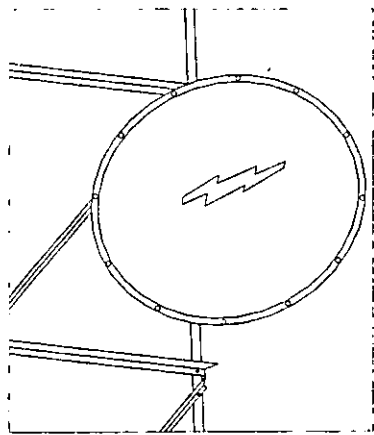
Las cargas sobre las torres pueden variar, dependiendo del uso o funcionamiento de las torres y del tipo de antena que se coloque en estas, esto debido a que existen varios tipos de antenas. En la siguiente figura se muestran algunas:



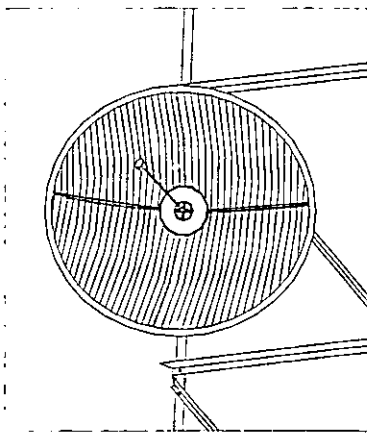
Antena Sólida o de Plato
(Parabólica).



Antena Radome Plano



Antena Radome Cónico



Antena de Rejilla

Fig. c. 2. Antenas.

Las antenas por su geometría, sufren un empuje debido al viento, siendo la *antena sólida o de plato* (antena parabólica) fig. c. 3, la que por su forma provoca mayores efectos sobre la torre. Los diámetros más comunes de esta son (0.6m), (1.20m), (2.40m), (3.20m) y (4.80m).



Fig. c. 3. Antena parabólica o de plato

Los efectos eólicos sobre las antenas son de diferentes magnitudes, dependiendo del tipo de antenas, por ejemplo: *antena sólida o de plato*, *radome plano*, *radome cónico* y *antena de rejilla*. Siendo la *antena sólida o de plato* la más afectada y la *antena de rejilla* la de menor impacto eólico.

I. Bases de diseño.

I. 1. Reglamentos y especificaciones.

Para el análisis por viento se tomó en cuenta el Manual de Diseño de Obras Civiles (Diseño por Viento), editado por la Comisión Federal de Electricidad (C. F. E.) en Octubre de 1993.

Así mismo para el análisis sísmico se consideró el Manual de Diseño de Obras Civiles (Diseño por Sismo), editado por la Comisión Federal de Electricidad (C. F. E.) en Octubre de 1993.

Para el estudio de fuerzas de viento sobre las antenas se consideraron las normas siguientes: Structural Standards for Steel Antenna Towers and Antenna Supporting Structures de junio de 1996, revisadas por la TIA (Telecommunications Industry Association) y EIA (Electronic Industries Association).

Para el diseño de la estructura, tanto de elementos principales como de conexiones se utilizó el American Institute of Steel Constructions (AISC, Novena Edición).

El proyecto se encuentra apegado al Reglamento de Construcciones del Departamento del Distrito Federal de 1987.

En tanto que para la cimentación se consideraron las Normas Técnicas Complementarias para Diseño y Construcción de Estructuras de Concreto así como las Normas Técnicas Complementarias para Diseño y Construcción de Cimentaciones.

I. 2. Materiales empleados.

Los materiales utilizados en este proyecto son:

Acero estructural A - 36 (36 000 lb/plg²) con un esfuerzo de fluencia (fy) de 2530 kg/cm² y módulo de elasticidad (Es) de 2 100 000 kg/cm²

Concreto de clase 1, con resistencia a la compresión (f'c) a los 28 días de edad de 350 Kg/cm²

El módulo de elasticidad será igual a:

$$E_c = 14000 \sqrt{f'_c} \quad \text{kg/cm}^2$$

$$E_c = 261916 \quad \text{kg/cm}^2$$

Acero de refuerzo con un esfuerzo de fluencia (f_y) de 4200 kg/cm^2 y módulo de elasticidad (E_s) de 2100000 kg/cm^2 .

II. Cargas o acciones permanentes, variables y accidentales.

La torre se diseñará con base en las siguientes acciones:

1) Carga muerta (permanente): Es el peso propio de la estructura incluyendo además el peso de antenas y equipos instalados sobre la torre.

2) Carga viva (variable): Es el peso de las personas y herramientas que se encontrarán en la estructura; ya sea instalando antenas o dando mantenimiento a estas y/o a la estructura.

3) Acciones eólicas (accidentales): Son las fuerzas del viento incidentes sobre la estructura y antenas.

4) Acciones sísmicas (accidentales): Son las fuerzas provocadas por movimientos telúricos, a las cuales puede someterse la estructura.

5) Es importante mencionar que la torre puede estar sometida a grandes cambios de temperatura o cargas adicionales como granizo y nieve, esto dependerá principalmente de la ubicación de la estructura.



Figura: II. 1. Torre expuesta a las acciones de nieve.

II. 1. Análisis por viento.

Para determinar las acciones ejercidas por el viento, es necesario definir que tipo de análisis se requiere; este análisis puede ser estático o dinámico, dependiendo principalmente de la rigidez y geometría de la estructura, además de las características topográficas, regionales y locales del sitio.

Para la determinación de las acciones de diseño es necesario realizar las clasificaciones y pasos siguientes:

II. 1. 1. Clasificación de la estructura según su importancia.

La seguridad necesaria para que una construcción cumpla con las funciones a las cuales fue destinada puede establecerse por niveles, dichos niveles se asocian con velocidades del viento que tengan una probabilidad de ser excedidas y a partir de esta se clasifican a las estructuras atendiendo al grado de seguridad aconsejable de la siguiente manera:

Grupo A

Estructuras en las que se recomienda un grado de seguridad elevado. Aquellas cuyo funcionamiento es imprescindible y deben continuar operando después de la ocurrencia de vientos fuertes tales como huracanes y esas que en caso de fallar causarían la pérdida de un número importante de vidas, altos perjuicios económicos y culturales. Algunos ejemplos son los hospitales, escuelas, estadios, museos, torres y postes de transmisión principal, inmuebles de telecomunicaciones, estaciones de bomberos, etc.

Grupo B

Estructuras en las que se recomienda un grado de seguridad moderado. Aquellas que en caso de fallar representan un bajo riesgo de pérdidas humanas y ocasionarían daños materiales de magnitud intermedia. Este es el caso de las plantas industriales, bodegas ordinarias, gasolineras, comercios, restaurantes, casas para habitación, hoteles, oficinas, etc.

Grupo C

Estructuras en las que se recomienda un bajo grado de seguridad. Son aquellas cuya falla no implica graves consecuencias. Es el caso de bodegas provisionales, cimbras, carteles, muros aislados, bardas no mayores a 2.5 m de altura, etc.

II. 1. 2. Clasificación de la estructura según su respuesta.

De acuerdo con la sensibilidad de las construcciones ante los efectos de las ráfagas de viento y a su correspondiente respuesta dinámica, las estructuras se clasifican en cuatro tipos.

Tipo 1

Estructuras poco sensibles a las ráfagas y a los efectos dinámicos del viento. Son aquellas en que la relación de aspecto λ , (definida como el cociente entre la altura y la dimensión menor en planta), es menor o igual a cinco y cuyo período natural de vibración es menor o igual a un segundo. Pertenecen a este tipo, bodegas, naves industriales, teatros, auditorios, puentes cortos, viaductos, casas habitación, etc.

Tipo 2

Estructuras que por su alta relación de aspecto λ , o también por sus dimensiones reducidas en su sección transversal, son especialmente sensibles a las ráfagas de viento de corta duración (entre 1 y 5 segundos) y cuyos períodos naturales largos favorecen la ocurrencia de oscilaciones importantes en la dirección del viento. Dentro de este tipo se encuentran las estructuras con relación de aspecto λ , mayor que cinco o con período fundamental mayor que un segundo. Se incluyen las torres de celosía atirantadas y las autosoportadas para líneas de transmisión, chimeneas, tanques elevados, antenas, bardas, anuncios, etc.

Tipo 3

Estructuras que además de reunir las características del Tipo 2, presentan oscilaciones importantes transversales al flujo del viento provocadas por la aparición periódica de vórtices o remolinos. En este tipo se incluyen las construcciones cilíndricas o prismáticas esbeltas, como chimeneas, arbotantes para iluminación, tuberías exteriores o elevadas, etc.

Tipo 4

Estructuras que por su forma o por lo largo de sus períodos de vibración presentan problemas aerodinámicos especiales. Entre ellas se hallan las formas aerodinámicamente inestables como cables de línea de transmisión, tuberías colgantes, antenas parabólicas, grandes puentes, etc.

II. 1. 3. Clasificación del terreno según su rugosidad.

Para determinar la categoría del terreno, se tiene que clasificar conforme a su rugosidad.

Categoría	Descripción	Ejemplos	Limitaciones
1	Terreno abierto, prácticamente plano y sin obstrucciones. <i>(No rugoso).</i>	Franjas costeras planas, zonas de pantanos, campos aéreos, pastizales y tierras de cultivo sin setos o bardas alrededor. Superficies nevadas planas.	La longitud mínima de este tipo de terreno en la dirección del viento debe ser de (2000m) o 10 veces la altura de la construcción por diseñar, la que sea mayor.
2	Terreno plano u ondulado con pocas obstrucciones. <i>(Poco rugoso).</i>	Campos de cultivo o granjas con pocas obstrucciones tales como setos o bardas alrededor, árboles y construcciones dispersas.	Las obstrucciones tienen alturas de 1.5 a (10m), en una longitud mínima de (1500m).
3	Terreno cubierto por numerosas obstrucciones estrechamente espaciadas. <i>(Rugoso).</i>	Áreas urbanas, suburbanas y de bosque, o cualquier terreno con numerosas obstrucciones estrechamente espaciadas. El tamaño de las construcciones corresponde al de las casas y viviendas.	Las obstrucciones presentan alturas de 3 a (5m). La longitud mínima de este tipo de terreno en la dirección del viento debe ser de (500m) ó 10 veces la altura de la construcción, la que sea mayor.
4	Terreno con numerosas obstrucciones largas, altas y estrechamente espaciadas. <i>(Muy rugoso).</i>	Centros de grandes ciudades y complejos industriales bien desarrollados.	Por lo menos el 50% de los edificios tiene una altura mayor que (20m). Las obstrucciones miden de (10 a 30m) de altura. La longitud mínima de este tipo de terreno en la dirección del viento debe ser la mayor entre (400m) y 10 veces la altura de la construcción.

Tabla. II. 1. 1.

II. 1. 4. Clasificación de la estructura según su tamaño.

Clase A

Todo elemento de recubrimiento de fachadas, de ventanerías y de techumbres y sus respectivos sujetadores. Todo elemento estructural aislado, expuesto directamente a la acción del viento. Así mismo, todas las construcciones cuya mayor dimensión, ya sea horizontal o vertical, sea menor que 20m.

Clase B

Todas las construcciones cuya mayor dimensión, ya sea horizontal o vertical, varíe entre 20 y 50m.

Clase C

Todas las construcciones cuya mayor dimensión, ya sea horizontal o vertical, sea mayor que 50m.

II. 1. 5. Determinación de la velocidad de diseño.

$$V_D = F_T F_\alpha V_R \quad (\text{km/h})$$

En donde:

F_T es un factor que depende de la topografía del sitio, adimensional. (Tabla II. 1. 2.).

F_α es el factor que toma en cuenta el efecto combinado de las características de exposición locales, del tamaño de la construcción y de la variación de la velocidad con la altura, adimensional. (Tablas II. 1. 3. y II. 1. 4.).

V_R es la velocidad regional que le corresponde al sitio en donde se construirá la estructura, en km/h. (Tabla II. 1. 5.).

- Factor de topografía, F_T

Este factor toma en cuenta el efecto topográfico local del sitio en donde desplantará la estructura.

Sitios	Topografía	F_T
Protegidos	Base de promontorios y faldas de serranías del lado de sotavento	0.8
	Valles cerrados	0.9
Normales	Terreno prácticamente plano, campo abierto, ausencia de cambios topográficos importantes, con pendientes menores que 5%.	1.0
Expuestos	Terrenos inclinados con pendientes entre 5 y 10%, valles abiertos y litorales planos.	1.1
	Cimas de promontorios, colinas o montañas, terrenos con pendientes mayores que 10%, cañadas cerradas y valles que formen un embudo o cañón, islas.	1.2

Tabla II. 1. 2.

- Factor de exposición, F_a

$$F_a = F_C F_{rz}$$

En donde:

F_C es el factor que determina la influencia del tamaño de la construcción, adimensional. (Tabla II. 1. 3.). Para aplicar la tabla véase el inciso II. 1. 4.

F_{rz} es el factor que establece la variación de la velocidad del viento con la altura Z en función de la rugosidad del terreno de los alrededores, adimensional.

- Factor de tamaño, F_C

El factor de tamaño, F_C , es el que toma en cuenta el tiempo en el que la ráfaga de viento actúa de manera efectiva sobre una construcción de dimensiones dadas.

Considerando la clasificación de las estructuras según su tamaño, se puede obtener el factor de tamaño.

Clase de estructura	F_C
A	1.00
B	0.95
C	0.90

Tabla. II. 1. 3.

- Factor de rugosidad y altura, F_{rz}

Este factor establece la variación de la velocidad del viento con la altura Z . Dicha variación está en función de la categoría del terreno y del tamaño de la construcción.

Se obtiene de acuerdo con las siguientes expresiones:

$$F_{rz} = 1.56 \left[\frac{10}{\delta} \right]^\alpha \quad \text{si } Z \leq 10$$

$$F_{rz} = 1.56 \left[\frac{Z}{\delta} \right]^\alpha \quad \text{si } 10 < Z < \delta$$

$$F_{rz} = 1.56 \quad \text{si } Z \geq \delta$$

En donde:

δ es la altura medida a partir del nivel del terreno de desplante, por encima de la cual la variación de la velocidad del viento no es importante y se puede suponer constante; a esta altura se le conoce como altura gradiente; δ y Z están dadas en metros.

α es el exponente que determina la forma de la variación de la velocidad del viento con la altura y es adimensional.

Los coeficientes α y δ están en función de la rugosidad del terreno y del tamaño de la construcción. En la tabla II. 1. 4. se consignan los valores que se recomiendan para estos coeficientes.

Categoría del Terreno	α			δ (m)
	Clase de estructura			
	A	B	C	
1	0.099	0.101	0.105	245
2	0.128	0.131	0.138	315
3	0.156	0.160	0.171	390
4	0.170	0.177	0.193	455

Tabla. II. 1. 4.

- Velocidad Regional, V_R

La velocidad regional del viento, es la máxima velocidad media probable de presentarse con un cierto período de recurrencia (conocido como período de retorno) en una zona o región determinada del país.

La velocidad regional se determina tomando en consideración tanto la localización geográfica del sitio de desplante de la estructura como su destino.

A continuación se presenta una tabla con las principales ciudades del país y sus correspondientes velocidades regionales para diferentes períodos de retorno (V_{10} , V_{50} y V_{200} son los períodos de retorno de 10, 50 y 200 años respectivamente), alturas sobre el nivel del mar y temperaturas medias anuales.

Ciudad	Velocidades (km/h).			A.S.N.M. (m).	Temp. media anual (°C).
	V ₁₀	V ₅₀	V ₂₀₀		
Acapulco, Gro.	129	162	181	28	27.5
Aguascalientes, Ags.	118	141	160	1908	18.2
Campeche, Camp.	98	132	159	5	26.1
Cd. Guzmán, Jal.	101	120	132	1507	21.5
Cd. Juárez, Chih.	116	144	158	1144	17.1
Cd. Obregón, Son.	147	169	186	100	26.1
Cd. Victoria, Tamps.	135	170	197	380	24.1
Coatzacoalcos, Ver.	117	130	145	14	26.0
Colima, Col.	105	128	147	494	24.8
Colotlán, Jal.	131	148	161	1589	21.4
Comitán, Chis.	72	99	124	1530	18.2
Cozumel, Q. Roo.	124	158	185	10	25.5
Cuernavaca, Mor.	93	108	120	1560	20.9
Culiacán, Sin.	94	118	140	84	24.9
Chapingo, Edo. Méx.	91	110	126	2250	15.0
Chetumal, Q. Roo.	119	150	180	3	26.0
Chihuahua, Chih.	122	136	147	1423	18.7
Chilpancingo, Gro.	109	120	131	1369	20.0
Durango, Dgo.	106	117	126	1889	17.5
Ensenada, B.C.	100	148	190	13	16.7
Guadalajara, Jal.	146	164	176	1589	19.1
Guanajuato, Gto.	127	140	148	2050	17.9
Guaymas, Son.	130	160	190	44	24.9
Hermosillo, Son.	122	151	179	237	25.2
Jalapa, Ver.	118	137	152	1427	17.9
La Paz, B.C.	135	171	200	10	24.0
Lagos de Moreno, Jal.	118	130	141	1942	18.1
León, Gto.	127	140	148	1885	19.2
Manzanillo, Col.	110	158	195	8	26.6
Mazatlán, Sin.	145	213	240	8	24.1
Mérida, Yuc.	122	156	186	9	25.9
Mexicali, B.C.	100	149	190	1	22.2
México, D.F.	98	115	129	2240	23.4
Monclova, Coah.	123	145	159	591	21.6
Monterrey, N.L.	123	143	158	538	22.1
Morelia, Mich.	79	92	102	1941	17.6
Nva. Casas Gdes, Chih.	117	134	148	1550	17.6
Oaxaca, Oax.	104	114	122	1550	20.6
Orizaba, Ver.	126	153	172	1284	19.0
Pachuca, Hgo.	117	128	137	2426	14.2
Parral de Hgo., Chih.	121	141	157	1661	17.7
Piedras Negras, Coah.	137	155	168	220	21.6
Progreso, Yuc.	103	163	198	8	25.4
Puebla, Pue.	93	106	117	2150	17.3

Ciudad	Velocidades (km/h).			A.S.N.M. (m).	Temp. media anual (°C).
	V ₁₀	V ₅₀	V ₂₀₀		
Puerto Cortés, B.C.	129	155	172	5	21.4
Puerto Vallarta, Jal.	108	146	171	2	26.2
Querétaro, Qro.	103	118	131	1842	18.7
Río Verde, SLP.	84	111	130	987	20.9
Salina Cruz, Oax.	109	126	146	6	26.0
Saltillo, Coah.	111	124	142	1609	17.7
S. C. de la Casas, Chis.	75	92	105	2276	14.8
San Luis Potosí, SLP.	126	141	153	1877	17.9
S. la Marina, Tamps.	130	167	204	25	24.1
Tampico, Tamps.	129	160	193	12	24.3
Tamuín, SLP.	121	138	155	140	24.7
Tapachula, Chi.	90	111	132	182	26.0
Tepic, Nay.	84	102	115	915	26.2
Tlaxcala, Tlax.	87	102	113	2252	16.2
Toluca, Edo. Méx.	81	93	102	2680	12.7
Torreón, Coah.	136	168	193	1013	20.5
Tulancingo, Hgo.	92	106	116	2222	14.9
Tuxpan, Ver.	122	151	172	14	24.2
Tuxtla Gutiérrez, Chis.	90	106	120	528	24.7
Valladolid, Yuc.	100	163	198	8	26.0
Veracruz, Ver.	150	175	194	16	25.2
Villahermosa, Tab.	114	127	138	10	26.8
Zacatecas, Zac.	110	122	131	2612	13.5

Tabla. II. 1. 5.

II. 1. 6. Presión dinámica de base, q_z .

Cuando el viento actúa sobre un obstáculo, genera presiones sobre su superficie que varían según la intensidad de la velocidad y la dirección del viento. La presión que ejerce el flujo del viento sobre una superficie plana perpendicular a él se denomina comúnmente *presión dinámica de base* y se determina con la siguiente ecuación:

$$q_z = 0.0048 G V_D^2$$

en donde:

G es el factor de corrección por temperatura y por altura con respecto al nivel del mar, adimensional.

V_D es la velocidad de diseño, en km/h, definida en el inciso II. 1. 5.

q_z es la presión dinámica de base a una altura Z sobre el nivel del terreno, en kg/m².

El factor de 0.0048 corresponde a un medio de la densidad del aire.

El valor de G se obtiene de la expresión:

$$G = \frac{0.392 \Omega}{273 + \tau}$$

en donde:

Ω es la presión barométrica, en mm de Hg.

τ es la temperatura ambiental en °C.

En la tabla siguiente se presenta la relación entre los valores de la altitud (h_m), en metros sobre el nivel del mar (msnm), y la presión barométrica (Ω).

Altitud (msnm).	Presión barométrica (mm de Hg).
0	760
500	720
1000	675
1500	635
2000	600
2500	565
3000	530
3500	495

Tabla II. 1. 6. Relación entre altitud y presión barométrica.
Podrá interpolarse en valores intermedios de altitud

- **Análisis Estático**

Los empujes medios que se evalúan con este procedimiento son aplicables al diseño de las estructuras pertenecientes al Tipo 1.

El método estático sólo puede usarse para diseñar estructuras o elementos estructurales poco sensibles a la acción turbulenta del viento. Esto se satisface cuando:

- a) La relación $H / D \leq 5$, en donde H es la altura y D la dimensión mínima de la base.
- b) El período fundamental de la estructura es menor o igual que un segundo.

Los empujes dinámicos correspondientes a las estructuras Tipo 2 y 3 se determinan conforme al análisis dinámico.

- Análisis Dinámico.

Este procedimiento permite evaluar los empujes ocasionados por la interacción dinámica entre el flujo del viento y las estructuras, principalmente las pertenecientes a los Tipos 2 y 3 definidos en el inciso II. 1. 2.

En particular, este método deberá emplearse en el diseño de las estructuras que cumplan con alguna de las siguientes condiciones:

- a) La relación $H / D > 5$, en donde H es la altura de la construcción y D la dimensión mínima de la base.
- b) El periodo fundamental de la estructura es mayor que 1 segundo.

- Velocidad de diseño, V_D .

La velocidad de diseño se calculará como fue explicado en el inciso II. 1. 5. Sin embargo, para el análisis dinámico, el factor que considera el tamaño de la estructura F_c , y del cual es función el factor de exposición F_a , se tomará igual a uno.

- Presiones en la dirección del viento.

La presión total en la dirección del viento se calculará con la siguiente ecuación:

$$P_z = F_g C_a q_z$$

en donde:

F_g es el factor de respuesta dinámica debida a ráfagas, adimensional.

C_a es el coeficiente de arrastre que depende de la forma de la estructura, adimensional.

q_z es la presión dinámica de base en la dirección del viento, en kg/m^2 , a una altura Z, en m, sobre el nivel del terreno.

- Fuerzas en la dirección del viento.

Las fuerzas generadas en la dirección del viento sobre las estructuras prismáticas de los Tipos 2 y 3, se calcularán multiplicando la presión P_z por el área A_z , en m^2 .

La fuerza total F sobre la estructura, en kg, resultará de sumar cada una de las fuerzas que actúan sobre el área expuesta de la estructura o parte de ella, a una altura z dada.

$$F = \sum F_z = \sum P_z A_z$$

El momento de volteo máximo de diseño se determinará mediante la suma de los momentos producidos por cada una de las fuerzas F_z .

- Factor de respuesta dinámica debida a ráfagas.

A fin de calcular la fuerza de diseño en la dirección del viento, para las estructuras Tipos 2 y 3 se considerarán dos componentes: uno llamado medio debido a la acción media del viento asociada a un lapso de promediación de 3 segundos, y otro dinámico, caracterizado por el valor pico de la acción del viento. Estos dos componentes se toman en cuenta implícitamente en el factor de respuesta dinámica debida a ráfagas.

En el diseño de construcciones del Tipo 2 y 3 se tomarán en cuenta los efectos dinámicos debidos a la turbulencia en la dirección del viento, utilizando el factor F_g , el cual se obtiene con la siguiente ecuación:

$$F_g = \frac{1}{g^2} \left[1 + g_p \left(\frac{\sigma}{\mu} \right) \right]$$

en donde:

g es un factor de ráfaga, variable con la altura Z , adimensional.

g_p es el factor pico o de efecto máximo de la carga por viento, adimensional.

σ/μ es la relación entre la desviación estándar (raíz cuadrada del valor cuadrático medio) de la carga por viento y el valor medio de la carga por viento, adimensional.

La variación del factor de ráfaga con la altura Z se calcula con las siguientes expresiones:

$$g = k' \left[\frac{10}{\delta} \right]^n \quad \text{si } Z \leq 10$$

$$g = k' \left[\frac{Z}{\delta} \right]^n \quad \text{si } 10 < Z < \delta$$

$$g = k' \quad \text{si } Z \geq \delta$$

En donde las variables k' y η , adimensionales, dependen de la rugosidad del sitio de desplante, y δ es la altura gradiente en m. Estas variables están definidas en la siguiente tabla:

Categoría	1	2	3	4
k'	1.224	1.288	1.369	1.457
η	-0.032	-0.054	-0.096	-0.151
δ	245	315	390	455

Tabla: II. 1. 7.

La relación σ / μ , que representa la variación de la carga debida a la turbulencia del viento, se calculará con:

$$\frac{\sigma}{\mu} = \sqrt{\frac{k_r}{C_a} \left(B + \frac{SE}{\zeta} \right)}$$

en donde:

k_r es un factor relacionado con la rugosidad del terreno:
Para terrenos con:

- Categoría 1 = 0.06
- Categoría 2 = 0.08
- Categoría 3 = 0.10
- Categoría 4 = 0.14

ζ es el coeficiente de amortiguamiento crítico:

Para construcciones formadas por:

- Armaduras = 0.005
- Marcos de acero = 0.01
- Marcos de concreto = 0.02

B es el factor de excitación de fondo.

S es el factor de reducción por tamaño.

E es el factor que representa la relación de la energía de ráfaga con la frecuencia natural de la estructura.

$C_{\alpha'}$ es un factor que se define con las siguientes expresiones:

$$C_{\alpha'} = 3.46(F_T)^2 \left[\frac{10}{\delta} \right]^{2\alpha'} \quad \text{si } H \leq 10$$

$$C_{\alpha'} = 3.46(F_T)^2 \left[\frac{H}{\delta} \right]^{2\alpha'} \quad \text{si } 10 < H < \delta$$

$$C_{\alpha'} = 3.46(F_T)^2 \quad \text{si } H \geq \delta$$

en donde:

F_T es el factor de topografía.

δ es la altura gradiente, en m. (Tabla: II. 1. 7.).

H es la altura total de la construcción, en m.

α' es igual a 0.13, 0.18, 0.245 ó 0.31 para la categoría del terreno 1, 2, 3 ó 4, respectivamente.

Todas las variables que intervienen en la ecuación de σ/μ son adimensionales.

Los parámetros B, S, E y g_p , pueden calcularse con ayuda de las gráficas siguientes:

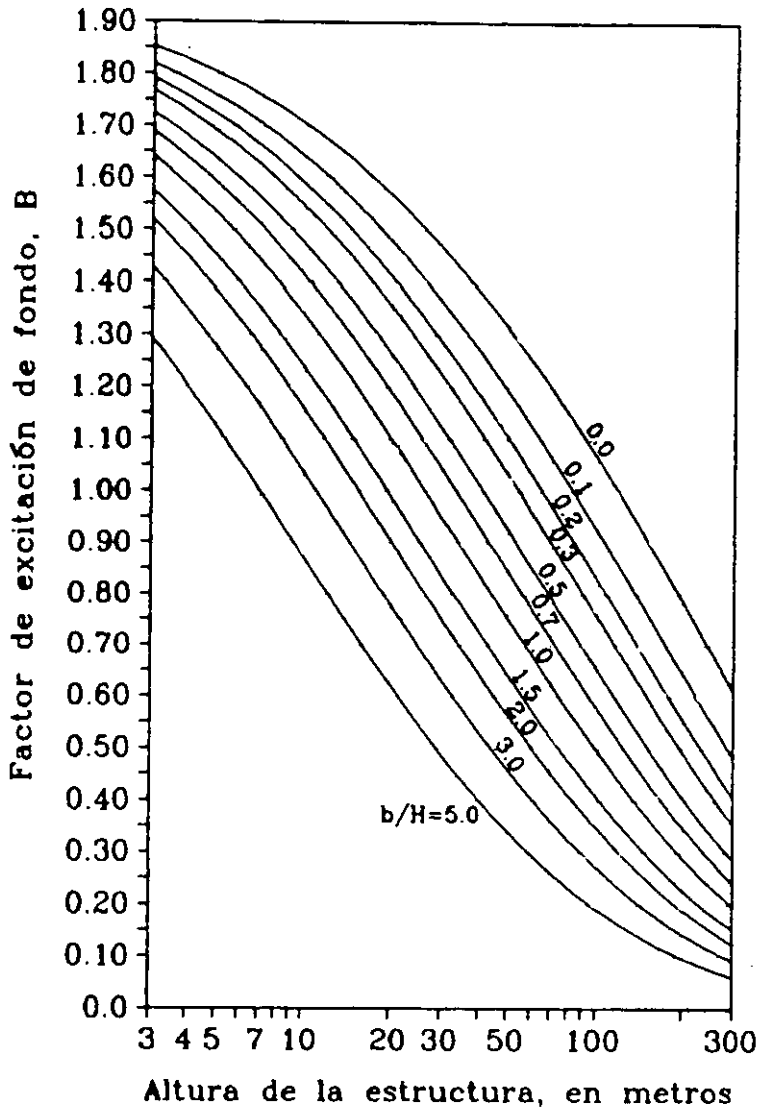


Figura: II. 1. 8. a. Parámetros que sirven para calcular el factor de respuesta dinámica

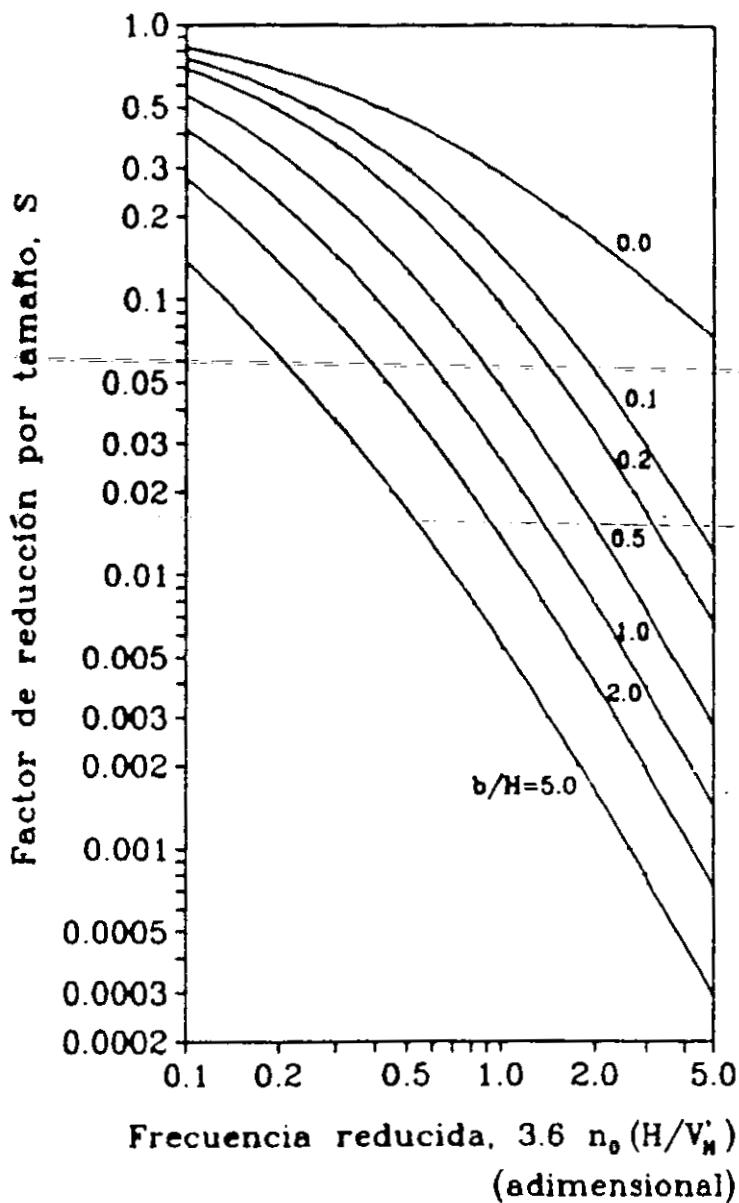


Figura: II. 1. 8. b. Parámetros que sirven para calcular el factor de respuesta dinámica

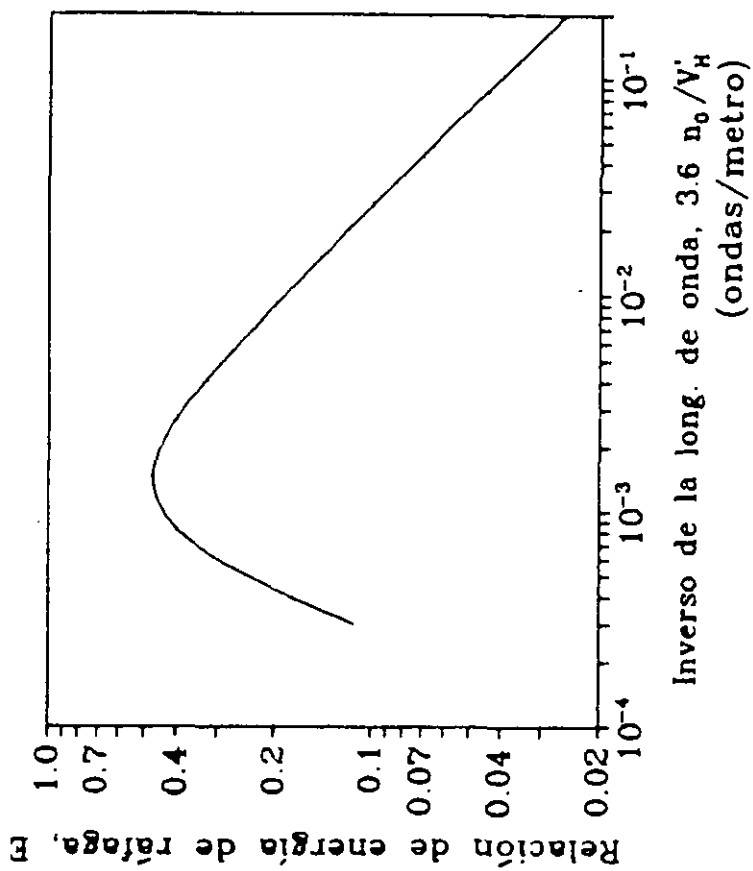


Figura: II. 8. c. Parámetros que sirven para calcular el factor de respuesta dinámica

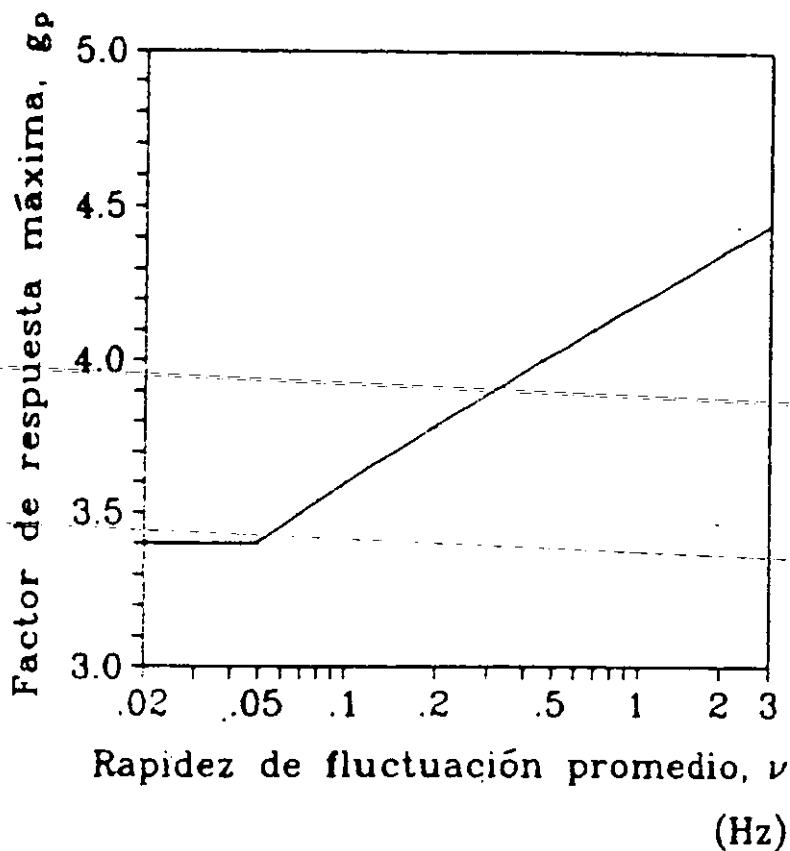


Figura: II. 1. 8. d. Parámetros que sirven para calcular el factor de respuesta dinámica

De las gráficas anteriores (II. 1. 8. a, b, c y d):

b/H es la relación entre el ancho b , y la altura H , de la construcción, ambos en metros y corresponden al lado de barlovento.

La relación $(3.6 n_0 H)/V_H$ es la frecuencia reducida, adimensional.

en donde:

n_0 es la frecuencia natural de vibración de la estructura, en Hz.

V'_H es la velocidad media de diseño del viento, en km/h. Dicha velocidad se calcula para la altura más elevada de la estructura, H, en m, y se determina mediante la ecuación siguiente:

$$V'_H = \frac{1}{g_H} V_H$$

en donde:

g_H es el factor de ráfaga y se calcula para $Z = H$.

V_H es la velocidad de diseño (V_D), para $Z = H$, en km/h.

Así mismo, en la figura II. 1. 8. c. aparece el número de ondas $(3.6 n_0)/V'_H$, en ondas/metro, en donde n_0 está en Hz. y V'_H en km/h, determinados en el párrafo anterior.

El factor de pico, g_p , figura (II. 1. 8. d), se obtiene en función del coeficiente de rapidez de fluctuación promedio v , en Hz, el cual se define mediante:

$$v = n_0 \sqrt{\frac{S E}{S E + \zeta B}}$$

Los términos que aparecen en esta fórmula, ya se establecieron con anterioridad.

- Fuerza de arrastre de diseño.

Para el viento que actúa sobre cualquier cara de la torre, la fuerza de arrastre de diseño deberá calcularse por medio de la ecuación siguiente:

$$F_a = C_a A_z q_z$$

en donde:

F_a es la fuerza de arrastre, en kg, que actúa paralelamente a la dirección del viento y es variable con la altura.

C_a es el coeficiente de arrastre en la dirección del flujo del viento, adimensional.

A_z es el área de los miembros de la cara frontal, a una altura Z, proyectada perpendicularmente a la dirección del viento, en m².

q_z es la presión dinámica de base del viento a la altura Z , en kg/m^2 .

Los valores del coeficiente de arrastre C_a , para torres de celosía con diferentes arreglos se presentan en las siguientes tablas:

Solidez de la cara frontal (ϕ)	Coeficiente de arrastre (C_a)	
	Torres de sección cuadrada.	Torres de sección triangular equilátera.
0.1	3.5	3.1
0.2	2.8	2.7
0.3	2.5	2.3
0.4	2.1	2.1
≥ 0.5	1.8	1.9

Tabla: II. 1. 9. Coeficiente de arrastre C_a , para torres de celosía con sección transversal cuadrada o triangular equilátera con miembros de lados planos.

Solidez de la cara frontal (ϕ)	Coeficiente de arrastre (C_a)	
	Partes de la torre dentro del flujo subcrítico. $bV_D < 3\text{m}^2/\text{s}$ (Cualquier dirección del viento).	Partes de la torre dentro del flujo supercrítico. $bV_D \geq 6\text{m}^2/\text{s}$ (Cualquier dirección del viento).
0.05	1.8	1.1
0.1	1.7	1.1
0.2	1.6	1.1
0.3	1.5	1.1
0.4	1.5	1.1
≥ 0.5	1.4	1.2

Tabla: II. 1. 10. Coeficiente de arrastre C_a , para torres de celosía con sección transversal triangular equilátera con miembros de sección transversal circular.

Nota: En las tablas II. 1. 9. y II. 1. 10.

ϕ es la relación de solidez definida como el cociente entre el área sólida y el área total encerrada por la cara frontal.

b es el diámetro promedio de los elementos de sección circular, en metros.

V_D es la velocidad de diseño del viento, convertida a m/s .

Para valores intermedios de bV_D se permite la interpolación lineal.

II. 1. 7. Viento sobre antenas.

Las fuerzas que actúan sobre una antena parabólica son las siguientes:

La fuerza axial (F_A), actúa a lo largo del eje de la antena. La fuerza lateral (F_S), actúa perpendicularmente al eje de la antena en el plano del eje de la antena y el vector del viento. El momento torsionante (M), actúa en el plano que contiene a F_A y F_S .

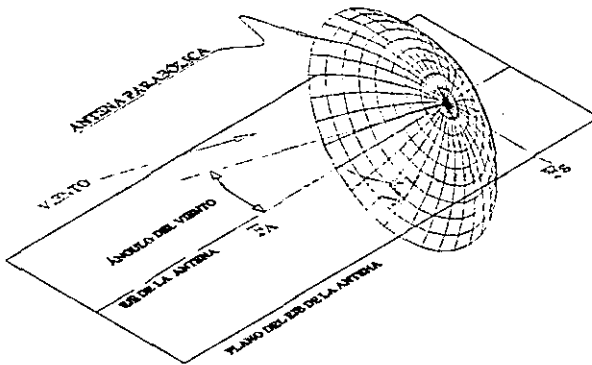
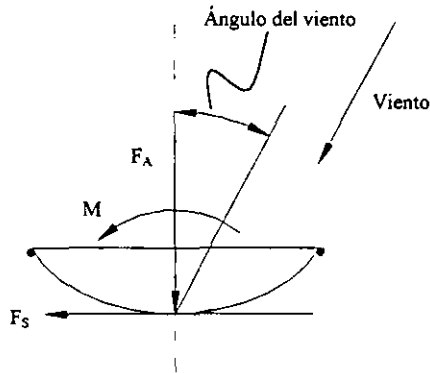


Fig. II. 1. 11. Fuerzas de viento sobre una antena parabólica, (vista en planta y 3D).

Las magnitudes F_A , F_S y M dependen de la presión dinámica del viento, el área de la proyección frontal de la antena y las características aerodinámicas de la propia antena. Las características aerodinámicas varían con el ángulo del viento. Los valores de F_A , F_S y M deberán calcularse con las siguientes ecuaciones:

$$F_A = C_A A K_z G_H V^2 \quad (1b)$$

$$F_s = C_s A K_z G_H V^2 \quad (\text{lb})$$

$$M = C_M A D K_z G_H V^2 \quad (\text{ft-lb})$$

donde:

G_H es el factor de ráfaga.

Para estructuras de celosía, el factor de ráfaga deberá ser calculado de la siguiente manera:

$$G_H = 0.65 + \frac{0.60}{\left(\frac{h}{10}\right)^{2/7}} \quad \text{para } h \text{ en metros}$$

$$G_H = 0.65 + \frac{0.60}{\left(\frac{h}{33}\right)^{2/7}} \quad \text{para } h \text{ en pies}$$

$$1.00 \leq G_H \leq 1.25$$

h es la altura total de la estructura. (m ó ft).

A es el área de la parábola, (m^2 ó ft^2).

D es el diámetro de la parábola, (m ó ft).

V es la velocidad regional, (m/s ó mph).

K_z es el factor de exposición con z igual a la altura deseada a partir del origen.

$$K_z = \left(\frac{z}{10}\right)^{2/7} \quad \text{para } z \text{ en metros}$$

$$K_z = \left(\frac{z}{33}\right)^{2/7} \quad \text{para } z \text{ en pies}$$

$$1.00 \leq K_z \leq 2.58$$

z es la altura desde el nivel del piso hasta el punto medio de la sección deseada, (m ó ft).

C_A , C_s y C_M son coeficientes que a continuación se presentan:

Wind Force Coefficients for Typical Paraboloid Without Radome

Wind Force Coefficients for Typical Paraboloid With Radome

Wind Angle

Wind Angle

Θ (Deg)	C _A	C _S	C _M
0	0.00397	0.00000	0.000000
10	0.00394	-0.00012	-0.000065
20	0.00396	-0.00013	-0.000097
30	0.00398	-0.00008	-0.000108
40	0.00408	0.00002	-0.000137
50	0.00426	0.00023	-0.000177
60	0.00422	0.00062	-0.000223
70	0.00350	0.00117	-0.000020
80	0.00195	0.00097	0.000256
90	-0.00003	0.00088	0.000336
100	-0.00103	0.00098	0.000338
110	-0.00118	0.00106	0.000343
120	-0.00117	0.00117	0.000366
130	-0.00120	0.00120	0.000374
140	-0.00147	0.00114	0.000338
150	-0.00198	0.00100	0.000278
160	-0.00222	0.00075	0.000214
170	-0.00242	0.00037	0.000130
180	-0.00270	0.00000	0.000000
190	-0.00242	-0.00037	-0.000130
200	-0.00222	-0.00075	-0.000214
210	-0.00198	-0.00100	-0.000278
220	-0.00147	-0.00114	-0.000338
230	-0.00120	-0.00120	-0.000374
240	-0.00117	-0.00117	-0.000366
250	-0.00118	-0.00106	-0.000343
260	-0.00103	-0.00098	-0.000338
270	-0.00003	-0.00088	-0.000336
280	0.00195	-0.00097	-0.000256
290	0.00350	-0.00117	0.000020
300	0.00422	-0.00062	0.000223
310	0.00426	-0.00023	0.000177
320	0.00408	-0.00002	0.000137
330	0.00398	0.00008	0.000108
340	0.00396	0.00013	0.000097
350	0.00394	0.00012	0.000065

Θ (Deg)	C _A	C _S	C _M
0	0.00221	0.00000	0.000000
10	0.00220	0.00038	-0.000204
20	0.00210	0.00076	-0.000285
30	0.00195	0.00105	-0.000277
40	0.00170	0.00125	-0.000205
50	0.00140	0.00136	-0.000114
60	0.00107	0.00128	-0.000002
70	0.00080	0.00118	0.000130
80	0.00058	0.00112	0.000268
90	0.00034	0.00104	0.000390
100	0.00008	0.00100	0.000434
110	-0.00017	0.00095	0.000422
120	-0.00042	0.00089	0.000404
130	-0.00075	0.00082	0.000357
140	-0.00105	0.00078	0.000232
150	-0.00133	0.00070	0.000132
160	-0.00154	0.00058	0.000063
170	-0.00168	0.00038	0.000022
180	-0.00177	0.00000	0.000000
190	-0.00168	-0.00038	-0.000022
200	-0.00154	-0.00058	-0.000063
210	-0.00133	-0.00070	-0.000132
220	-0.00105	-0.00078	-0.000232
230	-0.00075	-0.00082	-0.000357
240	-0.00042	-0.00089	-0.000404
250	-0.00017	-0.00095	-0.000422
260	0.00008	-0.00100	-0.000434
270	0.00034	-0.00104	-0.000390
280	0.00058	-0.00112	-0.000268
290	0.00080	-0.00118	-0.000130
300	0.00107	-0.00128	0.000002
310	0.00140	-0.00136	0.000114
320	0.00170	-0.00125	0.000205
330	0.00195	-0.00105	0.000277
340	0.00210	-0.00076	0.000285
350	0.00220	-0.00038	0.000204

Wind Force Coefficients for Typical Paraboloid With Cylindrical Shroud

Wind Force Coefficients for Typical Grid Antenna Without Ice

Wind Angle

Θ (Deg)	C_A	C_S	C_M
0	0.00323	0.00000	0.000000
10	0.00323	0.00025	-0.000072
20	0.00320	0.00045	-0.000116
30	0.00310	0.00060	-0.000133
40	0.00296	0.00072	-0.000125
50	0.00278	0.00078	-0.000083
60	0.00242	0.00094	-0.000022
70	0.00172	0.00122	0.000058
80	0.00070	0.00149	0.000178
90	-0.00028	0.00160	0.000251
100	-0.00088	0.00154	0.000288
110	-0.00138	0.00136	0.000292
120	-0.00182	0.00112	0.000266
130	-0.00220	0.00080	0.000237
140	-0.00239	0.00059	0.000199
150	-0.00245	0.00045	0.000158
160	-0.00249	0.00038	0.000112
170	-0.00255	0.00025	0.000059
180	-0.00260	0.00000	0.000000
190	-0.00255	-0.00025	-0.000059
200	-0.00249	-0.00038	-0.000112
210	-0.00245	-0.00045	-0.000158
220	-0.00239	-0.00059	-0.000199
230	-0.00220	-0.00080	-0.000237
240	-0.00182	-0.00112	-0.000266
250	-0.00138	-0.00136	-0.000292
260	-0.00088	-0.00154	-0.000288
270	-0.00028	-0.00160	-0.000251
280	0.00070	-0.00149	-0.000178
290	0.00172	-0.00122	-0.000058
300	0.00242	-0.00094	0.000022
310	0.00278	-0.00078	0.000083
320	0.00296	-0.00072	0.000125
330	0.00310	-0.00060	0.000133
340	0.00320	-0.00045	0.000116
350	0.00323	-0.00025	0.000072

Wind Angle

Θ (Deg)	C_A	C_S	C_M
0	0.00137	0.00000	0.000000
10	0.00134	0.00026	0.000043
20	0.00130	0.00046	0.000074
30	0.00118	0.00059	0.000098
40	0.00104	0.00067	0.000115
50	0.00088	0.00070	0.000127
60	0.00060	0.00072	0.000135
70	0.00033	0.00070	0.000142
80	0.00010	0.00064	0.000126
90	-0.00013	0.00062	0.000111
100	-0.00030	0.00070	0.000120
110	-0.00048	0.00073	0.000129
120	-0.00068	0.00071	0.000131
130	-0.00086	0.00067	0.000127
140	-0.00104	0.00060	0.000114
150	-0.00122	0.00052	0.000095
160	-0.00140	0.00040	0.000070
170	-0.00150	0.00022	0.000038
180	-0.00152	0.00000	0.000000
190	-0.00150	-0.00022	-0.000038
200	-0.00140	-0.00040	-0.000070
210	-0.00122	-0.00052	-0.000095
220	-0.00104	-0.00060	-0.000114
230	-0.00086	-0.00067	-0.000127
240	-0.00068	-0.00071	-0.000131
250	-0.00048	-0.00073	-0.000129
260	-0.00030	-0.00070	-0.000120
270	-0.00013	-0.00062	-0.000111
280	0.00010	-0.00064	-0.000126
290	0.00033	-0.00070	-0.000142
300	0.00060	-0.00072	-0.000135
310	0.00088	-0.00070	-0.000127
320	0.00104	-0.00067	-0.000115
330	0.00118	-0.00059	-0.000098
340	0.00130	-0.00046	-0.000074
350	0.00134	-0.00026	-0.000043

Wind Force Coefficients for Typical Conical Horn Reflector Antenna

Wind Force Coefficients for Typical Passive Reflector

Wind Angle

Wind Angle

Θ (Deg)	C _A	C _S	C _M
0	0.00338	0.00000	0.00000
10	0.00355	0.00004	-0.00005
20	0.00354	0.00025	-0.00007
30	0.00345	0.00077	-0.00001
40	0.00335	0.00142	0.00009
50	0.00299	0.00181	0.00023
60	0.00235	0.00208	0.00035
70	0.00154	0.00237	0.00044
80	0.00059	0.00248	0.00046
90	-0.00020	0.00245	0.00040
100	-0.00062	0.00240	0.00032
110	-0.00088	0.00235	0.00030
120	-0.00147	0.00225	0.00032
130	-0.00225	0.00201	0.00027
140	-0.00289	0.00167	0.00021
150	-0.00323	0.00113	0.00014
160	-0.00367	0.00052	0.00007
170	-0.00375	0.00010	0.00003
180	-0.00356	0.00000	0.00000
190	-0.00375	-0.00010	-0.00003
200	-0.00367	-0.00052	-0.00007
210	-0.00323	-0.00113	-0.00014
220	-0.00289	-0.00167	-0.00021
230	-0.00225	-0.00201	-0.00027
240	-0.00147	-0.00225	-0.00032
250	-0.00088	-0.00235	-0.00030
260	-0.00062	-0.00240	-0.00032
270	-0.00020	-0.00245	-0.00040
280	0.00059	-0.00248	-0.00046
290	0.00154	-0.00237	-0.00044
300	0.00235	-0.00208	-0.00035
310	0.00299	-0.00181	-0.00023
320	0.00335	-0.00142	-0.00009
330	0.00345	-0.00077	0.00001
340	0.00354	-0.00025	0.00007
350	0.00355	-0.00004	0.00005

Θ (Deg)	C _A	C _S	C _M
0	0.00351	0.00000	0.000000
10	0.00348	0.00003	-0.000070
20	0.00341	0.00008	-0.000134
30	0.00329	0.00010	-0.000180
40	0.00309	0.00013	-0.000198
50	0.00300	0.00018	-0.000208
60	0.00282	0.00021	-0.000262
70	0.00178	0.00023	-0.000225
80	0.00071	0.00027	-0.000129
90	-0.00010	0.00030	0.000030
100	-0.00108	0.00035	0.000180
110	-0.00235	0.00039	0.000225
120	-0.00348	0.00036	0.000210
130	-0.00348	0.00029	0.000148
140	-0.00360	0.00023	0.000126
150	-0.00376	0.00019	0.000109
160	-0.00390	0.00012	0.000080
170	-0.00400	0.00008	0.000042
180	-0.00403	0.00000	0.000000
190	-0.00400	-0.00008	-0.000042
200	-0.00390	-0.00012	-0.000080
210	-0.00376	-0.00019	-0.000109
220	-0.00360	-0.00023	-0.000126
230	-0.00348	-0.00029	-0.000148
240	-0.00348	-0.00036	-0.000210
250	-0.00235	-0.00039	-0.000225
260	-0.00108	-0.00035	-0.000180
270	-0.00010	-0.00030	-0.000030
280	0.00071	-0.00027	0.000129
290	0.00178	-0.00023	0.000225
300	0.00282	-0.00021	0.000262
310	0.00300	-0.00018	0.000208
320	0.00309	-0.00013	0.000198
330	0.00329	-0.00010	0.000180
340	0.00341	-0.00008	0.000134
350	0.00348	-0.00003	0.000070

Tabla II. 1. 12. Coeficientes de fuerza de viento.

II. 2. Análisis sísmico.

Para llevar a cabo el análisis sísmico, es necesario tomar en cuenta las siguientes clasificaciones:

II. 2. 1. Clasificación de Construcciones según su destino.

Grupo A

Estructuras en que se requiere un grado de seguridad alto. Construcciones cuya falla estructural causaría la pérdida de un número elevado de vidas o pérdidas económicas o culturales de magnitud excepcionalmente alta. Algunos ejemplos son los hospitales, escuelas, estadios, museos, centrales telefónicas, estaciones de bomberos, sistemas de abastecimiento de agua potable, etc.

Grupo B

Estructuras en que se requiere un grado de seguridad intermedio. Construcciones cuya falla estructural ocasionaría pérdidas de magnitud intermedia. Este es el caso de naves industriales, estructuras comunes destinadas a vivienda u oficinas, hoteles, etc.

Grupo C

Estructuras en que es admisible un grado de seguridad bajo. Construcciones cuya falla estructural ocasionaría pérdidas de magnitud sumamente pequeñas y sin pérdida de vidas. Se incluyen bodegas provisionales y bardas con altura o mayor de 2.5 m.

II. 2. 2. Clasificación de Construcciones según su estructuración.

Atendiendo a las características estructurales que influyen en la respuesta sísmica de la estructura, las construcciones se clasifican según su estructuración:

Tipo 1

Estructuras de edificios: Estructuras comunes tales como edificios urbanos, naves industriales típicas, salas de espectáculos y estructuras semejantes, en que las fuerzas laterales se resisten en cada nivel por marcos continuos contraventeados o no, por diafragmas o muros o por combinación de diversos sistemas como los mencionados.

Tipo 2

Péndulos invertidos y apéndices: Péndulos invertidos o estructuras en que el 50% o más de su masa se halle en el extremo superior y tenga un solo elemento resistente en la dirección de análisis o una sola hilera de columnas perpendicular a esta. Estructuras tales como tanques, parapetos, pretilas, anuncios, ventanales, muros y revestimientos entre otros.

Tipo 3

Muros de retención.

Tipo 4

Chimeneas, silos y similares: Chimeneas y silos, o estructuras semejantes en que la masa y rigidez se encuentran distribuidas continuamente a lo largo de su altura y donde dominan las deformaciones por flexión.

Tipo 5

Tanques, depósitos y similares: Tanques elevados y depósitos superficiales, o estructuras semejantes destinadas al almacenamiento de líquidos que originan importantes fuerzas hidrodinámicas sobre el recipiente.

Tipo 6

Estructuras industriales: Estructuras fabriles en las que se requieren grandes áreas libres de columnas, dejando grandes claros libres entre sus ejes.

Tipo 7

Puentes.

Tipo 8

Tuberías.

Tipo 9

Presas.

Tipo 10

Otras estructuras.

II. 2. 3. Factor de comportamiento sísmico.

En la actualidad, la forma mas adecuada de caracterizar a las estructuras en función de su ductilidad consiste en el empleo del factor del comportamiento sísmico Q , el cual en realidad no sólo está asociado a la ductilidad estructural, sino también a la estructuración misma, al deterioro o efecto que puede llegar a contrarrestar gran parte de la capacidad extra en resistencia que suministra la ductilidad y a reservas de capacidad ante carga sísmica que los métodos convencionales de diseño no consideran.

Para el tipo de estructura manejada, el factor de comportamiento sísmico según el Manual de la C.F.E. (Diseño por Sismo) 1993, será:

$$Q = 2.$$

II. 2. 4. Factor reductivo por ductilidad.

Para diseñar se necesita considerar el comportamiento inelástico de la estructura, aunque sea de manera aproximada. Para ello las ordenadas espectrales se podrán reducir dividiéndolas entre el factor reductivo Q' a fin de obtener las fuerzas sísmicas reducidas por ductilidad. Para cualquier tipo de estructura el factor reductivo se calcula como sigue:

$$Q' = 1 + (Q - 1) \frac{T}{T_a}; \quad \text{si } T < T_a$$

$$Q' = Q \quad ; \quad \text{si } T > T_a$$

donde:

T se tomará igual al período fundamental de vibración cuando se emplee en análisis estático e igual al período natural de vibración del modo que se considere cuando se emplee el análisis modal espectral.

T_a es el primer período característico del espectro de diseño.

II. 2. 5. Regionalización sísmica de la República Mexicana.

Con base en estudios de riesgo sísmico, se encontró que para fines de diseño sísmico, la República Mexicana se considera dividida en cuatro zonas.

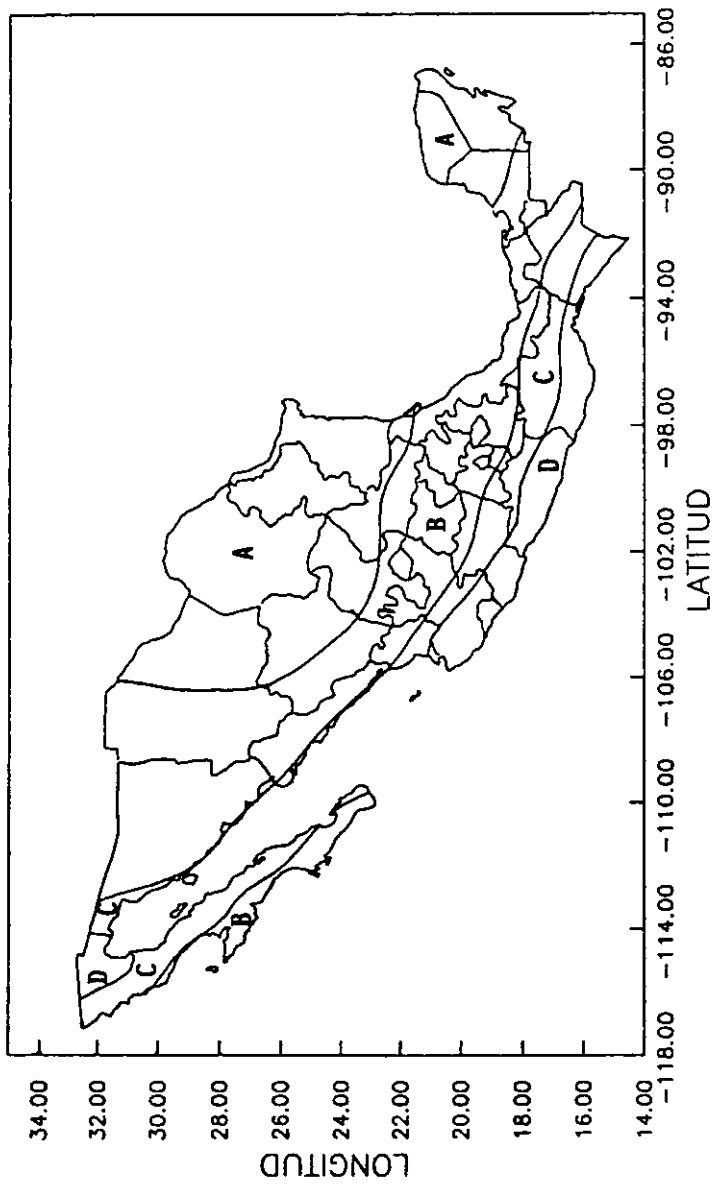


Fig. II. 2. 5. 1. Regionalización sísmica de la República Mexicana.

La República Mexicana está dividida en cuatro zonas A, B, C y D, la zona A es la de menor intensidad sísmica, mientras que la mayor es la zona D.

II. 2. 6. Espectros para Diseño Sísmico.

Las ordenadas del espectro de aceleraciones para diseño sísmico, "a", expresadas como fracción de la aceleración de la gravedad, están dadas por las siguientes expresiones:

$$a = a_0 + (c - a_0) \frac{T}{T_a} \quad ; \quad \text{si } T < T_a$$

$$a = c \quad ; \quad \text{si } T_a \leq T \leq T_b$$

$$a = c \left[\frac{T_b}{T} \right]^r \quad ; \quad \text{si } T > T_b$$

donde:

a_0 es el coeficiente de aceleración del terreno.

c es el coeficiente sísmico.

T es el período natural de interés.

T_a y T_b son dos períodos característicos que delimitan la meseta.

r es un exponente que define la parte curva del espectro de diseño.

Los valores de estos parámetros se encuentran en la tabla II. 2. 6. 1. para cada una de las zonas sísmicas y los distintos tipos de terreno de cimentación.

Zona Sísmica	Tipo de Suelo	a_0	c	T_a (s)	T_b (s)	r
A	I	0.02	0.08	0.2	0.6	1/2
	II	0.04	0.16	0.3	1.5	2/3
	III	0.05	0.20	0.6	2.9	1
B	I	0.04	0.14	0.2	0.6	1/2
	II	0.08	0.30	0.3	1.5	2/3
	III	0.10	0.36	0.6	2.9	1
C	I	0.36	0.36	0.0	0.6	1/2
	II	0.64	0.64	0.0	1.4	2/3
	III	0.64	0.64	0.0	1.9	1
D	I	0.50	0.50	0.0	0.6	1/2
	II	0.86	0.86	0.0	1.2	2/3
	III	0.86	0.86	0.0	1.7	1

Tabla II. 2. 6. 1. Espectros de diseño para estructuras del grupo B

Los espectros de diseño especificados son aplicables a estructuras del grupo B. Para estructuras del grupo A, los valores de las ordenadas espectrales deberán multiplicarse por "1.5" a fin de tener en cuenta la importancia de la estructura.

II. 2. 7. Tipos de Análisis.

Para el análisis sísmico de estructuras tales como torres, se puede recurrir a dos métodos:

a) Análisis Estático.

Es aplicable a estructuras que no pasen de 60 m. de altura. Para el análisis estático de torres, los efectos dinámicos inducidos por el sismo se simularán mediante una fuerza lateral equivalente, distribuida a lo largo de la altura de la estructura y actuando en dirección del movimiento del terreno. La magnitud de la resultante de la fuerza lateral distribuida verticalmente será igual a la fuerza cortante basal determinada de acuerdo con lo dispuesto para estructuras de edificios, pero amplificada por un factor de incremento por el que se aumentan las ordenadas espectrales con objeto de tener en cuenta que el amortiguamiento en torres es menor que en estructuras de edificios.

La distribución vertical de la fuerza cortante basal amplificada se llevará a cabo dividiendo la estructura en "N" segmentos de igual altura como se muestra en la figura II. 2. 7. 1. En el centro de masa del n-ésimo segmento se aplicará una fuerza horizontal que se define dependiendo del período característico T_b de las siguientes formas:

$$P_n = 0.85 W_n h_n \frac{\sum_{n=1}^N W_n}{\sum_{n=1}^N W_n h_n} \frac{a}{Q'} \xi ; \text{ si } T_c < T_b$$

$$P_n = 0.85 W_n (\alpha_1 h_n + \alpha_2 h_n^2) \frac{a}{Q} \xi ; \text{ si } T_c > T_b$$

donde:

W_n es el peso del n-ésimo segmento.

h_n es la altura de su centro de gravedad medida desde el desplante.

- a es la ordenada espectral correspondiente al período fundamental T_e de la estructura.
- T_e es el período fundamental de la estructura.
- T_b es el segundo período característico del espectro de diseño.
- Q es el factor de comportamiento sísmico.
- Q' es el factor reductor por ductilidad.
- α_1 y α_2 son los coeficientes de proporcionalidad que se especifican para estructuras de edificios.

ξ es el factor de incremento, este factor se podrá tomar como $\xi=1.25$ para estructuras de concreto ó $\xi=1.45$ para estructuras de acero. Estos valores son permitidos solo en terrenos tipo I.

Para tener en cuenta los efectos de los modos superiores de vibración, en el N-ésimo segmento se aplicará adicionalmente una fuerza horizontal que se define como:

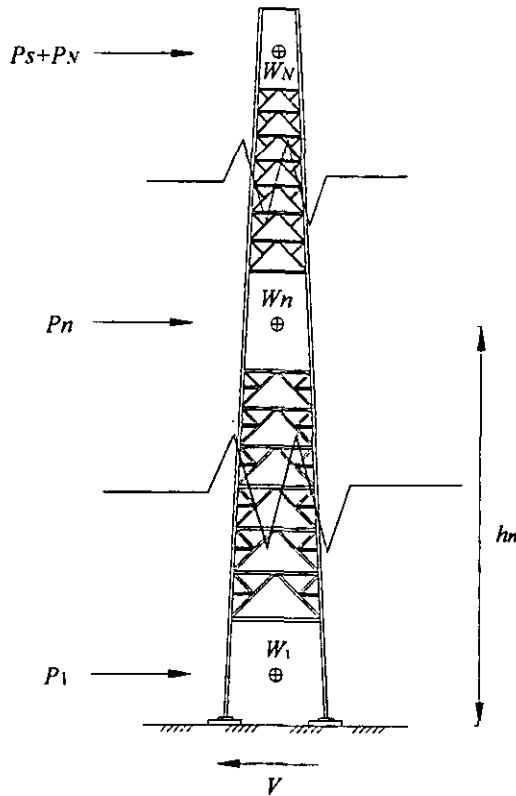
$$P_s = 0.15 W \frac{a}{Q'} \xi \quad ; \quad \text{si } T_e < T_b$$

$$P_s = 0.15 W (1 + 0.5 r - 0.5 r q) \frac{a}{Q} \xi \quad ; \quad \text{si } T_e > T_b$$

donde:

$$q = \left(\frac{T_b}{T_e} \right)^r$$

- W es el peso de la estructura.
- r es el exponente de la parte curva del espectro de diseño.



II. 2. 7. 1. Fuerzas sísmicas en una torre

b) Análisis Dinámico.

Para el análisis dinámico de esta torre autoportada, se empleará el análisis modal espectral que se aplica a estructuras cuyas alturas son superiores a 60 m, junto con las disposiciones correspondientes estipuladas para estructuras de edificios, teniendo en cuenta las siguientes recomendaciones:

1. Los parámetros dinámicos de una torre se determinarán suponiendo que la estructura posee modos clásicos de vibración, por lo que las frecuencias y los modos naturales de vibrar se obtendrán considerando nulo el amortiguamiento. Bastará con tener en cuenta las tres primeras formas modales para calcular las respuestas de diseño.

2. Al determinar las respuestas modales se aumentarán las ordenadas espectrales por un factor de incremento, "ξ", (ya explicado en el análisis estático), con objeto de tomar en cuenta que el amortiguamiento de torres puede ser menor que en estructuras de edificios.

3. Las respuestas de diseño se obtendrán mediante la combinación de las respuestas modales máximas, de acuerdo con la expresión:

$$R = \sqrt{\sum_{n=1}^3 R_n^2}$$

la cual representa la raíz cuadrada de la suma de los cuadrados de las respuestas modales R_n que pueden ser los desplazamientos, las fuerzas cortantes o los momentos de volteo.

4. En ninguna situación se permitirá que la fuerza cortante basal calculada dinámicamente sea menor que 75 por ciento de la calculada estáticamente con la opción que toma en cuenta el valor aproximado del período fundamental de la estructura. Cuando $V_d / V_e < 0.75$, las respuestas de diseño se incrementarán en $0.75 V_e / V_d$, siendo V_e y V_d las fuerzas cortantes basales calculadas estáticamente y dinámicamente, respectivamente.

5. Los momentos de volteo no serán reducidos. En el análisis de torres que no sean muy esbeltas, se podrán despreciar los efectos de segundo orden (P-delta), es decir, las fuerzas cortantes y los momentos flexionantes adicionales provocados por las cargas verticales actuando sobre la estructura deformada, así como por la influencia de la carga axial en la rigidez del fuste de la estructura.

Para los efectos combinados de los movimientos del terreno, se analizará ante la acción de dos componentes horizontales ortogonales del movimiento del terreno. En estructuras que no sean demasiado esbeltas se podrá despreciar la acción del componente vertical. Las fuerzas internas se combinarán sumando vectorialmente las gravitacionales, las del componente del movimiento del terreno paralelo a la dirección de análisis y 0.3 de las del otro componente con el signo que para cada concepto resulte más desfavorable. La elección de las direcciones ortogonales para las cuales se efectuará el análisis se hará atendiendo las direcciones más desfavorables que estarán definidas por la menor resistencia de la estructura, tanto a flexocompresión como a fuerza cortante.

III. Análisis de la estructura ante las acciones permanentes y eventuales.

III. 1. Datos de diseño.

A continuación se mencionan las principales características del proyecto, que son indispensables para la realización de este:

a) Ubicación.

La estructura se encontrará ubicada en la ciudad de Monterrey Nuevo León.

b) Parámetros de diseño.

- Velocidad Regional $V_R = 158 \text{ km/h}$, con un periodo de retorno de 200 años.
- La temperatura media anual es 22.1°C .
- La estructura se encuentra clasificada dentro del Grupo A.
- Es una estructura que se considera del Tipo 2.
- La categoría del terreno, (C.T.), es 2.
- El factor de topografía es $FT = 1.1$
- Debido a sus dimensiones la estructura se clasifica como Clase C.
- Altura Sobre el Nivel del Mar, A.S.N.M.= 538 m.
- Monterrey se localiza en la región sísmica A de la Rep. Mexicana.
- El tipo de terreno es: Tipo II.
- La estructura se clasifica según su destino como del Tipo 4
- La capacidad de carga admisible es: $\sigma_{adm.} = 16 \text{ Ton/m}^2$.
- El peso volumétrico del suelo es: $\gamma = 1600 \text{ kg/m}^3$.
- El nivel de aguas freáticas es: N.A.F.= 12 m.
- Profundidad de desplante $D_f = 2.5 \text{ m}$.

III. 2. Geometría de la estructura.

En la figura siguiente se presentan las características geométricas de la estructura que será analizada:

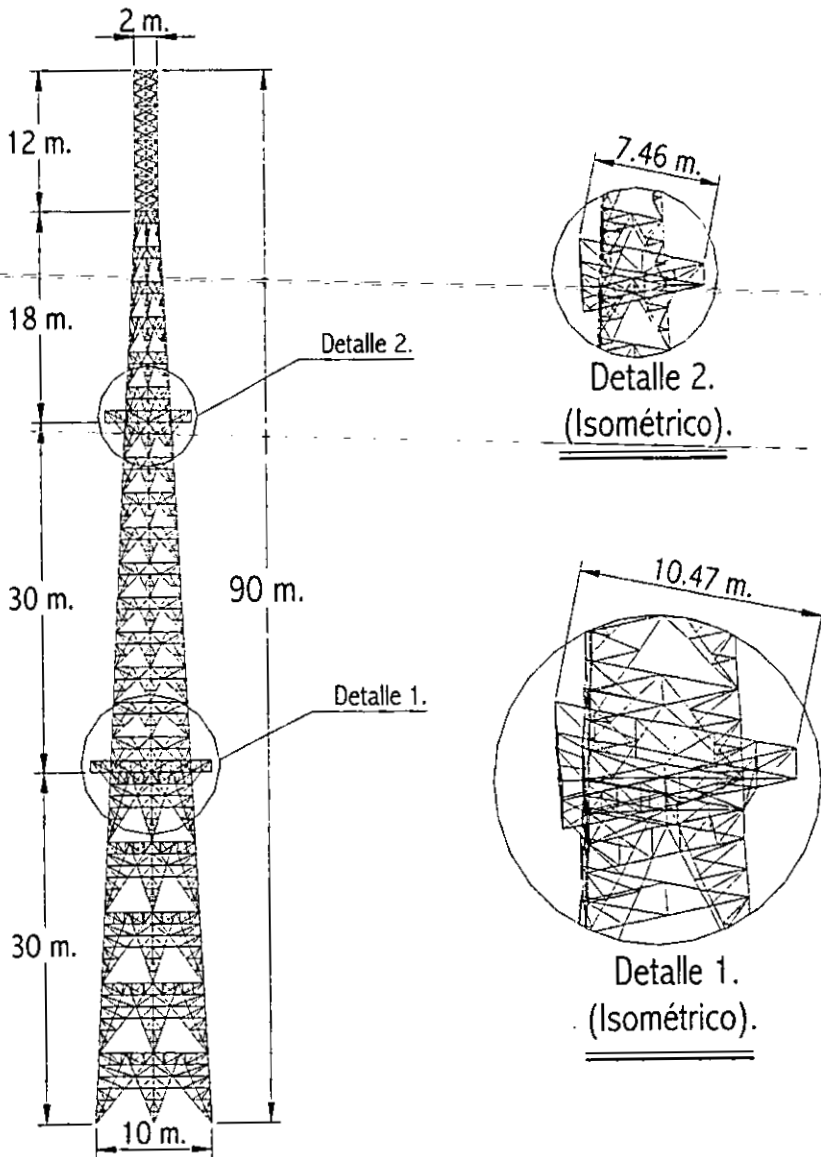


Fig. III. 2. 1. Torre Autosoportada

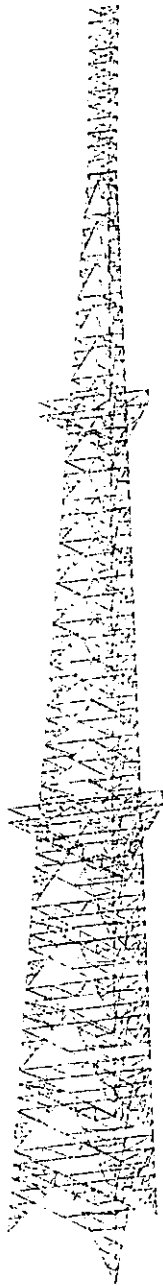


Figura: III. 2. 2. Torre en Perspectiva.

III. 3. Obtención de cargas permanentes, variables y accidentales.

III. 3. 1. Cargas permanentes.

La obtención de cargas permanentes se resume en el cálculo del peso de la estructura y de las antenas parabólicas. Para llevar a cabo el cálculo de las cargas, es necesario considerar el volumen de cada uno de los elementos y el peso volumétrico del acero empleado, con lo cual se obtiene el peso del material.

$$W = (V)(\gamma)$$

donde:

W es el peso del material, kg.

V es el volumen del material, m³.

γ es el peso volumétrico del material, kg/m³.

El peso de la estructura obtenido es de: 129 843 kg.

De la misma manera, se puede obtener el peso de las antenas parabólicas, aunque es recomendable considerar el valor del peso proporcionado por el fabricante.

El peso total considerado de las antenas parabólicas es de:

$$(9 \text{ parabólicas})(200 \text{ kg}) = 1 800 \text{ kg}$$

De esta manera sabemos que el total de carga permanente es de:

$$\underline{131 643 \text{ kg.}}$$

III. 3. 2. Cargas variables.

Para cargas variables se propusieron tres cargas puntuales de 100 kg en cada una de las dos plataformas, (una carga en cada vértice de las plataformas), simulando la presencia de tres personas en cada plataforma.

III. 3. 3. Viento.

Las cargas de viento en estructura y líneas de conducción se obtuvieron siguiendo las recomendaciones del Manual de Diseño por Viento (C.F.E. 93). En tanto que las cargas de viento sobre las antenas parabólicas se obtuvieron con ayuda de las TIA / EIA standard.

Debido a que la estructura tiene una relación de aspecto mayor a cinco, se llevará a cabo un análisis dinámico.

$$\lambda = \frac{\text{Altura}}{\text{Dim. menor en planta}} = \frac{90}{10} = 9$$

Por lo tanto: $9 > 5$

- Determinación de la velocidad de diseño.

$$V_D = F_T F_a V_R$$

- Factor topográfico, F_T

Este factor es obtenido de la tabla II. 1. 2., resultando ser: $F_T = 1.1$

- Factor de exposición, F_a

$$F_a = F_c F_{rz}$$

- Factor de tamaño, F_c .

Debido a que es un análisis dinámico, se considera al factor: $F_c = 1$

- Factor de rugosidad y altura, F_{rz}

De la tabla II. 1. 4. se obtiene que:

$$\alpha = 0.138$$
$$\delta = 315 \text{ m.}$$

Como: $10 < Z < 315$ m. entonces:

$$Frz = 1.56 \left[\frac{Z}{\delta} \right]^a$$

$$Frz = 1.56 \left[\frac{Z}{315} \right]^{0.138}$$

$$\boxed{Frz = 0.7053(Z)^{0.138}}$$

Sustituyendo Frz y Fc en:

$$F_a = F_c F_{rz}$$

Tenemos que:

$$F_a = (1) (0.7053) (Z)^{0.138}$$

$$\boxed{F_a = (0.7053) (Z)^{0.138}}$$

Por lo tanto la velocidad de diseño es:

$$V_D = (1.1) (0.7053) (Z)^{0.138} (158)$$

$$\boxed{V_D = 122.57(Z)^{0.138}}$$

- Presión dinámica de base, q_z .

$$q_z = 0.0048 G V_D^2$$

$$G = \frac{0.392 \Omega}{273 + \tau}$$

Sabiendo que A.S.N.M.= 538 m. obtenemos de la tabla II. 1. 6. que la presión barométrica es 716.58 mm de Hg. Además, conocida la temperatura ambiental (22.1 °C), sustituimos:

$$G = \frac{0.392 (716.58)}{273 + 22.1}$$

$$G = 0.9519$$

Por lo tanto:

$$q_z = 0.0048 G V_D^2$$

$$q_z = 68.6437(Z)^{0.276}$$

- Factor de respuesta dinámica debida a ráfagas.

$$F_s = \frac{1}{g^2} \left[1 + g_p \left(\frac{\sigma}{\mu} \right) \right]$$

Sabiendo que la C.T.= 2, entonces de la tabla II. 1. 7., obtenemos:

$$k' = 1.288$$

$$\eta = -0.054$$

$$\delta = 315 \text{ m.}$$

Como $10 < Z < 315$ m. entonces:

$$g = k' \left[\frac{Z}{\delta} \right]^\eta \quad \text{si } 10 < Z < \delta$$

Sustituyendo valores:

$$g = 1.7572 (Z)^{-0.054}$$

Por otro lado:

$$\frac{\sigma}{\mu} = \sqrt{\frac{k_r}{C_a} \left(B + \frac{SE}{\zeta} \right)}$$

Por ser un terreno con categoría igual a 2, entonces $k_r = 0.08$

$$C_a = 3.46(F_T)^2 \left[\frac{H}{\delta} \right]^{2\alpha'} \quad \text{si } 10 < H < \delta$$

$$C_a = 3.46(1.1)^2 \left[\frac{90}{315} \right]^{2(0.18)}$$

$$C_a = 2.6668$$

El coeficiente de amortiguamiento crítico para este tipo de estructuras es: $\zeta = 0.005$

Para obtener el factor de excitación de fondo (B), entramos a la gráfica II. 1. 8. a. con $H = 90\text{m}$ y $b/H = (10\text{m} / 90\text{m}) = 0.11$. y obtenemos que el factor es igual a $B = 0.94$.

$$V_H' = \frac{1}{g_H} V_H$$

$$g_H = k' \left[\frac{H}{\delta} \right]^q$$

Sustituyendo:

$$V_H' = \frac{1}{k' \left[\frac{H}{\delta} \right]^q} V_H$$

$$V_H' = \frac{1}{1.288 \left[\frac{90}{315} \right]^{-0.054}} (122.57)(90^{0.138})$$

$$V_H' = 165.49 \text{ km/h}$$

Los valores de la estructura S, E y g_p se obtendrán conjuntamente con los de las líneas de conducción y parábolas, esto se realizará de manera iterativa, proponiendo una frecuencia natural de $n_0 = 2 \text{ Hz}$, con lo cual al final obtendré las fuerzas de diseño tanto para la estructura como para las plataformas, líneas y parábolas.

Para la obtención estos parámetros, primero he calculado las áreas totales y expuestas (para cada sección de seis metros), que son indispensables para la obtención de la relación de solidez.

Áreas Totales en dirección Z-Z			
Elev. (m)	Secc. (m)	Tramo (m)	Área Total (m ²)
6	1-6	6	58.127
12	6-12	6	54.532
18	12-18	6	50.936
24	18-24	6	47.341
30	24-30	6	43.745
36	30-36	6	40.150
42	36-42	6	36.554
48	42-48	6	32.959
54	48-54	6	29.363
60	54-60	6	25.768
66	60-66	6	21.965
72	66-72	6	17.969
78	72-78	6	13.976
84	78-84	6	12.000
90	84-90	6	12.000

Áreas Totales en dirección X-X			
Elev. (m)	Secc. (m)	Tramo (m)	Área Total (m ²)
6	1-6	6	50.359
12	6-12	6	47.240
18	12-18	6	44.125
24	18-24	6	41.010
30	24-30	6	37.894
36	30-36	6	34.779
42	36-42	6	31.664
48	42-48	6	28.548
54	48-54	6	25.433
60	54-60	6	22.318
66	60-66	6	19.027
72	66-72	6	15.568
78	72-78	6	12.110
84	78-84	6	10.393
90	84-90	6	10.393

Áreas Expuestas en dirección Z-Z

Elev. (m)	Secc.	Tramo (m)	Largueros				Celosía Principal y Diafragmas				Celosía Secundaria				Área Expuesta	% Tub	% Ang
			Long. (m)	Tubo (plg)	Long Exp. (m)	A (m ²)	Long. (m)	Ángulo (plg)	Long Exp. (m)	A (m ²)	Long. (m)	Ángulo (plg)	Long Exp. (m)	A (m ²)			
6	1-6	6	12	18	0.457	5.486	39.51	4x4x10/16	0.102	4.014	45.97	3x3x8/16	0.076	3.503	13.003	42	58
12	6-12	6	12	18	0.457	5.486	37.63	4x4x10/16	0.102	3.823	43.65	3x3x8/16	0.076	3.326	12.636	43	57
18	12-18	6	12	18	0.457	5.486	35.77	4x4x10/16	0.102	3.634	41.37	3x3x8/16	0.076	3.152	12.273	45	55
24	18-24	6	12	18	0.457	5.486	33.93	4x4x10/16	0.102	3.447	39.09	3x3x8/16	0.076	2.978	11.912	46	54
30	24-30	6	12	18	0.457	5.486	32.09	4x4x10/16	0.102	3.261	36.83	3x3x8/16	0.076	2.806	11.553	47	53
36	30-36	6	12	14	0.356	4.267	31.31	3x3x8/16	0.076	2.386	28.3	3x3x7/16	0.076	2.156	8.809	48	52
42	36-42	6	12	14	0.356	4.267	29.22	3x3x8/16	0.076	2.226	26.1	3x3x7/16	0.076	1.989	8.483	50	50
48	42-48	6	12	14	0.356	4.267	27.17	3x3x8/16	0.076	2.071	23.93	3x3x7/16	0.076	1.823	8.161	52	48
54	48-54	6	12	14	0.356	4.267	25.18	3x3x8/16	0.076	1.918	21.8	3x3x7/16	0.076	1.661	7.847	54	46
60	54-60	6	12	14	0.356	4.267	23.23	3x3x8/16	0.076	1.770	19.72	3x3x7/16	0.076	1.503	7.540	57	43
66	60-66	6	12	10	0.254	3.048	21.23	3x3x7/16	0.076	1.618	17.5	3x3x6/16	0.076	1.334	5.999	51	49
72	66-72	6	12	10	0.254	3.048	19.22	3x3x7/16	0.076	1.465	15.34	3x3x6/16	0.076	1.169	5.682	54	46
78	72-78	6	12	10	0.254	3.048	17.32	3x3x7/16	0.076	1.320	13.32	3x3x6/16	0.076	1.015	5.383	57	43
84	78-84	6	12	8	0.203	2.438	4	3x3x6/16	0.076	0.305	26.83	3x3x5/16	0.076	2.045	4.788	51	49
90	84-90	6	12	8	0.203	2.438	4	3x3x6/16	0.076	0.305	26.83	3x3x5/16	0.076	2.045	4.788	51	49

Áreas Expuestas en dirección X-X																	
Elev. (m)	Secc.	Tramo (m)	Largueros				Celosía Principal y Diafragmas				Celosía Secundaria				Área Expuesta	% Tub	% Ang
			Long. (m)	Tubo (plg)	Long Exp. (m)	A (m ²)	Long. (m)	Ángulo (plg)	Long Exp. (m)	A (m ²)	Long. (m)	Ángulo (plg)	Long Exp. (m)	A (m ²)			
6	1-6	6	12	18	0.457	5.486	35.49	4x4x10/16	0.102	3.606	41.19	3x3x8/16	0.076	3.139	12.231	45	55
12	6-12	6	12	18	0.457	5.486	33.89	4x4x10/16	0.102	3.443	39.15	3x3x8/16	0.076	2.983	11.913	46	54
18	12-18	6	12	18	0.457	5.486	32.31	4x4x10/16	0.102	3.282	37.27	3x3x8/16	0.076	2.840	11.609	47	53
24	18-24	6	12	18	0.457	5.486	30.73	4x4x10/16	0.102	3.123	35.38	3x3x8/16	0.076	2.696	11.305	49	51
30	24-30	6	12	18	0.457	5.486	29.18	4x4x10/16	0.102	2.964	33.44	3x3x8/16	0.076	2.548	10.999	50	50
36	30-36	6	12	14	0.356	4.267	28.21	3x3x8/16	0.076	2.150	25.13	3x3x7/16	0.076	1.915	8.332	51	49
42	36-42	6	12	14	0.356	4.267	26.46	3x3x8/16	0.076	2.016	23.26	3x3x7/16	0.076	1.773	8.056	53	47
48	42-48	6	12	14	0.356	4.267	24.74	3x3x8/16	0.076	1.885	21.42	3x3x7/16	0.076	1.632	7.785	55	45
54	48-54	6	12	14	0.356	4.267	23.07	3x3x8/16	0.076	1.758	19.62	3x3x7/16	0.076	1.495	7.520	57	43
60	54-60	6	12	14	0.356	4.267	21.44	3x3x8/16	0.076	1.634	17.87	3x3x7/16	0.076	1.361	7.262	59	41
66	60-66	6	12	10	0.254	3.048	19.77	3x3x7/16	0.076	1.506	16	3x3x6/16	0.076	1.220	5.774	53	47
72	66-72	6	12	10	0.254	3.048	18.09	3x3x7/16	0.076	1.378	14.2	3x3x6/16	0.076	1.082	5.509	55	45
78	72-78	6	12	10	0.254	3.048	16.5	3x3x7/16	0.076	1.257	12.5	3x3x6/16	0.076	0.953	5.258	58	42
84	78-84	6	12	8	0.203	2.438	3.464	3x3x6/16	0.076	0.264	24	3x3x5/16	0.076	1.829	4.531	54	46
90	84-90	6	12	8	0.203	2.438	3.464	3x3x6/16	0.076	0.264	24	3x3x5/16	0.076	1.829	4.531	54	46

Para la obtención de presiones y fuerzas sobre las líneas de conducción, se consideraron nueve líneas (una por antena), de 1 5/8" de diámetro. Nueve líneas suben hasta la primera plataforma, seis suben hasta la segunda plataforma y tres suben hasta la punta de la torre.

Presiones y Fuerzas sobre la Estructura en dirección Z-Z.

Sección	Altura (m).	Frz	Fa	V _D (km/h).	q _z (kg/m ²).	Ae (m ²).	A _T (m ²).	Solidez φ	bV _D	Ca	g
1	90	1.31	1.31	228.07	237.67	4.7879	12	0.40	12.87	1.59	1.38
2	89	1.31	1.31	227.72	236.94	4.7879	12	0.40	12.85	1.59	1.38
3	88	1.31	1.31	227.36	236.20	4.7879	12	0.40	12.83	1.59	1.38
4	87	1.31	1.31	227.01	235.46	4.7879	12	0.40	12.81	1.59	1.38
5	86	1.30	1.30	226.64	234.71	4.7879	12	0.40	12.79	1.59	1.38
6	85	1.30	1.30	226.28	233.95	4.7879	12	0.40	12.77	1.59	1.38
7	84	1.30	1.30	225.91	233.19	4.7879	12	0.40	12.75	1.59	1.38
8	83	1.30	1.30	225.54	232.42	4.7879	12	0.40	12.73	1.59	1.38
9	82	1.30	1.30	225.16	231.64	4.7879	12	0.40	12.71	1.59	1.39
10	81	1.29	1.29	224.78	230.86	4.7879	12	0.40	12.68	1.59	1.39
11	80	1.29	1.29	224.39	230.07	4.7879	12	0.40	12.66	1.59	1.39
12	79	1.29	1.29	224.00	229.27	4.7879	12	0.40	12.64	1.59	1.39
13	78	1.29	1.29	223.61	228.46	5.383	13.9764	0.39	15.77	1.54	1.39
14	77	1.28	1.28	223.21	227.65	5.383	13.9764	0.39	15.74	1.54	1.39
15	76	1.28	1.28	222.81	226.83	5.383	13.9764	0.39	15.72	1.54	1.39
16	75	1.28	1.28	222.40	226.00	5.383	13.9764	0.39	15.69	1.54	1.39
17	74	1.28	1.28	221.99	225.17	5.383	13.9764	0.39	15.66	1.54	1.39
18	73	1.28	1.28	221.58	224.33	5.383	13.9764	0.39	15.63	1.54	1.39
19	72	1.27	1.27	221.15	223.47	5.6818	17.9692	0.32	15.60	1.63	1.39
20	71	1.27	1.27	220.73	222.61	5.6818	17.9692	0.32	15.57	1.63	1.40
21	70	1.27	1.27	220.30	221.74	5.6818	17.9692	0.32	15.54	1.63	1.40
22	69	1.27	1.27	219.86	220.86	5.6818	17.9692	0.32	15.51	1.63	1.40
23	68	1.26	1.26	219.42	219.98	5.6818	17.9692	0.32	15.48	1.63	1.40
24	67	1.26	1.26	218.97	219.08	5.6818	17.9692	0.32	15.45	1.63	1.40
25	66	1.26	1.26	218.51	218.17	5.9991	21.965	0.27	15.41	1.75	1.40
26	65	1.25	1.25	218.06	217.25	5.9991	21.965	0.27	15.38	1.75	1.40
27	64	1.25	1.25	217.59	216.32	5.9991	21.965	0.27	15.35	1.75	1.40
28	63	1.25	1.25	217.12	215.39	5.9991	21.965	0.27	15.31	1.75	1.40
29	62	1.25	1.25	216.64	214.44	5.9991	21.965	0.27	15.28	1.75	1.41
30	61	1.24	1.24	216.15	213.48	5.9991	21.965	0.27	15.25	1.75	1.41
31	60	1.24	1.24	215.66	212.51	7.5398	25.7677	0.29	21.30	1.63	1.41
32	59	1.24	1.24	215.16	211.52	7.5398	25.7677	0.29	21.25	1.63	1.41
33	58	1.24	1.24	214.65	210.53	7.5398	25.7677	0.29	21.20	1.63	1.41
34	57	1.23	1.23	214.14	209.52	7.5398	25.7677	0.29	21.15	1.63	1.41
35	56	1.23	1.23	213.62	208.50	7.5398	25.7677	0.29	21.09	1.63	1.41
36	55	1.23	1.23	213.09	207.46	7.5398	25.7677	0.29	21.04	1.63	1.42
37	54	1.22	1.22	212.55	206.42	7.8471	29.3632	0.27	20.99	1.71	1.42
38	53	1.22	1.22	212.00	205.35	7.8471	29.3632	0.27	20.93	1.71	1.42
39	52	1.22	1.22	211.44	204.28	7.8471	29.3632	0.27	20.88	1.71	1.42
40	51	1.21	1.21	210.88	203.18	7.8471	29.3632	0.27	20.82	1.71	1.42
41	50	1.21	1.21	210.30	202.08	7.8471	29.3632	0.27	20.77	1.71	1.42
42	49	1.21	1.21	209.72	200.95	7.8471	29.3632	0.27	20.71	1.71	1.42
43	48	1.20	1.20	209.12	199.81	8.1612	32.9587	0.25	20.65	1.77	1.43
44	47	1.20	1.20	208.51	198.66	8.1612	32.9587	0.25	20.59	1.77	1.43
45	46	1.20	1.20	207.90	197.48	8.1612	32.9587	0.25	20.53	1.77	1.43
46	45	1.19	1.19	207.27	196.29	8.1612	32.9587	0.25	20.47	1.77	1.43
47	44	1.19	1.19	206.62	195.07	8.1612	32.9587	0.25	20.40	1.77	1.43

Sección	Altura (m).	Frz	Fa	V _D (km/h).	q _z (kg/m ²).	A _e (m ²).	A _T (m ²).	Solidez φ	bV _D	Ca	g
48	43	1.19	1.19	205.97	193.84	8.1612	32.9587	0.25	20.34	1.77	1.43
49	42	1.18	1.18	205.30	192.58	8.4825	36.5542	0.23	20.27	1.84	1.44
50	41	1.18	1.18	204.62	191.31	8.4825	36.5542	0.23	20.21	1.84	1.44
51	40	1.17	1.17	203.92	190.01	8.4825	36.5542	0.23	20.14	1.84	1.44
52	39	1.17	1.17	203.21	188.68	8.4825	36.5542	0.23	20.07	1.84	1.44
53	38	1.17	1.17	202.49	187.34	8.4825	36.5542	0.23	20.00	1.84	1.44
54	37	1.16	1.16	201.74	185.96	8.4825	36.5542	0.23	19.92	1.84	1.45
55	36	1.16	1.16	200.98	184.56	8.8091	40.1497	0.22	19.85	1.89	1.45
56	35	1.15	1.15	200.20	183.13	8.8091	40.1497	0.22	19.77	1.89	1.45
57	34	1.15	1.15	199.40	181.67	8.8091	40.1497	0.22	19.69	1.89	1.45
58	33	1.14	1.14	198.58	180.18	8.8091	40.1497	0.22	19.61	1.89	1.45
59	32	1.14	1.14	197.74	178.66	8.8091	40.1497	0.22	19.53	1.89	1.46
60	31	1.13	1.13	196.88	177.10	8.8091	40.1497	0.22	19.44	1.89	1.46
61	30	1.13	1.13	195.99	175.50	11.5534	43.7452	0.26	24.88	1.82	1.46
62	29	1.12	1.12	195.07	173.87	11.5534	43.7452	0.26	24.77	1.82	1.47
63	28	1.12	1.12	194.13	172.19	11.5534	43.7452	0.26	24.65	1.82	1.47
64	27	1.11	1.11	193.16	170.47	11.5534	43.7452	0.26	24.52	1.82	1.47
65	26	1.11	1.11	192.15	168.71	11.5534	43.7452	0.26	24.40	1.82	1.47
66	25	1.10	1.10	191.12	166.89	11.5534	43.7452	0.26	24.27	1.82	1.48
67	24	1.09	1.09	190.04	165.02	11.9114	47.3407	0.25	24.13	1.86	1.48
68	23	1.09	1.09	188.93	163.09	11.9114	47.3407	0.25	23.99	1.86	1.48
69	22	1.08	1.08	187.78	161.11	11.9114	47.3407	0.25	23.84	1.86	1.49
70	21	1.07	1.07	186.57	159.05	11.9114	47.3407	0.25	23.69	1.86	1.49
71	20	1.07	1.07	185.32	156.92	11.9114	47.3407	0.25	23.53	1.86	1.49
72	19	1.06	1.06	184.01	154.72	11.9114	47.3407	0.25	23.36	1.86	1.50
73	18	1.05	1.05	182.65	152.42	12.2725	50.9362	0.24	23.19	1.89	1.50
74	17	1.04	1.04	181.21	150.04	12.2725	50.9362	0.24	23.01	1.89	1.51
75	16	1.03	1.03	179.70	147.55	12.2725	50.9362	0.24	22.82	1.89	1.51
76	15	1.02	1.02	178.11	144.94	12.2725	50.9362	0.24	22.61	1.89	1.52
77	14	1.02	1.02	176.42	142.21	12.2725	50.9362	0.24	22.40	1.89	1.52
78	13	1.00	1.00	174.63	139.33	12.2725	50.9362	0.24	22.17	1.89	1.53
79	12	0.99	0.99	172.71	136.29	12.6356	54.5317	0.23	21.93	1.94	1.54
80	11	0.98	0.98	170.65	133.05	12.6356	54.5317	0.23	21.67	1.94	1.54
81	10	0.97	0.97	168.42	129.60	12.6356	54.5317	0.23	21.38	1.94	1.55
82	9	0.96	0.96	165.99	125.88	12.6356	54.5317	0.23	21.07	1.94	1.56
83	8	0.94	0.94	163.31	121.86	12.6356	54.5317	0.23	20.73	1.94	1.57
84	7	0.92	0.92	160.33	117.45	12.6356	54.5317	0.23	20.36	1.94	1.58
85	6	0.90	0.90	156.95	112.56	13.0035	58.1272	0.22	19.93	1.98	1.60
86	5	0.88	0.88	153.05	107.03	13.0035	58.1272	0.22	19.43	1.98	1.61
87	4	0.85	0.85	148.41	100.64	13.0035	58.1272	0.22	18.84	1.98	1.63
88	3	0.82	0.82	142.64	92.96	13.0035	58.1272	0.22	18.11	1.98	1.66
89	2	0.78	0.78	134.87	83.12	13.0035	58.1272	0.22	17.12	1.98	1.69
90	1	0.71	0.71	122.57	68.64	13.0035	58.1272	0.22	15.56	1.98	1.76

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
Valor propuesto							
2.00	3.92	0.019	0.04	0.075	0.96	4.18	0.19
Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos	
0.9484	358.40	301.55	286.00	2	953 954 FZ	143.00	
0.9473	356.87	300.62	284.77	2	998 1000 FZ	142.39	
0.9461	355.32	299.69	283.54	2	997 999 FZ	141.77	
0.9450	353.77	298.74	282.30	2	950 951 FZ	141.15	
0.9438	352.20	297.79	281.05	2	994 996 FZ	140.52	
0.9426	350.62	296.83	279.79	2	993 995 FZ	139.89	
0.9414	349.03	295.86	278.52	2	947 948 FZ	139.26	
0.9402	347.43	294.89	277.24	2	990 992 FZ	138.62	
0.9389	345.82	293.90	275.96	2	989 991 FZ	137.98	
0.9377	344.19	292.91	274.66	2	944 945 FZ	137.33	
0.9364	342.55	291.91	273.35	2	977 979 FZ	136.68	
0.9352	340.90	290.90	272.03	2	976 978 FZ	136.02	
0.9339	328.27	315.37	294.51	3	752 754 826 FZ	98.17	
0.9326	326.65	314.25	293.06	4	773 807 864 867 FZ	73.26	
0.9313	325.01	313.12	291.59	4	774 808 865 866 FZ	72.90	
0.9299	323.37	311.97	290.11	3	775 809 828 FZ	96.70	
0.9286	321.70	310.82	288.62	4	776 810 868 871 FZ	72.16	
0.9272	320.03	309.65	287.12	4	777 811 869 870 FZ	71.78	
0.9258	337.99	345.70	320.07	3	778 812 860 FZ	106.69	
0.9244	336.18	344.37	318.35	4	779 813 872 875 FZ	79.59	
0.9230	334.35	343.03	316.62	4	780 814 873 874 FZ	79.16	
0.9216	332.51	341.67	314.88	3	781 815 861 FZ	104.96	
0.9201	330.65	340.29	313.12	4	782 816 877 878 FZ	78.28	
0.9187	328.78	338.91	311.34	4	783 817 876 879 FZ	77.84	
0.9172	349.54	381.04	349.48	3	784 818 862 FZ	116.49	
0.9157	347.49	379.44	347.44	4	785 819 880 883 FZ	86.86	
0.9141	345.43	377.82	345.38	4	786 820 881 882 FZ	86.34	
0.9126	343.35	376.18	343.30	3	787 821 863 FZ	114.43	
0.9110	341.24	374.52	341.19	4	788 822 884 887 FZ	85.30	
0.9094	339.12	372.85	339.07	4	789 823 885 886 FZ	84.77	
0.9078	315.06	436.13	395.92	3	512 514 630 FZ	131.97	
0.9061	313.03	434.11	393.37	4	544 601 633 634 FZ	98.34	
0.9045	310.99	432.07	390.79	4	545 600 632 635 FZ	97.70	
0.9028	308.92	430.00	388.19	3	546 599 627 FZ	129.40	
0.9010	306.82	427.91	385.56	4	547 598 637 638 FZ	96.39	
0.8993	304.71	425.78	382.91	4	548 597 636 639 FZ	95.73	
0.8975	316.28	460.88	413.64	3	549 596 624 FZ	137.88	
0.8957	314.02	458.50	410.68	4	550 595 641 642 FZ	102.67	
0.8939	311.73	456.10	407.69	4	551 594 640 643 FZ	101.92	
0.8920	309.41	453.66	404.66	3	552 593 621 FZ	134.89	
0.8901	307.07	451.19	401.60	4	553 592 644 647 FZ	100.40	
0.8881	304.69	448.68	398.49	4	554 591 645 646 FZ	99.62	
0.8862	313.76	481.60	426.78	3	555 590 618 FZ	142.26	

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.8842	311.24	478.81	423.35	4	556 589 649 650 FZ	105.84
0.8821	308.68	475.98	419.86	4	557 588 648 651 FZ	104.97
0.8800	306.08	473.10	416.34	3	558 587 615 FZ	138.78
0.8779	303.45	470.18	412.76	4	559 586 653 654 FZ	103.19
0.8757	300.79	467.20	409.13	4	560 585 652 655 FZ	102.28
0.8735	309.52	500.97	437.58	3	561 584 612 FZ	145.86
0.8712	306.67	497.65	433.55	4	562 583 656 659 FZ	108.39
0.8689	303.78	494.27	429.46	4	563 582 657 658 FZ	107.37
0.8665	300.84	490.82	425.31	3	564 581 609 FZ	141.77
0.8641	297.85	487.32	421.09	4	565 580 661 662 FZ	105.27
0.8616	294.82	483.74	416.80	4	566 579 660 663 FZ	104.20
0.8591	299.72	512.24	440.05	3	567 578 606 FZ	146.68
0.8565	296.50	508.27	435.31	4	568 577 665 666 FZ	108.83
0.8538	293.21	504.22	430.49	4	569 576 664 667 FZ	107.62
0.8510	289.87	500.09	425.59	3	570 575 603 FZ	141.86
0.8482	286.47	495.86	420.59	4	571 574 669 670 FZ	105.15
0.8453	283.00	491.53	415.49	4	572 573 668 671 FZ	103.87
0.8423	269.17	615.33	518.30	7	4 6 372 443 444 446 447 FZ	74.04
0.8392	265.69	609.60	511.59	6	383 388 393 394 449 451 FZ	85.27
0.8361	262.13	603.72	504.75	4	382 387 392 395 FZ	126.19
0.8328	258.49	597.69	497.75	4	381 386 391 396 FZ	124.44
0.8294	254.77	591.50	490.59	4	380 385 390 397 FZ	122.65
0.8259	250.97	585.13	483.25	4	379 384 389 398 FZ	120.81
0.8223	251.84	608.04	499.96	7	14 18 302 360 361 363 364 FZ	71.42
0.8185	247.76	600.93	491.86	6	295 300 318 319 367 369 FZ	81.98
0.8146	243.56	593.61	483.53	4	294 299 317 320 FZ	120.88
0.8105	239.25	586.03	474.97	4	293 298 316 321 FZ	118.74
0.8062	234.81	578.20	466.15	4	292 297 315 322 FZ	116.54
0.8018	230.23	570.07	457.06	4	291 296 314 323 FZ	114.27
0.7971	229.87	589.87	470.19	7	13 17 205 277 278 280 281 FZ	67.17
0.7922	224.88	580.64	459.98	6	216 221 265 274 283 285 FZ	76.66
0.7870	219.71	571.01	449.40	4	215 220 266 273 FZ	112.35
0.7816	214.33	560.93	438.40	4	214 219 267 272 FZ	109.60
0.7758	208.73	550.35	426.93	4	213 218 268 271 FZ	106.73
0.7696	202.87	539.20	414.96	4	212 217 269 270 FZ	103.74
0.7629	202.10	557.84	425.60	7	12 16 111 144 145 147 148 FZ	60.80
0.7558	195.45	544.60	411.61	6	116 121 126 127 150 152 FZ	68.60
0.7481	188.43	530.46	396.82	4	115 120 125 128 FZ	99.21
0.7396	180.96	515.26	381.09	4	114 119 124 129 FZ	95.27
0.7303	172.96	498.78	364.24	4	113 118 123 130 FZ	91.06
0.7198	164.31	480.73	348.03	4	112 117 122 131 FZ	86.51
0.7079	157.90	483.39	342.20	7	11 15 20 68 69 71 72 FZ	48.89
0.6941	147.22	459.67	319.06	6	31 36 51 52 74 76 FZ	53.18
0.6776	135.13	432.21	292.86	4	30 35 50 53 FZ	73.22
0.6569	121.00	399.22	262.23	4	29 34 49 54 FZ	65.56
0.6287	103.55	356.95	224.42	4	28 33 48 55 FZ	56.11
0.5834	79.35	294.80	171.98	4	27 32 47 56 FZ	42.99

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
1 ^{ra} itera							
0.92276	1.81	0.066	0.020	0.13	0.74	4.12	0.28
Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos	
1.14	430.36	301.55	343.42	2	953 954 FZ	171.71	
1.14	428.52	300.62	341.95	2	998 1000 FZ	170.97	
1.14	426.66	299.69	340.47	2	997 999 FZ	170.23	
1.13	424.79	298.74	338.98	2	950 951 FZ	169.49	
1.13	422.91	297.79	337.48	2	994 996 FZ	168.74	
1.13	421.02	296.83	335.96	2	993 995 FZ	167.98	
1.13	419.11	295.86	334.44	2	947 948 FZ	167.22	
1.13	417.18	294.89	332.91	2	990 992 FZ	166.45	
1.13	415.25	293.90	331.36	2	989 991 FZ	165.68	
1.13	413.30	292.91	329.80	2	944 945 FZ	164.90	
1.12	411.33	291.91	328.23	2	977 979 FZ	164.12	
1.12	409.35	290.90	326.65	2	976 978 FZ	163.33	
1.12	394.18	315.37	353.65	3	752 754 826 FZ	117.88	
1.12	392.23	314.25	351.90	4	773-807-864-867-FZ	87.97	
1.12	390.27	313.12	350.14	4	774 808 865 866 FZ	87.53	
1.12	388.29	311.97	348.36	3	775 809 828 FZ	116.12	
1.12	386.29	310.82	346.57	4	776 810 868 871 FZ	86.64	
1.11	384.28	309.65	344.76	4	777 811 869 870 FZ	86.19	
1.11	405.85	345.70	384.33	3	778 812 860 FZ	128.11	
1.11	403.68	344.37	382.27	4	779 813 872 875 FZ	95.57	
1.11	401.48	343.03	380.19	4	780 814 873 874 FZ	95.05	
1.11	399.27	341.67	378.10	3	781 815 861 FZ	126.03	
1.10	397.04	340.29	375.98	4	782 816 877 878 FZ	94.00	
1.10	394.79	338.91	373.85	4	783 817 876 879 FZ	93.46	
1.10	419.71	381.04	419.65	3	784 818 862 FZ	139.88	
1.10	417.26	379.44	417.20	4	785 819 880 883 FZ	104.30	
1.10	414.78	377.82	414.72	4	786 820 881 882 FZ	103.68	
1.10	412.28	376.18	412.22	3	787 821 863 FZ	137.41	
1.09	409.76	374.52	409.70	4	788 822 884 887 FZ	102.42	
1.09	407.21	372.85	407.15	4	789 823 885 886 FZ	101.79	
1.09	378.32	436.13	475.40	3	512 514 630 FZ	158.47	
1.09	375.88	434.11	472.35	4	544 601 633 634 FZ	118.09	
1.09	373.42	432.07	469.26	4	545 600 632 635 FZ	117.31	
1.08	370.94	430.00	466.13	3	546 599 627 FZ	155.38	
1.08	368.42	427.91	462.97	4	547 598 637 638 FZ	115.74	
1.08	365.88	425.78	459.78	4	548 597 636 639 FZ	114.95	
1.08	379.78	460.88	496.69	3	549 596 624 FZ	165.56	
1.08	377.06	458.50	493.14	4	550 595 641 642 FZ	123.28	
1.07	374.31	456.10	489.55	4	551 594 640 643 FZ	122.39	
1.07	371.53	453.66	485.91	3	552 593 621 FZ	161.97	
1.07	368.72	451.19	482.23	4	553 592 644 647 FZ	120.56	
1.07	365.87	448.68	478.50	4	554 591 645 646 FZ	119.63	
1.06	376.76	481.60	512.47	3	555 590 618 FZ	170.82	

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
1.06	373.73	478.81	508.34	4	556 589 649 650 FZ	127.09
1.06	370.65	475.98	504.16	4	557 588 648 651 FZ	126.04
1.06	367.54	473.10	499.93	3	558 587 615 FZ	166.64
1.05	364.38	470.18	495.63	4	559 586 653 654 FZ	123.91
1.05	361.18	467.20	491.27	4	560 585 652 655 FZ	122.82
1.05	371.66	500.97	525.44	3	561 584 612 FZ	175.15
1.05	368.24	497.65	520.60	4	562 583 656 659 FZ	130.15
1.04	364.77	494.27	515.69	4	563 582 657 658 FZ	128.92
1.04	361.24	490.82	510.70	3	564 581 609 FZ	170.23
1.04	357.65	487.32	505.63	4	565 580 661 662 FZ	126.41
1.03	354.01	483.74	500.48	4	566 579 660 663 FZ	125.12
1.03	359.90	512.24	528.40	3	567 578 606 FZ	176.13
1.03	356.03	508.27	522.71	4	568 577 665 666 FZ	130.68
1.03	352.08	504.22	516.92	4	569 576 664 667 FZ	129.23
1.02	348.07	500.09	511.03	3	570 575 603 FZ	170.34
1.02	343.98	495.86	505.03	4	571 574 669 670 FZ	126.26
1.02	339.81	491.53	498.91	4	572 573 668 671 FZ	124.73
1.01	323.21	615.33	622.36	7	4 6 372 443 444 446 447 FZ	88.91
1.01	319.03	609.60	614.31	6	383 388 393 394 449 451 FZ	102.38
1.00	314.76	603.72	606.09	4	382 387 392 395 FZ	151.52
1.00	310.39	597.69	597.68	4	381 386 391 396 FZ	149.42
1.00	305.93	591.50	589.08	4	380 385 390 397 FZ	147.27
0.99	301.35	585.13	580.28	4	379 384 389 398 FZ	145.07
0.99	302.40	608.04	600.34	7	14 18 302 360 361 363 364 FZ	85.76
0.98	297.50	600.93	590.61	6	295 300 318 319 367 369 FZ	98.43
0.98	292.46	593.61	580.61	4	294 299 317 320 FZ	145.15
0.97	287.29	586.03	570.33	4	293 298 316 321 FZ	142.58
0.97	281.95	578.20	559.74	4	292 297 315 322 FZ	139.94
0.96	276.45	570.07	548.83	4	291 296 314 323 FZ	137.21
0.96	276.03	589.87	564.59	7	13 17 205 277 278 280 281 FZ	80.66
0.95	270.03	580.64	552.33	6	216 221 265 274 283 285 FZ	92.06
0.95	263.82	571.01	539.62	4	215 220 266 273 FZ	134.91
0.94	257.36	560.93	526.41	4	214 219 267 272 FZ	131.60
0.93	250.63	550.35	512.65	4	213 218 268 271 FZ	128.16
0.92	243.60	539.20	498.27	4	212 217 269 270 FZ	124.57
0.92	242.67	557.84	511.05	7	12 16 111 144 145 147 148 FZ	73.01
0.91	234.70	544.60	494.26	6	116 121 126 127 150 152 FZ	82.38
0.90	226.26	530.46	476.49	4	115 120 125 128 FZ	119.12
0.89	217.29	515.26	457.60	4	114 119 124 129 FZ	114.40
0.88	207.68	498.78	437.36	4	113 118 123 130 FZ	109.34
0.86	197.30	480.73	415.50	4	112 117 122 131 FZ	103.88
0.85	189.60	483.39	410.90	7	11 15 20 68 69 71 72 FZ	58.70
0.83	176.78	459.67	383.12	6	31 36 51 52 74 76 FZ	63.85
0.81	162.26	432.21	351.66	4	30 35 50 53 FZ	87.91
0.79	145.29	399.22	314.88	4	29 34 49 54 FZ	78.72
0.75	124.34	356.95	269.48	4	28 33 48 55 FZ	67.37
0.70	95.28	294.80	206.50	4	27 32 47 56 FZ	51.63

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
2 ^a itera							
0.89166	1.75	0.07	0.019	0.138	0.73	4.11	0.29

Fg	Pz (kg/m ²)	Fa (kg)	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
1.16	439.0	301.55	350.3	2	953 954 FZ	175.16
1.16	437.1	300.62	348.8	2	998 1000 FZ	174.41
1.16	435.2	299.69	347.3	2	997 999 FZ	173.65
1.16	433.3	298.74	345.8	2	950 951 FZ	172.89
1.16	431.4	297.79	344.3	2	994 996 FZ	172.13
1.15	429.5	296.83	342.7	2	993 995 FZ	171.35
1.15	427.5	295.86	341.2	2	947 948 FZ	170.58
1.15	425.6	294.89	339.6	2	990 992 FZ	169.80
1.15	423.6	293.90	338.0	2	989 991 FZ	169.01
1.15	421.6	292.91	336.4	2	944 945 FZ	168.21
1.15	419.6	291.91	334.8	2	977 979 FZ	167.41
1.15	417.6	290.90	333.2	2	976 978 FZ	166.61
1.14	402.1	315.37	360.7	3	752 754 826 FZ	120.25
1.14	400.1	314.25	359.0	4	773 807 864 867 FZ	89.74
1.14	398.1	313.12	357.2	4	774 808 865 866 FZ	89.29
1.14	396.1	311.97	355.4	3	775 809 828 FZ	118.45
1.14	394.0	310.82	353.5	4	776 810 868 871 FZ	88.38
1.14	392.0	309.65	351.7	4	777 811 869 870 FZ	87.92
1.13	414.0	345.70	392.0	3	778 812 860 FZ	130.68
1.13	411.8	344.37	389.9	4	779 813 872 875 FZ	97.49
1.13	409.5	343.03	387.8	4	780 814 873 874 FZ	96.96
1.13	407.3	341.67	385.7	3	781 815 861 FZ	128.56
1.13	405.0	340.29	383.5	4	782 816 877 878 FZ	95.88
1.13	402.7	338.91	381.4	4	783 817 876 879 FZ	95.34
1.12	428.1	381.04	428.1	3	784 818 862 FZ	142.69
1.12	425.6	379.44	425.6	4	785 819 880 883 FZ	106.39
1.12	423.1	377.82	423.0	4	786 820 881 882 FZ	105.76
1.12	420.6	376.18	420.5	3	787 821 863 FZ	140.17
1.12	418.0	374.52	417.9	4	788 822 884 887 FZ	104.48
1.11	415.4	372.85	415.3	4	789 823 885 886 FZ	103.83
1.11	385.9	436.13	484.9	3	512 514 630 FZ	161.65
1.11	383.4	434.11	481.8	4	544 601 633 634 FZ	120.46
1.11	380.9	432.07	478.7	4	545 600 632 635 FZ	119.67
1.11	378.4	430.00	475.5	3	546 599 627 FZ	158.50
1.10	375.8	427.91	472.3	4	547 598 637 638 FZ	118.07
1.10	373.2	425.78	469.0	4	548 597 636 639 FZ	117.25
1.10	387.4	460.88	506.7	3	549 596 624 FZ	168.89
1.10	384.6	458.50	503.0	4	550 595 641 642 FZ	125.76
1.09	381.8	456.10	499.4	4	551 594 640 643 FZ	124.84
1.09	379.0	453.66	495.7	3	552 593 621 FZ	165.22
1.09	376.1	451.19	491.9	4	553 592 644 647 FZ	122.98
1.09	373.2	448.68	488.1	4	554 591 645 646 FZ	122.03
1.09	384.3	481.60	522.8	3	555 590 618 FZ	174.25

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
1.08	381.2	478.81	518.6	4	556 589 649 650 FZ	129.64
1.08	378.1	475.98	514.3	4	557 588 648 651 FZ	128.57
1.08	374.9	473.10	510.0	3	558 587 615 FZ	169.99
1.08	371.7	470.18	505.6	4	559 586 653 654 FZ	126.40
1.07	368.4	467.20	501.1	4	560 585 652 655 FZ	125.28
1.07	379.1	500.97	536.0	3	561 584 612 FZ	178.66
1.07	375.6	497.65	531.1	4	562 583 656 659 FZ	132.76
1.06	372.1	494.27	526.0	4	563 582 657 658 FZ	131.51
1.06	368.5	490.82	521.0	3	564 581 609 FZ	173.65
1.06	364.8	487.32	515.8	4	565 580 661 662 FZ	128.95
1.06	361.1	483.74	510.5	4	566 579 660 663 FZ	127.63
1.05	367.1	512.24	539.0	3	567 578 606 FZ	179.67
1.05	363.2	508.27	533.2	4	568 577 665 666 FZ	133.30
1.05	359.2	504.22	527.3	4	569 576 664 667 FZ	131.83
1.04	355.1	500.09	521.3	3	570 575 603 FZ	173.76
1.04	350.9	495.86	515.2	4	571 574 669 670 FZ	128.79
1.04	346.6	491.53	508.9	4	572 573 668 671 FZ	127.23
1.03	329.7	615.33	634.9	7	4 6 372 443 444 446 447 FZ	90.69
1.03	325.4	609.60	626.6	6	383 388 393 394 449 451 FZ	104.44
1.02	321.1	603.72	618.3	4	382 387 392 395 FZ	154.56
1.02	316.6	597.69	609.7	4	381 386 391 396 FZ	152.42
1.02	312.1	591.50	600.9	4	380 385 390 397 FZ	150.23
1.01	307.4	585.13	591.9	4	379 384 389 398 FZ	147.98
1.01	308.5	608.04	612.4	7	14 18 302 360 361 363 364 FZ	87.48
1.00	303.5	600.93	602.5	6	295 300 318 319 367 369 FZ	100.41
1.00	298.3	593.61	592.3	4	294 299 317 320 FZ	148.07
0.99	293.1	586.03	581.8	4	293 298 316 321 FZ	145.45
0.99	287.6	578.20	571.0	4	292 297 315 322 FZ	142.75
0.98	282.0	570.07	559.8	4	291 296 314 323 FZ	139.96
0.98	281.6	589.87	575.9	7	13 17 205 277 278 280 281 FZ	82.28
0.97	275.5	580.64	563.4	6	216 221 265 274 283 285 FZ	93.90
0.96	269.1	571.01	550.5	4	215 220 266 273 FZ	137.61
0.96	262.5	560.93	537.0	4	214 219 267 272 FZ	134.25
0.95	255.7	550.35	522.9	4	213 218 268 271 FZ	130.74
0.94	248.5	539.20	508.3	4	212 217 269 270 FZ	127.07
0.93	247.5	557.84	521.3	7	12 16 111 144 145 147 148 FZ	74.47
0.93	239.4	544.60	504.2	6	116 121 126 127 150 152 FZ	84.03
0.92	230.8	530.46	486.1	4	115 120 125 128 FZ	121.52
0.91	221.7	515.26	466.8	4	114 119 124 129 FZ	116.70
0.89	211.9	498.78	446.1	4	113 118 123 130 FZ	111.54
0.88	201.3	480.73	423.8	4	112 117 122 131 FZ	105.96
0.87	193.4	483.39	419.2	7	11 15 20 68 69 71 72 FZ	59.88
0.85	180.3	459.67	390.8	6	31 36 51 52 74 76 FZ	65.14
0.83	165.5	432.21	358.7	4	30 35 50 53 FZ	89.68
0.80	148.2	399.22	321.2	4	29 34 49 54 FZ	80.30
0.77	126.8	356.95	274.9	4	28 33 48 55 FZ	68.72
0.71	97.2	294.80	210.7	4	27 32 47 56 FZ	52.66

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
3^{ra} itera							
0.88811	1.74	0.07	0.019	0.138	0.73	4.11	0.29
Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos	
1.16	439.0	301.55	350.3	2	953 954 FZ	175.16	
1.16	437.1	300.62	348.8	2	998 1000 FZ	174.41	
1.16	435.2	299.69	347.3	2	997 999 FZ	173.65	
1.16	433.3	298.74	345.8	2	950 951 FZ	172.89	
1.16	431.4	297.79	344.3	2	994 996 FZ	172.13	
1.15	429.5	296.83	342.7	2	993 995 FZ	171.35	
1.15	427.5	295.86	341.2	2	947 948 FZ	170.58	
1.15	425.6	294.89	339.6	2	990 992 FZ	169.80	
1.15	423.6	293.90	338.0	2	989 991 FZ	169.01	
1.15	421.6	292.91	336.4	2	944 945 FZ	168.21	
1.15	419.6	291.91	334.8	2	977 979 FZ	167.41	
1.15	417.6	290.90	333.2	2	976 978 FZ	166.61	
1.14	402.1	315.37	360.7	3	752 754 826 FZ	120.25	
1.14	400.1	314.25	359.0	4	773 807 864 867 FZ	89.74	
1.14	398.1	313.12	357.2	4	774 808 865 866 FZ	89.29	
1.14	396.1	311.97	355.4	3	775 809 828 FZ	118.45	
1.14	394.0	310.82	353.5	4	776 810 868 871 FZ	88.38	
1.14	392.0	309.65	351.7	4	777 811 869 870 FZ	87.92	
1.13	414.0	345.70	392.0	3	778 812 860 FZ	130.68	
1.13	411.8	344.37	389.9	4	779 813 872 875 FZ	97.49	
1.13	409.5	343.03	387.8	4	780 814 873 874 FZ	96.96	
1.13	407.3	341.67	385.7	3	781 815 861 FZ	128.56	
1.13	405.0	340.29	383.5	4	782 816 877 878 FZ	95.88	
1.13	402.7	338.91	381.4	4	783 817 876 879 FZ	95.34	
1.12	428.1	381.04	428.1	3	784 818 862 FZ	142.69	
1.12	425.6	379.44	425.6	4	785 819 880 883 FZ	106.39	
1.12	423.1	377.82	423.0	4	786 820 881 882 FZ	105.76	
1.12	420.6	376.18	420.5	3	787 821 863 FZ	140.17	
1.12	418.0	374.52	417.9	4	788 822 884 887 FZ	104.48	
1.11	415.4	372.85	415.3	4	789 823 885 886 FZ	103.83	
1.11	385.9	436.13	484.9	3	512 514 630 FZ	161.65	
1.11	383.4	434.11	481.8	4	544 601 633 634 FZ	120.46	
1.11	380.9	432.07	478.7	4	545 600 632 635 FZ	119.67	
1.11	378.4	430.00	475.5	3	546 599 627 FZ	158.50	
1.10	375.8	427.91	472.3	4	547 598 637 638 FZ	118.07	
1.10	373.2	425.78	469.0	4	548 597 636 639 FZ	117.25	
1.10	387.4	460.88	506.7	3	549 596 624 FZ	168.89	
1.10	384.6	458.50	503.0	4	550 595 641 642 FZ	125.76	
1.09	381.8	456.10	499.4	4	551 594 640 643 FZ	124.84	
1.09	379.0	453.66	495.7	3	552 593 621 FZ	165.22	
1.09	376.1	451.19	491.9	4	553 592 644 647 FZ	122.98	
1.09	373.2	448.68	488.1	4	554 591 645 646 FZ	122.03	
1.09	384.3	481.60	522.8	3	555 590 618 FZ	174.25	

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
1.08	381.2	478.81	518.6	4	556 589 649 650 FZ	129.64
1.08	378.1	475.98	514.3	4	557 588 648 651 FZ	128.57
1.08	374.9	473.10	510.0	3	558 587 615 FZ	169.99
1.08	371.7	470.18	505.6	4	559 586 653 654 FZ	126.40
1.07	368.4	467.20	501.1	4	560 585 652 655 FZ	125.28
1.07	379.1	500.97	536.0	3	561 584 612 FZ	178.66
1.07	375.6	497.65	531.1	4	562 583 656 659 FZ	132.76
1.06	372.1	494.27	526.0	4	563 582 657 658 FZ	131.51
1.06	368.5	490.82	521.0	3	564 581 609 FZ	173.65
1.06	364.8	487.32	515.8	4	565 580 661 662 FZ	128.95
1.06	361.1	483.74	510.5	4	566 579 660 663 FZ	127.63
1.05	367.1	512.24	539.0	3	567 578 606 FZ	179.67
1.05	363.2	508.27	533.2	4	568 577 665 666 FZ	133.30
1.05	359.2	504.22	527.3	4	569 576 664 667 FZ	131.83
1.04	355.1	500.09	521.3	3	570 575 603 FZ	173.76
1.04	350.9	495.86	515.2	4	571 574 669 670 FZ	128.79
1.04	346.6	491.53	508.9	4	572 573 668 671 FZ	127.23
1.03	329.7	615.33	634.9	7	4 6 372 443 444 446 447 FZ	90.69
1.03	325.4	609.60	626.6	6	383 388 393 394 449 451 FZ	104.44
1.02	321.1	603.72	618.3	4	382 387 392 395 FZ	154.56
1.02	316.6	597.69	609.7	4	381 386 391 396 FZ	152.42
1.02	312.1	591.50	600.9	4	380 385 390 397 FZ	150.23
1.01	307.4	585.13	591.9	4	379 384 389 398 FZ	147.98
1.01	308.5	608.04	612.4	7	14 18 302 360 361 363 364 FZ	87.48
1.00	303.5	600.93	602.5	6	295 300 318 319 367 369 FZ	100.41
1.00	298.3	593.61	592.3	4	294 299 317 320 FZ	148.07
0.99	293.1	586.03	581.8	4	293 298 316 321 FZ	145.45
0.99	287.6	578.20	571.0	4	292 297 315 322 FZ	142.75
0.98	282.0	570.07	559.8	4	291 296 314 323 FZ	139.96
0.98	281.6	589.87	575.9	7	13 17 205 277 278 280 281 FZ	82.28
0.97	275.5	580.64	563.4	6	216 221 265 274 283 285 FZ	93.90
0.96	269.1	571.01	550.5	4	215 220 266 273 FZ	137.61
0.96	262.5	560.93	537.0	4	214 219 267 272 FZ	134.25
0.95	255.7	550.35	522.9	4	213 218 268 271 FZ	130.74
0.94	248.5	539.20	508.3	4	212 217 269 270 FZ	127.07
0.93	247.5	557.84	521.3	7	12 16 111 144 145 147 148 FZ	74.47
0.93	239.4	544.60	504.2	6	116 121 126 127 150 152 FZ	84.03
0.92	230.8	530.46	486.1	4	115 120 125 128 FZ	121.52
0.91	221.7	515.26	468.8	4	114 119 124 129 FZ	116.70
0.89	211.9	498.78	446.1	4	113 118 123 130 FZ	111.54
0.88	201.3	480.73	423.8	4	112 117 122 131 FZ	105.96
0.87	193.4	483.39	419.2	7	11 15 20 68 69 71 72 FZ	59.88
0.85	180.3	459.67	390.8	6	31 36 51 52 74 76 FZ	65.14
0.83	165.5	432.21	358.7	4	30 35 50 53 FZ	89.68
0.80	148.2	399.22	321.2	4	29 34 49 54 FZ	80.30
0.77	126.8	356.95	274.9	4	28 33 48 55 FZ	68.72
0.71	97.2	294.80	210.7	4	27 32 47 56 FZ	52.66

Presiones y Fuerzas sobre las Líneas en dirección Z-Z.

Sección	Altura (m).	Frz	Fa	V ₀ (km/h).	q _z (kg/m ²).	Ae (m ²).	AT (m ²).	Solidez ϕ	bV _D	Ca	g
1	90	1.31	1.31	228.07	237.67	0.123825	0.123825	1.00	12.87	1.2	1.38
2	89	1.31	1.31	227.72	236.94	0.123825	0.123825	1.00	12.85	1.2	1.38
3	88	1.31	1.31	227.36	236.20	0.123825	0.123825	1.00	12.83	1.2	1.38
4	87	1.31	1.31	227.01	235.46	0.123825	0.123825	1.00	12.81	1.2	1.38
5	86	1.30	1.30	226.64	234.71	0.123825	0.123825	1.00	12.79	1.2	1.38
6	85	1.30	1.30	226.28	233.95	0.123825	0.123825	1.00	12.77	1.2	1.38
7	84	1.30	1.30	225.91	233.19	0.123825	0.123825	1.00	12.75	1.2	1.38
8	83	1.30	1.30	225.54	232.42	0.123825	0.123825	1.00	12.73	1.2	1.38
9	82	1.30	1.30	225.16	231.64	0.123825	0.123825	1.00	12.71	1.2	1.39
10	81	1.29	1.29	224.78	230.86	0.123825	0.123825	1.00	12.68	1.2	1.39
11	80	1.29	1.29	224.39	230.07	0.123825	0.123825	1.00	12.66	1.2	1.39
12	79	1.29	1.29	224.00	229.27	0.123825	0.123825	1.00	12.64	1.2	1.39
13	78	1.29	1.29	223.61	228.46	0.123825	0.123825	1.00	15.77	1.2	1.39
14	77	1.28	1.28	223.21	227.65	0.123825	0.123825	1.00	15.74	1.2	1.39
15	76	1.28	1.28	222.81	226.83	0.123825	0.123825	1.00	15.72	1.2	1.39
16	75	1.28	1.28	222.40	226.00	0.123825	0.123825	1.00	15.69	1.2	1.39
17	74	1.28	1.28	221.99	225.17	0.123825	0.123825	1.00	15.66	1.2	1.39
18	73	1.28	1.28	221.58	224.33	0.123825	0.123825	1.00	15.63	1.2	1.39
19	72	1.27	1.27	221.15	223.47	0.123825	0.123825	1.00	15.60	1.2	1.39
20	71	1.27	1.27	220.73	222.61	0.123825	0.123825	1.00	15.57	1.2	1.40
21	70	1.27	1.27	220.30	221.74	0.123825	0.123825	1.00	15.54	1.2	1.40
22	69	1.27	1.27	219.86	220.86	0.123825	0.123825	1.00	15.51	1.2	1.40
23	68	1.26	1.26	219.42	219.98	0.123825	0.123825	1.00	15.48	1.2	1.40
24	67	1.26	1.26	218.97	219.08	0.123825	0.123825	1.00	15.45	1.2	1.40
25	66	1.26	1.26	218.51	218.17	0.123825	0.123825	1.00	15.41	1.2	1.40
26	65	1.25	1.25	218.06	217.25	0.123825	0.123825	1.00	15.38	1.2	1.40
27	64	1.25	1.25	217.59	216.32	0.123825	0.123825	1.00	15.35	1.2	1.40
28	63	1.25	1.25	217.12	215.39	0.123825	0.123825	1.00	15.31	1.2	1.40
29	62	1.25	1.25	216.64	214.44	0.123825	0.123825	1.00	15.28	1.2	1.41
30	61	1.24	1.24	216.15	213.48	0.123825	0.123825	1.00	15.25	1.2	1.41
31	60	1.24	1.24	215.66	212.51	0.24765	0.24765	1.00	21.30	1.2	1.41
32	59	1.24	1.24	215.16	211.52	0.24765	0.24765	1.00	21.25	1.2	1.41
33	58	1.24	1.24	214.65	210.53	0.24765	0.24765	1.00	21.20	1.2	1.41
34	57	1.23	1.23	214.14	209.52	0.24765	0.24765	1.00	21.15	1.2	1.41
35	56	1.23	1.23	213.62	208.50	0.24765	0.24765	1.00	21.09	1.2	1.41
36	55	1.23	1.23	213.09	207.46	0.24765	0.24765	1.00	21.04	1.2	1.42
37	54	1.22	1.22	212.55	206.42	0.24765	0.24765	1.00	20.99	1.2	1.42
38	53	1.22	1.22	212.00	205.35	0.24765	0.24765	1.00	20.93	1.2	1.42
39	52	1.22	1.22	211.44	204.28	0.24765	0.24765	1.00	20.88	1.2	1.42
40	51	1.21	1.21	210.88	203.18	0.24765	0.24765	1.00	20.82	1.2	1.42
41	50	1.21	1.21	210.30	202.08	0.24765	0.24765	1.00	20.77	1.2	1.42
42	49	1.21	1.21	209.72	200.95	0.24765	0.24765	1.00	20.71	1.2	1.42
43	48	1.20	1.20	209.12	199.81	0.24765	0.24765	1.00	20.65	1.2	1.43
44	47	1.20	1.20	208.51	198.66	0.24765	0.24765	1.00	20.59	1.2	1.43
45	46	1.20	1.20	207.90	197.48	0.24765	0.24765	1.00	20.53	1.2	1.43
46	45	1.19	1.19	207.27	196.29	0.24765	0.24765	1.00	20.47	1.2	1.43
47	44	1.19	1.19	206.62	195.07	0.24765	0.24765	1.00	20.40	1.2	1.43

Sección	Altura (m).	Frz	Fa	V _D (km/h).	q _z (kg/m ²).	Ae (m ²).	AT (m ²).	Solidez ϕ	bV _D	Ca	g
48	43	1.19	1.19	205.97	193.84	0.24765	0.24765	1.00	20.34	1.2	1.43
49	42	1.18	1.18	205.30	192.58	0.24765	0.24765	1.00	20.27	1.2	1.44
50	41	1.18	1.18	204.62	191.31	0.24765	0.24765	1.00	20.21	1.2	1.44
51	40	1.17	1.17	203.92	190.01	0.24765	0.24765	1.00	20.14	1.2	1.44
52	39	1.17	1.17	203.21	188.68	0.24765	0.24765	1.00	20.07	1.2	1.44
53	38	1.17	1.17	202.49	187.34	0.24765	0.24765	1.00	20.00	1.2	1.44
54	37	1.16	1.16	201.74	185.96	0.24765	0.24765	1.00	19.92	1.2	1.45
55	36	1.16	1.16	200.98	184.56	0.24765	0.24765	1.00	19.85	1.2	1.45
56	35	1.15	1.15	200.20	183.13	0.24765	0.24765	1.00	19.77	1.2	1.45
57	34	1.15	1.15	199.40	181.67	0.24765	0.24765	1.00	19.69	1.2	1.45
58	33	1.14	1.14	198.58	180.18	0.24765	0.24765	1.00	19.61	1.2	1.45
59	32	1.14	1.14	197.74	178.66	0.24765	0.24765	1.00	19.53	1.2	1.46
60	31	1.13	1.13	196.88	177.10	0.24765	0.24765	1.00	19.44	1.2	1.46
61	30	1.13	1.13	195.99	175.50	0.371475	0.371475	1.00	24.88	1.2	1.46
62	29	1.12	1.12	195.07	173.87	0.371475	0.371475	1.00	24.77	1.2	1.47
63	28	1.12	1.12	194.13	172.19	0.371475	0.371475	1.00	24.65	1.2	1.47
64	27	1.11	1.11	193.16	170.47	0.371475	0.371475	1.00	24.52	1.2	1.47
65	26	1.11	1.11	192.15	168.71	0.371475	0.371475	1.00	24.40	1.2	1.47
66	25	1.10	1.10	191.12	166.89	0.371475	0.371475	1.00	24.27	1.2	1.48
67	24	1.09	1.09	190.04	165.02	0.371475	0.371475	1.00	24.13	1.2	1.48
68	23	1.09	1.09	188.93	163.09	0.371475	0.371475	1.00	23.99	1.2	1.48
69	22	1.08	1.08	187.78	161.11	0.371475	0.371475	1.00	23.84	1.2	1.49
70	21	1.07	1.07	186.57	159.05	0.371475	0.371475	1.00	23.69	1.2	1.49
71	20	1.07	1.07	185.32	156.92	0.371475	0.371475	1.00	23.53	1.2	1.49
72	19	1.06	1.06	184.01	154.72	0.371475	0.371475	1.00	23.36	1.2	1.50
73	18	1.05	1.05	182.65	152.42	0.371475	0.371475	1.00	23.19	1.2	1.50
74	17	1.04	1.04	181.21	150.04	0.371475	0.371475	1.00	23.01	1.2	1.51
75	16	1.03	1.03	179.70	147.55	0.371475	0.371475	1.00	22.82	1.2	1.51
76	15	1.02	1.02	178.11	144.94	0.371475	0.371475	1.00	22.61	1.2	1.52
77	14	1.02	1.02	176.42	142.21	0.371475	0.371475	1.00	22.40	1.2	1.52
78	13	1.00	1.00	174.63	139.33	0.371475	0.371475	1.00	22.17	1.2	1.53
79	12	0.99	0.99	172.71	136.29	0.371475	0.371475	1.00	21.93	1.2	1.54
80	11	0.98	0.98	170.65	133.05	0.371475	0.371475	1.00	21.67	1.2	1.54
81	10	0.97	0.97	168.42	129.60	0.371475	0.371475	1.00	21.38	1.2	1.55
82	9	0.96	0.96	165.99	125.88	0.371475	0.371475	1.00	21.07	1.2	1.56
83	8	0.94	0.94	163.31	121.86	0.371475	0.371475	1.00	20.73	1.2	1.57
84	7	0.92	0.92	160.33	117.45	0.371475	0.371475	1.00	20.36	1.2	1.58
85	6	0.90	0.90	156.95	112.56	0.371475	0.371475	1.00	19.93	1.2	1.60
86	5	0.88	0.88	153.05	107.03	0.371475	0.371475	1.00	19.43	1.2	1.61
87	4	0.85	0.85	148.41	100.64	0.371475	0.371475	1.00	18.84	1.2	1.63
88	3	0.82	0.82	142.64	92.96	0.371475	0.371475	1.00	18.11	1.2	1.66
89	2	0.78	0.78	134.87	83.12	0.371475	0.371475	1.00	17.12	1.2	1.69
90	1	0.71	0.71	122.57	68.64	0.371475	0.371475	1.00	15.56	1.2	1.76

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
Valor propuesto							
2.00	3.92	0.019	0.04	0.075	0.96	4.18	0.19
Fg	Pz (kg/m ²)	Fa (kg)	Fz (kg)	No. de nodos	Nodos	Fz / #nodos	
0.9484	270.49	35.32	33.49	2	953 954 FZ	16.75	
0.9473	269.33	35.21	33.35	2	998 1000 FZ	16.68	
0.9461	268.17	35.10	33.21	2	997 999 FZ	16.60	
0.9450	266.99	34.99	33.06	2	950 951 FZ	16.53	
0.9438	265.81	34.87	32.91	2	994 996 FZ	16.46	
0.9426	264.62	34.76	32.77	2	993 995 FZ	16.38	
0.9414	263.42	34.65	32.62	2	947 948 FZ	16.31	
0.9402	262.21	34.53	32.47	2	990 992 FZ	16.23	
0.9389	260.99	34.42	32.32	2	989 991 FZ	16.16	
0.9377	259.77	34.30	32.17	2	944 945 FZ	16.08	
0.9364	258.53	34.19	32.01	2	977 979 FZ	16.01	
0.9352	257.28	34.07	31.86	2	976 978 FZ	15.93	
0.9339	256.03	33.95	31.70	3	752 754 826 FZ	10.57	
0.9326	254.76	33.83	31.55	4	773 807 864 867 FZ	7.89	
0.9313	253.49	33.71	31.39	4	774 808 865 866 FZ	7.85	
0.9299	252.20	33.58	31.23	3	775 809 828 FZ	10.41	
0.9286	250.91	33.46	31.07	4	776 810 868 871 FZ	7.77	
0.9272	249.60	33.33	30.91	4	777 811 869 870 FZ	7.73	
0.9258	248.28	33.21	30.74	3	778 812 860 FZ	10.25	
0.9244	246.95	33.08	30.58	4	779 813 872 875 FZ	7.64	
0.9230	245.61	32.95	30.41	4	780 814 873 874 FZ	7.60	
0.9216	244.25	32.82	30.24	3	781 815 861 FZ	10.08	
0.9201	242.89	32.69	30.08	4	782 816 877 878 FZ	7.52	
0.9187	241.51	32.55	29.91	4	783 817 876 879 FZ	7.48	
0.9172	240.12	32.42	29.73	3	784 818 862 FZ	9.91	
0.9157	238.72	32.28	29.56	4	785 819 880 883 FZ	7.39	
0.9141	237.30	32.14	29.38	4	786 820 881 882 FZ	7.35	
0.9126	235.87	32.00	29.21	3	787 821 863 FZ	9.74	
0.9110	234.42	31.86	29.03	4	788 822 884 887 FZ	7.26	
0.9094	232.97	31.72	28.85	4	789 823 885 886 FZ	7.21	
0.9078	231.49	63.15	57.33	3	512 514 630 FZ	19.11	
0.9061	230.00	62.86	56.96	4	544 601 633 634 FZ	14.24	
0.9045	228.50	62.56	56.59	4	545 600 632 635 FZ	14.15	
0.9028	226.98	62.26	56.21	3	546 599 627 FZ	18.74	
0.9010	225.44	61.96	55.83	4	547 598 637 638 FZ	13.96	
0.8993	223.88	61.65	55.45	4	548 597 636 639 FZ	13.86	
0.8975	222.31	61.34	55.06	3	549 596 624 FZ	18.35	
0.8957	220.72	61.03	54.66	4	550 595 641 642 FZ	13.67	
0.8939	219.11	60.71	54.26	4	551 594 640 643 FZ	13.57	
0.8920	217.49	60.38	53.86	3	552 593 621 FZ	17.95	
0.8901	215.84	60.05	53.45	4	553 592 644 647 FZ	13.36	
0.8881	214.17	59.72	53.04	4	554 591 645 646 FZ	13.26	
0.8862	212.48	59.38	52.62	3	555 590 618 FZ	17.54	

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.8842	210.77	59.04	52.20	4	556 589 649 650 FZ	13.05
0.8821	209.04	58.69	51.77	4	557 588 648 651 FZ	12.94
0.8800	207.28	58.33	51.33	3	558 587 615 FZ	17.11
0.8779	205.50	57.97	50.89	4	559 586 653 654 FZ	12.72
0.8757	203.69	57.60	50.44	4	560 585 652 655 FZ	12.61
0.8735	201.86	57.23	49.99	3	561 584 612 FZ	16.66
0.8712	200.00	56.85	49.53	4	562 583 656 659 FZ	12.38
0.8689	198.11	56.47	49.06	4	563 582 657 658 FZ	12.27
0.8665	196.20	56.07	48.59	3	564 581 609 FZ	16.20
0.8641	194.25	55.67	48.11	4	565 580 661 662 FZ	12.03
0.8616	192.27	55.26	47.62	4	566 579 660 663 FZ	11.90
0.8591	190.26	54.85	47.12	3	567 578 606 FZ	15.71
0.8565	188.21	54.42	46.61	4	568 577 665 666 FZ	11.65
0.8538	186.13	53.99	46.09	4	569 576 664 667 FZ	11.52
0.8510	184.01	53.55	45.57	3	570 575 603 FZ	15.19
0.8482	181.85	53.09	45.03	4	571 574 669 670 FZ	11.26
0.8453	179.64	52.63	44.49	4	572 573 668 671 FZ	11.12
0.8423	177.39	78.23	65.90	7	4 6 372 443 444 446 447 FZ	9.41
0.8392	175.10	77.51	65.05	6	383 388 393 394 449 451 FZ	10.84
0.8361	172.76	76.76	64.17	4	382 387 392 395 FZ	16.04
0.8328	170.36	75.99	63.28	4	381 386 391 396 FZ	15.82
0.8294	167.91	75.20	62.37	4	380 385 390 397 FZ	15.59
0.8259	165.40	74.39	61.44	4	379 384 389 398 FZ	15.36
0.8223	162.83	73.56	60.49	7	14 18 302 360 361 363 364 FZ	8.64
0.8185	160.19	72.70	59.51	6	295 300 318 319 367 369 FZ	9.92
0.8146	157.48	71.82	58.50	4	294 299 317 320 FZ	14.62
0.8105	154.69	70.90	57.46	4	293 298 316 321 FZ	14.37
0.8062	151.82	69.95	56.40	4	292 297 315 322 FZ	14.10
0.8018	148.86	68.97	55.30	4	291 296 314 323 FZ	13.82
0.7971	145.80	67.95	54.16	7	13 17 205 277 278 280 281 FZ	7.74
0.7922	142.63	66.88	52.98	6	216 221 265 274 283 285 FZ	8.83
0.7870	139.35	65.77	51.77	4	215 220 266 273 FZ	12.94
0.7816	135.94	64.61	50.50	4	214 219 267 272 FZ	12.62
0.7758	132.38	63.39	49.18	4	213 218 268 271 FZ	12.29
0.7696	128.67	62.11	47.80	4	212 217 269 270 FZ	11.95
0.7629	124.78	60.75	46.35	7	12 16 111 144 145 147 148 FZ	6.62
0.7558	120.68	59.31	44.83	6	116 121 126 127 150 152 FZ	7.47
0.7481	116.34	57.77	43.22	4	115 120 125 128 FZ	10.80
0.7396	111.73	56.12	41.50	4	114 119 124 129 FZ	10.38
0.7303	106.79	54.32	39.67	4	113 118 123 130 FZ	9.92
0.7198	101.45	52.36	37.69	4	112 117 122 131 FZ	9.42
0.7079	95.62	50.17	35.52	7	11 15 20 68 69 71 72 FZ	5.07
0.6941	89.15	47.71	33.12	6	31 36 51 52 74 76 FZ	5.52
0.6776	81.83	44.86	30.40	4	30 35 50 53 FZ	7.60
0.6569	73.27	41.44	27.22	4	29 34 49 54 FZ	6.80
0.6287	62.71	37.05	23.29	4	28 33 48 55 FZ	5.82
0.5834	48.05	30.60	17.85	4	27 32 47 56 FZ	4.46

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	v (Hz)	g_p	σ/μ
1 ^{ra} itera							
0.92609	1.81	0.066	0.020	0.13	0.74	4.12	0.28

Fg	Pz (kg/m ²)	Fa (kg)	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
1.14	324.80	35.32	40.22	2	953 954 FZ	20.11
1.14	323.41	35.21	40.05	2	998 1000 FZ	20.02
1.14	322.01	35.10	39.87	2	997 999 FZ	19.94
1.13	320.60	34.99	39.70	2	950 951 FZ	19.85
1.13	319.18	34.87	39.52	2	994 996 FZ	19.76
1.13	317.75	34.76	39.35	2	993 995 FZ	19.67
1.13	316.31	34.65	39.17	2	947 948 FZ	19.58
1.13	314.86	34.53	38.99	2	990 992 FZ	19.49
1.13	313.39	34.42	38.81	2	989 991 FZ	19.40
1.13	311.92	34.30	38.62	2	944 945 FZ	19.31
1.12	310.44	34.19	38.44	2	977 979 FZ	19.22
1.12	308.94	34.07	38.25	2	976 978 FZ	19.13
1.12	307.43	33.95	38.07	3	752 754 826 FZ	12.69
1.12	305.91	33.83	37.88	4	773-807-864-867 FZ	9.47
1.12	304.38	33.71	37.69	4	774 808 865 866 FZ	9.42
1.12	302.84	33.58	37.50	3	775 809 828 FZ	12.50
1.12	301.28	33.46	37.31	4	776 810 868 871 FZ	9.33
1.11	299.71	33.33	37.11	4	777 811 869 870 FZ	9.28
1.11	298.13	33.21	36.92	3	778 812 860 FZ	12.31
1.11	296.53	33.08	36.72	4	779 813 872 875 FZ	9.18
1.11	294.92	32.95	36.52	4	780 814 873 874 FZ	9.13
1.11	293.29	32.82	36.32	3	781 815 861 FZ	12.11
1.10	291.66	32.69	36.11	4	782 816 877 878 FZ	9.03
1.10	290.00	32.55	35.91	4	783 817 876 879 FZ	8.98
1.10	288.33	32.42	35.70	3	784 818 862 FZ	11.90
1.10	286.65	32.28	35.49	4	785 819 880 883 FZ	8.87
1.10	284.94	32.14	35.28	4	786 820 881 882 FZ	8.82
1.10	283.23	32.00	35.07	3	787 821 863 FZ	11.69
1.09	281.49	31.86	34.86	4	788 822 884 887 FZ	8.71
1.09	279.74	31.72	34.64	4	789 823 885 886 FZ	8.66
1.09	277.97	63.15	68.84	3	512 514 630 FZ	22.95
1.09	276.18	62.86	68.40	4	544 601 633 634 FZ	17.10
1.09	274.37	62.56	67.95	4	545 600 632 635 FZ	16.99
1.08	272.55	62.26	67.50	3	546 599 627 FZ	22.50
1.08	270.70	61.96	67.04	4	547 598 637 638 FZ	16.76
1.08	268.83	61.65	66.58	4	548 597 636 639 FZ	16.64
1.08	266.95	61.34	66.11	3	549 596 624 FZ	22.04
1.08	265.04	61.03	65.64	4	550 595 641 642 FZ	16.41
1.07	263.11	60.71	65.16	4	551 594 640 643 FZ	16.29
1.07	261.15	60.38	64.67	3	552 593 621 FZ	21.56
1.07	259.17	60.05	64.18	4	553 592 644 647 FZ	16.05
1.07	257.17	59.72	63.69	4	554 591 645 646 FZ	15.92
1.06	255.14	59.38	63.19	3	555 590 618 FZ	21.06

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
1.06	253.09	59.04	62.68	4	556 589 649 650 FZ	15.67
1.06	251.01	58.69	62.16	4	557 588 648 651 FZ	15.54
1.06	248.90	58.33	61.64	3	558 587 615 FZ	20.55
1.05	246.76	57.97	61.11	4	559 586 653 654 FZ	15.28
1.05	244.59	57.60	60.57	4	560 585 652 655 FZ	15.14
1.05	242.39	57.23	60.03	3	561 584 612 FZ	20.01
1.05	240.16	56.85	59.47	4	562 583 656 659 FZ	14.87
1.04	237.89	56.47	58.91	4	563 582 657 658 FZ	14.73
1.04	235.59	56.07	58.34	3	564 581 609 FZ	19.45
1.04	233.25	55.67	57.76	4	565 580 661 662 FZ	14.44
1.03	230.87	55.26	57.18	4	566 579 660 663 FZ	14.29
1.03	228.46	54.85	56.58	3	567 578 606 FZ	18.86
1.03	226.00	54.42	55.97	4	568 577 665 666 FZ	13.99
1.03	223.50	53.99	55.35	4	569 576 664 667 FZ	13.84
1.02	220.95	53.55	54.72	3	570 575 603 FZ	18.24
1.02	218.36	53.09	54.08	4	571 574 669 670 FZ	13.52
1.02	215.71	52.63	53.42	4	572 573 668 671 FZ	13.36
1.01	213.01	78.23	79.13	7	4 6 372 443 444 446 447 FZ	11.30
1.01	210.26	77.51	78.10	6	383 388 393 394 449 451 FZ	13.02
1.00	207.44	76.76	77.06	4	382 387 392 395 FZ	19.26
1.00	204.56	75.99	75.99	4	381 386 391 396 FZ	19.00
1.00	201.62	75.20	74.90	4	380 385 390 397 FZ	18.72
0.99	198.61	74.39	73.78	4	379 384 389 398 FZ	18.44
0.99	195.52	73.56	72.63	7	14 18 302 360 361 363 364 FZ	10.38
0.98	192.35	72.70	71.45	6	295 300 318 319 367 369 FZ	11.91
0.98	189.09	71.82	70.24	4	294 299 317 320 FZ	17.56
0.97	185.75	70.90	69.00	4	293 298 316 321 FZ	17.25
0.97	182.30	69.95	67.72	4	292 297 315 322 FZ	16.93
0.96	178.74	68.97	66.40	4	291 296 314 323 FZ	16.60
0.96	175.07	67.95	65.03	7	13 17 205 277 278 280 281 FZ	9.29
0.95	171.27	66.88	63.62	6	216 221 265 274 283 285 FZ	10.60
0.95	167.33	65.77	62.16	4	215 220 266 273 FZ	15.54
0.94	163.23	64.61	60.64	4	214 219 267 272 FZ	15.16
0.93	158.96	63.39	59.05	4	213 218 268 271 FZ	14.76
0.92	154.50	62.11	57.39	4	212 217 269 270 FZ	14.35
0.92	149.83	60.75	55.66	7	12 18 111 144 145 147 148 FZ	7.95
0.91	144.90	59.31	53.83	6	116 121 126 127 150 152 FZ	8.97
0.90	139.70	57.77	51.89	4	115 120 125 128 FZ	12.97
0.89	134.16	56.12	49.84	4	114 119 124 129 FZ	12.46
0.88	128.23	54.32	47.63	4	113 118 123 130 FZ	11.91
0.86	121.82	52.36	45.25	4	112 117 122 131 FZ	11.31
0.85	114.81	50.17	42.65	7	11 15 20 68 69 71 72 FZ	6.09
0.83	107.05	47.71	39.77	6	31 36 51 52 74 76 FZ	6.63
0.81	98.26	44.86	36.50	4	30 35 50 53 FZ	9.13
0.79	87.98	41.44	32.68	4	29 34 49 54 FZ	8.17
0.75	75.30	37.05	27.97	4	28 33 48 55 FZ	6.99
0.70	57.70	30.60	21.43	4	27 32 47 56 FZ	5.36

n_0 (Hz)	$3.6n_0(H/V)_H$	S	$(3.6n_0)/V_H$	E	v (Hz)	g_p	σ/μ
2 ^{da} itera 0.89166	1.75	0.07	0.019	0.138	0.73	4.11	0.29
Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos	
1.16	331.32	35.32	41.03	2	953 954 FZ	20.51	
1.16	329.90	35.21	40.85	2	998 1000 FZ	20.43	
1.16	328.47	35.10	40.67	2	997 999 FZ	20.34	
1.16	327.04	34.99	40.50	2	950 951 FZ	20.25	
1.16	325.59	34.87	40.32	2	994 996 FZ	20.16	
1.15	324.13	34.76	40.14	2	993 995 FZ	20.07	
1.15	322.66	34.65	39.95	2	947 948 FZ	19.98	
1.15	321.18	34.53	39.77	2	990 992 FZ	19.88	
1.15	319.69	34.42	39.59	2	989 991 FZ	19.79	
1.15	318.18	34.30	39.40	2	944 945 FZ	19.70	
1.15	316.67	34.19	39.21	2	977 979 FZ	19.61	
1.15	315.14	34.07	39.02	2	976 978 FZ	19.51	
1.14	313.61	33.95	38.83	3	752 754 826 FZ	12.94	
1.14	312.06	33.83	38.64	4	773 807 864 867 FZ	9.66	
1.14	310.49	33.71	38.45	4	774 808 865 866 FZ	9.61	
1.14	308.92	33.58	38.25	3	775 809 828 FZ	12.75	
1.14	307.33	33.46	38.06	4	776 810 868 871 FZ	9.51	
1.14	305.73	33.33	37.86	4	777 811 869 870 FZ	9.46	
1.13	304.11	33.21	37.66	3	778 812 860 FZ	12.55	
1.13	302.48	33.08	37.46	4	779 813 872 875 FZ	9.36	
1.13	300.84	32.95	37.25	4	780 814 873 874 FZ	9.31	
1.13	299.18	32.82	37.05	3	781 815 861 FZ	12.35	
1.13	297.51	32.69	36.84	4	782 816 877 878 FZ	9.21	
1.13	295.82	32.55	36.63	4	783 817 876 879 FZ	9.16	
1.12	294.12	32.42	36.42	3	784 818 862 FZ	12.14	
1.12	292.40	32.28	36.21	4	785 819 880 883 FZ	9.05	
1.12	290.67	32.14	35.99	4	786 820 881 882 FZ	9.00	
1.12	288.91	32.00	35.77	3	787 821 863 FZ	11.92	
1.12	287.14	31.86	35.56	4	788 822 884 887 FZ	8.89	
1.11	285.36	31.72	35.33	4	789 823 885 886 FZ	8.83	
1.11	283.55	63.15	70.22	3	512 514 630 FZ	23.41	
1.11	281.73	62.86	69.77	4	544 601 633 634 FZ	17.44	
1.11	279.88	62.56	69.31	4	545 600 632 635 FZ	17.33	
1.11	278.02	62.26	68.85	3	546 599 627 FZ	22.95	
1.10	276.14	61.96	68.39	4	547 598 637 638 FZ	17.10	
1.10	274.23	61.65	67.91	4	548 597 636 639 FZ	16.98	
1.10	272.31	61.34	67.44	3	549 596 624 FZ	22.48	
1.10	270.36	61.03	66.95	4	550 595 641 642 FZ	16.74	
1.09	268.39	60.71	66.47	4	551 594 640 643 FZ	16.62	
1.09	266.40	60.38	65.97	3	552 593 621 FZ	21.99	
1.09	264.38	60.05	65.47	4	553 592 644 647 FZ	16.37	
1.09	262.33	59.72	64.97	4	554 591 645 646 FZ	16.24	
1.09	260.27	59.38	64.45	3	555 590 618 FZ	21.48	

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
1.08	258.17	59.04	63.94	4	556 589 649 650 FZ	15.98
1.08	256.05	58.69	63.41	4	557 588 648 651 FZ	15.85
1.08	253.89	58.33	62.88	3	558 587 615 FZ	20.96
1.08	251.71	57.97	62.34	4	559 586 653 654 FZ	15.58
1.07	249.50	57.60	61.79	4	560 585 652 655 FZ	15.45
1.07	247.26	57.23	61.23	3	561 584 612 FZ	20.41
1.07	244.98	56.85	60.67	4	562 583 656 659 FZ	15.17
1.06	242.67	56.47	60.10	4	563 582 657 658 FZ	15.02
1.06	240.32	56.07	59.52	3	564 581 609 FZ	19.84
1.06	237.93	55.67	58.92	4	565 580 661 662 FZ	14.73
1.06	235.51	55.26	58.32	4	566 579 660 663 FZ	14.58
1.05	233.05	54.85	57.71	3	567 578 606 FZ	19.24
1.05	230.54	54.42	57.09	4	568 577 665 666 FZ	14.27
1.05	227.99	53.99	56.46	4	569 576 664 667 FZ	14.12
1.04	225.39	53.55	55.82	3	570 575 603 FZ	18.61
1.04	222.74	53.09	55.16	4	571 574 669 670 FZ	13.79
1.04	220.04	52.63	54.49	4	572 573 668 671 FZ	13.62
1.03	217.29	78.23	80.72	7	4 6 372 443 444 446 447 FZ	11.53
1.03	214.48	77.51	79.67	6	383 388 393 394 449 451 FZ	13.28
1.02	211.61	76.76	78.61	4	382 387 392 395 FZ	19.65
1.02	208.67	75.99	77.52	4	381 386 391 396 FZ	19.38
1.02	205.67	75.20	76.40	4	380 385 390 397 FZ	19.10
1.01	202.60	74.39	75.26	4	379 384 389 398 FZ	18.81
1.01	199.44	73.56	74.09	7	14 18 302 360 361 363 364 FZ	10.58
1.00	196.21	72.70	72.89	6	295 300 318 319 367 369 FZ	12.15
1.00	192.89	71.82	71.65	4	294 299 317 320 FZ	17.91
0.99	189.48	70.90	70.39	4	293 298 316 321 FZ	17.60
0.99	185.96	69.95	69.08	4	292 297 315 322 FZ	17.27
0.98	182.33	68.97	67.73	4	291 296 314 323 FZ	16.93
0.98	178.58	67.95	66.34	7	13 17 205 277 278 280 281 FZ	9.48
0.97	174.71	66.88	64.90	6	216 221 265 274 283 285 FZ	10.82
0.96	170.69	65.77	63.41	4	215 220 266 273 FZ	15.85
0.96	166.51	64.61	61.85	4	214 219 267 272 FZ	15.46
0.95	162.16	63.39	60.24	4	213 218 268 271 FZ	15.06
0.94	157.61	62.11	58.55	4	212 217 269 270 FZ	14.64
0.93	152.84	60.75	56.77	7	12 16 111 144 145 147 148 FZ	8.11
0.93	147.81	59.31	54.91	6	116 121 126 127 150 152 FZ	9.15
0.92	142.50	57.77	52.94	4	115 120 125 128 FZ	13.23
0.91	136.85	56.12	50.84	4	114 119 124 129 FZ	12.71
0.89	130.80	54.32	48.59	4	113 118 123 130 FZ	12.15
0.88	124.26	52.36	46.16	4	112 117 122 131 FZ	11.54
0.87	117.12	50.17	43.51	7	11 15 20 68 69 71 72 FZ	6.22
0.85	109.20	47.71	40.57	6	31 36 51 52 74 76 FZ	6.76
0.83	100.23	44.86	37.23	4	30 35 50 53 FZ	9.31
0.80	89.75	41.44	33.34	4	29 34 49 54 FZ	8.33
0.77	76.81	37.05	28.53	4	28 33 48 55 FZ	7.13
0.71	58.86	30.60	21.87	4	27 32 47 56 FZ	5.47

n_0 (Hz)	$3.6n_0(H/V)_H$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
3 ^{ra} itera							
0.88811	1.74	0.07	0.019	0.138	0.73	4.11	0.29

Fg	Pz (kg/m ²)	Fa (kg)	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
1.16	331.32	35.32	41.03	2	953 954 FZ	20.51
1.16	329.90	35.21	40.85	2	998 1000 FZ	20.43
1.16	328.47	35.10	40.67	2	997 999 FZ	20.34
1.16	327.04	34.99	40.50	2	950 951 FZ	20.25
1.16	325.59	34.87	40.32	2	994 996 FZ	20.16
1.15	324.13	34.76	40.14	2	993 995 FZ	20.07
1.15	322.66	34.65	39.95	2	947 948 FZ	19.98
1.15	321.18	34.53	39.77	2	990 992 FZ	19.88
1.15	319.69	34.42	39.59	2	989 991 FZ	19.79
1.15	318.18	34.30	39.40	2	944 945 FZ	19.70
1.15	316.67	34.19	39.21	2	977 979 FZ	19.61
1.15	315.14	34.07	39.02	2	976 978 FZ	19.51
1.14	313.61	33.95	38.83	3	752 754 826 FZ	12.94
1.14	312.06	33.83	38.64	4	773 807 864 867 FZ	9.66
1.14	310.49	33.71	38.45	4	774 808 865 866 FZ	9.61
1.14	308.92	33.58	38.25	3	775 809 828 FZ	12.75
1.14	307.33	33.46	38.06	4	776 810 868 871 FZ	9.51
1.14	305.73	33.33	37.86	4	777 811 869 870 FZ	9.46
1.13	304.11	33.21	37.66	3	778 812 860 FZ	12.55
1.13	302.48	33.08	37.46	4	779 813 872 875 FZ	9.36
1.13	300.84	32.95	37.25	4	780 814 873 874 FZ	9.31
1.13	299.18	32.82	37.05	3	781 815 861 FZ	12.35
1.13	297.51	32.69	36.84	4	782 816 877 878 FZ	9.21
1.13	295.82	32.55	36.63	4	783 817 876 879 FZ	9.16
1.12	294.12	32.42	36.42	3	784 818 862 FZ	12.14
1.12	292.40	32.28	36.21	4	785 819 880 883 FZ	9.05
1.12	290.67	32.14	35.99	4	786 820 881 882 FZ	9.00
1.12	288.91	32.00	35.77	3	787 821 863 FZ	11.92
1.12	287.14	31.86	35.56	4	788 822 884 887 FZ	8.89
1.11	285.36	31.72	35.33	4	789 823 885 886 FZ	8.83
1.11	283.55	63.15	70.22	3	512 514 630 FZ	23.41
1.11	281.73	62.86	69.77	4	544 601 633 634 FZ	17.44
1.11	279.88	62.56	69.31	4	545 600 632 635 FZ	17.33
1.11	278.02	62.26	68.85	3	546 599 627 FZ	22.95
1.10	276.14	61.96	68.39	4	547 598 637 638 FZ	17.10
1.10	274.23	61.65	67.91	4	548 597 636 639 FZ	16.98
1.10	272.31	61.34	67.44	3	549 596 624 FZ	22.48
1.10	270.36	61.03	66.95	4	550 595 641 642 FZ	16.74
1.09	268.39	60.71	66.47	4	551 594 640 643 FZ	16.62
1.09	266.40	60.38	65.97	3	552 593 621 FZ	21.99
1.09	264.38	60.05	65.47	4	553 592 644 647 FZ	16.37
1.09	262.33	59.72	64.97	4	554 591 645 646 FZ	16.24
1.09	260.27	59.38	64.45	3	555 590 618 FZ	21.48

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
1.08	258.17	59.04	63.94	4	556 589 649 650 FZ	15.98
1.08	256.05	58.69	63.41	4	557 588 648 651 FZ	15.85
1.08	253.89	58.33	62.88	3	558 587 615 FZ	20.96
1.08	251.71	57.97	62.34	4	559 586 653 654 FZ	15.58
1.07	249.50	57.60	61.79	4	560 585 652 655 FZ	15.45
1.07	247.26	57.23	61.23	3	561 584 612 FZ	20.41
1.07	244.98	56.85	60.67	4	562 583 656 659 FZ	15.17
1.06	242.67	56.47	60.10	4	563 582 657 658 FZ	15.02
1.06	240.32	56.07	59.52	3	564 581 609 FZ	19.84
1.06	237.93	55.67	58.92	4	565 580 661 662 FZ	14.73
1.06	235.51	55.26	58.32	4	566 579 660 663 FZ	14.58
1.05	233.05	54.85	57.71	3	567 578 606 FZ	19.24
1.05	230.54	54.42	57.09	4	568 577 665 666 FZ	14.27
1.05	227.99	53.99	56.46	4	569 576 664 667 FZ	14.12
1.04	225.39	53.55	55.82	3	570 575 603 FZ	18.61
1.04	222.74	53.09	55.16	4	571 574 669 670 FZ	13.79
1.04	220.04	52.63	54.49	4	572 573 668 671 FZ	13.62
1.03	217.29	78.23	80.72	7	4 6 372 443 444 446 447 FZ	11.53
1.03	214.48	77.51	79.67	6	383 388 393 394 449 451 FZ	13.28
1.02	211.61	76.76	78.61	4	382 387 392 395 FZ	19.65
1.02	208.67	75.99	77.52	4	381 386 391 396 FZ	19.38
1.02	205.67	75.20	76.40	4	380 385 390 397 FZ	19.10
1.01	202.60	74.39	75.26	4	379 384 389 398 FZ	18.81
1.01	199.44	73.56	74.09	7	14 18 302 360 361 363 364 FZ	10.58
1.00	196.21	72.70	72.89	6	295 300 318 319 367 369 FZ	12.15
1.00	192.89	71.82	71.65	4	294 299 317 320 FZ	17.91
0.99	189.48	70.90	70.39	4	293 298 316 321 FZ	17.60
0.99	185.96	69.95	69.08	4	292 297 315 322 FZ	17.27
0.98	182.33	68.97	67.73	4	291 296 314 323 FZ	16.93
0.98	178.58	67.95	66.34	7	13 17 205 277 278 280 281 FZ	9.48
0.97	174.71	66.88	64.90	6	216 221 265 274 283 285 FZ	10.82
0.96	170.69	65.77	63.41	4	215 220 266 273 FZ	15.85
0.96	166.51	64.61	61.85	4	214 219 267 272 FZ	15.46
0.95	162.16	63.39	60.24	4	213 218 268 271 FZ	15.06
0.94	157.61	62.11	58.55	4	212 217 269 270 FZ	14.64
0.93	152.84	60.75	56.77	7	12 16 111 144 145 147 148 FZ	8.11
0.93	147.81	59.31	54.91	6	116 121 126 127 150 152 FZ	9.15
0.92	142.50	57.77	52.94	4	115 120 125 128 FZ	13.23
0.91	136.85	56.12	50.84	4	114 119 124 129 FZ	12.71
0.89	130.80	54.32	48.59	4	113 118 123 130 FZ	12.15
0.88	124.26	52.36	46.16	4	112 117 122 131 FZ	11.54
0.87	117.12	50.17	43.51	7	11 15 20 68 69 71 72 FZ	6.22
0.85	109.20	47.71	40.57	6	31 36 51 52 74 76 FZ	6.76
0.83	100.23	44.86	37.23	4	30 35 50 53 FZ	9.31
0.80	89.75	41.44	33.34	4	29 34 49 54 FZ	8.33
0.77	76.81	37.05	28.53	4	28 33 48 55 FZ	7.13
0.71	58.86	30.60	21.87	4	27 32 47 56 FZ	5.47

Fuerzas sobre las Parábolas en dirección Z-Z.

Antenas Parabólicas	Altura (ft).	Grados (γ)	Diámetro (φ) (ft).	Wind Force Coefficients for Typical Paraboloid Without Radome			Ap (ft²).
				C _A	C _S	C _M	
1	295.2756	0	9.8425	0.00397	0.00000	0.000000	76.0856
2	295.2756	0	9.8425	0.00397	0.00000	0.000000	76.0856
3	295.2756	180	9.8425	0.00270	0.00000	0.000000	76.0856
4	196.8504	0	9.8425	0.00397	0.00000	0.000000	76.0856
5	196.8504	0	9.8425	0.00397	0.00000	0.000000	76.0856
6	196.8504	180	9.8425	0.00270	0.00000	0.000000	76.0856
7	98.4252	0	9.8425	0.00397	0.00000	0.000000	76.0856
8	98.4252	0	9.8425	0.00397	0.00000	0.000000	76.0856
9	98.4252	180	9.8425	0.00270	0.00000	0.000000	76.0856
PLATAF-1	98.4252	0	8.0410	0.00397	0.00000	0.000000	50.7809
PLATAF-2	196.8504	0	6.7274	0.00397	0.00000	0.000000	35.5457

K _Z (ft)	G _H (ft).	V _R (mi/h)	F _A (lb).	F _S (lb).	M (lb-ft).	FX (lb).
1.87033	1.08872	98.18	5928.51	0.00	0.00	0.00
1.87033	1.08872	98.18	5928.51	0.00	0.00	0.00
1.87033	1.08872	98.18	4031.99	0.00	0.00	0.00
1.66574	1.08872	98.18	5280.00	0.00	0.00	0.00
1.66574	1.08872	98.18	5280.00	0.00	0.00	0.00
1.66574	1.08872	98.18	3590.93	0.00	0.00	0.00
1.36646	1.08872	98.18	4331.37	0.00	0.00	0.00
1.36646	1.08872	98.18	4331.37	0.00	0.00	0.00
1.36646	1.08872	98.18	2945.77	0.00	0.00	0.00
1.36646	1.08872	98.18	2890.83	0.00	0.00	0.00
1.66574	1.08872	98.18	2466.71	0.00	0.00	0.00

FZ (lb).	Nodos										FZ (kg).	FZ / # nodos (kg).																
5928.51											2689.17																	
5928.51											2689.17																	
4031.99											1828.91																	
5280.00											2395.01																	
5280.00											2395.01																	
3590.93											1628.85																	
4331.37											1964.71																	
4331.37											1964.71																	
2945.77											1336.20																	
2890.83	496	511	510	509	508	507	495	460	493	492	491	490	489	459	FZ	1311.28	93.66											
2466.71																848	859	858	857	850	830	847	846	845	832	FZ	1118.90	111.89

El valor obtenido de frecuencia natural en dirección Z-Z, con las iteraciones conjuntas de la estructura, líneas y parábolas es:

n_0 (Hz)

Definitivo

0.88811

Por lo tanto las fuerzas obtenidas de la tercera iteración, son las fuerzas de diseño.

Presiones y Fuerzas sobre la Estructura en dirección X-X.

Sección	Altura (m).	Frz	Fa	V _D (km/h).	q _z (kg/m ²).	Ae (m ²).	AT (m ²).	Solidez ϕ	bV _D	Ca	g
1	90	1.31	1.31	228.07	237.67	4.531	10.393	0.44	12.87	1.54	1.38
2	89	1.31	1.31	227.72	236.94	4.531	10.393	0.44	12.85	1.54	1.38
3	88	1.31	1.31	227.36	236.20	4.531	10.393	0.44	12.83	1.54	1.38
4	87	1.31	1.31	227.01	235.46	4.531	10.393	0.44	12.81	1.54	1.38
5	86	1.30	1.30	226.64	234.71	4.531	10.393	0.44	12.79	1.54	1.38
6	85	1.30	1.30	226.28	233.95	4.531	10.393	0.44	12.77	1.54	1.38
7	84	1.30	1.30	225.91	233.19	4.531	10.393	0.44	12.75	1.54	1.38
8	83	1.30	1.30	225.54	232.42	4.531	10.393	0.44	12.73	1.54	1.38
9	82	1.30	1.30	225.16	231.64	4.531	10.393	0.44	12.71	1.54	1.39
10	81	1.29	1.29	224.78	230.86	4.531	10.393	0.44	12.68	1.54	1.39
11	80	1.29	1.29	224.39	230.07	4.531	10.393	0.44	12.66	1.54	1.39
12	79	1.29	1.29	224.00	229.27	4.531	10.393	0.44	12.64	1.54	1.39
13	78	1.29	1.29	223.61	228.46	5.258	12.11	0.43	15.77	1.51	1.39
14	77	1.28	1.28	223.21	227.65	5.258	12.11	0.43	15.74	1.51	1.39
15	76	1.28	1.28	222.81	226.83	5.258	12.11	0.43	15.72	1.51	1.39
16	75	1.28	1.28	222.40	226.00	5.258	12.11	0.43	15.69	1.51	1.39
17	74	1.28	1.28	221.99	225.17	5.258	12.11	0.43	15.66	1.51	1.39
18	73	1.28	1.28	221.58	224.33	5.258	12.11	0.43	15.63	1.51	1.39
19	72	1.27	1.27	221.15	223.47	5.508	15.568	0.35	15.60	1.60	1.39
20	71	1.27	1.27	220.73	222.61	5.508	15.568	0.35	15.57	1.60	1.40
21	70	1.27	1.27	220.30	221.74	5.508	15.568	0.35	15.54	1.60	1.40
22	69	1.27	1.27	219.86	220.86	5.508	15.568	0.35	15.51	1.60	1.40
23	68	1.26	1.26	219.42	219.98	5.508	15.568	0.35	15.48	1.60	1.40
24	67	1.26	1.26	218.97	219.08	5.508	15.568	0.35	15.45	1.60	1.40
25	66	1.26	1.26	218.51	218.17	5.774	19.027	0.30	15.41	1.66	1.40
26	65	1.25	1.25	218.06	217.25	5.774	19.027	0.30	15.38	1.66	1.40
27	64	1.25	1.25	217.59	216.32	5.774	19.027	0.30	15.35	1.66	1.40
28	63	1.25	1.25	217.12	215.39	5.774	19.027	0.30	15.31	1.66	1.40
29	62	1.25	1.25	216.64	214.44	5.774	19.027	0.30	15.28	1.66	1.41
30	61	1.24	1.24	216.15	213.48	5.774	19.027	0.30	15.25	1.66	1.41
31	60	1.24	1.24	215.66	212.51	7.2622	22.318	0.33	21.30	1.57	1.41
32	59	1.24	1.24	215.16	211.52	7.2622	22.318	0.33	21.25	1.57	1.41
33	58	1.24	1.24	214.65	210.53	7.2622	22.318	0.33	21.20	1.57	1.41
34	57	1.23	1.23	214.14	209.52	7.2622	22.318	0.33	21.15	1.57	1.41
35	56	1.23	1.23	213.62	208.50	7.2622	22.318	0.33	21.09	1.57	1.41
36	55	1.23	1.23	213.09	207.46	7.2622	22.318	0.33	21.04	1.57	1.42
37	54	1.22	1.22	212.55	206.42	7.5202	25.433	0.30	20.99	1.62	1.42
38	53	1.22	1.22	212.00	205.35	7.5202	25.433	0.30	20.93	1.62	1.42
39	52	1.22	1.22	211.44	204.28	7.5202	25.433	0.30	20.88	1.62	1.42
40	51	1.21	1.21	210.88	203.18	7.5202	25.433	0.30	20.82	1.62	1.42
41	50	1.21	1.21	210.30	202.08	7.5202	25.433	0.30	20.77	1.62	1.42
42	49	1.21	1.21	209.72	200.95	7.5202	25.433	0.30	20.71	1.62	1.42
43	48	1.20	1.20	209.12	199.81	7.7842	28.548	0.27	20.65	1.69	1.43
44	47	1.20	1.20	208.51	198.66	7.7842	28.548	0.27	20.59	1.69	1.43
45	46	1.20	1.20	207.90	197.48	7.7842	28.548	0.27	20.53	1.69	1.43

Sección	Altura (m).	F _r	F _a	V _D (km/h).	q _z (kg/m ²).	A _e (m ²).	AT (m ²).	Solidez ϕ	bV _D	Ca	g
46	45	1.19	1.19	207.27	196.29	7.7842	28.548	0.27	20.47	1.69	1.43
47	44	1.19	1.19	206.62	195.07	7.7842	28.548	0.27	20.40	1.69	1.43
48	43	1.19	1.19	205.97	193.84	7.7842	28.548	0.27	20.34	1.69	1.43
49	42	1.18	1.18	205.30	192.58	8.0562	31.664	0.25	20.27	1.76	1.44
50	41	1.18	1.18	204.62	191.31	8.0562	31.664	0.25	20.21	1.76	1.44
51	40	1.17	1.17	203.92	190.01	8.0562	31.664	0.25	20.14	1.76	1.44
52	39	1.17	1.17	203.21	188.68	8.0562	31.664	0.25	20.07	1.76	1.44
53	38	1.17	1.17	202.49	187.34	8.0562	31.664	0.25	20.00	1.76	1.44
54	37	1.16	1.16	201.74	185.96	8.0562	31.664	0.25	19.92	1.76	1.45
55	36	1.16	1.16	200.98	184.56	8.3322	34.779	0.24	19.85	1.81	1.45
56	35	1.15	1.15	200.20	183.13	8.3322	34.779	0.24	19.77	1.81	1.45
57	34	1.15	1.15	199.40	181.67	8.3322	34.779	0.24	19.69	1.81	1.45
58	33	1.14	1.14	198.58	180.18	8.3322	34.779	0.24	19.61	1.81	1.45
59	32	1.14	1.14	197.74	178.66	8.3322	34.779	0.24	19.53	1.81	1.46
60	31	1.13	1.13	196.88	177.10	8.3322	34.779	0.24	19.44	1.81	1.46
61	30	1.13	1.13	195.99	175.50	10.9984	37.894	0.29	24.88	1.72	1.46
62	29	1.12	1.12	195.07	173.87	10.9984	37.894	0.29	24.77	1.72	1.47
63	28	1.12	1.12	194.13	172.19	10.9984	37.894	0.29	24.65	1.72	1.47
64	27	1.11	1.11	193.16	170.47	10.9984	37.894	0.29	24.52	1.72	1.47
65	26	1.11	1.11	192.15	168.71	10.9984	37.894	0.29	24.40	1.72	1.47
66	25	1.10	1.10	191.12	166.89	10.9984	37.894	0.29	24.27	1.72	1.48
67	24	1.09	1.09	190.04	165.02	11.3054	41.01	0.28	24.13	1.75	1.48
68	23	1.09	1.09	188.93	163.09	11.3054	41.01	0.28	23.99	1.75	1.48
69	22	1.08	1.08	187.78	161.11	11.3054	41.01	0.28	23.84	1.75	1.49
70	21	1.07	1.07	186.57	159.05	11.3054	41.01	0.28	23.69	1.75	1.49
71	20	1.07	1.07	185.32	156.92	11.3054	41.01	0.28	23.53	1.75	1.49
72	19	1.06	1.06	184.01	154.72	11.3054	41.01	0.28	23.36	1.75	1.50
73	18	1.05	1.05	182.65	152.42	11.6084	44.125	0.26	23.19	1.82	1.50
74	17	1.04	1.04	181.21	150.04	11.6084	44.125	0.26	23.01	1.82	1.51
75	16	1.03	1.03	179.70	147.55	11.6084	44.125	0.26	22.82	1.82	1.51
76	15	1.02	1.02	178.11	144.94	11.6084	44.125	0.26	22.61	1.82	1.52
77	14	1.02	1.02	176.42	142.21	11.6084	44.125	0.26	22.40	1.82	1.52
78	13	1.00	1.00	174.63	139.33	11.6084	44.125	0.26	22.17	1.82	1.53
79	12	0.99	0.99	172.71	136.29	11.9124	47.24	0.25	21.93	1.86	1.54
80	11	0.98	0.98	170.65	133.05	11.9124	47.24	0.25	21.67	1.86	1.54
81	10	0.97	0.97	168.42	129.60	11.9124	47.24	0.25	21.38	1.86	1.55
82	9	0.96	0.96	165.99	125.88	11.9124	47.24	0.25	21.07	1.86	1.56
83	8	0.94	0.94	163.31	121.86	11.9124	47.24	0.25	20.73	1.86	1.57
84	7	0.92	0.92	160.33	117.45	11.9124	47.24	0.25	20.36	1.86	1.58
85	6	0.90	0.90	156.95	112.56	12.2264	50.359	0.24	19.93	1.89	1.60
86	5	0.88	0.88	153.05	107.03	12.2264	50.359	0.24	19.43	1.89	1.61
87	4	0.85	0.85	148.41	100.64	12.2264	50.359	0.24	18.84	1.89	1.63
88	3	0.82	0.82	142.64	92.96	12.2264	50.359	0.24	18.11	1.89	1.66
89	2	0.78	0.78	134.87	83.12	12.2264	50.359	0.24	17.12	1.89	1.69
90	1	0.71	0.71	122.57	68.64	12.2264	50.359	0.24	15.56	1.89	1.76

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
Valor propuesto							
2.00	3.92	0.019	0.04	0.075	0.96	4.18	0.19
Fg	Pz	(kg/m ²). Fa	(kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.9484	348.21	277.26	262.96	2	953 955 FX	131.48	
0.9473	346.72	276.41	261.83	2	998 1015 FX	130.92	
0.9461	345.22	275.55	260.70	2	997 1014 FX	130.35	
0.9450	343.71	274.68	259.56	2	950 952 FX	129.78	
0.9438	342.19	273.80	258.41	2	994 1013 FX	129.20	
0.9426	340.65	272.92	257.25	2	993 1012 FX	128.63	
0.9414	339.11	272.03	256.08	2	947 949 FX	128.04	
0.9402	337.55	271.13	254.91	2	990 1011 FX	127.45	
0.9389	335.99	270.23	253.73	2	989 1010 FX	126.86	
0.9377	334.41	269.31	252.53	2	944 946 FX	126.27	
0.9364	332.82	268.39	251.33	2	975 979 FX	125.67	
0.9352	331.21	267.46	250.12	2	974 978 FX	125.06	
0.9339	322.64	302.76	282.74	3	752 755 825 FX	94.25	
0.9326	321.04	301.68	281.34	4	790 807 892 895 FX	70.34	
0.9313	319.44	300.60	279.93	4	791 808 893 894 FX	69.98	
0.9299	317.82	299.50	278.51	3	792 809 829 FX	92.84	
0.9286	316.18	298.39	277.08	4	793 810 896 899 FX	69.27	
0.9272	314.54	297.27	275.64	4	794 811 897 898 FX	68.91	
0.9258	330.00	327.21	302.94	3	795 812 888 FX	100.98	
0.9244	328.24	325.95	301.32	4	796 813 902 905 FX	75.33	
0.9230	326.45	324.68	299.69	4	797 814 903 904 FX	74.92	
0.9216	324.66	323.39	298.03	3	798 815 889 FX	99.34	
0.9201	322.84	322.09	296.37	4	799 816 906 909 FX	74.09	
0.9187	321.01	320.78	294.69	4	800 817 907 908 FX	73.67	
0.9172	332.97	349.36	320.43	3	801 818 890 FX	106.81	
0.9157	331.02	347.89	318.55	4	802 819 910 913 FX	79.64	
0.9141	329.06	346.41	316.66	4	803 820 911 912 FX	79.17	
0.9126	327.07	344.90	314.75	3	804 821 891 FX	104.92	
0.9110	325.07	343.38	312.82	4	805 822 900 915 FX	78.21	
0.9094	323.05	341.85	310.88	4	806 823 901 914 FX	77.72	
0.9078	302.37	403.15	365.97	3	512 513 631 FX	121.99	
0.9061	300.42	401.28	363.62	4	543 601 672 675 FX	90.91	
0.9045	298.46	399.40	361.24	4	542 600 673 674 FX	90.31	
0.9028	296.47	397.48	358.84	3	541 599 628 FX	119.61	
0.9010	294.46	395.55	356.41	4	540 598 676 679 FX	89.10	
0.8993	292.43	393.58	353.95	4	539 597 677 678 FX	88.49	
0.8975	299.38	418.08	375.23	3	538 596 625 FX	125.08	
0.8957	297.24	415.93	372.55	4	537 595 680 683 FX	93.14	
0.8939	295.07	413.75	369.84	4	536 594 681 682 FX	92.46	
0.8920	292.88	411.54	367.09	3	535 593 622 FX	122.36	
0.8901	290.66	409.29	364.31	4	534 592 684 687 FX	91.08	
0.8881	288.42	407.02	361.49	4	533 591 685 686 FX	90.37	
0.8862	299.95	439.14	389.15	3	532 590 619 FX	129.72	

Fg	Pz	(kg/m ²). Fa	(kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.8842	297.54	436.59	386.02	4	531 589 688 691 FX	96.50	
0.8821	295.09	434.01	382.84	4	530 588 689 690 FX	95.71	
0.8800	292.61	431.38	379.62	3	529 587 616 FX	126.54	
0.8779	290.10	428.72	376.36	4	528 586 692 695 FX	94.09	
0.8757	287.55	426.00	373.05	4	527 585 693 694 FX	93.26	
0.8735	295.73	454.59	397.07	3	526 584 613 FX	132.36	
0.8712	293.00	451.57	393.42	4	525 583 696 699 FX	98.35	
0.8689	290.24	448.51	389.70	4	524 582 697 698 FX	97.43	
0.8665	287.43	445.38	385.93	3	523 581 610 FX	128.64	
0.8641	284.58	442.20	382.10	4	522 580 701 702 FX	95.53	
0.8616	281.68	438.96	378.21	4	521 579 700 703 FX	94.55	
0.8591	286.28	462.78	397.55	3	520 578 607 FX	132.52	
0.8565	283.20	459.19	393.27	4	519 577 704 707 FX	98.32	
0.8538	280.06	455.53	388.92	4	518 576 705 706 FX	97.23	
0.8510	276.87	451.79	384.49	3	517 575 604 FX	128.16	
0.8482	273.62	447.97	379.97	4	516 574 709 710 FX	94.99	
0.8453	270.30	444.07	375.37	4	515 573 708 711 FX	93.84	
0.8423	254.27	553.34	466.09	7	4 5 373 421 422 424 425 FX	66.58	
0.8392	250.98	548.19	460.06	6	378 383 403 404 427 429 FX	76.68	
0.8361	247.62	542.90	453.90	4	377 382 402 405 FX	113.47	
0.8328	244.18	537.48	447.60	4	376 381 401 406 FX	111.90	
0.8294	240.67	531.91	441.16	4	375 380 400 407 FX	110.29	
0.8259	250.97	557.02	460.04	4	374 379 399 408 FX	115.01	
0.8223	237.84	545.01	448.14	7	10 18 303 339 340 342 343 FX	64.02	
0.8185	233.98	538.65	440.87	6	290 300 328 329 345 347 FX	73.48	
0.8146	230.02	532.08	433.41	4	289 299 327 330 FX	108.35	
0.8105	225.95	525.29	425.74	4	288 298 326 331 FX	106.43	
0.8062	221.75	518.27	417.84	4	287 297 325 332 FX	104.46	
0.8018	217.43	510.98	409.69	4	286 296 324 333 FX	102.42	
0.7971	221.22	536.96	428.01	7	9 17 206 234 235 237 238 FX	61.14	
0.7922	216.42	528.55	418.72	6	211 216 226 227 240 242 FX	69.79	
0.7870	211.44	519.78	409.08	4	210 215 225 228 FX	102.27	
0.7816	206.26	510.61	399.07	4	209 214 224 229 FX	99.77	
0.7758	200.87	500.97	388.63	4	208 213 223 230 FX	97.16	
0.7696	195.24	490.83	377.73	4	207 212 222 231 FX	94.43	
0.7629	192.99	502.20	383.16	7	8 16 154 172 173 175 176 FX	54.74	
0.7558	186.65	490.29	370.57	6	121 159 160 169 178 180 FX	61.76	
0.7481	179.94	477.56	357.25	4	120 158 161 168 FX	89.31	
0.7396	172.80	463.87	343.08	4	119 157 162 167 FX	85.77	
0.7303	165.16	449.03	327.91	4	118 156 163 166 FX	81.98	
0.7198	156.91	432.79	311.52	4	117 155 164 165 FX	77.88	
0.7079	150.76	433.95	307.20	7	7 15 21 88 89 91 92 FX	43.89	
0.6941	140.56	412.65	286.43	6	26 36 57 66 94 96 FX	47.74	
0.6776	129.02	388.01	262.91	4	25 35 58 65 FX	65.73	
0.6569	115.53	358.39	235.41	4	24 34 59 64 FX	58.85	
0.6287	98.87	320.45	201.47	4	23 33 60 63 FX	50.37	
0.5834	75.76	264.65	154.39	4	22 32 61 62 FX	38.60	

ESTA TESIS NO SALE
DE LA BIBLIOTECA

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
1 ^{ra} itera							
1.33196	2.61	0.032	0.029	0.091	0.83	4.15	0.21

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.99	364.76	277.26	275.45	2	953 955 FX	137.73
0.99	363.19	276.41	274.27	2	998 1015 FX	137.14
0.99	361.62	275.55	273.09	2	997 1014 FX	136.54
0.99	360.04	274.68	271.89	2	950 952 FX	135.94
0.99	358.44	273.80	270.68	2	994 1013 FX	135.34
0.99	356.84	272.92	269.47	2	993 1012 FX	134.74
0.99	355.22	272.03	268.25	2	947 949 FX	134.13
0.98	353.59	271.13	267.02	2	990 1011 FX	133.51
0.98	351.95	270.23	265.78	2	989 1010 FX	132.89
0.98	350.29	269.31	264.53	2	944 946 FX	132.26
0.98	348.63	268.39	263.27	2	975 979 FX	131.64
0.98	346.95	267.46	262.00	2	974 978 FX	131.00
0.98	337.97	302.76	296.17	3	752 755 825 FX	98.72
0.98	336.30	301.68	294.71	4	790 807 892 895 FX	73.68
0.98	334.61	300.60	293.23	4	791 808 893 894 FX	73.31
0.97	332.92	299.50	291.75	3	792 809 829 FX	97.25
0.97	331.20	298.39	290.25	4	793 810 896 899 FX	72.56
0.97	329.48	297.27	288.73	4	794 811 897 898 FX	72.18
0.97	345.68	327.21	317.34	3	795 812 888 FX	105.78
0.97	343.83	325.95	315.64	4	796 813 902 905 FX	78.91
0.97	341.96	324.68	313.92	4	797 814 903 904 FX	78.48
0.97	340.08	323.39	312.19	3	798 815 889 FX	104.06
0.96	338.18	322.09	310.45	4	799 816 906 909 FX	77.61
0.96	336.26	320.78	308.69	4	800 817 907 908 FX	77.17
0.96	348.79	349.36	335.65	3	801 818 890 FX	111.88
0.96	346.75	347.89	333.69	4	802 819 910 913 FX	83.42
0.96	344.69	346.41	331.71	4	803 820 911 912 FX	82.93
0.96	342.61	344.90	329.71	3	804 821 891 FX	109.90
0.95	340.51	343.38	327.69	4	805 822 900 915 FX	81.92
0.95	338.39	341.85	325.65	4	806 823 901 914 FX	81.41
0.95	316.73	403.15	383.36	3	512 513 631 FX	127.79
0.95	314.69	401.28	380.90	4	543 601 672 675 FX	95.22
0.95	312.64	399.40	378.40	4	542 600 673 674 FX	94.60
0.95	310.55	397.48	375.88	3	541 599 628 FX	125.29
0.94	308.45	395.55	373.34	4	540 598 676 679 FX	93.33
0.94	306.32	393.58	370.76	4	539 597 677 678 FX	92.69
0.94	313.60	418.08	393.06	3	538 596 625 FX	131.02
0.94	311.36	415.93	390.25	4	537 595 680 683 FX	97.56
0.94	309.09	413.75	387.41	4	536 594 681 682 FX	96.85
0.93	306.80	411.54	384.53	3	535 593 622 FX	128.18
0.93	304.47	409.29	381.62	4	534 592 684 687 FX	95.40
0.93	302.12	407.02	378.67	4	533 591 685 686 FX	94.67
0.93	314.20	439.14	407.64	3	532 590 619 FX	135.88

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.93	311.67	436.59	404.36	4	531 589 688 691 FX	101.09
0.92	309.11	434.01	401.03	4	530 588 689 690 FX	100.26
0.92	306.51	431.38	397.66	3	529 587 616 FX	132.55
0.92	303.88	428.72	394.24	4	528 586 692 695 FX	98.56
0.92	301.21	426.00	390.78	4	527 585 693 694 FX	97.69
0.91	309.78	454.59	415.94	3	526 584 613 FX	138.65
0.91	306.92	451.57	412.11	4	525 583 696 699 FX	103.03
0.91	304.03	448.51	408.22	4	524 582 697 698 FX	102.05
0.91	301.09	445.38	404.27	3	523 581 610 FX	134.76
0.91	298.10	442.20	400.25	4	522 580 701 702 FX	100.06
0.90	295.06	438.96	396.18	4	521 579 700 703 FX	99.04
0.90	299.88	462.78	416.44	3	520 578 607 FX	138.81
0.90	296.65	459.19	411.96	4	519 577 704 707 FX	102.99
0.89	293.37	455.53	407.40	4	518 576 705 706 FX	101.85
0.89	290.02	451.79	402.75	3	517 575 604 FX	134.25
0.89	286.62	447.97	398.02	4	516 574 709 710 FX	99.51
0.89	283.14	444.07	393.20	4	515 573 708 711 FX	98.30
0.88	266.35	553.34	488.23	7	4 5 373 421 422 424 425 FX	69.75
0.88	262.90	548.19	481.91	6	378 383 403 404 427 429 FX	80.32
0.88	259.38	542.90	475.46	4	377 382 402 405 FX	118.87
0.87	255.78	537.48	468.87	4	376 381 401 406 FX	117.22
0.87	252.10	531.91	462.12	4	375 380 400 407 FX	115.53
0.87	262.89	557.02	481.89	4	374 379 399 408 FX	120.47
0.86	249.14	545.01	469.43	7	10 18 303 339 340 342 343 FX	67.06
0.86	245.10	538.65	461.82	6	290 300 328 329 345 347 FX	76.97
0.85	240.95	532.08	454.00	4	289 299 327 330 FX	113.50
0.85	236.68	525.29	445.97	4	288 298 326 331 FX	111.49
0.84	232.29	518.27	437.69	4	287 297 325 332 FX	109.42
0.84	227.76	510.98	429.15	4	286 296 324 333 FX	107.29
0.83	231.73	536.96	448.34	7	9 17 206 234 235 237 238 FX	64.05
0.83	226.70	528.55	438.61	6	211 216 226 227 240 242 FX	73.10
0.82	221.49	519.78	428.52	4	210 215 225 228 FX	107.13
0.82	216.06	510.61	418.03	4	209 214 224 229 FX	104.51
0.81	210.42	500.97	407.10	4	208 213 223 230 FX	101.77
0.81	204.51	490.83	395.68	4	207 212 222 231 FX	98.92
0.80	202.16	502.20	401.36	7	8 16 154 172 173 175 176 FX	57.34
0.79	195.51	490.29	388.17	6	121 159 160 169 178 180 FX	64.70
0.78	188.49	477.56	374.22	4	120 158 161 168 FX	93.56
0.77	181.01	463.87	359.38	4	119 157 162 167 FX	89.85
0.76	173.01	449.03	343.49	4	118 156 163 166 FX	85.87
0.75	164.36	432.79	326.32	4	117 155 164 165 FX	81.58
0.74	157.92	433.95	321.80	7	7 15 21 88 89 91 92 FX	45.97
0.73	147.24	412.65	300.04	6	26 36 57 66 94 96 FX	50.01
0.71	135.15	388.01	275.40	4	25 35 58 65 FX	68.85
0.69	121.01	358.39	246.60	4	24 34 59 64 FX	61.65
0.66	103.57	320.45	211.04	4	23 33 60 63 FX	52.76
0.61	79.36	264.65	161.72	4	22 32 61 62 FX	40.43

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
2 ^{aa} itera.							
1.31116	2.57	0.032	0.029	0.091	0.81	4.15	0.21
Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos	
0.99	364.8	277.26	275.5	2	953 955 FX	137.73	
0.99	363.2	276.41	274.3	2	998 1015 FX	137.14	
0.99	361.6	275.55	273.1	2	997 1014 FX	136.54	
0.99	360.0	274.68	271.9	2	950 952 FX	135.94	
0.99	358.4	273.80	270.7	2	994 1013 FX	135.34	
0.99	356.8	272.92	269.5	2	993 1012 FX	134.74	
0.99	355.2	272.03	268.3	2	947 949 FX	134.13	
0.98	353.6	271.13	267.0	2	990 1011 FX	133.51	
0.98	351.9	270.23	265.8	2	989 1010 FX	132.89	
0.98	350.3	269.31	264.5	2	944 946 FX	132.26	
0.98	348.6	268.39	263.3	2	975 979 FX	131.64	
0.98	346.9	267.46	262.0	2	974 978 FX	131.00	
0.98	338.0	302.76	296.2	3	752 755 825 FX	98.72	
0.98	336.3	301.68	294.7	4	790 807 892 895 FX	73.68	
0.98	334.6	300.60	293.2	4	791 808 893 894 FX	73.31	
0.97	332.9	299.50	291.7	3	792 809 829 FX	97.25	
0.97	331.2	298.39	290.2	4	793 810 896 899 FX	72.56	
0.97	329.5	297.27	288.7	4	794 811 897 898 FX	72.18	
0.97	345.7	327.21	317.3	3	795 812 888 FX	105.78	
0.97	343.8	325.95	315.6	4	796 813 902 905 FX	78.91	
0.97	342.0	324.68	313.9	4	797 814 903 904 FX	78.48	
0.97	340.1	323.39	312.2	3	798 815 889 FX	104.06	
0.96	338.2	322.09	310.4	4	799 816 906 909 FX	77.61	
0.96	336.3	320.78	308.7	4	800 817 907 908 FX	77.17	
0.96	348.8	349.36	335.6	3	801 818 890 FX	111.88	
0.96	346.7	347.89	333.7	4	802 819 910 913 FX	83.42	
0.96	344.7	346.41	331.7	4	803 820 911 912 FX	82.93	
0.96	342.6	344.90	329.7	3	804 821 891 FX	109.90	
0.95	340.5	343.38	327.7	4	805 822 900 915 FX	81.92	
0.95	338.4	341.85	325.6	4	806 823 901 914 FX	81.41	
0.95	316.7	403.15	383.4	3	512 513 631 FX	127.79	
0.95	314.7	401.28	380.9	4	543 601 672 675 FX	95.22	
0.95	312.6	399.40	378.4	4	542 600 673 674 FX	94.60	
0.95	310.6	397.48	375.9	3	541 599 628 FX	125.29	
0.94	308.5	395.55	373.3	4	540 598 676 679 FX	93.33	
0.94	306.3	393.58	370.8	4	539 597 677 678 FX	92.69	
0.94	313.6	418.08	393.1	3	538 596 625 FX	131.02	
0.94	311.4	415.93	390.3	4	537 595 680 683 FX	97.56	
0.94	309.1	413.75	387.4	4	536 594 681 682 FX	96.85	
0.93	306.8	411.54	384.5	3	535 593 622 FX	128.18	
0.93	304.5	409.29	381.6	4	534 592 684 687 FX	95.40	
0.93	302.1	407.02	378.7	4	533 591 685 686 FX	94.67	
0.93	314.2	439.14	407.6	3	532 590 619 FX	135.88	

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.93	311.7	436.59	404.4	4	531 589 688 691 FX	101.09
0.92	309.1	434.01	401.0	4	530 588 689 690 FX	100.26
0.92	306.5	431.38	397.7	3	529 587 616 FX	132.55
0.92	303.9	428.72	394.2	4	528 586 692 695 FX	98.56
0.92	301.2	426.00	390.8	4	527 585 693 694 FX	97.69
0.91	309.8	454.59	415.9	3	526 584 613 FX	138.65
0.91	306.9	451.57	412.1	4	525 583 696 699 FX	103.03
0.91	304.0	448.51	408.2	4	524 582 697 698 FX	102.05
0.91	301.1	445.38	404.3	3	523 581 610 FX	134.76
0.91	298.1	442.20	400.3	4	522 580 701 702 FX	100.06
0.90	295.1	438.96	396.2	4	521 579 700 703 FX	99.04
0.90	299.9	462.78	416.4	3	520 578 607 FX	138.81
0.90	296.7	459.19	412.0	4	519 577 704 707 FX	102.99
0.89	293.4	455.53	407.4	4	518 576 705 706 FX	101.85
0.89	290.0	451.79	402.8	3	517 575 604 FX	134.25
0.89	286.6	447.97	398.0	4	516 574 709 710 FX	99.51
0.89	283.1	444.07	393.2	4	515 573 708 711 FX	98.30
0.88	266.3	553.34	488.2	7	4 5 373 421 422 424 425 FX	69.75
0.88	262.9	548.19	481.9	6	378 383 403 404 427 429 FX	80.32
0.88	259.4	542.90	475.5	4	377 382 402 405 FX	118.87
0.87	255.8	537.48	468.9	4	376 381 401 406 FX	117.22
0.87	252.1	531.91	462.1	4	375 380 400 407 FX	115.53
0.87	262.9	557.02	481.9	4	374 379 399 408 FX	120.47
0.86	249.1	545.01	469.4	7	10 18 303 339 340 342 343 FX	67.06
0.86	245.1	538.65	461.8	6	290 300 328 329 345 347 FX	76.97
0.85	240.9	532.08	454.0	4	289 299 327 330 FX	113.50
0.85	236.7	525.29	446.0	4	288 298 326 331 FX	111.49
0.84	232.3	518.27	437.7	4	287 297 325 332 FX	109.42
0.84	227.8	510.98	429.2	4	286 296 324 333 FX	107.29
0.83	231.7	536.96	448.3	7	9 17 206 234 235 237 238 FX	64.05
0.83	226.7	528.55	438.6	6	211 216 226 227 240 242 FX	73.10
0.82	221.5	519.78	428.5	4	210 215 225 228 FX	107.13
0.82	216.1	510.61	418.0	4	209 214 224 229 FX	104.51
0.81	210.4	500.97	407.1	4	208 213 223 230 FX	101.77
0.81	204.5	490.83	395.7	4	207 212 222 231 FX	98.92
0.80	202.2	502.20	401.4	7	8 16 154 172 173 175 176 FX	57.34
0.79	195.5	490.29	388.2	6	121 159 160 169 178 180 FX	64.70
0.78	188.5	477.56	374.2	4	120 158 161 168 FX	93.56
0.77	181.0	463.87	359.4	4	119 157 162 167 FX	89.85
0.76	173.0	449.03	343.5	4	118 156 163 166 FX	85.87
0.75	164.4	432.79	326.3	4	117 155 164 165 FX	81.58
0.74	157.9	433.95	321.8	7	7 15 21 88 89 91 92 FX	45.97
0.73	147.2	412.65	300.0	6	26 36 57 66 94 96 FX	50.01
0.71	135.1	388.01	275.4	4	25 35 58 65 FX	68.85
0.69	121.0	358.39	246.6	4	24 34 59 64 FX	61.65
0.66	103.6	320.45	211.0	4	23 33 60 63 FX	52.76
0.61	79.4	264.65	161.7	4	22 32 61 62 FX	40.43

Presiones y Fuerzas sobre las Líneas en dirección X-X.

Sección	Altura (m).	Frz	Fa	V ₀ (km/h).	q _z (kg/m ²).	Ae (m ²).	AT (m ²).	Solidez ϕ	bV ₀	Ca	g
1	90	1.31	1.31	228.07	237.67	0.0413	0.0413	1.00	12.87	1.2	1.38
2	89	1.31	1.31	227.72	236.94	0.0413	0.0413	1.00	12.85	1.2	1.38
3	88	1.31	1.31	227.36	236.20	0.0413	0.0413	1.00	12.83	1.2	1.38
4	87	1.31	1.31	227.01	235.46	0.0413	0.0413	1.00	12.81	1.2	1.38
5	86	1.30	1.30	226.64	234.71	0.0413	0.0413	1.00	12.79	1.2	1.38
6	85	1.30	1.30	226.28	233.95	0.0413	0.0413	1.00	12.77	1.2	1.38
7	84	1.30	1.30	225.91	233.19	0.0413	0.0413	1.00	12.75	1.2	1.38
8	83	1.30	1.30	225.54	232.42	0.0413	0.0413	1.00	12.73	1.2	1.38
9	82	1.30	1.30	225.16	231.64	0.0413	0.0413	1.00	12.71	1.2	1.39
10	81	1.29	1.29	224.78	230.86	0.0413	0.0413	1.00	12.68	1.2	1.39
11	80	1.29	1.29	224.39	230.07	0.0413	0.0413	1.00	12.66	1.2	1.39
12	79	1.29	1.29	224.00	229.27	0.0413	0.0413	1.00	12.64	1.2	1.39
13	78	1.29	1.29	223.61	228.46	0.0413	0.0413	1.00	12.62	1.2	1.39
14	77	1.28	1.28	223.21	227.65	0.0413	0.0413	1.00	12.60	1.2	1.39
15	76	1.28	1.28	222.81	226.83	0.0413	0.0413	1.00	12.57	1.2	1.39
16	75	1.28	1.28	222.40	226.00	0.0413	0.0413	1.00	12.55	1.2	1.39
17	74	1.28	1.28	221.99	225.17	0.0413	0.0413	1.00	12.53	1.2	1.39
18	73	1.28	1.28	221.58	224.33	0.0413	0.0413	1.00	12.50	1.2	1.39
19	72	1.27	1.27	221.15	223.47	0.0413	0.0413	1.00	12.48	1.2	1.39
20	71	1.27	1.27	220.73	222.61	0.0413	0.0413	1.00	12.46	1.2	1.40
21	70	1.27	1.27	220.30	221.74	0.0413	0.0413	1.00	12.43	1.2	1.40
22	69	1.27	1.27	219.86	220.86	0.0413	0.0413	1.00	12.41	1.2	1.40
23	68	1.26	1.26	219.42	219.98	0.0413	0.0413	1.00	12.38	1.2	1.40
24	67	1.26	1.26	218.97	219.08	0.0413	0.0413	1.00	12.36	1.2	1.40
25	66	1.26	1.26	218.51	218.17	0.0413	0.0413	1.00	12.33	1.2	1.40
26	65	1.25	1.25	218.06	217.25	0.0413	0.0413	1.00	12.30	1.2	1.40
27	64	1.25	1.25	217.59	216.32	0.0413	0.0413	1.00	12.28	1.2	1.40
28	63	1.25	1.25	217.12	215.39	0.0413	0.0413	1.00	12.25	1.2	1.40
29	62	1.25	1.25	216.64	214.44	0.0413	0.0413	1.00	12.22	1.2	1.41
30	61	1.24	1.24	216.15	213.48	0.0413	0.0413	1.00	12.20	1.2	1.41
31	60	1.24	1.24	215.66	212.51	0.0413	0.0413	1.00	15.21	1.2	1.41
32	59	1.24	1.24	215.16	211.52	0.0413	0.0413	1.00	15.18	1.2	1.41
33	58	1.24	1.24	214.65	210.53	0.0413	0.0413	1.00	15.14	1.2	1.41
34	57	1.23	1.23	214.14	209.52	0.0413	0.0413	1.00	15.10	1.2	1.41
35	56	1.23	1.23	213.62	208.50	0.0413	0.0413	1.00	15.07	1.2	1.41
36	55	1.23	1.23	213.09	207.46	0.0413	0.0413	1.00	15.03	1.2	1.42
37	54	1.22	1.22	212.55	206.42	0.0413	0.0413	1.00	14.99	1.2	1.42
38	53	1.22	1.22	212.00	205.35	0.0413	0.0413	1.00	14.95	1.2	1.42
39	52	1.22	1.22	211.44	204.28	0.0413	0.0413	1.00	14.91	1.2	1.42
40	51	1.21	1.21	210.88	203.18	0.0413	0.0413	1.00	14.87	1.2	1.42
41	50	1.21	1.21	210.30	202.08	0.0413	0.0413	1.00	14.83	1.2	1.42
42	49	1.21	1.21	209.72	200.95	0.0413	0.0413	1.00	14.79	1.2	1.42
43	48	1.20	1.20	209.12	199.81	0.0413	0.0413	1.00	14.75	1.2	1.43
44	47	1.20	1.20	208.51	198.66	0.0413	0.0413	1.00	14.71	1.2	1.43
45	46	1.20	1.20	207.90	197.48	0.0413	0.0413	1.00	14.66	1.2	1.43
46	45	1.19	1.19	207.27	196.29	0.0413	0.0413	1.00	14.62	1.2	1.43
47	44	1.19	1.19	206.62	195.07	0.0413	0.0413	1.00	14.57	1.2	1.43
48	43	1.19	1.19	205.97	193.84	0.0413	0.0413	1.00	14.53	1.2	1.43
49	42	1.18	1.18	205.30	192.58	0.0413	0.0413	1.00	14.48	1.2	1.44

Sección	Altura (m).	Frz	Fa	V _D (km/h).	q _z (kg/m ²).	Ae (m ²).	AT (m ²).	Solidez φ	bV _D	Ca	g
50	41	1.18	1.18	204.62	191.31	0.0413	0.0413	1.00	14.43	1.2	1.44
51	40	1.17	1.17	203.92	190.01	0.0413	0.0413	1.00	14.38	1.2	1.44
52	39	1.17	1.17	203.21	188.68	0.0413	0.0413	1.00	14.33	1.2	1.44
53	38	1.17	1.17	202.49	187.34	0.0413	0.0413	1.00	14.28	1.2	1.44
54	37	1.16	1.16	201.74	185.96	0.0413	0.0413	1.00	14.23	1.2	1.45
55	36	1.16	1.16	200.98	184.56	0.0413	0.0413	1.00	14.18	1.2	1.45
56	35	1.15	1.15	200.20	183.13	0.0413	0.0413	1.00	14.12	1.2	1.45
57	34	1.15	1.15	199.40	181.67	0.0413	0.0413	1.00	14.06	1.2	1.45
58	33	1.14	1.14	198.58	180.18	0.0413	0.0413	1.00	14.01	1.2	1.45
59	32	1.14	1.14	197.74	178.66	0.0413	0.0413	1.00	13.95	1.2	1.46
60	31	1.13	1.13	196.88	177.10	0.0413	0.0413	1.00	13.89	1.2	1.46
61	30	1.13	1.13	195.99	175.50	0.0413	0.0413	1.00	19.35	1.2	1.46
62	29	1.12	1.12	195.07	173.87	0.0413	0.0413	1.00	19.26	1.2	1.47
63	28	1.12	1.12	194.13	172.19	0.0413	0.0413	1.00	19.17	1.2	1.47
64	27	1.11	1.11	193.16	170.47	0.0413	0.0413	1.00	19.07	1.2	1.47
65	26	1.11	1.11	192.15	168.71	0.0413	0.0413	1.00	18.98	1.2	1.47
66	25	1.10	1.10	191.12	166.89	0.0413	0.0413	1.00	18.87	1.2	1.48
67	24	1.09	1.09	190.04	165.02	0.0413	0.0413	1.00	18.77	1.2	1.48
68	23	1.09	1.09	188.93	163.09	0.0413	0.0413	1.00	18.66	1.2	1.48
69	22	1.08	1.08	187.78	161.11	0.0413	0.0413	1.00	18.54	1.2	1.49
70	21	1.07	1.07	186.57	159.05	0.0413	0.0413	1.00	18.42	1.2	1.49
71	20	1.07	1.07	185.32	156.92	0.0413	0.0413	1.00	18.30	1.2	1.49
72	19	1.06	1.06	184.01	154.72	0.0413	0.0413	1.00	18.17	1.2	1.50
73	18	1.05	1.05	182.65	152.42	0.0413	0.0413	1.00	18.04	1.2	1.50
74	17	1.04	1.04	181.21	150.04	0.0413	0.0413	1.00	17.89	1.2	1.51
75	16	1.03	1.03	179.70	147.55	0.0413	0.0413	1.00	17.75	1.2	1.51
76	15	1.02	1.02	178.11	144.94	0.0413	0.0413	1.00	17.59	1.2	1.52
77	14	1.02	1.02	176.42	142.21	0.0413	0.0413	1.00	17.42	1.2	1.52
78	13	1.00	1.00	174.63	139.33	0.0413	0.0413	1.00	17.24	1.2	1.53
79	12	0.99	0.99	172.71	136.29	0.0413	0.0413	1.00	17.05	1.2	1.54
80	11	0.98	0.98	170.65	133.05	0.0413	0.0413	1.00	16.85	1.2	1.54
81	10	0.97	0.97	168.42	129.60	0.0413	0.0413	1.00	16.63	1.2	1.55
82	9	0.96	0.96	165.99	125.88	0.0413	0.0413	1.00	16.39	1.2	1.56
83	8	0.94	0.94	163.31	121.86	0.0413	0.0413	1.00	16.13	1.2	1.57
84	7	0.92	0.92	160.33	117.45	0.0413	0.0413	1.00	15.83	1.2	1.58
85	6	0.90	0.90	156.95	112.56	0.0413	0.0413	1.00	15.50	1.2	1.60
86	5	0.88	0.88	153.05	107.03	0.0413	0.0413	1.00	15.11	1.2	1.61
87	4	0.85	0.85	148.41	100.64	0.0413	0.0413	1.00	14.66	1.2	1.63
88	3	0.82	0.82	142.64	92.96	0.0413	0.0413	1.00	14.09	1.2	1.66
89	2	0.78	0.78	134.87	83.12	0.0413	0.0413	1.00	13.32	1.2	1.69
90	1	0.71	0.71	122.57	68.64	0.0413	0.0413	1.00	12.10	1.2	1.76

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
Valor propuesto							
2.00	3.92	0.019	0.04	0.075	0.96	4.18	0.19
Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos	
0.9484	270.49	11.77	11.16	2	953 955 FX	5.58	
0.9473	269.33	11.74	11.12	2	998 1015 FX	5.56	
0.9461	268.17	11.70	11.07	2	997 1014 FX	5.53	
0.9450	266.99	11.66	11.02	2	950 952 FX	5.51	
0.9438	265.81	11.62	10.97	2	994 1013 FX	5.49	
0.9426	264.62	11.59	10.92	2	993 1012 FX	5.46	
0.9414	263.42	11.55	10.87	2	947 949 FX	5.44	
0.9402	262.21	11.51	10.82	2	990 1011 FX	5.41	
0.9389	260.99	11.47	10.77	2	989 1010 FX	5.39	
0.9377	259.77	11.43	10.72	2	944 946 FX	5.36	
0.9364	258.53	11.40	10.67	2	975 979 FX	5.34	
0.9352	257.28	11.36	10.62	2	974 978 FX	5.31	
0.9339	256.03	11.32	10.57	3	752 755 825 FX	3.52	
0.9326	254.76	11.28	10.52	4	790 807 892 895 FX	2.63	
0.9313	253.49	11.24	10.46	4	791-808-893-894-FX	2.62	
0.9299	252.20	11.19	10.41	3	792 809 829 FX	3.47	
0.9286	250.91	11.15	10.36	4	793 810 896 899 FX	2.59	
0.9272	249.60	11.11	10.30	4	794 811 897 898 FX	2.58	
0.9258	248.28	11.07	10.25	3	795 812 888 FX	3.42	
0.9244	246.95	11.03	10.19	4	796 813 902 905 FX	2.55	
0.9230	245.61	10.98	10.14	4	797 814 903 904 FX	2.53	
0.9216	244.25	10.94	10.08	3	798 815 889 FX	3.36	
0.9201	242.89	10.90	10.03	4	799 816 906 909 FX	2.51	
0.9187	241.51	10.85	9.97	4	800 817 907 908 FX	2.49	
0.9172	240.12	10.81	9.91	3	801 818 890 FX	3.30	
0.9157	238.72	10.76	9.85	4	802 819 910 913 FX	2.46	
0.9141	237.30	10.71	9.79	4	803 820 911 912 FX	2.45	
0.9126	235.87	10.67	9.74	3	804 821 891 FX	3.25	
0.9110	234.42	10.62	9.68	4	805 822 900 915 FX	2.42	
0.9094	232.97	10.57	9.62	4	806 823 901 914 FX	2.40	
0.9078	231.49	10.53	9.55	3	512 513 631 FX	3.18	
0.9061	230.00	10.48	9.49	4	543 601 672 675 FX	2.37	
0.9045	228.50	10.43	9.43	4	542 600 673 674 FX	2.36	
0.9028	226.98	10.38	9.37	3	541 599 628 FX	3.12	
0.9010	225.44	10.33	9.31	4	540 598 676 679 FX	2.33	
0.8993	223.88	10.28	9.24	4	539 597 677 678 FX	2.31	
0.8975	222.31	10.22	9.18	3	538 596 625 FX	3.06	
0.8957	220.72	10.17	9.11	4	537 595 680 683 FX	2.28	
0.8939	219.11	10.12	9.04	4	536 594 681 682 FX	2.26	
0.8920	217.49	10.06	8.98	3	535 593 622 FX	2.99	
0.8901	215.84	10.01	8.91	4	534 592 684 687 FX	2.23	
0.8881	214.17	9.95	8.84	4	533 591 685 686 FX	2.21	
0.8862	212.48	9.90	8.77	3	532 590 619 FX	2.92	
0.8842	210.77	9.84	8.70	4	531 589 688 691 FX	2.17	
0.8821	209.04	9.78	8.63	4	530 588 689 690 FX	2.16	

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.8800	207.28	9.72	8.56	3	529 587 616 FX	2.85
0.8779	205.50	9.66	8.48	4	528 586 692 695 FX	2.12
0.8757	203.69	9.60	8.41	4	527 585 693 694 FX	2.10
0.8735	201.86	9.54	8.33	3	526 584 613 FX	2.78
0.8712	200.00	9.48	8.26	4	525 583 696 699 FX	2.06
0.8689	198.11	9.41	8.18	4	524 582 697 698 FX	2.04
0.8665	196.20	9.35	8.10	3	523 581 610 FX	2.70
0.8641	194.25	9.28	8.02	4	522 580 701 702 FX	2.00
0.8616	192.27	9.21	7.94	4	521 579 700 703 FX	1.98
0.8591	190.26	9.14	7.85	3	520 578 607 FX	2.62
0.8565	188.21	9.07	7.77	4	519 577 704 707 FX	1.94
0.8538	186.13	9.00	7.68	4	518 576 705 706 FX	1.92
0.8510	184.01	8.92	7.59	3	517 575 604 FX	2.53
0.8482	181.85	8.85	7.51	4	516 574 709 710 FX	1.88
0.8453	179.64	8.77	7.41	4	515 573 708 711 FX	1.85
0.8423	177.39	8.69	7.32	7	4 5 373 421 422 424 425 FX	1.05
0.8392	175.10	8.61	7.23	6	378 383 403 404 427 429 FX	1.20
0.8361	172.76	8.53	7.13	4	377 382 402 405 FX	1.78
0.8328	170.36	8.44	7.03	4	376 381 401 406 FX	1.76
0.8294	167.91	8.36	6.93	4	375 380 400 407 FX	1.73
0.8259	165.40	8.27	6.83	4	374 379 399 408 FX	1.71
0.8223	162.83	8.17	6.72	7	10 18 303 339 340 342 343 FX	0.96
0.8185	160.19	8.08	6.61	6	290 300 328 329 345 347 FX	1.10
0.8146	157.48	7.98	6.50	4	289 299 327 330 FX	1.62
0.8105	154.69	7.88	6.38	4	288 298 326 331 FX	1.60
0.8062	151.82	7.77	6.27	4	287 297 325 332 FX	1.57
0.8018	148.86	7.66	6.14	4	286 296 324 333 FX	1.54
0.7971	145.80	7.55	6.02	7	9 17 206 234 235 237 238 FX	0.86
0.7922	142.63	7.43	5.89	6	211 216 226 227 240 242 FX	0.98
0.7870	139.35	7.31	5.75	4	210 215 225 228 FX	1.44
0.7816	135.94	7.18	5.61	4	209 214 224 229 FX	1.40
0.7758	132.38	7.04	5.46	4	208 213 223 230 FX	1.37
0.7696	128.67	6.90	5.31	4	207 212 222 231 FX	1.33
0.7629	124.78	6.75	5.15	7	8 16 154 172 173 175 176 FX	0.74
0.7558	120.68	6.59	4.98	6	121 159 160 169 178 180 FX	0.83
0.7481	116.34	6.42	4.80	4	120 158 161 168 FX	1.20
0.7396	111.73	6.24	4.61	4	119 157 162 167 FX	1.15
0.7303	106.79	6.04	4.41	4	118 156 163 166 FX	1.10
0.7198	101.45	5.82	4.19	4	117 155 164 165 FX	1.05
0.7079	95.62	5.57	3.95	7	7 15 21 88 89 91 92 FX	0.56
0.6941	89.15	5.30	3.68	6	26 36 57 66 94 96 FX	0.61
0.6776	81.83	4.98	3.38	4	25 35 58 65 FX	0.84
0.6569	73.27	4.60	3.02	4	24 34 59 64 FX	0.76
0.6287	62.71	4.12	2.59	4	23 33 60 63 FX	0.65
0.5834	48.05	3.40	1.98	4	22 32 61 62 FX	0.50

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	ν (Hz)	g_p	σ/μ
1 ^{ra} itera							
1.33196	2.61	0.032	0.029	0.091	0.82	4.15	0.21
Fg	Pz (kg/m ²)	Fa (kg)	Fz (kg)	No. de nodos	Nodos	Fz / #nodos	
0.99	283.34	11.77	11.69	2	953 955 FX	5.85	
0.99	282.13	11.74	11.64	2	998 1015 FX	5.82	
0.99	280.91	11.70	11.59	2	997 1014 FX	5.80	
0.99	279.68	11.66	11.54	2	950 952 FX	5.77	
0.99	278.44	11.62	11.49	2	994 1013 FX	5.75	
0.99	277.19	11.59	11.44	2	993 1012 FX	5.72	
0.99	275.93	11.55	11.39	2	947 949 FX	5.69	
0.98	274.67	11.51	11.34	2	990 1011 FX	5.67	
0.98	273.39	11.47	11.28	2	989 1010 FX	5.64	
0.98	272.11	11.43	11.23	2	944 946 FX	5.62	
0.98	270.81	11.40	11.18	2	975 979 FX	5.59	
0.98	269.51	11.36	11.12	2	974 978 FX	5.56	
0.98	268.19	11.32	11.07	3	752 755 825 FX	3.69	
0.98	266.87	11.28	11.01	4	790 807 892 895 FX	2.75	
0.98	265.53	11.24	10.96	4	791 808 893 894 FX	2.74	
0.97	264.18	11.19	10.90	3	792 809 829 FX	3.63	
0.97	262.83	11.15	10.85	4	793 810 896 899 FX	2.71	
0.97	261.46	11.11	10.79	4	794 811 897 898 FX	2.70	
0.97	260.07	11.07	10.73	3	795 812 888 FX	3.58	
0.97	258.68	11.03	10.68	4	796 813 902 905 FX	2.67	
0.97	257.28	10.98	10.62	4	797 814 903 904 FX	2.65	
0.97	255.86	10.94	10.56	3	798 815 889 FX	3.52	
0.96	254.43	10.90	10.50	4	799 816 906 909 FX	2.63	
0.96	252.99	10.85	10.44	4	800 817 907 908 FX	2.61	
0.96	251.53	10.81	10.38	3	801 818 890 FX	3.46	
0.96	250.06	10.76	10.32	4	802 819 910 913 FX	2.58	
0.96	248.57	10.71	10.26	4	803 820 911 912 FX	2.56	
0.96	247.08	10.67	10.20	3	804 821 891 FX	3.40	
0.95	245.56	10.62	10.14	4	805 822 900 915 FX	2.53	
0.95	244.03	10.57	10.07	4	806 823 901 914 FX	2.52	
0.95	242.49	10.53	10.01	3	512 513 631 FX	3.34	
0.95	240.93	10.48	9.94	4	543 601 672 675 FX	2.49	
0.95	239.35	10.43	9.88	4	542 600 673 674 FX	2.47	
0.95	237.76	10.38	9.81	3	541 599 628 FX	3.27	
0.94	236.15	10.33	9.75	4	540 598 676 679 FX	2.44	
0.94	234.52	10.28	9.68	4	539 597 677 678 FX	2.42	
0.94	232.87	10.22	9.61	3	538 596 625 FX	3.20	
0.94	231.21	10.17	9.54	4	537 595 680 683 FX	2.39	
0.94	229.52	10.12	9.47	4	536 594 681 682 FX	2.37	
0.93	227.82	10.06	9.40	3	535 593 622 FX	3.13	
0.93	226.09	10.01	9.33	4	534 592 684 687 FX	2.33	
0.93	224.35	9.95	9.26	4	533 591 685 686 FX	2.31	
0.93	222.58	9.90	9.19	3	532 590 619 FX	3.06	

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.93	220.78	9.84	9.11	4	531 589 688 691 FX	2.28
0.92	218.97	9.78	9.04	4	530 588 689 690 FX	2.26
0.92	217.13	9.72	8.96	3	529 587 616 FX	2.99
0.92	215.26	9.66	8.88	4	528 586 692 695 FX	2.22
0.92	213.37	9.60	8.81	4	527 585 693 694 FX	2.20
0.91	211.45	9.54	8.73	3	526 584 613 FX	2.91
0.91	209.50	9.48	8.65	4	525 583 696 699 FX	2.16
0.91	207.53	9.41	8.57	4	524 582 697 698 FX	2.14
0.91	205.52	9.35	8.48	3	523 581 610 FX	2.83
0.91	203.48	9.28	8.40	4	522 580 701 702 FX	2.10
0.90	201.41	9.21	8.31	4	521 579 700 703 FX	2.08
0.90	199.30	9.14	8.23	3	520 578 607 FX	2.74
0.90	197.15	9.07	8.14	4	519 577 704 707 FX	2.03
0.89	194.97	9.00	8.05	4	518 576 705 706 FX	2.01
0.89	192.75	8.92	7.96	3	517 575 604 FX	2.65
0.89	190.48	8.85	7.86	4	516 574 709 710 FX	1.97
0.89	188.18	8.77	7.77	4	515 573 708 711 FX	1.94
0.88	185.82	8.69	7.67	7	4 5 373 421 422 424 425 FX	1.10
0.88	183.42	8.61	7.57	6	378 383 403 404 427 429 FX	1.26
0.88	180.96	8.53	7.47	4	377 382 402 405 FX	1.87
0.87	178.45	8.44	7.37	4	376 381 401 406 FX	1.84
0.87	175.89	8.36	7.26	4	375 380 400 407 FX	1.81
0.87	173.26	8.27	7.15	4	374 379 399 408 FX	1.79
0.86	170.56	8.17	7.04	7	10 18 303 339 340 342 343 FX	1.01
0.86	167.80	8.08	6.93	6	290 300 328 329 345 347 FX	1.15
0.85	164.96	7.98	6.81	4	289 299 327 330 FX	1.70
0.85	162.04	7.88	6.69	4	288 298 326 331 FX	1.67
0.84	159.03	7.77	6.56	4	287 297 325 332 FX	1.64
0.84	155.93	7.66	6.44	4	286 296 324 333 FX	1.61
0.83	152.72	7.55	6.30	7	9 17 206 234 235 237 238 FX	0.90
0.83	149.41	7.43	6.17	6	211 216 226 227 240 242 FX	1.03
0.82	145.97	7.31	6.02	4	210 215 225 228 FX	1.51
0.82	142.40	7.18	5.88	4	209 214 224 229 FX	1.47
0.81	138.67	7.04	5.72	4	208 213 223 230 FX	1.43
0.81	134.78	6.90	5.56	4	207 212 222 231 FX	1.39
0.80	130.70	6.75	5.39	7	8 16 154 172 173 175 176 FX	0.77
0.79	126.41	6.59	5.22	6	121 159 160 169 178 180 FX	0.87
0.78	121.87	6.42	5.03	4	120 158 161 168 FX	1.26
0.77	117.03	6.24	4.83	4	119 157 162 167 FX	1.21
0.76	111.86	6.04	4.62	4	118 156 163 166 FX	1.15
0.75	106.27	5.82	4.39	4	117 155 164 165 FX	1.10
0.74	100.16	5.57	4.13	7	7 15 21 88 89 91 92 FX	0.59
0.73	93.39	5.30	3.85	6	26 36 57 66 94 96 FX	0.64
0.71	85.72	4.98	3.54	4	25 35 58 65 FX	0.88
0.69	76.75	4.60	3.17	4	24 34 59 64 FX	0.79
0.66	65.69	4.12	2.71	4	23 33 60 63 FX	0.68
0.61	50.34	3.40	2.08	4	22 32 61 62 FX	0.52

n_0 (Hz)	$3.6n_0(H/V_H)$	S	$(3.6n_0)/V_H$	E	u (Hz)	g_p	σ/μ
2 ^{da} itera							
1.31116	2.57	0.032	0.029	0.091	0.81	4.15	0.21

Fg	Pz (kg/m ²)	Fa (kg)	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.99	283.34	11.77	11.69	2	953 955 FX	5.85
0.99	282.13	11.74	11.64	2	998 1015 FX	5.82
0.99	280.91	11.70	11.59	2	997 1014 FX	5.80
0.99	279.68	11.66	11.54	2	950 952 FX	5.77
0.99	278.44	11.62	11.49	2	994 1013 FX	5.75
0.99	277.19	11.59	11.44	2	993 1012 FX	5.72
0.99	275.93	11.55	11.39	2	947 949 FX	5.69
0.98	274.67	11.51	11.34	2	990 1011 FX	5.67
0.98	273.39	11.47	11.28	2	989 1010 FX	5.64
0.98	272.11	11.43	11.23	2	944 946 FX	5.62
0.98	270.81	11.40	11.18	2	975 979 FX	5.59
0.98	269.51	11.36	11.12	2	974 978 FX	5.56
0.98	268.19	11.32	11.07	3	752 755 825 FX	3.69
0.98	266.87	11.28	11.01	4	790 807 892 895 FX	2.75
0.98	265.53	11.24	10.96	4	791 808 893 894 FX	2.74
0.97	264.18	11.19	10.90	3	792 809 829 FX	3.63
0.97	262.83	11.15	10.85	4	793 810 896 899 FX	2.71
0.97	261.46	11.11	10.79	4	794 811 897 898 FX	2.70
0.97	260.07	11.07	10.73	3	795 812 888 FX	3.58
0.97	258.68	11.03	10.68	4	796 813 902 905 FX	2.67
0.97	257.28	10.98	10.62	4	797 814 903 904 FX	2.65
0.97	255.86	10.94	10.56	3	798 815 889 FX	3.52
0.96	254.43	10.90	10.50	4	799 816 906 909 FX	2.63
0.96	252.99	10.85	10.44	4	800 817 907 908 FX	2.61
0.96	251.53	10.81	10.38	3	801 818 890 FX	3.46
0.96	250.06	10.76	10.32	4	802 819 910 913 FX	2.58
0.96	248.57	10.71	10.26	4	803 820 911 912 FX	2.56
0.96	247.08	10.67	10.20	3	804 821 891 FX	3.40
0.95	245.56	10.62	10.14	4	805 822 900 915 FX	2.53
0.95	244.03	10.57	10.07	4	806 823 901 914 FX	2.52
0.95	242.49	10.53	10.01	3	512 513 631 FX	3.34
0.95	240.93	10.48	9.94	4	543 601 672 675 FX	2.49
0.95	239.35	10.43	9.88	4	542 600 673 674 FX	2.47
0.95	237.76	10.38	9.81	3	541 599 628 FX	3.27
0.94	236.15	10.33	9.75	4	540 598 676 679 FX	2.44
0.94	234.52	10.28	9.68	4	539 597 677 678 FX	2.42
0.94	232.87	10.22	9.61	3	538 596 625 FX	3.20
0.94	231.21	10.17	9.54	4	537 595 680 683 FX	2.39
0.94	229.52	10.12	9.47	4	536 594 681 682 FX	2.37
0.93	227.82	10.06	9.40	3	535 593 622 FX	3.13
0.93	226.09	10.01	9.33	4	534 592 684 687 FX	2.33
0.93	224.35	9.95	9.26	4	533 591 685 686 FX	2.31
0.93	222.58	9.90	9.19	3	532 590 619 FX	3.06

Fg	Pz (kg/m ²).	Fa (kg).	Fz (kg)	No. de nodos	Nodos	Fz / #nodos
0.93	220.78	9.84	9.11	4	531 589 688 691 FX	2.28
0.92	218.97	9.78	9.04	4	530 588 689 690 FX	2.26
0.92	217.13	9.72	8.96	3	529 587 616 FX	2.99
0.92	215.26	9.66	8.88	4	528 586 692 695 FX	2.22
0.92	213.37	9.60	8.81	4	527 585 693 694 FX	2.20
0.91	211.45	9.54	8.73	3	526 584 613 FX	2.91
0.91	209.50	9.48	8.65	4	525 583 696 699 FX	2.16
0.91	207.53	9.41	8.57	4	524 582 697 698 FX	2.14
0.91	205.52	9.35	8.48	3	523 581 610 FX	2.83
0.91	203.48	9.28	8.40	4	522 580 701 702 FX	2.10
0.90	201.41	9.21	8.31	4	521 579 700 703 FX	2.08
0.90	199.30	9.14	8.23	3	520 578 607 FX	2.74
0.90	197.15	9.07	8.14	4	519 577 704 707 FX	2.03
0.89	194.97	9.00	8.05	4	518 576 705 706 FX	2.01
0.89	192.75	8.92	7.96	3	517 575 604 FX	2.65
0.89	190.48	8.85	7.86	4	516 574 709 710 FX	1.97
0.89	188.18	8.77	7.77	4	515 573 708 711 FX	1.94
0.88	185.82	8.69	7.67	7	4 5 373 421 422 424 425 FX	1.10
0.88	183.42	8.61	7.57	6	378 383 403 404 427 429 FX	1.26
0.88	180.96	8.53	7.47	4	377 382 402 405 FX	1.87
0.87	178.45	8.44	7.37	4	376 381 401 406 FX	1.84
0.87	175.89	8.36	7.26	4	375 380 400 407 FX	1.81
0.87	173.26	8.27	7.15	4	374 379 399 408 FX	1.79
0.86	170.56	8.17	7.04	7	10 18 303 339 340 342 343 FX	1.01
0.86	167.80	8.08	6.93	6	290 300 328 329 345 347 FX	1.15
0.85	164.96	7.98	6.81	4	289 299 327 330 FX	1.70
0.85	162.04	7.88	6.69	4	288 298 326 331 FX	1.67
0.84	159.03	7.77	6.56	4	287 297 325 332 FX	1.64
0.84	155.93	7.66	6.44	4	286 296 324 333 FX	1.61
0.83	152.72	7.55	6.30	7	9 17 206 234 235 237 238 FX	0.90
0.83	149.41	7.43	6.17	6	211 216 226 227 240 242 FX	1.03
0.82	145.97	7.31	6.02	4	210 215 225 228 FX	1.51
0.82	142.40	7.18	5.88	4	209 214 224 229 FX	1.47
0.81	138.67	7.04	5.72	4	208 213 223 230 FX	1.43
0.81	134.78	6.90	5.56	4	207 212 222 231 FX	1.39
0.80	130.70	6.75	5.39	7	8 16 154 172 173 175 176 FX	0.77
0.79	126.41	6.59	5.22	6	121 159 160 169 178 180 FX	0.87
0.78	121.87	6.42	5.03	4	120 158 161 168 FX	1.26
0.77	117.03	6.24	4.83	4	119 157 162 167 FX	1.21
0.76	111.86	6.04	4.62	4	118 156 163 166 FX	1.15
0.75	106.27	5.82	4.39	4	117 155 164 165 FX	1.10
0.74	100.16	5.57	4.13	7	7 15 21 88 89 91 92 FX	0.59
0.73	93.39	5.30	3.85	6	26 36 57 66 94 96 FX	0.64
0.71	85.72	4.98	3.54	4	25 35 58 65 FX	0.88
0.69	76.75	4.60	3.17	4	24 34 59 64 FX	0.79
0.66	65.69	4.12	2.71	4	23 33 60 63 FX	0.68
0.61	50.34	3.40	2.08	4	22 32 61 62 FX	0.52

Fuerzas sobre las Parábolas en dirección X-X.

Antenas Parabólicas	Altura (ft).	Grados (γ)	Diámetro (φ) (ft).	Wind Force Coefficients for Typical Paraboloid Without Radome			Ap (ft²).	Kz (ft)	GH (ft).
				CA	CS	CM			
1	295.2756	270	9.8425	-0.00003	-0.00088	-0.000336	76.0856	1.8703	1.0887
2	295.2756	270	9.8425	-0.00003	-0.00088	-0.000336	76.0856	1.8703	1.0887
3	295.2756	90	9.8425	-0.00003	0.00088	0.000336	76.0856	1.8703	1.0887
4	196.8504	270	9.8425	-0.00003	-0.00088	-0.000336	76.0856	1.6657	1.0887
5	196.8504	90	9.8425	-0.00003	0.00088	0.000336	76.0856	1.6657	1.0887
6	196.8504	270	9.8425	-0.00003	-0.00088	-0.000336	76.0856	1.6657	1.0887
7	98.4252	270	9.8425	-0.00003	-0.00088	-0.000336	76.0856	1.3665	1.0887
8	98.4252	90	9.8425	-0.00003	0.00088	0.000336	76.0856	1.3665	1.0887
9	98.4252	270	9.8425	-0.00003	-0.00088	-0.000336	76.0856	1.3665	1.0887
PLATAF-1	98.4252	0	7.3281	0.00397	0.00000	0.000000	42.1765	1.3665	1.0887
PLATAF-2	196.8504	0	6.1112	0.00397	0.00000	-0.000000	29.3323	1.6657	1.0887

V _R (mi/h)	F _A (lb).	F _s (lb).	M (lb-ft).	F _Z (lb).	F _X (lb).	F _X (kg).	F _Z (kg).	M (kg-m).
98.18	-44.80	-1314.13	-4938.56	-44.80	-1314.13	-596.09	-20.32	-682.78
98.18	-44.80	-1314.13	-4938.56	-44.80	-1314.13	-596.09	-20.32	-682.78
98.18	-44.80	1314.13	4938.56	-44.80	1314.13	596.09	-20.32	682.78
98.18	-39.90	-1170.38	-4398.34	-39.90	-1170.38	-530.88	-18.10	-608.09
98.18	-39.90	1170.38	4398.34	-39.90	1170.38	530.88	-18.10	608.09
98.18	-39.90	-1170.38	-4398.34	-39.90	-1170.38	-530.88	-18.10	-608.09
98.18	-32.73	-960.10	-3608.11	-32.73	-960.10	-435.50	-14.85	-498.84
98.18	-32.73	960.10	3608.11	-32.73	960.10	435.50	-14.85	498.84
98.18	-32.73	-960.10	-3608.11	-32.73	-960.10	-435.50	-14.85	-498.84
98.18	2401.01	0.00	0.00	0.00	2401.01	1089.10	0.00	0.00
98.18	2035.53	0.00	0.00	0.00	2035.53	923.32	0.00	0.00

Por convención del programa STAAD, las direcciones de las Fuerzas y Momentos serán los siguientes:

Nodos	FX (kg).	FX / # nodos (kg).	Nodos	FZ (kg).	Nodos	M (kg-m).
953 FX	596.09		953 FZ	-20.32	953 MY	-682.78
954 FX	596.09		954 FZ	-20.32	954 MY	-682.78
955 FX	596.09		955 FZ	20.32	955 MY	682.78
512 FX	530.88		512 FZ	-18.10	512 MY	-608.09
513 FX	530.88		513 FZ	18.10	513 MY	608.09
514 FX	530.88		514 FZ	-18.10	514 MY	-608.09
4 FX	435.50		4 FZ	-14.85	4 MY	-498.84
5 FX	435.50		5 FZ	14.85	5 MY	498.84
6 FX	435.50		6 FZ	-14.85	6 MY	-498.84
496 497 498 499 500 501 494 460 464 465 466 467 468 458 FX	1089.10	77.79				
848 854 855 856 849 830 842 843 844 831 FX	923.32	92.33				

El valor obtenido de frecuencia natural en dirección X-X, con las iteraciones conjuntas de la estructura, líneas y parábolas es:

n_0 (Hz)

Definitivo

1.31116

Por lo tanto las fuerzas obtenidas de la segunda iteración, son las fuerzas de diseño.

III. 3. 4. Sismo.

Para el sismo se realizó el espectro de diseño, el cual se hizo de la siguiente manera:

Clasificación de la construcción según su destino:	Grupo A
Clasificación de la construcción según su estructuración:	Tipo 4
Terreno:	Tipo II
Región sísmica:	Zona A.
Factor de coeficiente sísmico:	Q = 2

Para diseñar se necesita considerar el comportamiento inelástico de la estructura, aunque sea de manera aproximada. Para ello las ordenadas espectrales se reducirán dividiéndolas entre el factor reductivo Q' a fin de obtener las fuerzas sísmicas reducidas por ductilidad. El factor reductivo se calcula de la siguiente manera:

$$Q' = 1 + (Q - 1) \frac{T}{T_a}; \quad \text{si } T < T_a$$

$$Q' = Q; \quad \text{si } T > T_a$$

De la tabla II. 2. 6. 1.

$$a_0 = 0.04$$

$$c = 0.16$$

$$T_a (s) = 0.3$$

$$T_b (s) = 1.5$$

$$r = 0.6667$$

$$F_a = 1.5$$

Espectro de diseño

$$a = a_0 + (c - a_0) \frac{T}{T_a}; \quad \text{si } T < T_a$$

$$a = c; \quad \text{si } T_a \leq T \leq T_b$$

$$a = c \left[\frac{T_b}{T} \right]^r; \quad \text{si } T > T_b$$

Espectro de Diseño

T (s)	Q'	a/g
0.00	1.00	0.06
0.05	1.17	0.08
0.10	1.33	0.09
0.15	1.50	0.10
0.20	1.67	0.11
0.25	1.83	0.11
0.30	2.00	0.12
0.50	2.00	0.12
0.70	2.00	0.12
0.90	2.00	0.12
1.10	2.00	0.12
1.30	2.00	0.12
1.50	2.00	0.12
1.60	2.00	0.11
1.90	2.00	0.10
2.20	2.00	0.09
2.50	2.00	0.09
2.80	2.00	0.08

Continuación...

T (s)	Q'	a/g
3.10	2.00	0.07
3.40	2.00	0.07
3.70	2.00	0.07
4.00	2.00	0.06
4.30	2.00	0.06
4.60	2.00	0.06
4.90	2.00	0.05
5.20	2.00	0.05
5.50	2.00	0.05
5.80	2.00	0.05
6.10	2.00	0.05
6.40	2.00	0.05
6.70	2.00	0.04
7.00	2.00	0.04
7.30	2.00	0.04
7.60	2.00	0.04
7.90	2.00	0.04
8.20	2.00	0.04

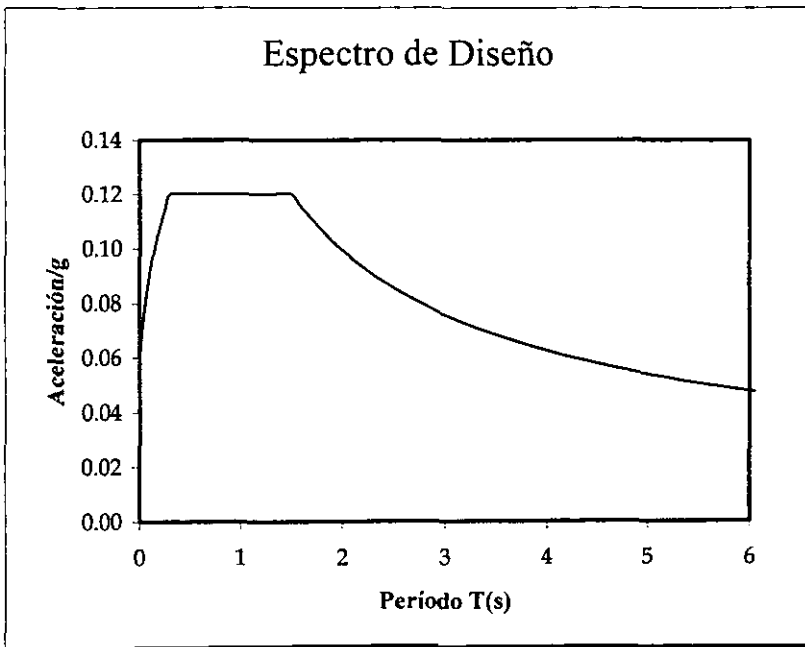


Figura III. 3. 1. Espectro de diseño.

Los valores del espectro del diseño fueron introducidos al programa STAAD para llevar a cabo el análisis de fuerzas.

- Factores de escalamiento para el análisis sísmico.

Para la obtención de los factores de escalamiento se debe obtener primero el peso total de la estructura y el período correspondiente al primer modo de vibración de la esta.

Peso de la estructura	129 843 kg
Peso de las parábolas	1 800 kg
<hr/>	
Peso total	131 643 kg

 Período de la estructura = **0.94545 s**

Con el valor del período fundamental entramos a la gráfica del espectro de diseño (figura III. 3. 1), con la cual obtenemos la siguiente aceleración:

 $a = 0.12$

Con este valor y el peso total de la estructura obtenemos el cortante basal:

$$V = 0.12 (131\ 643)$$

$$V = 15\ 797\ \text{kg}$$

Los cortantes obtenidos en la corrida del STAAD son:

$$\text{Cortante en dirección Z-Z} = 7\ 345.36\ \text{kg}$$

$$\text{Cortante en dirección X-X} = 7345.08\ \text{kg}$$

Los factores de escalamiento son aplicados para cada dirección del análisis. La forma de obtener los factores de escalamiento se llevará a cabo de la siguiente manera:

$$\text{Factor de escalamiento en Z-Z} = (15\ 797 / 7\ 345.36)$$

$$\text{Factor de escalamiento en Z-Z} = \mathbf{2.151}$$

Factor de escalamiento en X-X = (15 797 / 7345.08)

Factor de escalamiento en X-X = 2.151

	Para Cimentación		Para Diseño	
	100%	30%	100%	30%
Factor de escalamiento en Z-Z =	2.151	0.645	1.613	0.484
Factor de escalamiento en X-X =	2.151	0.645	1.613	0.484

Estos factores serán considerados en las combinaciones de cargas.

III. 4. Estados de carga y combinaciones.

- Estados de carga.

- 1.- Peso propio y peso de parábolas. (CM).
- 2.- Carga viva (CV).
- 3.- Viento en dirección Z-Z (VZ).
- 4.- Viento en dirección X-X (VX).
- 5.- Sismo en dirección de Z-Z (SZ).
- 6.- Sismo en dirección de X-X (SX).

- Combinaciones de carga.

Cimentación.

- 7.- $(CM + CV) * 1$
- 8.- $(CM + CV + VZ) * 1$
- 9.- $(CM + CV - VZ) * 1$
- 10.- $(CM + CV + VX) * 1$
- 11.- $(CM + CV - VX) * 1$
- 12.- $(CM + CV + 100\% SZ + 30\% SX) * 1$
- 13.- $(CM + CV + 100\% SZ - 30\% SX) * 1$
- 14.- $(CM + CV - 100\% SZ + 30\% SX) * 1$
- 15.- $(CM + CV - 100\% SZ - 30\% SX) * 1$
- 16.- $(CM + CV + 30\% SZ + 100\% SX) * 1$
- 17.- $(CM + CV + 30\% SZ - 100\% SX) * 1$
- 18.- $(CM + CV - 30\% SZ + 100\% SX) * 1$
- 19.- $(CM + CV - 30\% SZ - 100\% SX) * 1$

Diseño.

- 20.- $(CM + CV) * 1$
- 21.- $(CM + CV + VZ) * 0.75$
- 22.- $(CM + CV - VZ) * 0.75$
- 23.- $(CM + CV + VX) * 0.75$
- 24.- $(CM + CV - VX) * 0.75$
- 25.- $(CM + CV + 100\% SZ + 30\% SX) * 0.75$
- 26.- $(CM + CV + 100\% SZ - 30\% SX) * 0.75$
- 27.- $(CM + CV - 100\% SZ + 30\% SX) * 0.75$
- 28.- $(CM + CV - 100\% SZ - 30\% SX) * 0.75$
- 29.- $(CM + CV + 30\% SZ + 100\% SX) * 0.75$
- 30.- $(CM + CV + 30\% SZ - 100\% SX) * 0.75$
- 31.- $(CM + CV - 30\% SZ + 100\% SX) * 0.75$
- 32.- $(CM + CV - 30\% SZ - 100\% SX) * 0.75$

A continuación se presentan las mismas combinaciones de carga, las cuales a diferencia de las anteriores, fueron modificadas por los factores de escalamiento en los factores de carga.

Cimentación.

- 7.- $(CM + CV) * 1$
- 8.- $(CM + CV + VZ) * 1$
- 9.- $(CM + CV - VZ) * 1$
- 10.- $(CM + CV + VX) * 1$
- 11.- $(CM + CV - VX) * 1$
- 12.- $(CM + CV) * 1 + (100\% SZ * 2.151) + (30\% SX * 0.6453)$

$$13.- (CM + CV)*1 + (100\% SZ * 2.151) - (30\% SX * 0.6453)$$

$$14 (CM + CV)*1 - (100\% SZ * 2.151) + (30\% SX * 0.6453)$$

$$15.- (CM + CV)*1 - (100\% SZ * 2.151) - (30\% SX * 0.6453)$$

$$16.- (CM + CV)*1 + (30\% SZ * 0.6453) + (100\% SX * 2.151)$$

$$17.- (CM + CV)*1 + (30\% SZ * 0.6453) - (100\% SX * 2.151)$$

$$18.- (CM + CV)*1 - (30\% SZ * 0.6453) + (100\% SX * 2.151)$$

$$19.- (CM + CV)*1 - (30\% SZ * 0.6453) - (100\% SX * 2.151)$$

Diseño.

$$20.- (CM + CV)*1$$

$$21.- (CM + CV + VZ)*0.75$$

$$22.- (CM + CV - VZ)*0.75$$

$$23.- (CM + CV + VX)*0.75$$

$$24.- (CM + CV - VX)*0.75$$

$$25.- (CM + CV)*0.75 + (100\% SZ * 1.61325) + (30\% SX * 0.48397)$$

$$26.- (CM + CV)*0.75 + (100\% SZ * 1.61325) - (30\% SX * 0.48397)$$

$$27.- (CM + CV)*0.75 - (100\% SZ * 1.61325) + (30\% SX * 0.48397)$$

$$28.- (CM + CV)*0.75 - (100\% SZ * 1.61325) - (30\% SX * 0.48397)$$

$$29.- (CM + CV)*0.75 + (30\% SZ * 0.48397) + (100\% SX * 1.61325)$$

$$30.- (CM + CV)*0.75 + (30\% SZ * 0.48397) - (100\% SX * 1.61325)$$

$$31.- (CM + CV)*0.75 - (30\% SZ * 0.48397) + (100\% SX * 1.61325)$$

$$32.- (CM + CV)*0.75 - (30\% SZ * 0.48397) - (100\% SX * 1.61325)$$

III. 5. Modelo matemático.

La estructura esquelética es analizada por medio del programa STAAD-Pro versión 2001, este programa nos ayuda a realizar un análisis tridimensional matricial, para la aplicación del método de análisis, con este programa se elabora un archivo de entrada de datos que consiste en los cuatro puntos siguientes:

- 1) Topología de la estructura esquelética (Definición de coordenadas y elementos de incidencia).
- 2) Propiedades geométricas de los elementos estructurales.
- 3) Estados y combinaciones de carga.
- 4) Condiciones de apoyo y restricciones de la estructura esquelética.

El tipo de análisis que se utiliza en este programa, consiste esencialmente en lo siguiente:

El método que lleva a cabo el programa STAAD-Pro, es el *método de rigideces en tres dimensiones*. Primero obtiene la matriz de rigideces de cada uno de los elementos estructurales $[k^m]$, para posteriormente ensamblar con cada una de estas, la matriz global de rigideces $[K^m]$.

Una vez obtenidas las fuerzas externas $[F]$, se deben introducir en el archivo de datos, el programa procede a calcular los desplazamientos $[d]$ en cada nudo de la estructura, es decir, resuelve el sistema matricial siguiente:

$$[F] = [K] [d]$$

Una vez obtenidos los desplazamientos, el programa procede a calcular las deformaciones de cada elemento y con estas sus esfuerzos principales, así mismo, calcula los elementos mecánicos (fuerzas axiales, fuerzas cortantes y momentos flexionantes) en cada uno de los elementos de la estructura.

III. 6. Análisis del modelo matemático.

El análisis del modelo matemático se presenta en su totalidad en los apéndices A y B. El apéndice A contiene los archivos de entrada y salida de datos; en tanto que en el apéndice B se presentan algunos gráficos ilustrativos de la estructura.

IV. Diseño.

Criterios de Diseño Estructural

Toda estructura y cada una de sus partes deberán diseñarse para cumplir con los requisitos básicos siguientes:

I. Tener seguridad adecuada contra la aparición de todo estado límite de falla posible ante las combinaciones de acciones más desfavorables que puedan presentarse durante su vida esperada.

II. No rebasar ningún estado límite de servicio ante combinaciones de acciones que corresponden a condiciones normales de operación

Estado límite de falla.

Se considera como estado límite de falla cualquier situación que corresponda al agotamiento de la capacidad de carga de la estructura o de cualesquiera de sus componentes, incluyendo la cimentación, o al hecho de que ocurran daños irreversibles que afecten significativamente la resistencia ante nuevas aplicaciones de carga.

Estado límite de servicio.

Se considerará como estado límite de servicio la ocurrencia de desplazamientos, agrietamientos vibraciones o daños que afecten el correcto funcionamiento de la edificación, pero que no perjudiquen su capacidad para soportar cargas.

Los estados límite de falla y de servicio se revisarán conforme el avance de este trabajo.

IV. 1. Diseño de la estructura

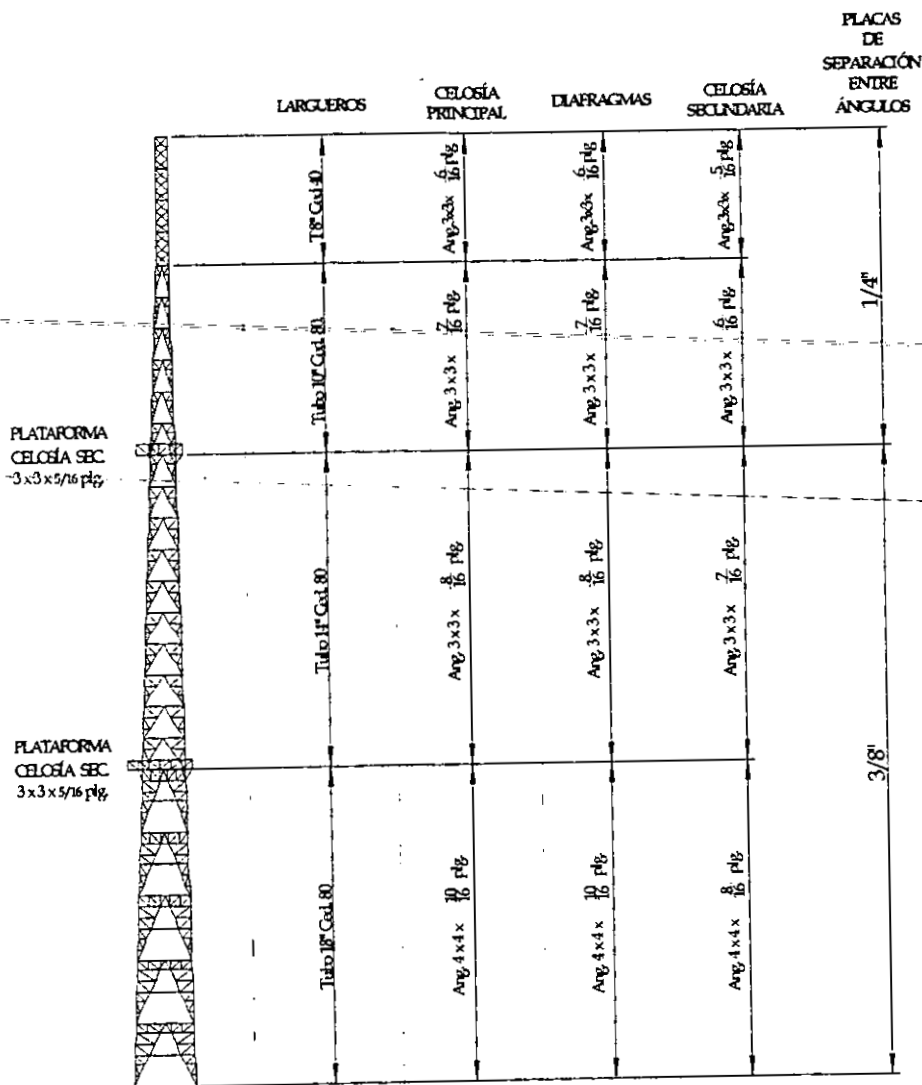


Figura: IV. 1. 1. Propiedades geométricas de la estructura

Las propiedades geométricas obtenidas en el diseño de la estructura, son las que se presentan en la figura (IV. 1. 1.). En esta figura también se puede observar las propiedades de las placas que se encuentran entre los ángulos de la celosía.

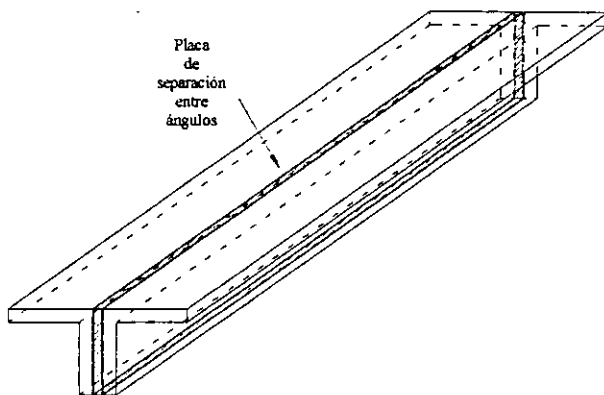


Figura: IV. 1. 2. Placa entre ángulos.

Las propiedades de los largueros (tubo galvanizado) empleados son:

Diámetro exterior = ϕ ext.

Diámetro interior = ϕ int.

Área a cortante = A_c

Tubo 18" cédula 80: ϕ ext. = 457 mm.
 ϕ int. = 410 mm.
 $A_c = 325 \text{ cm}^2$.

Tubo 14" cédula 80: ϕ ext. = 356 mm.
 ϕ int. = 317 mm.
 $A_c = 201.42 \text{ cm}^2$.

Tubo 10" cédula 80: ϕ ext. = 273 mm.
 ϕ int. = 243 mm.
 $A_c = 122.3 \text{ cm}^2$.

Tubo 8" cédula 40:

ϕ ext. = 219 mm.

ϕ int. = 203 mm.

$A_c = 54.3 \text{ cm}^2$.

Cálculo de máxima deflexión.

Deflexión permisible:

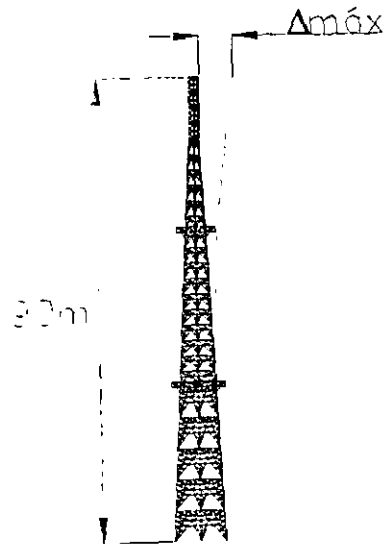
$$\Delta_{\text{perm}} = 0.006 (H)$$

$$\Delta_{\text{perm}} = 0.006 (90 \text{ m}) = 0.54 \text{ m} = 54 \text{ cm.}$$

Máxima deflexión:

$$\Delta_{\text{máx}} = 48.79 \text{ cm.} \quad (\text{Ver apéndice A}).$$

Como: $\Delta_{\text{perm}} > \Delta_{\text{máx}}$ entonces, cumple por deformaciones.



IV. 2. Diseño de la cimentación.

Revisión de Presiones en la Cimentación

Revisión de losa de cimentación en dirección "Z"

De la página 34 de resultados del STAAD obtenemos:

Combinación de Carga No. 8
CM+CV+Viento en "Z"

Nodo 1		Nodo 2	
Tensión =	-145431 kg	Compresión =	423104 kg
Vz =	-11519 kg	Vz =	-45055 kg
Mx =	-3162 kg-m	Mx =	7111 kg-m
Nodo 3			
Tensión =	-145430 kg		
Vz =	-11147 kg		
Mx =	-2806 kg-m		

Parámetros

Capacidad admisible del terreno	$\sigma_{adm} =$	16000 kg / m ²
Peso volumétrico del concreto	$\gamma_{concreto} =$	2400 kg / m ³
Peso volumétrico del relleno	$\gamma_{relleno} =$	1600 Kg / m ³
Factor de seguridad	F.S. =	1.1

Dimensiones Propuestas

$B_{Losa} =$	13 m.	$h_{Peralte} =$	1.0 m.
$L_{Losa} =$	13 m.	$B_{Columna} =$	0.7 m
$D_f =$	2.5 m.	$L_{Columna} =$	0.7 m

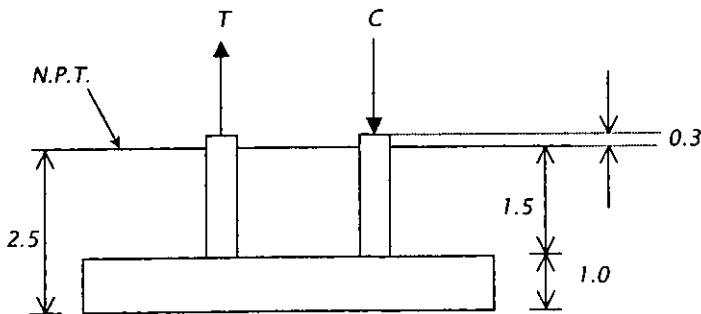


Fig. IV. 2. 1. Esquema de Losa de Cimentación (Unidades en metros).

Fuerzas de Diseño

Nodo 1	Nodo 2
Tensión = 145431 kg	Compresión = 423104 kg
$Mx_{total} = 29089 \text{ kg-m}$	$Mx_{total} = 133264 \text{ kg-m}$
Nodo 3	
Tensión = 145430 kg	
$Mx_{total} = 28405 \text{ kg-m}$	

Momento de Volteo

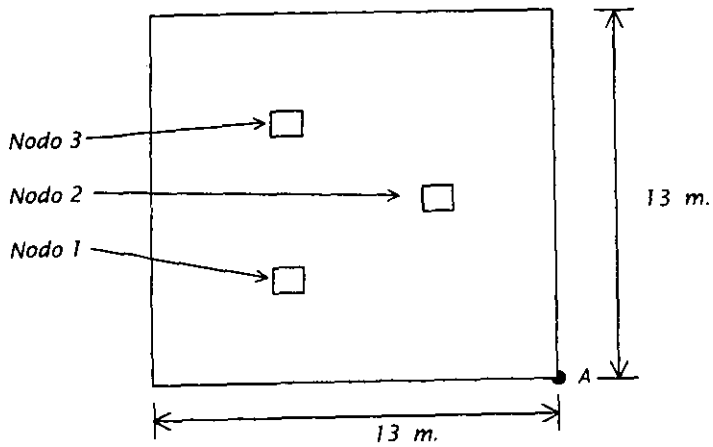
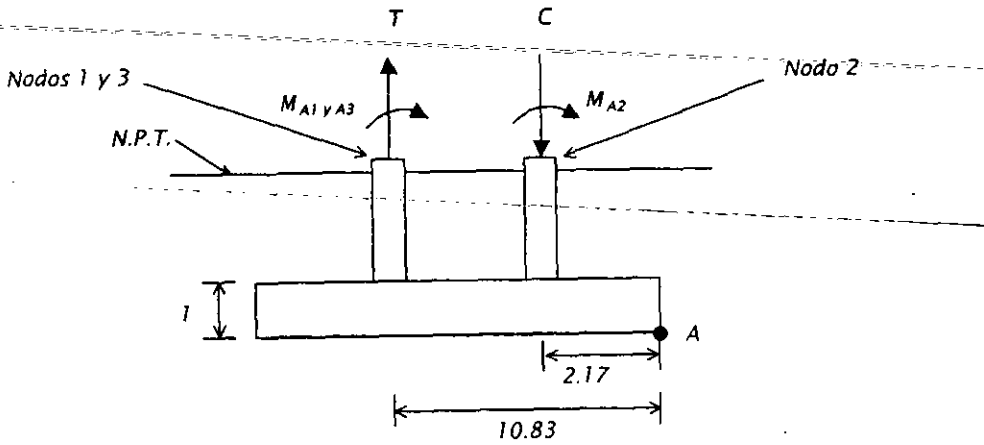


Fig. IV. 2. 2 Esquema de Losa en Planta y Elevación (Unidades en metros).

Suma de momentos respecto de "A"

$$M_t = 255681 \text{ kg-m}$$

$$M_v = (M_t)(F.S.)$$

$$M_v = 281249 \text{ kg-m}$$

Esfuerzo ejercido

$$\text{Tensión Nudo 1} = 145431 \text{ kg}$$

$$\text{Compresión Nudo 2} = 423104 \text{ kg}$$

$$\text{Tensión Nudo 3} = 145430 \text{ kg}$$

$$\text{Área de dados} = 1.47 \text{ m}^2$$

$$\text{Volumen de dados} = 2.646 \text{ m}^3$$

$$\text{Área de losa} = 169 \text{ m}^2$$

$$\text{Volumen losa} = 169 \text{ m}^3$$

$$\text{Volumen de concreto} = 171.646 \text{ m}^3$$

$$\text{Peso de losa y dados} = 411950.4 \text{ kg}$$

$$\text{Área de relleno} = 167.53 \text{ m}^2$$

$$\text{Volumen del relleno} = 251.295 \text{ m}^3$$

$$\text{Peso del relleno} = 402072 \text{ kg}$$

Fuerza Resultante

$$P = 946265 \text{ kg}$$

$$P_{\text{Total}} = (P)(F.S.)$$

$$P_{\text{Total}} = 1040892 \text{ kg}$$

Cálculo de la excentricidad

$$e = \frac{M_v}{P_{\text{Total}}}$$

$$e = 0.27 \text{ m.}$$

$$a = 6.23 \text{ m.}$$

$$B/3 = 4.33 \text{ m.}$$

$$B/6 = 2.17 \text{ m.}$$

$$\text{Si } e = 0 \quad \sigma = \frac{P_{\text{Total}}}{A_{\text{Losa}}} \quad \dots(1)$$

$$\text{Si } e < B/6 \text{ ó } a > B/3 \quad \sigma = \left(\frac{P_{\text{Total}}}{A_{\text{Losa}}} \right) \left[1 \pm \left(\frac{6e}{B} \right) \right] \quad \dots(2)$$

$$\text{Si } e = B/6 \text{ ó } a = B/3 \quad \sigma = 2 \left(\frac{P_{\text{Total}}}{A_{\text{Losa}}} \right) \quad \dots(3)$$

$$\text{Si } e > B/6 \quad \sigma = \frac{4P_{\text{Total}}}{3L(B - 2e)} \quad \dots(4)$$

Se empleará la ecuación que conforme a los parámetros calculados sea la indicada

$$\begin{array}{l} \sigma_{\text{máx}} = 6927 \text{ kg/m}^2 < \sigma_{\text{admisible}} = 16000 \text{ kg/m}^2 \\ \sigma_{\text{mín}} = 5391 \text{ kg/m}^2 < \sigma_{\text{admisible}} = 16000 \text{ kg/m}^2 \end{array}$$

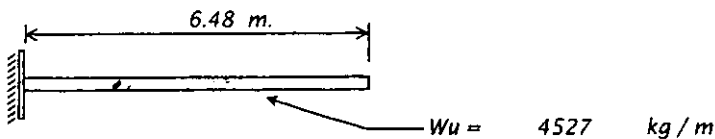
Por lo tanto se acepta.

Diseño de Losa de Cimentación

Esfuerzo empleado para el diseño: Esf_D

$$Esf_D = \sigma_{\text{máx}} \cdot esf_{\text{relleno}}$$

$$Esf_D = 4527 \text{ kg/m}^2$$



"L" para la sección más crítica:

6.48 m

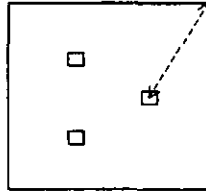


Fig. IV. 2. 3. Esquema de la losa vista en planta

$$M_u = \frac{W_u(L)^2}{2} \quad ; \quad V_u = W_u(L)$$

$$M_u = 95156 \text{ kg-m} \quad ; \quad V_u = 29353 \text{ kg}$$

Constantes de cálculo:

Resistencia del concreto a compresión a los 28 días: $f'c = 350 \text{ Kg/cm}^2$
 Esfuerzo de fluencia del acero de refuerzo: $f_y = 4200 \text{ Kg/cm}^2$

$$f^*c = 0.8 f'c \quad f^*c = 280 \text{ Kg/cm}^2$$

$$f''c = [1.05 - (f^*c/1250)](f^*c) \quad f''c = 231.28 \text{ Kg/cm}^2 \quad \text{Si } f^*c > 250 \text{ kg/cm}^2$$

$$f''c = 0.85 f^*c \quad f''c = 238 \text{ Kg/cm}^2 \quad \text{Si } f^*c \leq 250 \text{ kg/cm}^2$$

Definitivo $f''c = 231.28 \text{ Kg/cm}^2$

Porcentaje balanceado de acero para losas de concreto: ρ_b

$$\rho_b = \frac{f''c}{f_y} \frac{4800}{f_y + 6000} \quad \rho_b = 0.0259$$

$$\rho_{m\acute{a}x} = 0.75 \rho_b \quad \rho_{m\acute{a}x} = 0.0194$$

$$\rho_{m\acute{i}n} = \frac{0.7 \sqrt{f''c}}{f_y} \quad \rho_{m\acute{i}n} = 0.0031$$

Se propondrá $\rho = 0.0045$ de porcentaje para cálculo

$$q = \frac{\rho f_y}{f'c}$$

$$q = 0.0541$$

Revisión del peralte por flexión.

$$df = \sqrt{\frac{Mu}{F_R f'' c b q (1 - 0.5q)}}$$

$$df = 93.17 \text{ cm.}$$

Peralte total propuesto	$h =$	100	cm.
Recubrimiento libre	$r =$	5	cm.
Peralte efectivo	$dp =$	95	cm.

Como: $dp = 95 \text{ cm.} > df = 93.17 \text{ cm.}$

entonces el peralte propuesto es correcto. (ok).

Revisión por cortante como viga ancha.

$$Vud = (L - d)Wu$$

$$Vud = 25052 \text{ kg}$$

Capacidad del concreto para tomar cortante.

$$\text{Si } \rho < 0.01 \quad V_{CR} = F_R b d (0.2 + 30\rho) \sqrt{f^* c}$$

$$V_{CR} = 42643 \text{ kg}$$

$$\text{Si } \rho \geq 0.01 \quad V_{CR} = 0.5 F_R b d \sqrt{f^* c}$$

$$V_{CR} = 63586 \text{ kg}$$

Debido al porcentaje de acero propuesto se toma:

$$V_{CR} = 42643 \text{ kg}$$

Como el peralte es mayor de 70 cm, se reducirá en 30% la capacidad del cortante:

$$\text{Definitivo} \quad V_{CR} = 29850 \text{ kg}$$

Como: $V_{CR} > Vud$ entonces el peralte es correcto. (ok).

Revisión por penetración.

$$\begin{aligned}C1 &= 70 \text{ cm.} \\C2 &= 70 \text{ cm.} \\dp &= 95 \text{ cm.} \\C1+dp &= 165 \text{ cm.} \\C2+dp &= 165 \text{ cm.}\end{aligned}$$

$$\alpha = 1 - \frac{1}{1 + 0.67 \sqrt{(C1+dp)/(C2+dp)}}$$

$$\alpha = 0.401198$$

Momento que toma la losa:

$$\alpha Mx = 5346532.52 \text{ kg-cm}$$

$$V_{Total} = P \cdot [(C1+dp)(C2+dp)(Esf_D)]$$

$$V_{Total} = 682119 \text{ kg}$$

$$Ac = 2[(C1+dp)+(C2+dp)](dp)$$

$$Ac = 62700 \text{ cm}^2$$

$$Jc = \frac{dp(C1+dp)^3}{6} + \frac{(C1+dp)dp^3}{6} + \frac{dp(C2+dp)(C1+dp)^2}{2}$$

$$Jc = 71125312.5 + 23577813 + 213375938$$

$$Jc = 308079063 \text{ cm}^4$$

Sustituyendo valores en la fórmula de la escuadría.

$$Vu = \frac{V_{Total}}{Ac} \pm \frac{\alpha Mx C_{AB}}{Jc}$$

$$Vu_{\text{máx.}} = 12.31 \text{ Kg/cm}^2$$

$$Vu_{\text{mín.}} = 9.45 \text{ Kg/cm}^2$$

El esfuerzo cortante que resiste el concreto por penetración es:

$$Vcr = F_R \sqrt{f^* c} \quad Vcr = 13.39 \text{ Kg/cm}^2$$

Como $Vcr > Vu_{\text{máx.}}$ no existe penetración de la columna en la losa de cimentación.

Por lo tanto se aceptan las dimensiones propuestas para la cimentación.

Acero de refuerzo por flexión:

$$A_s = \rho \quad b d \qquad A_s = 42.85 \quad \text{cm}^2$$

Separación de varillas para la losa en su cara inferior.

Varilla que se utilizará 1 " Barra del No. 8

$$a_s = 5.07 \quad \text{cm}^2$$

No. de barras: 1

$$s = \frac{100 \quad a_s}{A_s} \qquad s = 11.82 \quad \text{cm}$$

Utilizar paquetes de: 1 barras

Con varillas de: 1 " a cada 12 cm.

Acero requerido por Cambios Volumétricos

Separación de varillas para la losa en su cara superior.

$$a_{ST} = \frac{660 \quad x_1}{f_y (x_1 + 100)} \qquad a_{ST} = 7.86 \quad \text{cm}^2/\text{cm}$$

Por ser un elemento que está en contacto con el terreno, el área será aumentada en un 50%

$$a_{ST} = 11.79 \quad \text{cm}^2/\text{cm}$$

Varilla que se utilizará 1/2 " Barra del No. 4

$$a_s = 1.27 \quad \text{cm}^2$$

$$s = \frac{100 \quad a_s}{a_{ST}} \qquad s = 10.75 \quad \text{cm} \leq 50 \text{ cm por norma}$$

Utilizar barras de: 1/2 " a cada 10.00 cm.

Diseño de Dados o Columnas.

Diseño de Columna (Nodo 2)

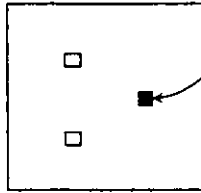


Fig. IV. 2. 4. Losa vista en planta

Acero de refuerzo por flexocompresión:

Constantes de cálculo:

Resistencia del concreto a compresión a los 28 días: $f'c = 350 \text{ Kg/cm}^2$
 Esfuerzo de fluencia del acero de refuerzo: $fy = 4200 \text{ Kg/cm}^2$

$$f^*c = 0.8 f'c \qquad f^*c = 280 \text{ Kg/cm}^2$$

$$f''c = [1.05 - (f^*c/1250)](f^*c) \qquad f''c = 231.28 \text{ Kg/cm}^2 \quad \text{Si } f^*c > 250 \text{ kg/cm}^2$$

$$f''c = 0.85 f^*c \qquad f''c = 238 \text{ Kg/cm}^2 \quad \text{Si } f^*c \leq 250 \text{ kg/cm}^2$$

Definitivo $f''c = 231.28 \text{ Kg/cm}^2$

Sección de la columna.

$h =$	70	cm
$b =$	70	cm
$r =$	5	cm
$d =$	65	cm

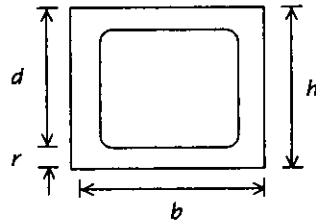


Fig. IV. 2. 5. Columna vista en planta

$$e = Mu / Pu \qquad e = 0.31 \text{ m.}$$

$$e / h = 0.45$$

$$d / h = 0.93$$

$$K = \frac{Pu}{F_R bh f''c} \quad K = 0.51$$

$$R = \frac{Mu}{F_R bh^2 f''c} \quad R = 0.23$$

Con los valores calculados entramos a diagramas de interacción, para obtener "q"

$$q = 0.38$$

Sustituyendo en:

$$\rho = q \frac{f''c}{fy} \quad \rho = 0.02093$$

$$\rho_{min} = \frac{20}{fy} = 0.00476 \leq 0.020925 \leq \rho_{max} = 0.06$$

Diseño del acero longitudinal de la columna.

$$\rho = 0.02093$$

$$As = \rho bh$$

$$As = 102.53 \text{ cm}^2$$

Varilla que se utilizará 1 3/8 " Barra del No. 11

Área de la barra: $as = 9.58 \text{ cm}^2$

No. de Varillas $= \frac{As}{as}$; No. de Varillas = 10.70 12

Utilizar 12 varillas de 1 3/8 "

Revisión por Cortante

$$\text{Si } \rho < 0.01 \text{ y } Pu \leq 0.7f''cAg + 2000As \quad V_{CR} = [F_R bd(0.2 + 30\rho)\sqrt{f''c}] \left[1 + 0.007 \left(\frac{Pu}{Ag} \right) \right]$$

$$\text{Si } \rho \geq 0.01 \text{ y } Pu \leq 0.7f''cAg + 2000As \quad V_{CR} = 0.5F_R bd \sqrt{f''c} \left[1 + 0.007 \left(\frac{Pu}{Ag} \right) \right]$$

$$A_s = 102.53 \text{ cm}^2 ; \quad A_g = 4900 \text{ cm}^2$$

$$P_u = 465414 \text{ kg} ; \quad 0.7f_c A_g + 2000A_s = 1165468 \text{ kg}$$

Si $\rho < 0.01$ y $P_u \leq 0.7f_c A_g + 2000A_s$ $V_{CR} = 83939.64 \text{ kg}$

Si $\rho \geq 0.01$ y $P_u \leq 0.7f_c A_g + 2000A_s$ $V_{CR} = 50702.88 \text{ kg}$

De los parámetros anteriores se obtiene que el cortante que resiste el concreto es: $V_{CR} = 50703 \text{ kg}$

Diseño del acero transversal (estribos) para la columna.

Los estribos serán de: $3/8$ " Estribos del No. 3

Separación de estribos.

S_2

La menor separación de:

1.- $S = \frac{850}{\sqrt{f_y}} \phi b$ $s = 45.81 \text{ cm}$

2.- $s = 48$ diámetros del estribo. $s = 45.72 \text{ cm}$

3.- $s = b_{\text{Columna}} / 2$ $s = 35.00 \text{ cm}$

4.- Por cortante $s = \frac{F_R A_v f_y d}{V_Z - V_{CR}}$ $s = -272.42 \text{ cm}$

$S_2 = 35 \text{ cm}$

S_1

La menor separación de:

1.- $S = \frac{425}{\sqrt{f_y}} \phi b$ $s = 22.90 \text{ cm}$

2.- $s = 20 \text{ cm}$. $s = 20.00 \text{ cm}$

3.- $s = b_{\text{Columna}} / 4$ $s = 17.50 \text{ cm}$

4.- Por cortante $s = \frac{F_R A_v f_y d}{V_Z - V_{CR}}$ $s = -272.42 \text{ cm}$

$S_1 = 17 \text{ cm}$

Hc

La mayor separación de:

- 1.- $s = \text{Dim. mayor } (C_1 \text{ ó } C_2)$ $s = 70.00 \text{ cm}$
 - 2.- $s = H_{\text{Libre}} / 6$ $s = 30.00 \text{ cm}$
 - 3.- $s = 60 \text{ cm.}$ $s = 60.00 \text{ cm}$
- $Hc = 70 \text{ cm}$

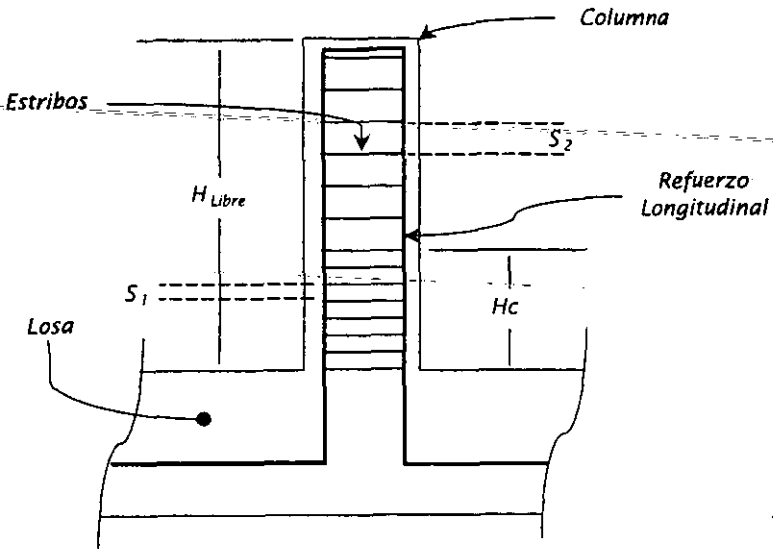


Fig. IV. 2. 6. Representación de la separación de estribos

El cortante que resiste el acero es:

$$V_{SR} = \frac{F_R A_v f_y d}{s} \quad V_{SR} = 8893 \text{ kg}$$

Cortante que actúa en la columna:

$$V_z = 49560 \text{ kg}$$

El cortante total que resiste el elemento es:

$$V_R = V_{CR} + V_{SR} \quad V_R = 59596 \text{ kg}$$

Como $V_{CR} > V_z$, entonces no hay problemas por cortante.

Por lo tanto se aceptan las dimensiones propuestas para la columna (dados).

Revisión de Columnas (Nodos 1 y 3)

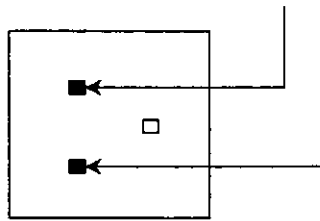


Fig. IV. 2. 7. Losa vista en planta

Acero de refuerzo por flexotensión:

Constantes de cálculo:

Resistencia del concreto a compresión a los 28 días: $f'c = 350 \text{ Kg/cm}^2$
 Esfuerzo de fluencia del acero de refuerzo: $fy = 4200 \text{ Kg/cm}^2$

$$f^*c = 0.8 f'c \qquad f^*c = 280 \text{ Kg/cm}^2$$

$$f''c = [1.05 - (f^*c/1250)](f^*c) \qquad f''c = 231.28 \text{ Kg/cm}^2 \quad \text{Si } f^*c > 250 \text{ kg/cm}^2$$

$$f''c = 0.85 f^*c \qquad f''c = 238 \text{ Kg/cm}^2 \quad \text{Si } f^*c \leq 250 \text{ kg/cm}^2$$

Definitivo $f''c = 231.28 \text{ Kg/cm}^2$

Sección de la columna.

$$\begin{aligned} h &= 70 \text{ cm} \\ b &= 70 \text{ cm} \\ r &= 5 \text{ cm} \\ d &= 65 \text{ cm} \end{aligned}$$

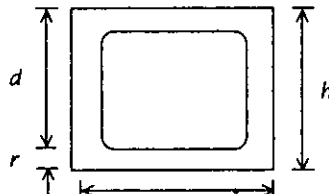


Fig. IV. 2. 8. Columna vista en planta

$$e = Mu / Pu \qquad e = 0.20 \text{ m.}$$

$$e / h = 0.29 \qquad d / h = 0.93$$

$$K = \frac{Pu}{F_R b h f''c} \qquad K = 0.18$$

$$R = \frac{Mu}{F_R b h^2 f''c} \qquad R = 0.05$$

Con los valores calculados entramos a diagramas de interacción, para obtener "q"

$$q = 0.3$$

Sustituyendo en:

$$\rho = q \frac{f'_c}{f_y} \quad \rho = 0.01652$$

$$\rho_{\min} = \frac{20}{f_y} = 0.00476 \leq 0.01652 \leq \rho_{\max} = 0.06$$

Diseño del acero longitudinal de la columna.

$$\rho = 0.01652$$

$$A_s = \rho bh \quad A_s = 80.95 \text{ cm}^2$$

Varilla que se utilizará $1 \text{ } 3/8 \text{ ''}$ Barra del No. 11

Área de la barra: $a_s = 9.58 \text{ cm}^2$

No. de Varillas $= \frac{A_s}{a_s}$; No. de Varillas = 8.45 10

Utilizar 10 varillas de $1 \text{ } 3/8 \text{ ''}$

Revisión por Cortante

$$\text{Si } \rho < 0.01 \text{ y } Pu \leq 0.7f'_cAg + 2000A_s \quad V_{CR} = [F_R bd(0.2 + 30\rho)\sqrt{f'_c}] \left[1 - 0.03 \left(\frac{Pu}{Ag} \right) \right]$$

$$\text{Si } \rho \geq 0.01 \text{ y } Pu \leq 0.7f'_cAg + 2000A_s \quad V_{CR} = 0.5F_R bd \sqrt{f'_c} \left[1 - 0.03 \left(\frac{Pu}{Ag} \right) \right]$$

$$A_s = 80.95 \text{ cm}^2 ; \quad Ag = 4900 \text{ cm}^2$$

$$Pu = 159974 \text{ kg} ; \quad 0.7f'_cAg + 2000A_s = 1122296 \text{ ton}$$

$$\text{Si } \rho < 0.01 \text{ y } Pu \leq 0.7f'_cAg + 2000A_s \quad V_{CR} = 747.1813 \text{ kg}$$

$$\text{Si } \rho \geq 0.01 \text{ y } Pu \leq 0.7f'_cAg + 2000A_s \quad V_{CR} = 626.3255 \text{ kg}$$

De los parámetros anteriores se obtiene que el cortante que resiste el concreto es:

$$V_{CR} = 626 \text{ kg}$$

Diseño del acero transversal (estribos) para la columna.

Los estribos serán de: 3/8 "

Estribos del No. 3

Separación de estribos.

S_2

La menor separación de:

1.- $S = \frac{850}{\sqrt{f_y}} \phi b$ $s = 45.81 \text{ cm}$

2.- $s = 48$ diámetros del estribo. $s = 45.72 \text{ cm}$

3.- $s = b_{\text{Columna}} / 2$ $s = 35.00 \text{ cm}$

4.- Por cortante $s = \frac{F_R A_v f_y d}{V_z - V_{CR}}$ $s = 25.84 \text{ cm}$

$S_2 = 25 \text{ cm}$

S_1

La menor separación de:

1.- $S = \frac{425}{\sqrt{f_y}} \phi b$ $s = 22.90 \text{ cm}$

2.- $s = 20 \text{ cm}$. $s = 20.00 \text{ cm}$

3.- $s = b_{\text{Columna}} / 4$ $s = 17.50 \text{ cm}$

4.- Por cortante $s = \frac{F_R A_v f_y d}{V_z - V_{CR}}$ $s = 25.84 \text{ cm}$

$S_1 = 17 \text{ cm}$

H_c

La mayor separación de:

1.- $s = \text{Dim. mayor } (C_1 \text{ ó } C_2)$ $s = 70.00 \text{ cm}$

2.- $s = H_{\text{Libre}} / 6$ $s = 30.00 \text{ cm}$

3.- $s = 60 \text{ cm}$. $s = 60.00 \text{ cm}$

$H_c = 70 \text{ cm}$

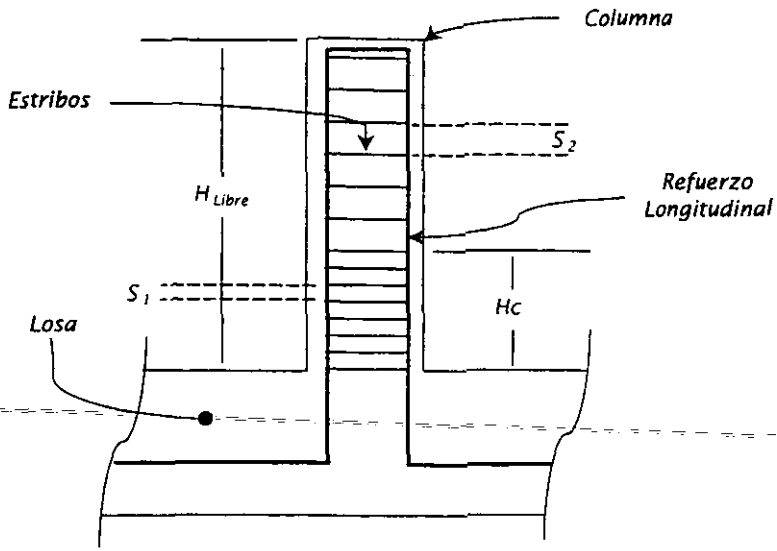


Fig. IV. 2. 9. Representación de la separación de estribos

El cortante que resiste el acero es:

$$V_{SR} = \frac{F_R A_v f_y d}{s} \quad V_{SR} = 12450 \text{ kg}$$

Cortante que actúa en la columna: $V_z = 12670 \text{ kg}$

El cortante total que resiste el elemento es:

$$V_R = V_{CR} + V_{SR} \quad V_R = 13076 \text{ kg}$$

Como $V_{CR} > V_z$, entonces no hay problemas por cortante.

Por lo tanto se aceptan las dimensiones propuestas para la columna (dados).

Revisión de Presiones en la Cimentación

Revisión de losa de cimentación en dirección "Z"

De la página 34 de resultados del STAAD obtenemos:

Combinación de Carga No. 9
CM+CV - Viento en "Z"

<p>Nodo 1</p> <p>Compresión = 233589 kg</p> <p>Vz = 16007 kg</p> <p>Mx = 1773 kg-m</p>	<p>Nodo 2</p> <p>Tensión = -334937 kg</p> <p>Vz = 36078 kg</p> <p>Mx = -4390 kg-m</p>
---	--

<p>Nodo 3</p> <p>Compresión = 233590 kg</p> <p>Vz = 15635 kg</p> <p>Mx = 1416 kg-m</p>

Parámetros

Capacidad admisible del terreno	$\sigma_{adm} = 16000 \text{ kg/m}^2$
Peso volumétrico del concreto	$\gamma_{concreto} = 2400 \text{ kg/m}^3$
Peso volumétrico del relleno	$\gamma_{relleno} = 1600 \text{ Kg/m}^3$
Factor de seguridad	F.S. = 1.1

Dimensiones Propuestas

B Losa = 13 m.	h Peralte = 1.0 m.
L Losa = 13 m.	B Columna = 0.7 m
Df = 2.5 m.	L Columna = 0.7 m

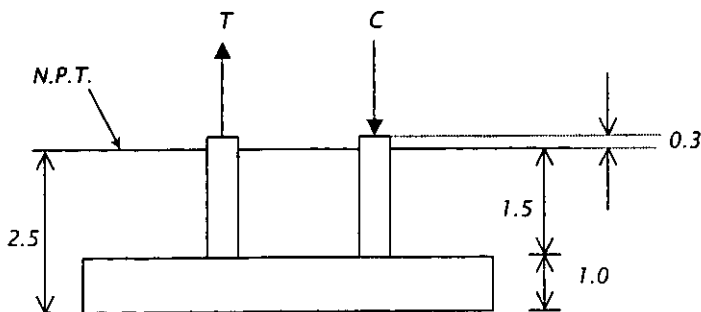


Fig. IV. 2. 10. Losa en elevación (Unidades en metros).

Fuerzas de Diseño

Nodo 1	Nodo 2
Compresión = 233589 kg	Tensión = 334937 kg
$Mx_{total} = 43046 \text{ kg}\cdot\text{m}$	$Mx_{total} = 105408 \text{ kg}\cdot\text{m}$
Nodo 3	
Compresión = 233590 kg	
$Mx_{total} = 42363 \text{ kg}\cdot\text{m}$	

Momento de Volteo

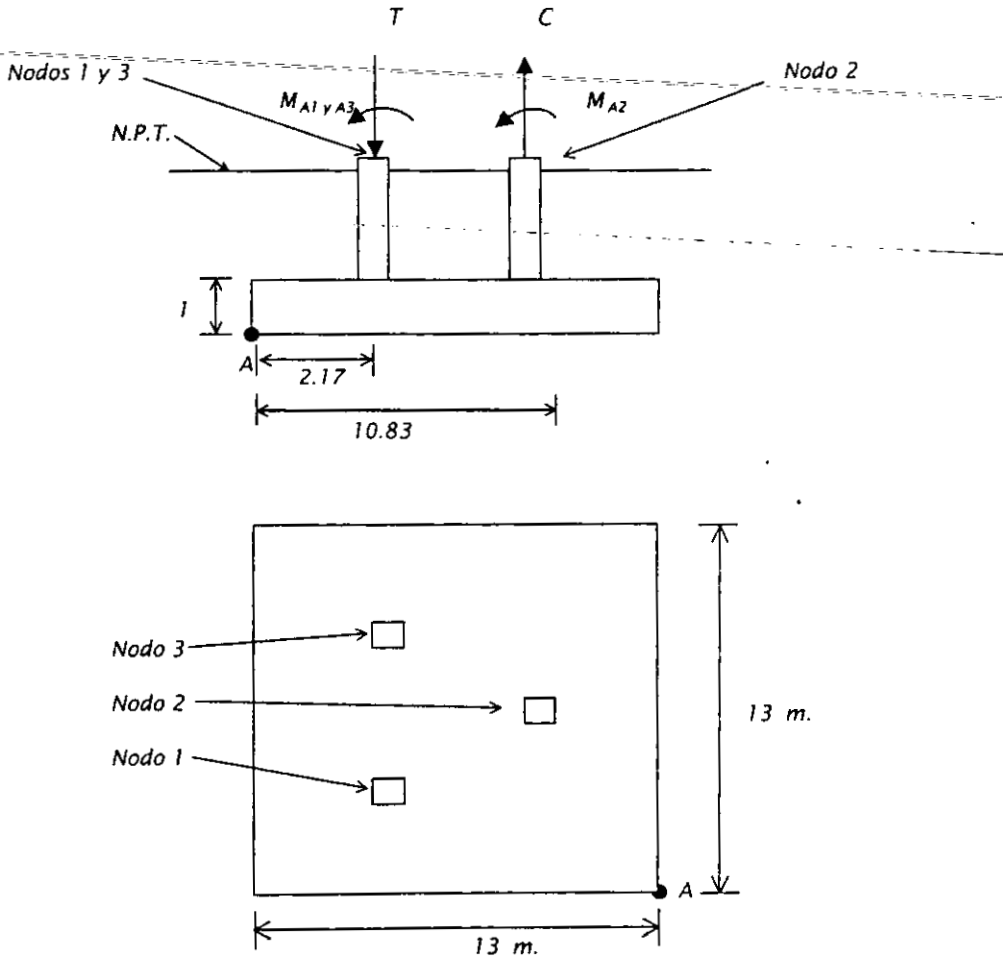


Fig. IV. 2. 11. Losa en planta y elevación (Unidades en metros).

Suma de momentos respecto de "A"

$$M_t = 141365 \text{ kg-m}$$

$$M_v = (M_t)(F.S.)$$

$$M_v = 155502 \text{ kg-m}$$

Esfuerzo ejercido

$$\text{Compresión Nudo 1} = 233589 \text{ kg}$$

$$\text{Tensión Nudo 2} = 334937 \text{ kg}$$

$$\text{Compresión Nudo 3} = 233590 \text{ kg}$$

$$\text{Área de dados} = 1.47 \text{ m}^2$$

$$\text{Volumen de dados} = 2.646 \text{ m}^3$$

$$\text{Área de losa} = 169 \text{ m}^2$$

$$\text{Volumen losa} = 169 \text{ m}^3$$

$$\text{Volumen de concreto} = 171.646 \text{ m}^3$$

$$\text{Peso de losa y dados} = 411950.4 \text{ kg}$$

$$\text{Área de relleno} = 167.53 \text{ m}^2$$

$$\text{Volumen del relleno} = 251.295 \text{ m}^3$$

$$\text{Peso del relleno} = 402072 \text{ kg}$$

Fuerza Resultante

$$P = 946265 \text{ kg}$$

$$P_{\text{Total}} = (P)(F.S.)$$

$$P_{\text{Total}} = 1040892 \text{ kg}$$

Cálculo de la excentricidad

$$e = \frac{M_v}{P_{\text{Total}}}$$

$$e = 0.15 \text{ m.}$$

$$a = 6.35 \text{ m.}$$

$$B/3 = 4.33 \text{ m.}$$

$$B/6 = 2.17 \text{ m.}$$

$$\text{Si } e = 0 \quad \sigma = \frac{P_{\text{Total}}}{A_{\text{Losa}}} \quad \dots(1)$$

$$\text{Si } e < B/6 \text{ ó } a > B/3 \quad \sigma = \left(\frac{P_{\text{Total}}}{A_{\text{Losa}}} \right) \left[1 \pm \left(\frac{6e}{B} \right) \right] \quad \dots(2)$$

$$\text{Si } e = B/6 \text{ ó } a = B/3 \quad \sigma = 2 \left(\frac{P_{\text{Total}}}{A_{\text{Losa}}} \right) \quad \dots(3)$$

$$\text{Si } e > B/6 \quad \sigma = \frac{4P_{\text{Total}}}{[3L(B-2e)]} \quad \dots(4)$$

Se empleará la ecuación que conforme a los parámetros calculados sea la indicada

$$\begin{array}{llll} \sigma_{\text{máx}} = & 6584 & \text{kg/m}^2 & < & \sigma_{\text{admisible}} = & 16000 & \text{kg/m}^2 \\ \sigma_{\text{mín}} = & 5734 & \text{kg/m}^2 & < & \sigma_{\text{admisible}} = & 16000 & \text{kg/m}^2 \end{array}$$

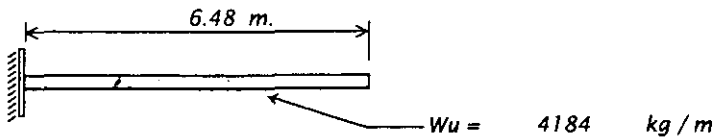
Por lo tanto se acepta.

Diseño de Losa de Cimentación

Esfuerzo empleado para el diseño: Esf_D

$$Esf_D = \sigma_{\text{máx}} \cdot esf_{\text{relleno}}$$

$$Esf_D = 4184 \text{ kg/m}^2$$



"L" para la sección más crítica:

6.48 m

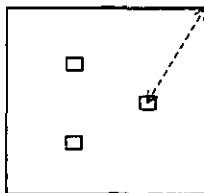


Fig. IV. 2. 12. Losa vista en planta

$$Mu = \frac{Wu(L)^2}{2} \quad ; \quad Vu = Wu(L)$$

$$Mu = 87938 \text{ kg-m} \quad ; \quad Vu = 27126 \text{ kg}$$

Constantes de cálculo:

Resistencia del concreto a compresión a los 28 días: $f'c = 350 \text{ Kg/cm}^2$
 Esfuerzo de fluencia del acero de refuerzo: $fy = 4200 \text{ Kg/cm}^2$

$$f^*c = 0.8 f'c \quad f^*c = 280 \text{ Kg/cm}^2$$

$$f''c = [1.05 - (f^*c/1250)](f^*c) \quad f''c = 231.28 \text{ Kg/cm}^2 \quad \text{Si } f^*c > 250 \text{ kg/cm}^2$$

$$f''c = 0.85 f^*c \quad f''c = 238 \text{ Kg/cm}^2 \quad \text{Si } f^*c \leq 250 \text{ kg/cm}^2$$

Definitivo $f''c = 231.28 \text{ Kg/cm}^2$

Porcentaje balanceado de acero para losas de concreto: ρb

$$\rho b = \frac{f''c}{fy} \frac{4800}{fy + 6000} \quad \rho b = 0.0259$$

$$\rho_{m\acute{a}x} = 0.75 \rho b \quad \rho_{m\acute{a}x} = 0.0194$$

$$\rho_{m\acute{i}n} = \frac{0.7 \sqrt{f'c}}{fy} \quad \rho_{m\acute{i}n} = 0.0031$$

Se propondrá $\rho = 0.0045$ de porcentaje para cálculo

$$q = \frac{\rho fy}{f'c}$$

$$q = 0.0541$$

Revisión del peralte por flexión.

$$df = \sqrt{\frac{Mu}{F_R f'' c b q (1 - 0.5q)}}$$

$$df = 89.57 \text{ cm.}$$

Peralte total propuesto	$h =$	100	cm.
Recubrimiento libre	$r =$	5	cm.
Peralte efectivo	$dp =$	95	cm.

Como: $dp = 95 \text{ cm.} > df = 89.57 \text{ cm.}$

entonces el peralte propuesto es correcto. (ok).

Revisión por cortante como viga ancha.

$$Vud = (L - d)Wu$$

$$Vud = 23152 \text{ kg}$$

Capacidad del concreto para tomar cortante.

$$\text{Si } \rho < 0.01 \quad V_{CR} = F_R b d (0.2 + 30\rho) \sqrt{f^* c}$$

$$V_{CR} = 42643 \text{ kg}$$

$$\text{Si } \rho \geq 0.01 \quad V_{CR} = 0.5 F_R b d \sqrt{f^* c}$$

$$V_{CR} = 63586 \text{ kg}$$

Debido al porcentaje de acero propuesto se toma:

$$V_{CR} = 42643 \text{ kg}$$

Como el peralte es mayor de 70 cm, se reducirá en 30% la capacidad del cortante:

$$\text{Definitivo} \quad V_{CR} = 29850 \text{ kg}$$

Como: $V_{CR} > Vud$ entonces el peralte es correcto. (ok).

Revisión por penetración.

$$\begin{aligned}C1 &= 70 \text{ cm.} \\C2 &= 70 \text{ cm.} \\dp &= 95 \text{ cm.} \\C1+dp &= 165 \text{ cm.} \\C2+dp &= 165 \text{ cm.}\end{aligned}$$

$$\alpha = 1 - \frac{1}{1 + 0.67 \sqrt{(C1+dp)/(C2+dp)}}$$

$$\alpha = 0.401198$$

Momento que toma la losa:

$$\alpha Mx = 4228941.95 \text{ kg-cm}$$

$$V_{Total} = P - [(C1+dp)(C2+dp)(Esf_D)]$$

$$V_{Total} = 594887 \text{ kg}$$

$$Ac = 2[(C1+dp)+(C2+dp)](dp)$$

$$Ac = 62700 \text{ cm}^2$$

$$Jc = \frac{dp(C1+dp)^3}{6} + \frac{(C1+dp)dp^3}{6} + \frac{dp(C2+dp)(C1+dp)^2}{2}$$

$$Jc = 71125312.5 + 23577813 + 213375938$$

$$Jc = 308079063 \text{ cm}^4$$

Sustituyendo valores en la fórmula de la escuadría.

$$Vu = \frac{V_{Total}}{Ac} \pm \frac{\alpha Mx C_{AB}}{Jc}$$

$$Vu_{m\acute{a}x.} = 10.62 \text{ Kg/cm}^2$$

$$Vu_{m\acute{i}n.} = 8.36 \text{ Kg/cm}^2$$

El esfuerzo cortante que resiste el concreto por penetración es:

$$Vcr = F_R \sqrt{f^* c} \quad Vcr = 13.39 \text{ Kg/cm}^2$$

Como $Vcr > Vu_{m\acute{a}x.}$ no existe penetración de la columna en la losa de cimentación.

Por lo tanto se aceptan las dimensiones propuestas para la cimentación.

Acero de refuerzo por flexión:

$$A_s = \rho \quad b d \qquad A_s = 42.85 \quad \text{cm}^2$$

Separación de varillas para la losa en su cara inferior.

Varilla que se utilizará 1 " Barra del No. 8

$$a_s = 5.07 \quad \text{cm}^2$$

No. de barras: 1

$$s = \frac{100 \quad a_s}{A_s} \qquad s = 11.82 \quad \text{cm}$$

Utilizar paquetes de: 1 barras

Con varillas de: 1 " a cada 12 cm.

Acero requerido por Cambios Volumétricos

Separación de varillas para la losa en su cara superior.

$$a_{ST} = \frac{660 \quad x_1}{f_y (x_1 + 100)} \qquad a_{ST} = 7.86 \quad \text{cm}^2/\text{cm}$$

Por ser un elemento que está en contacto con el terreno, el área será aumentada en un 50%

$$a_{ST} = 11.79 \quad \text{cm}^2/\text{cm}$$

Varilla que se utilizará 1/2 " Barra del No. 4

$$a_s = 1.27 \quad \text{cm}^2$$

$$s = \frac{100 \quad a_s}{a_{ST}} \qquad s = 10.75 \quad \text{cm} \leq 50 \text{ cm por norma}$$

Utilizar barras de: 1/2 " a cada 10 cm.

Diseño de Dados o Columnas.

Diseño de Columna (Nodo 2)

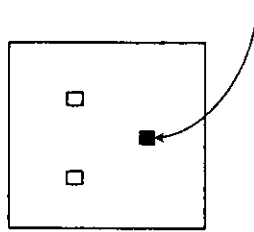


Fig. IV. 2. 13. Losa vista en planta

Acero de refuerzo por flexocompresión:

Constantes de cálculo:

Resistencia del concreto a compresión a los 28 días: $f'c = 350 \text{ Kg/cm}^2$
 Esfuerzo de fluencia del acero de refuerzo: $fy = 4200 \text{ Kg/cm}^2$

$$f^*c = 0.8 f'c \qquad f^*c = 280 \text{ Kg/cm}^2$$

$$f''c = [1.05 - (f^*c/1250)](f^*c) \qquad f''c = 231.28 \text{ Kg/cm}^2 \quad \text{Si } f^*c > 250 \text{ kg/cm}^2$$

$$f''c = 0.85 f^*c \qquad f''c = 238 \text{ Kg/cm}^2 \quad \text{Si } f^*c \leq 250 \text{ kg/cm}^2$$

Definitivo $f''c = 231.28 \text{ Kg/cm}^2$

Sección de la columna.

$$h = 70 \text{ cm}$$

$$b = 70 \text{ cm}$$

$$r = 5 \text{ cm}$$

$$d = 65 \text{ cm}$$

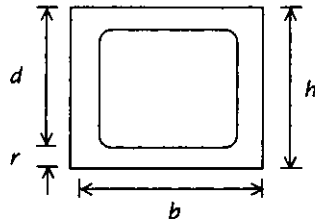


Fig. IV. 2. 14. Columna vista en planta

$$e = Mu / Pu \qquad e = 0.18 \text{ m.}$$

$$e / h = 0.26$$

$$d / h = 0.93$$

$$K = \frac{Pu}{F_R bh f''c} \quad K = 0.28$$

$$R = \frac{Mu}{F_R bh^2 f''c} \quad R = 0.07$$

Con los valores calculados entramos a diagramas de interacción, para obtener "q"

$$q = 0$$

Sustituyendo en:

$$\rho = q \frac{f''c}{f_y} \quad \rho = 0.00000$$

$$\rho_{min} = \frac{20}{f_y} = 0.00476 \leq 0.004762 \leq \rho_{max} = 0.06$$

Diseño del acero longitudinal de la columna.

$$\rho = 0.00476$$

$$As = \rho bh \quad As = 23.33 \text{ cm}^2$$

Varilla que se utilizará 1 3/8 " Barra del No. 11

Área de la barra: $as = 9.58 \text{ cm}^2$

No. de Varillas = $\frac{As}{as}$; No. de Varillas = 2.44 4

Utilizar 4 varillas de 1 3/8 "

Revisión por Cortante

$$\text{Si } \rho < 0.01 \text{ y } Pu \leq 0.7f''cAg + 2000As \quad V_{CR} = [F_R bd(0.2 + 30\rho)\sqrt{f''c}] \left[1 + 0.007 \left(\frac{Pu}{Ag} \right) \right]$$

$$\text{Si } \rho \geq 0.01 \text{ y } Pu \leq 0.7f''cAg + 2000As \quad V_{CR} = 0.5F_R bd \sqrt{f''c} \left[1 + 0.007 \left(\frac{Pu}{Ag} \right) \right]$$

$$A_s = 23.33 \text{ cm}^2 ; \quad A_g = 4900 \text{ cm}^2$$

$$P_u = 256948 \text{ kg} ; \quad 0.7f^*cA_g + 2000A_s = 1007067 \text{ kg}$$

$$\text{Si } \rho < 0.01 \text{ y } P_u \leq 0.7f^*cA_g + 2000A_s \quad V_{CR} = 22600.94 \text{ kg}$$

$$\text{Si } \rho \geq 0.01 \text{ y } P_u \leq 0.7f^*cA_g + 2000A_s \quad V_{CR} = 41633.31 \text{ kg}$$

De los parámetros anteriores se obtiene que el cortante que resiste el concreto es: $V_{CR} = 22601 \text{ kg}$

Diseño del acero transversal (estribos) para la columna.

Los estribos serán de: $3/8$ " Estribos del No. 3

Separación de estribos.

S_2

La menor separación de:

$$1.- \quad S = \frac{850}{\sqrt{f_y}} \phi b \quad s = 45.81 \text{ cm}$$

$$2.- \quad s = 48 \text{ diámetros del estribo.} \quad s = 45.72 \text{ cm}$$

$$3.- \quad s = b_{\text{Columna}} / 2 \quad s = 35.00 \text{ cm}$$

$$4.- \quad \text{Por cortante} \quad s = \frac{F_R A_v f_y d}{V_Z - V_{CR}} \quad s = -62.33 \text{ cm}$$

$$S_2 = 35 \text{ cm}$$

S_1

La menor separación de:

$$1.- \quad S = \frac{425}{\sqrt{f_y}} \phi b \quad s = 22.90 \text{ cm}$$

$$2.- \quad s = 20 \text{ cm.} \quad s = 20.00 \text{ cm}$$

$$3.- \quad s = b_{\text{Columna}} / 4 \quad s = 17.50 \text{ cm}$$

$$4.- \quad \text{Por cortante} \quad s = \frac{F_R A_v f_y d}{V_Z - V_{CR}} \quad s = -62.33 \text{ cm}$$

$$S_1 = 17 \text{ cm}$$

Hc

La mayor separación de:

- 1.- $s = \text{Dim. mayor } (C_1 \text{ ó } C_2)$ $s = 70.00 \text{ cm}$
 - 2.- $s = H_{\text{Libre}} / 6$ $s = 30.00 \text{ cm}$
 - 3.- $s = 60 \text{ cm.}$ $s = 60.00 \text{ cm}$
- $Hc = 70 \text{ cm}$

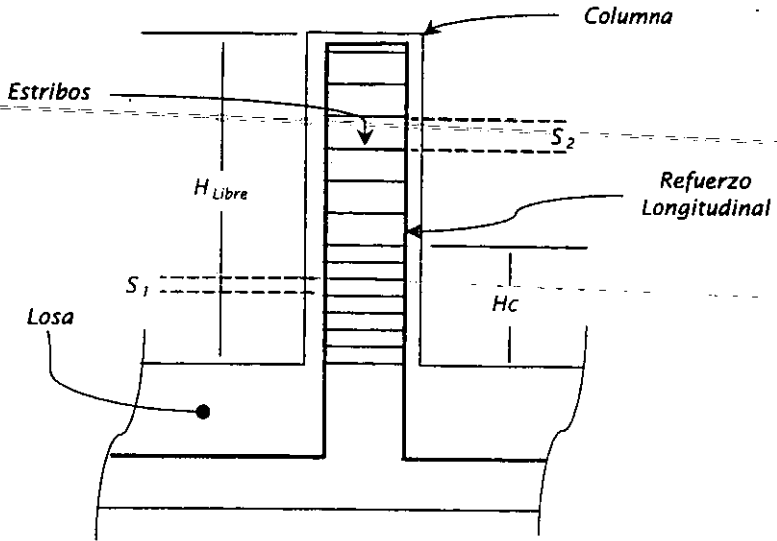


Fig. IV. 2. 15. Esquema representativo de la separación de estribos

El cortante que resiste el acero es:

$$V_{SR} = \frac{F_R A_v f_y d}{s} \quad V_{SR} = 8893 \text{ kg}$$

Cortante que actúa en la columna:

$$V_z = 17608 \text{ kg}$$

El cortante total que resiste el elemento es:

$$V_R = V_{CR} + V_{SR} \quad V_R = 31494 \text{ kg}$$

Como $V_{CR} > V_z$, entonces no hay problemas por cortante.

Por lo tanto se aceptan las dimensiones propuestas para la columna (dados).

Revisión de Columnas (Nodos 1 y 3)

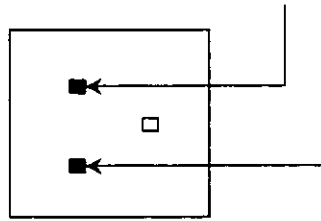


Fig. Iv. 2. 16. Losa vista en planta

Acero de refuerzo por flexotensión:

Constantes de cálculo:

Resistencia del concreto a compresión a los 28 días: $f'c = 350 \text{ Kg/cm}^2$
 Esfuerzo de fluencia del acero de refuerzo: $fy = 4200 \text{ Kg/cm}^2$

$f^*c = 0.8 f'c$	$f^*c = 280 \text{ Kg/cm}^2$	
$f''c = [1.05 \cdot (f^*c/1250)](f^*c)$	$f''c = 231.28 \text{ Kg/cm}^2$	Si $f^*c > 250 \text{ kg/cm}^2$
$f''c = 0.85 f^*c$	$f''c = 238 \text{ Kg/cm}^2$	Si $f^*c \leq 250 \text{ kg/cm}^2$
Definitivo	$f''c = 231.28 \text{ Kg/cm}^2$	

Sección de la columna.

$h = 70 \text{ cm}$
 $b = 70 \text{ cm}$
 $r = 5 \text{ cm}$
 $d = 65 \text{ cm}$

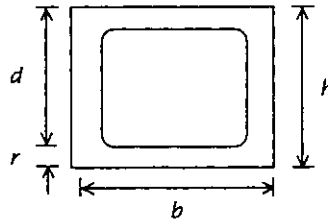


Fig. Iv. 2. 17. Columna vista en planta

$e = Mu / Pu \quad e = 0.31 \text{ m.}$

$e / h = 0.45 \quad d / h = 0.93$

$K = \frac{Pu}{F_R b h f''c} \quad K = 0.41$

$R = \frac{Mu}{F_R b h^2 f''c} \quad R = 0.18$

Con los valores calculados entramos a diagramas de interacción, para obtener "q"

$q = 0.79$

Sustituyendo en:

$$\rho = q \frac{f''c}{f_y} \quad \rho = 0.04350$$

$$\rho_{\min} = \frac{20}{f_y} = 0.00476 \leq 0.043503 \leq \rho_{\max} = 0.06$$

Diseño del acero longitudinal de la columna.

$$\rho = 0.04350$$

$$A_s = \rho b h \quad A_s = 213.16 \text{ cm}^2$$

Varilla que se utilizará 1 3/8 " Barra del No. 11

Área de la barra: $a_s = 9.58 \text{ cm}^2$

No. de Varillas = $\frac{A_s}{a_s}$; No. de Varillas = 22.25 24

Utilizar 24 varillas de 1 3/8 "

Revisión por Cortante

$$\text{Si } \rho < 0.01 \text{ y } P_u \leq 0.7f''cA_g + 2000A_s \quad V_{CR} = [F_R b d (0.2 + 30\rho) \sqrt{f''c}] \left[1 - 0.03 \left(\frac{P_u}{A_g} \right) \right]$$

$$\text{Si } \rho \geq 0.01 \text{ y } P_u \leq 0.7f''cA_g + 2000A_s \quad V_{CR} = 0.5 F_R b d \sqrt{f''c} \left[1 - 0.03 \left(\frac{P_u}{A_g} \right) \right]$$

$$A_s = 213.16 \text{ cm}^2 ; \quad A_g = 4900 \text{ cm}^2$$

$$P_u = 368431 \text{ kg} ; \quad 0.7f''cA_g + 2000A_s = 1386726 \text{ ton}$$

$$\text{Si } \rho < 0.01 \text{ y } P_u \leq 0.7f''cA_g + 2000A_s \quad V_{CR} = -48568.8 \text{ kg}$$

$$\text{Si } \rho \geq 0.01 \text{ y } P_u \leq 0.7f''cA_g + 2000A_s \quad V_{CR} = -38241.6 \text{ kg}$$

De los parámetros anteriores se obtiene que el cortante que resiste el concreto es:

$$V_{CR} = 0 \text{ kg}$$

Diseño del acero transversal (estribos) para la columna.

Los estribos serán de: 3/8 "

Estribos del No. 3

Separación de estribos.

S_2

La menor separación de:

1.- $S = \frac{850}{\sqrt{f_y}} \phi b$ $s = 45.81 \text{ cm}$

2.- $s = 48 \text{ diámetros del estribo.}$ $s = 45.72 \text{ cm}$

3.- $s = b_{\text{Columna}} / 2$ $s = 35.00 \text{ cm}$

4.- Por cortante $s = \frac{F_R A_v f_y d}{V_z - V_{CR}}$ $s = 7.84 \text{ cm}$

$S_2 = 7 \text{ cm}$

S_1

La menor separación de:

1.- $S = \frac{425}{\sqrt{f_y}} \phi b$ $s = 22.90 \text{ cm}$

2.- $s = 20 \text{ cm.}$ $s = 20.00 \text{ cm}$

3.- $s = b_{\text{Columna}} / 4$ $s = 17.50 \text{ cm}$

4.- Por cortante $s = \frac{F_R A_v f_y d}{V_z - V_{CR}}$ $s = 7.84 \text{ cm}$

$S_1 = 7 \text{ cm}$

H_c

La mayor separación de:

1.- $s = \text{Dim. mayor } (C_1 \text{ ó } C_2)$ $s = 70.00 \text{ cm}$

2.- $s = H_{\text{Libre}} / 6$ $s = 30.00 \text{ cm}$

3.- $s = 60 \text{ cm.}$ $s = 60.00 \text{ cm}$

$H_c = 70 \text{ cm}$

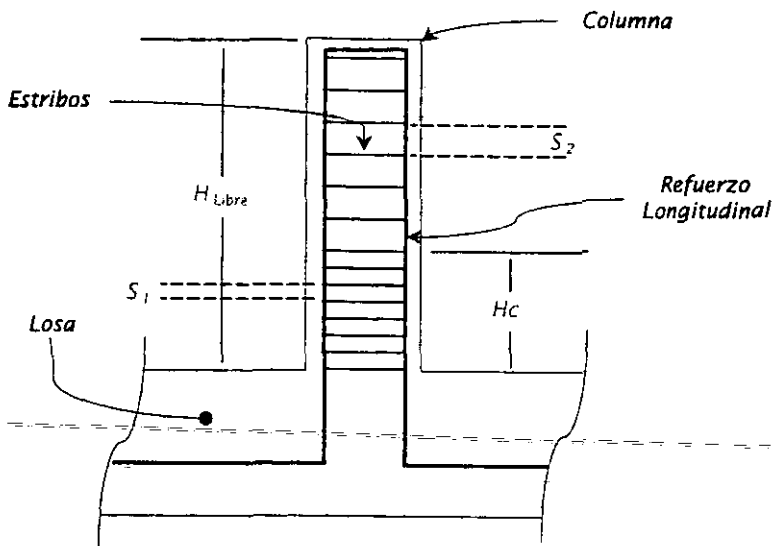


Fig. IV. 2. 18. Esquema representativo de la separación de estribos

El cortante que resiste el acero es:

$$V_{SR} = \frac{F_R A_v f_y d}{S} \quad V_{SR} = 44464 \text{ kg}$$

Cortante que actúa en la columna:

$$V_z = 39686 \text{ kg}$$

El cortante total que resiste el elemento es:

$$V_R = V_{CR} + V_{SR} \quad V_R = 44464 \text{ kg}$$

Como $V_{CR} > V_z$, entonces no hay problemas por cortante.

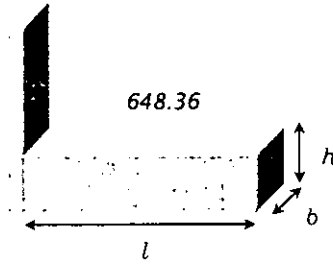
Por lo tanto se aceptan las dimensiones propuestas para la columna (dados).

Revisión de Flechas

Datos:

$$\begin{array}{llll} f'c = & 350 & \text{kg/cm}^2 & p' = & 0.00451 \\ b = & 100 & \text{cm} & w = & 45.27 \text{ kg/cm} \\ h = & 100 & \text{cm} & & \end{array}$$

Sección más crítica: $l = 648.36 \text{ cm}$



$$Ec = 14000 \sqrt{f'c}$$

$$Ec = 261916 \text{ kg/cm}^2$$

$$I = \frac{b h^3}{12}$$

$$I = 8333333 \text{ cm}^4$$

$$\Delta i = \frac{w l^4}{8 Ec I}$$

$$\Delta i = 0.46 \text{ cm}$$

$$\Delta d = \frac{2}{1 + 50 p'} \Delta i$$

$$\Delta d = 0.75 \text{ cm}$$

$$\Delta_{TOTAL} = \Delta i + \Delta d$$

$$\Delta_{TOTAL} = 1.2 \text{ cm}$$

Desplazamiento máximo permitido.

$$\Delta_{MÁX} = \frac{2l}{240} + 0.5$$

$$\Delta_{MÁX} = 5.9 \text{ cm}$$

Como: $\Delta_{MÁX} \gg \Delta_{TOTAL}$ entonces el dimensionamiento es correcto.

ARMADO DE LOSA Y COLUMNAS

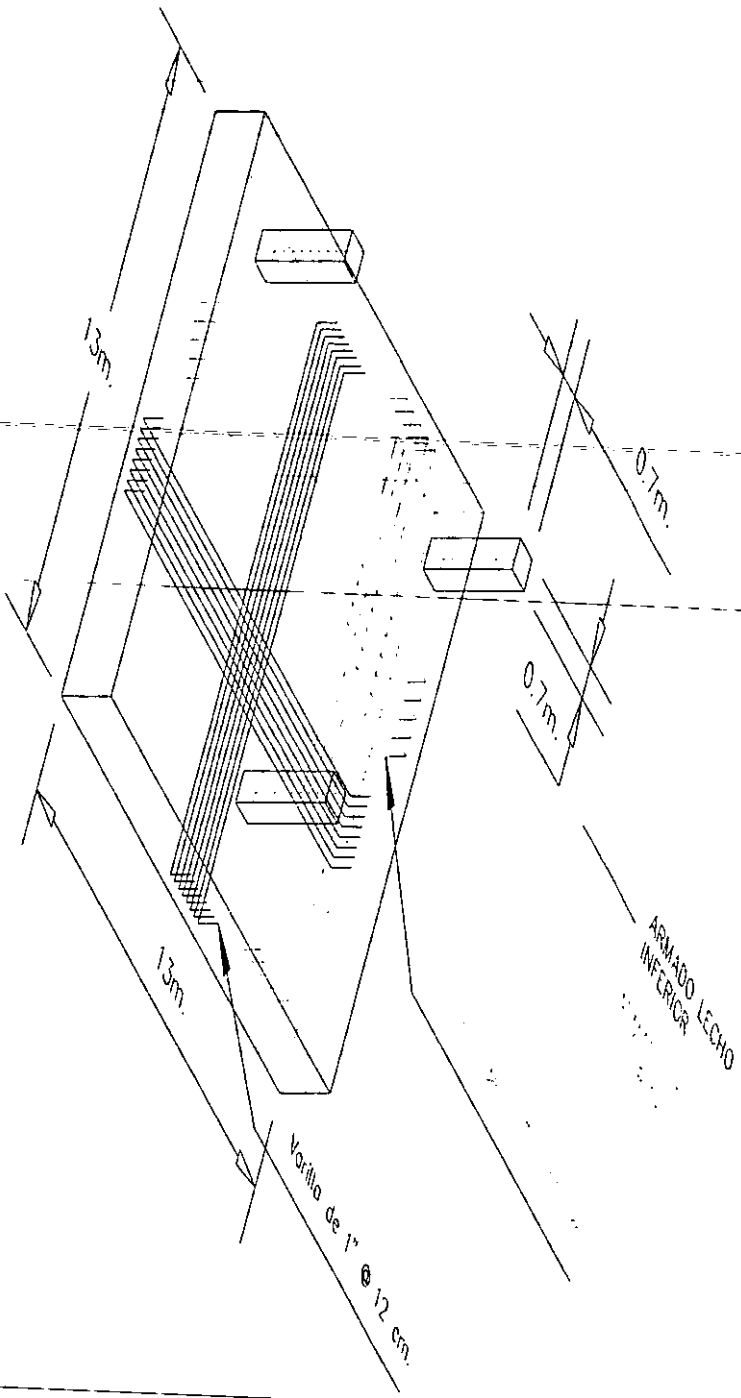


Fig. IV. 2. 19. Armado de Losa.

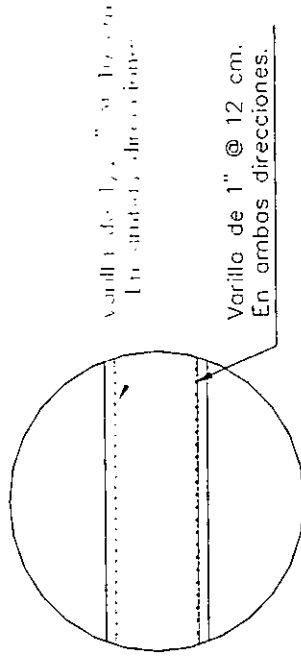
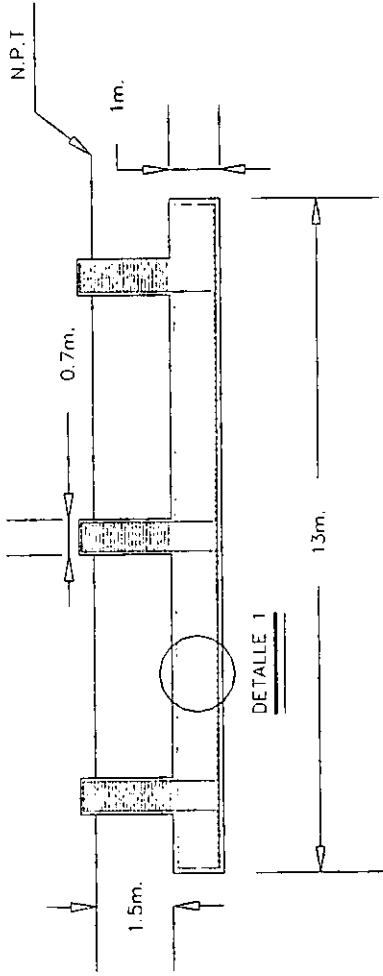
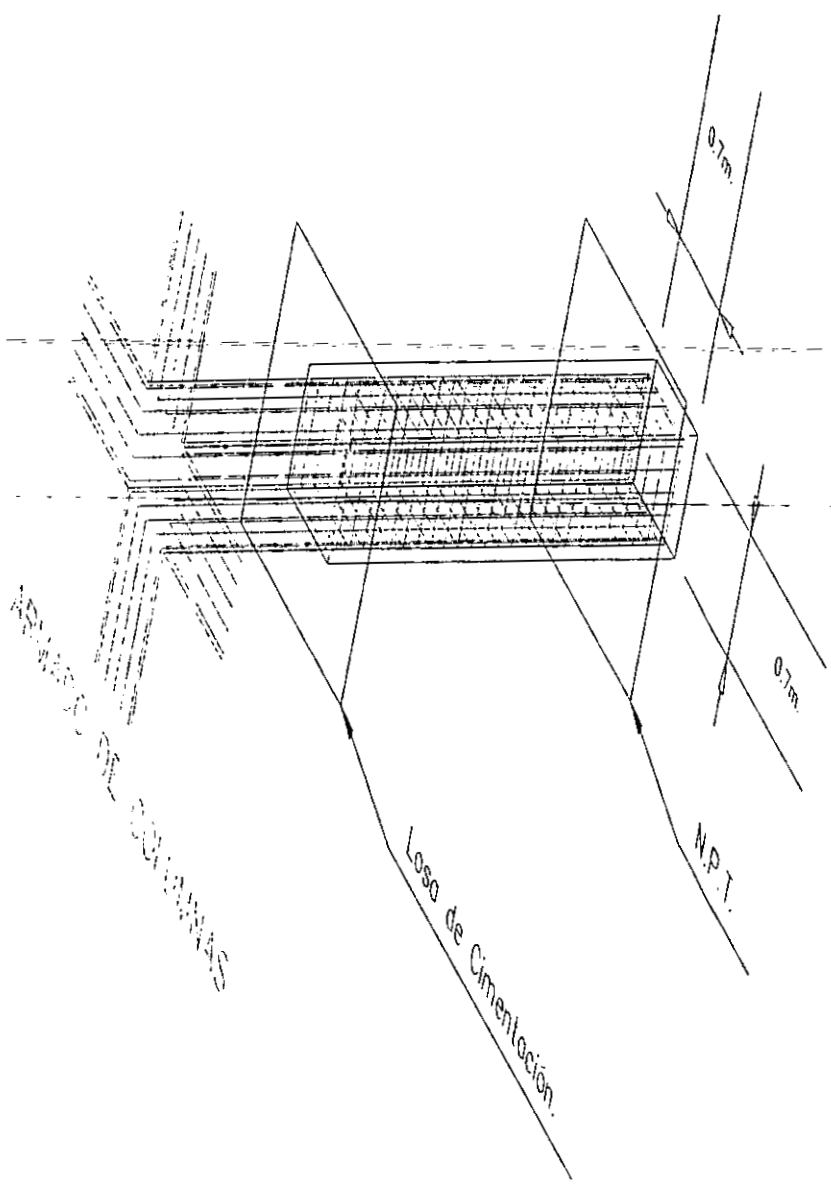
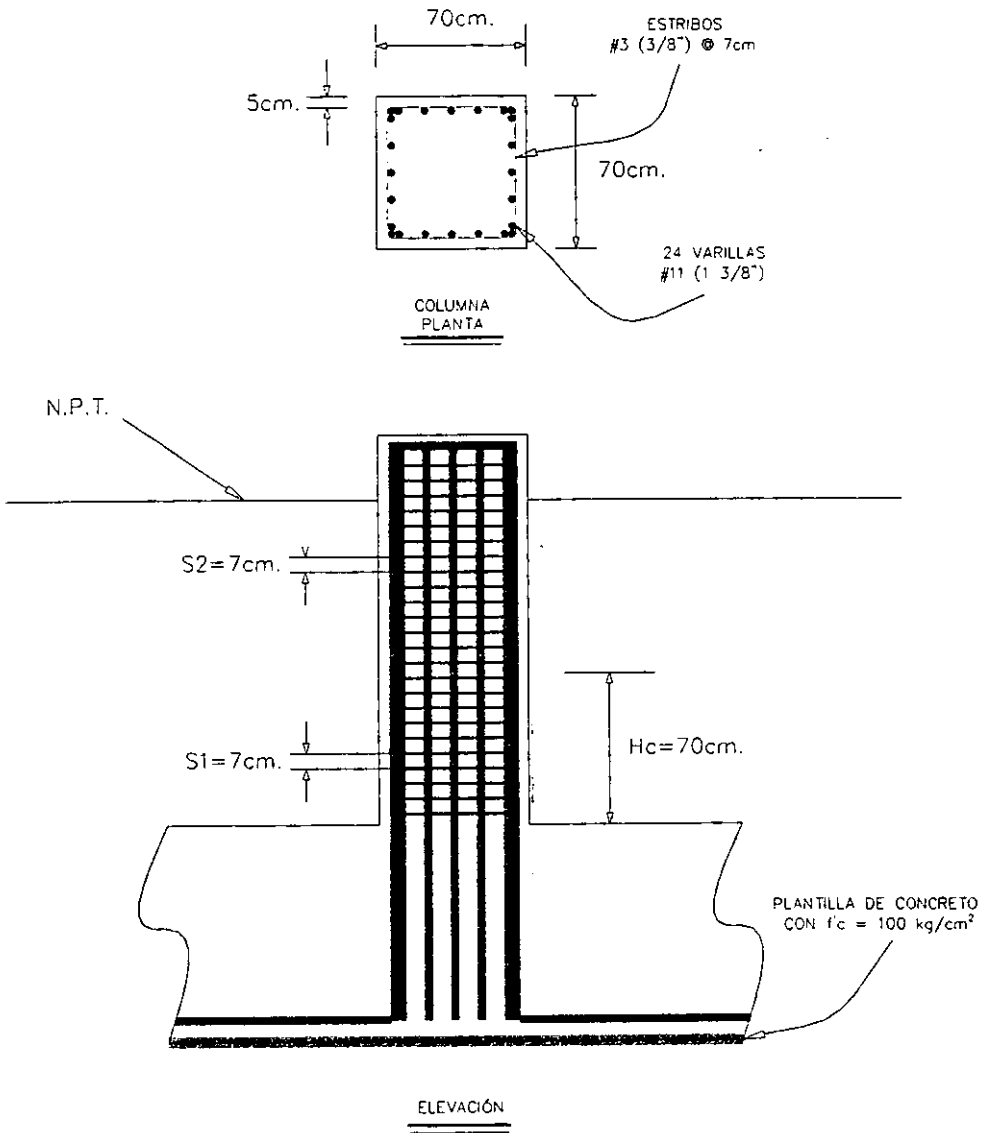


Fig. IV. 20. Armado de Losa y Columnas.

Fig. IV. 2. 21. Armado de Columnas.





ARMADO DE COLUMNAS A ESCALA

Fig. IV. 2. 22. Armado de Columnas.

V. Instalación de la estructura.

V. 1. Tolerancias en el montaje.

El montaje debe efectuarse con equipo apropiado, que ofrezca la mayor seguridad posible. Durante la carga, transporte y descarga del material, y durante el montaje, se adoptarán las precauciones necesarias para no producir deformaciones ni esfuerzos excesivos. Si a pesar de ello algunas de las piezas se maltratan y deforman, deben ser enderezadas o repuestas, según el caso, antes de montarlas.

Plomada: La distancia horizontal medida entre el punto inferior de la vertical en cuestión y un punto cualquiera en la misma horizontal, no excederá del 0.25 % de la distancia vertical tomada entre dos elevaciones.

Torsión: La torsión (rotación angular en el plano horizontal) entre dos puntos de elevación no excederá de 0.5° en una altura de 3 m (10 pies), por otro lado, la torsión total en la estructura no deberá exceder de 5° .



Figura: V. 1. 1. Montaje de Torre Autosoportada.

Existe un método para torres triangulares mediante el cual se puede determinar el desplazamiento de la estructura, este método se describe a continuación:

Se requiere de tres tránsitos, uno en cada larguero, esto es indispensable para la obtención de las siguientes longitudes:

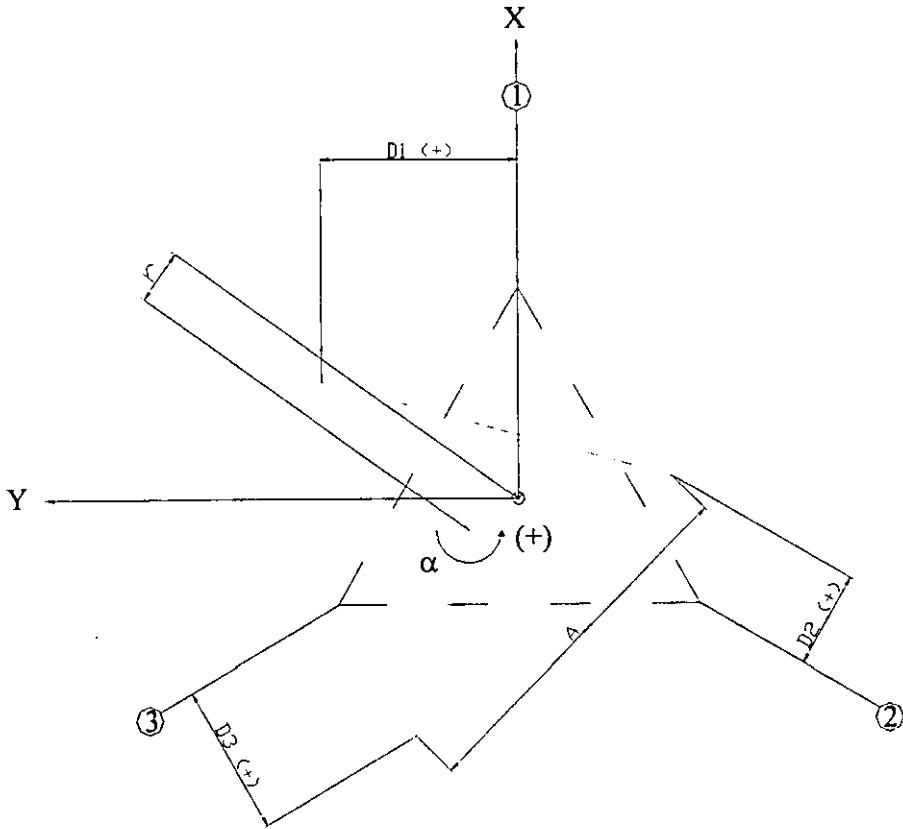


Figura: V. 1. 2. Determinación de torsión y plomada en torres triangulares.

Las dimensiones a obtener son: $D1$, $D2$, $D3$ y A .

Con estos parámetros y aplicando las siguientes ecuaciones, obtenemos los desplazamientos en torsión y plomada.

$$d = \frac{(D1 + D2 + D3)}{3}$$

$$e = \frac{d\sqrt{3}}{A}$$

$$\alpha = \arcsen(e)$$

$$x = \frac{D2 - D3}{\sqrt{3}}$$

$$y = \frac{(2D1 - D2 - D3)}{3}$$

$$r = \sqrt{x^2 + y^2}$$

Estos resultados se pueden presentar en una tabulación para diferentes elevaciones.

Datos					Torsión Calculada			Plomada Calculada (desplazamiento).		
Elev. (m).	A. (m).	D1 (m).	D2 (m)	D3 (m).	d (m).	e	α (Grados).	x (m).	y (m).	r (m).

Tabla: V. 1. 3. Resultados de la determinación de torsión y plomada en torres triangulares.

V. 2. Inspección y mantenimiento.

Los propietarios de las torres deben llevar a cabo una inspección inicial, y periódicamente un mantenimiento e inspección, para garantizar la integridad de la estructura y extender su período de servicio. Es recomendable que las inspecciones sean realizadas a un mínimo de 3 años en torres arriostradas y a cada 5 años en torres autoportadas.

Períodos de inspección cortos deberán ser considerados para estructuras que se localizan en ambientes agresivos, como en las costas. Esta rutina también se deberá llevar a cabo en zonas sujetas a frecuente vandalismo.

Además es recomendable que las estructuras sean inspeccionadas después de vientos severos y/o grandes nevadas u otras condiciones de carga extrema.

VI. Conclusiones.

Para mayor objetividad en la presentación de conclusiones y comentarios, se llevará a cabo una enumeración de los puntos relevantes:

1.- En este trabajo se presenta el Análisis y Diseño de una Torre Autosoportada de Comunicación. Para la realización de este proyecto, se emplearon las bases y criterios de diseño estructural que usualmente se llevan a cabo en la práctica profesional. Se consideraron los efectos a los cuales comúnmente son sometidas este tipo de estructuras, por esto podemos concluir que se llevaron a cabo los objetivos marcados al inicio de este trabajo.

2.- La realización de este proyecto estuvo apegada en su totalidad a reglamentos, normas y especificaciones.

3.- Debido principalmente a la altura de la torre, se puede concluir que los efectos provocados por el viento, son en gran medida mayores que los efectos provocados por sismo.

4.- Las fuerzas eólicas ejercidas sobre las antenas parabólicas son de gran magnitud, esto se debe a su forma cóncava. De lo anterior se concluye que las fuerzas de viento ejercidas sobre antenas parabólicas, contribuyen en gran medida a la flexión de la estructura, provocando mayores desplazamientos laterales.

5.- Debido a los grandes esfuerzos a los cuales fue sometida la losa de cimentación, se tuvo la necesidad de contemplar una mayor resistencia del concreto ($f'c = 350 \text{ kg/cm}^2$), con el fin de reducir las dimensiones de la cimentación.

6.- La elevada cantidad de acero requerido para columnas (dados), fue debido a la gran tensión ejercida sobre ellas. (La cantidad de acero requerido se encuentra dentro de los límites establecidos por los reglamentos).

7.- En la realización de proyectos como este, resulta indispensable el empleo de computadoras y programas, estas herramientas son sin duda alguna de gran ayuda ya que de no utilizarlas, la realización de proyectos sería mucho más prolongada.

8.- En la realización de proyectos como este, los métodos empleados son aproximados a la realidad, por lo cual el seguimiento del riguroso análisis debe ser complementado con el conocimiento acumulado que da la experiencia profesional.

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APÉNDICE

“A”

TORRE AUTOSOPORTADA

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98. 297 -4 19.967 -2.309; 298 -3.95 20.965 -2.281; 299 -3.9 21.963 -2.252
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266. 784 1.667 65.888 -0.962; 785 1.722 64.89 -0.995
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 796. 2577 668 6; 2578 6 572; 2579 712 6; 2580 708 5; 2581 5 515; 2582 715 5
 797. 2583 371 373; 2584 373 372; 2585 372 371
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 802. PIPE
 803. PIPX180
 804. 0.457 0.41 0.0325 0.0325
 805. TABLE 2
 806. UNIT METER KG
 807. PIPE
 808. PIPX140
 809. 0.356 0.3175 0.020142 0.020142
 810. END
 811. MEMBER PROPERTY AMERICAN
 812. *TRAMO 1(GM.)
 813. *LARGUEROS
 814. 8 42 TO 46 UPTABLE 1 PIPX180
 815. 9 47 TO 51 UPTABLE 1 PIPX180
 816. 7 37 TO 41 UPTABLE 1 PIPX180
 817. *CELOSLIA PRINCIPAL Y DIAFRAGMAS
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 821. *CELOSLIA SECUNDARIA

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 829. 295 301 TO 312 321 TO 323 332 333 335 336 338 339 341 343 -
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 835. *TRAMO 3(GM.)
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 837. 11 15 19 414 TO 428 UPTABLE 1 PIPX180
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 839. 28 TO 30 411 TO 413 429 TO 440 449 TO 451 460 462 463 465 466 468 -
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 841. 534 TO 542 551 TO 553 562 563 565 566 568 569 571 573 582 TO 586 -
 842. 587 TABLE LD L404010 SP 0.01
 843. *CELOSLIA SECUNDARIA
 844. 441 TO 448 452 TO 459 472 TO 479 494 TO 501 505 TO 512 525 TO 532 -
 845. 543 TO 550 554 TO 561 574 TO 581 TABLE LD L30308 SP 0.01
 846. *TRAMO 4(GM.)
 847. *LARGUEROS
 848. 12 16 20 588 TO 602 UPTABLE 1 PIPX180
 849. *CELOSLIA PRINCIPAL Y DIAFRAGMAS
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 851. 688 TO 690 699 701 TO 703 705 706 708 710 727 TO 729 738 740 741 -
 852. 743 TO 745 747 749 758 TO 764 TABLE LD L404010 SP 0.01
 853. *CELOSLIA SECUNDARIA
 854. 642 TO 649 653 TO 660 672 TO 687 691 TO 698 711 TO 726 730 TO 737 -
 855. 750 TO 757 TABLE LD L30308 SP 0.01
 856. *TRAMO 5(GM.)
 857. *LARGUEROS
 858. 13 17 21 768 TO 782 UPTABLE 1 PIPX180
 859. *CELOSLIA PRINCIPAL Y DIAFRAGMAS
 860. 4 TO 6 765 TO 767 783 TO 818 827 TO 829 838 848 849 858 867 TO 869 -
 861. 878 879 881 882 884 885 887 889 898 900 901 903 TO 905 907 909 918 -
 862. 920 921 923 924 926 928 937 TO 939 978 981 988 991 993 996 -
 863. 2583 TO 2585 TABLE LD L404010 SP 0.01
 864. *CELOSLIA SECUNDARIA
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 866. 890 TO 897 910 TO 917 929 TO 936 TABLE LD L30308 SP 0.01
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 869. 1011 TO 1014 1016 1017 1027 TO 1080 2562 TO 2572 -
 870. 2573 TABLE LD L30305 SP 0.01
 871. *TRAMO 6(GM.)
 872. *LARGUEROS
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 884. *CELOSIA PRINCIPAL Y DIAFRAGMAS
 885. 1315 TO 1320 1342 TO 1347 1371 TO 1374 1417 TO 1424 1511 TO 1514 -
 886. 1557 TO 1564 1651 TO 1654 1673 TO 1680 1789 1790 1792 1793 1795 -
 887. 1796 TABLE LD L30308 SP 0.01
 888. *CELOSIA SECUNDARIA
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 891. *TRAMOS 9(3M.) Y 10(3M.)
 892. *LARGUEROS
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 897. 1806 TABLE LD L30308 SP 0.01
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 901. *TRAMOS 11(3M.), 12(3M.) Y 13(3M.)
 902. *LARGUEROS
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 905. 1327 TO 1335 1354 TO 1362 1379 TO 1384 1397 TO 1408 1519 TO 1524 -
 906. 1537 TO 1548 1659 TO 1664 1689 TO 1700 1800 1801 1804 1807 TO 1811 -
 907. 1812 TABLE LD L30308 SP 0.01
 908. *CELOSIA SECUNDARIA
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 910. 1780 TABLE LD L30307 SP 0.01
 911. *TRAMOS 14(3M.) Y 15(3M.)
 912. *LARGUEROS
 913. 1223 1248 TO 1258 1306 TO 1311 UPTABLE 2 PIPX140
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 915. 1081 TO 1083 1336 TO 1338 1363 TO 1368 1385 TO 1396 1525 TO 1536 -
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 918. *CELOSIA SECUNDARIA
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 921. *PLATAFORMA 2
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 923. *TRAMOS 16(3M.) Y 17(3M.)
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 925. 1841 TO 1846 1858 TO 1863 1875 TO 1880 TABLE ST PIPX100
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 928. 2084 TO 2089 2098 TO 2103 2155 TO 2160 2183 TO 2190 2239 TO 2243 -
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 930. *CELOSIA SECUNDARIA
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 932. *TRAMOS 18(3M.) Y 19(3M.)
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 939. *CELOSIA SECUNDARIA
 940. 2034 TO 2049 2121 TO 2136 2207 TO 2222 TABLE LD L30306 SP 0.006
 941. *TRAMOS 20(3M.) Y 21(3M.)
 942. *LARGUEROS
 943. 1827 TO 1834 1847 TO 1851 1864 TO 1868 TABLE ST PIPX100
 944. *CELOSIA PRINCIPAL Y DIAFRAGMAS
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 946. 2067 2072 TO 2079 2165 TO 2174 2251 TO 2255 -
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 948. *CELOSIA SECUNDARIA
 949. 2018 TO 2033 2104 TO 2115 2117 TO 2120 2223 TO 2237 -
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 954. 2526 TO 2529 2536 2537 TABLE ST PIPX80
 955. *CELOSIA PRINCIPAL Y DIAFRAGMAS
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 957. 2535 TABLE LD L30306 SP 0.006
 958. *CELOSIA SECUNDARIA
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 960. 2435 2454 TO 2456 2464 TO 2469 2538 TO 2540 -
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 978. 1 TO 3 FIXED
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 985. 4 TO 6 FY -200
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 988. CALCULATE NATURAL FREQUENCY
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 997. LOAD 3 VIENTO EN Z-2
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 1001. 990 1000 FZ 174.41
 1002. 997 999 FZ 173.65
 1003. 950 951 FZ 172.89
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 1006. 947 948 FZ 170.50
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 1010. 977 979 FZ 167.41
 1011. 976 978 FZ 166.61
 1012. 752 754 826 FZ 120.25
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 1021. 781 815 861 FZ 128.56
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 1057. 570 575 603 FZ 173.76
 1058. 571 574 669 670 FZ 128.79
 1059. 572 573 668 671 FZ 127.23
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 1061. 383 388 393 394 449 451 FZ 104.44
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 1093. 997 999 FZ 20.34
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 1119. 789 823 865 866 FZ 8.83
 1120. 789 823 865 866 FZ 8.83
 1121. 512 534 630 FZ 23.41
 1122. 544 601 633 634 FZ 17.44
 1123. 545 600 632 635 FZ 17.33
 1124. 546 599 627 FZ 22.95
 1125. 547 598 637 638 FZ 17.11
 1126. 548 597 636 639 FZ 16.98
 1127. 549 596 624 FZ 22.48
 1128. 550 595 641 642 FZ 16.74
 1129. 551 594 640 643 FZ 16.62
 1130. 552 593 621 FZ 21.69
 1131. 553 592 644 647 FZ 16.37
 1132. 554 591 645 646 FZ 16.24
 1133. 555 590 618 FZ 21.48
 1134. 556 589 649 650 FZ 15.98
 1135. 557 588 648 651 FZ 15.85
 1136. 558 587 615 FZ 20.96
 1137. 559 586 653 654 FZ 15.58
 1138. 560 585 612 613 FZ 20.41
 1139. 561 584 611 612 FZ 15.45
 1140. 562 583 656 659 FZ 15.17
 1141. 563 582 657 658 FZ 15.02
 1142. 564 581 609 FZ 19.84
 1143. 565 580 661 662 FZ 14.73
 1144. 566 579 660 663 FZ 14.58
 1145. 567 578 606 FZ 19.24
 1146. 568 577 665 666 FZ 14.27
 1147. 569 576 664 667 FZ 14.12
 1148. 570 575 603 FZ 18.61
 1149. 571 574 669 670 FZ 13.79
 1150. 572 573 668 671 FZ 13.62
 1151. 4 6 372 443 444 446 447 FZ 11.53
 1152. 383 388 393 394 449 451 FZ 13.20
 1153. 382 387 392 395 FZ 19.65
 1154. 381 386 391 386 FZ 19.39
 1155. 380 385 390 397 FZ 19.1
 1156. 379 384 389 398 FZ 18.81
 1157. 14 18 302 366 361 363 364 FZ 10.58

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1162. 294 296 314 323 FZ 15.83
 1163. 11 20 257 277 285 280 281 FZ 9.48
 1164. 216 220 265 274 283 285 FZ 10.82
 1165. 217 219 263 272 FZ 15.85
 1166. 218 268 271 FZ 15.46
 1167. 219 268 271 FZ 15.06
 1168. 212 211 269 270 FZ 14.64
 1169. 12 16 111 144 145 147 148 FZ 8.11
 1170. 116 121 126 127 150 152 FZ 9.15
 1171. 115 120 125 128 FZ 13.23
 1172. 114 119 124 129 FZ 12.71
 1173. 113 118 122 131 FZ 11.54
 1174. 112 117 122 131 FZ 11.54
 1175. 11 15 20 48 69 71 72 FZ 6.72
 1176. 31 36 51 52 74 76 FZ 6.76
 1177. 30 35 50 53 FZ 9.31
 1178. 29 34 49 54 FZ 8.33
 1179. 28 33 48 53 FZ 7.13
 1180. 27 32 47 56 FZ 5.47
 1181. -VIENTO SOBRE PARABOLAS (3H DTM) 2-2
 1182. 953 FZ 2689.17
 1183. 954 FZ 2689.17
 1184. 955 FZ 1828.91
 1185. 512 FZ 2395.01
 1186. 513 FZ 2395.01
 1187. 514 FZ 628.85
 1188. 5 FZ 1864.71
 1189. 6 FZ 1336.2
 1190. 6 FZ 1336.2
 1191. -VIENTO SOBRE PLATAFORMAS
 1192. 459 460 489 TO 493 495 496 507 TO 511 FZ 93.66
 1193. 830 832 845 TO 848 850 857 TO 859 FZ 111.89
 1194. CALCULATE NATURAL FREQUENCY
 1195. *
 1196. LOAD 4 VIENTO EN X-X
 1197. JOINT LOAD
 1198. -VIENTO SOBRE ESTRUCTURA X-X
 1200. 998 1015 FX 137.14
 1201. 997 1014 FX 136.54
 1202. 950 952 FX 135.94
 1203. 994 1013 FX 135.34
 1204. 993 1012 FX 134.74
 1205. 947 949 FX 134.13
 1206. 990 1011 FX 133.51
 1207. 969 1010 FX 132.89
 1208. 946 948 FX 132.26
 1209. 975 979 FX 131.64
 1210. 974 978 FX 131.64
 1211. 752 755 825 FX 98.72
 1212. 750 807 892 895 FX 73.68
 1213. 791 808 893 894 FX 73.31

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1218. 796 813 902 905 FX 78.91
 1219. 797 814 903 904 FX 78.48
 1220. 798 815 889 FX 104.06
 1221. 799 816 906 908 FX 77.61
 1222. 800 817 907 908 FX 77.17
 1223. 801 818 890 FX 111.88
 1224. 802 819 910 913 FX 83.42
 1225. 803 820 911 912 FX 82.93
 1226. 804 821 891 FX 109.9
 1227. 805 872 900 915 FX 81.92
 1228. 806 823 901 914 FX 81.41
 1229. 512 513 631 FX 127.79
 1230. 543 601 672 675 FX 95.22
 1231. 542 600 673 674 FX 94.6
 1232. 541 599 628 FX 125.29
 1233. 540 598 676 679 FX 93.33
 1234. 539 597 677 678 FX 92.69
 1235. 538 596 625 FX 131.02
 1236. 537 595 680 683 FX 97.56
 1237. 536 594 681 682 FX 96.85
 1238. 535 593 622 FX 128.18
 1239. 534 592 684 687 FX 95.4
 1240. 533 591 685 686 FX 94.67
 1241. 532 590 619 FX 135.88
 1242. 531 589 688 691 FX 101.09
 1243. 530 588 689 690 FX 100.26
 1244. 529 587 616 FX 132.55
 1245. 528 586 692 695 FX 98.56
 1246. 527 585 693 694 FX 97.69
 1247. 526 584 633 FX 138.65
 1248. 525 583 696 699 FX 103.03
 1249. 524 582 697 698 FX 102.05
 1250. 523 581 610 FX 134.76
 1251. 522 580 701 702 FX 100.06
 1252. 521 579 700 703 FX 99.04
 1253. 520 578 607 FX 138.81
 1254. 519 577 704 707 FX 102.99
 1255. 518 576 705 706 FX 101.85
 1256. 517 575 604 FX 134.25
 1257. 516 574 709 710 FX 99.51
 1258. 515 573 708 711 FX 98.3
 1259. 4 5 373 421 422 424 425 FX 69.75
 1260. 378 383 403 404 427 429 FX 80.32
 1261. 377 382 402 405 FX 118.87
 1262. 376 381 401 406 FX 117.22
 1263. 375 380 400 407 FX 115.53
 1264. 374 379 399 408 FX 120.47
 1265. 10 16 303 339 340 342 343 FX 67.06
 1266. 290 300 328 329 345 347 FX 76.97
 1267. 289 299 327 330 FX 113.5
 1268. 288 298 326 331 FX 111.49
 1269. 287 297 325 332 FX 109.42

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1274. 209 214 224 229 FX 104.51
 1275. 208 213 223 230 FX 101.77
 1276. 207 212 222 231 FX 98.92
 1277. 8 14 154 172 173 175 176 FX 57.34
 1278. 121 159 160 169 178 180 FX 64.7
 1279. 120 158 161 168 FX 93.56
 1280. 119 157 162 167 FX 89.85
 1281. 118 156 163 166 FX 85.87
 1282. 117 155 164 165 FX 81.58
 1283. 7 15 21 88 89 91 92 FX 45.97
 1284. 26 36 57 66 94 96 FX 50.01
 1285. 25 35 58 65 FX 68.85
 1286. 24 34 59 64 FX 61.65
 1287. 23 33 60 63 FX 52.76
 1288. 22 32 61 62 FX 40.43
 1289. *VIENTO SOBRE LINEAS X-X
 1290. 953 955 FX 5.85
 1291. 998 1015 FX 5.82
 1292. 997 1014 FX 5.8
 1293. 950 952 FX 5.77
 1294. 994 1013 FX 5.75
 1295. 993 1012 FX 5.72
 1296. 947 949 FX 5.69
 1297. 990 1011 FX 5.67
 1298. 989 1010 FX 5.64
 1299. 944 946 FX 5.62
 1300. 975 979 FX 5.59
 1301. 974 978 FX 5.56
 1302. 752 755 825 FX 3.69
 1303. 790 807 892 895 FX 2.75
 1304. 791 808 893 894 FX 2.74
 1305. 792 809 829 FX 3.63
 1306. 793 810 896 899 FX 2.71
 1307. 794 811 897 898 FX 2.7
 1308. 795 812 888 FX 3.58
 1309. 796 813 902 905 FX 2.67
 1310. 797 814 903 904 FX 2.65
 1311. 798 815 889 FX 3.52
 1312. 799 816 906 809 FX 2.63
 1313. 800 817 907 908 FX 2.61
 1314. 801 818 890 FX 3.46
 1315. 802 819 910 913 FX 2.58
 1316. 803 820 911 912 FX 2.56
 1317. 804 821 891 FX 3.4
 1318. 805 822 900 915 FX 2.53
 1319. 806 823 901 914 FX 2.52
 1320. 512 513 631 FX 3.34
 1321. 543 601 672 675 FX 2.49
 1322. 542 600 673 674 FX 2.47
 1323. 541 599 628 FX 3.27
 1324. 540 598 676 679 FX 2.44
 1325. 539 597 677 678 FX 2.42

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1330. 534 592 684 687 FX 2.33
 1331. 533 591 685 686 FX 2.31
 1332. 532 590 619 FX 3.06
 1333. 531 589 688 691 FX 2.28
 1334. 530 588 689 690 FX 2.26
 1335. 529 587 616 FX 2.99
 1336. 528 586 692 695 FX 2.22
 1337. 527 585 693 694 FX 2.2
 1338. 526 584 613 FX 2.91
 1339. 525 583 696 699 FX 2.16
 1340. 524 582 697 698 FX 2.14
 1341. 523 581 610 FX 2.83
 1342. 522 580 701 702 FX 2.1
 1343. 521 579 700 703 FX 2.08
 1344. 520 578 607 FX 2.74
 1345. 519 577 704 707 FX 2.03
 1346. 518 576 705 706 FX 2.01
 1347. 517 575 604 FX 2.65
 1348. 516 574 709 710 FX 1.97
 1349. 515 573 708 711 FX 1.94
 1350. 4 5 373 421 422 424 425 FX 1.1
 1351. 378 383 405 404 427 429 FX 1.26
 1352. 377 382 402 405 FX 1.87
 1353. 376 381 401 406 FX 1.84
 1354. 375 380 400 407 FX 1.81
 1355. 374 379 399 408 FX 1.79
 1356. 10 18 303 339 340 342 343 FX 1.01
 1357. 290 300 328 329 345 347 FX 1.15
 1358. 289 299 327 330 FX 1.7
 1359. 288 298 326 331 FX 1.67
 1360. 287 297 325 332 FX 1.64
 1361. 286 296 324 333 FX 1.61
 1362. 9 17 206 234 235 237 238 FX 0.9
 1363. 211 216 226 227 240 242 FX 1.03
 1364. 210 215 225 228 FX 1.51
 1365. 209 214 224 229 FX 1.47
 1366. 208 213 223 230 FX 1.43
 1367. 207 212 222 231 FX 1.39
 1368. 8 16 154 172 173 175 176 FX 0.77
 1369. 121 159 160 169 178 180 FX 0.87
 1370. 120 158 161 168 FX 1.26
 1371. 119 157 162 167 FX 1.21
 1372. 118 156 163 166 FX 1.15
 1373. 117 155 164 165 FX 1.1
 1374. 7 15 21 88 89 91 92 FX 0.59
 1375. 26 36 57 66 94 96 FX 0.64
 1376. 25 35 58 65 FX 0.88
 1377. 24 34 59 64 FX 0.79
 1378. 23 33 60 63 FX 0.68
 1379. 22 32 61 62 FX 0.52
 1380. *VIENTO SOBRE PARABOLAS (3M DIAM) X-X
 1381. 953 FX 596.06 FZ -20.32 MY -682.78

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1386. 514 FX 530.88 FZ -18.1 MY -608.09
 1387. 4 FX 435.5 FZ -14.85 MY -608.09
 1388. 5 FX 435.5 FZ 14.85 MY 498.84
 1389. 6 FX 435.5 FZ -14.85 MY -498.84
 1390. *VIENTO SOBRE PLATAFORMAS
 1391. 458 460 464 TO 468 494 496 TO 501 FX 77.79
 1392. 830 831 842 TO 844 848 849 854 TO 856 FX 92.33
 1393. CALCULATE NATURAL FREQUENCY
 1394. *
 1395. *SISMO
 1396. *
 1397. LOAD 5 SISMO EN Z-Z
 1398. SELFWEIGHT X 1
 1399. SELFWEIGHT Y 1
 1400. SELFWEIGHT Z 1
 1401. JOINT LOAD
 1402. 4 TO 6 FX 200
 1403. 512 TO 514 FX 200
 1404. 953 TO 955 FX 200
 1405. 4 TO 6 FY 200
 1406. 512 TO 514 FY 200
 1407. 953 TO 955 FY 200
 1408. 4 TO 6 FZ 200
 1409. 512 TO 514 FZ 200
 1410. 953 TO 955 FZ 200
 1411. *GRUPO A, ORDENADAS ESPECTRALES MULTIPLICADAS POR 1.5
 1412. SPECTRUM SRSS 2 1 ACC SCALE 9.81
 1413. 0 0.06; 0.05 0.08; 0.1 0.09; 0.15 0.1; 0.2 0.11; 0.25 0.11; 0.3 0.12
 1414. 0.5 0.12; 0.7 0.12; 0.9 0.12; 1.1 0.12; 1.3 0.12; 1.5 0.12; 1.6 0.11
 1415. 1.9 0.1; 2.2 0.09; 2.5 0.09; 2.8 0.08; 3.1 0.07; 3.4 0.07; 3.7 0.07
 1416. 4 0.06; 4.3 0.06; 4.6 0.06; 4.9 0.05; 5.2 0.05; 5.5 0.05; 5.8 0.05
 1417. 6.1 0.05; 6.4 0.05; 6.7 0.04; 7 0.04; 7.3 0.04; 7.6 0.04; 7.9 0.04
 1418. 8.2 0.04
 1419. CALCULATE NATURAL FREQUENCY
 1420. *
 1421. LOAD 6 SISMO EN X-X
 1422. *GRUPO A, ORDENADAS ESPECTRALES MULTIPLICADAS POR 1.5
 1423. SPECTRUM SRSS X 1 ACC SCALE 9.81
 1424. 0 0.06; 0.05 0.08; 0.1 0.09; 0.15 0.1; 0.2 0.11; 0.25 0.11; 0.3 0.12
 1425. 0.5 0.12; 0.7 0.12; 0.9 0.12; 1.1 0.12; 1.3 0.12; 1.5 0.12; 1.6 0.11
 1426. 1.9 0.1; 2.2 0.09; 2.5 0.09; 2.8 0.08; 3.1 0.07; 3.4 0.07; 3.7 0.07
 1427. 4 0.06; 4.3 0.06; 4.6 0.06; 4.9 0.05; 5.2 0.05; 5.5 0.05; 5.8 0.05
 1428. 6.1 0.05; 6.4 0.05; 6.7 0.04; 7 0.04; 7.3 0.04; 7.6 0.04; 7.9 0.04
 1429. 8.2 0.04
 1430. CALCULATE NATURAL FREQUENCY
 1431. *
 1432. *CIMENTACION
 1433. LOAD COMB 7 (CM + CV) *1
 1434. 1 1.0 2 1.0
 1435. LOAD COMB 8 (CM + CV + VZ) *1
 1436. 1 1.0 2 1.0 3 1.0
 1437. LOAD COMB 9 (CM + CV - VZ) *1

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1442. 1 1.0 2 1.0 4 -1.0
 1443. LOAD COMB 12 (CH + CV + 100% SZ + 30% SX)*1
 1444. 1 1.0 2 1.0 5 2.151 6 0.6453
 1445. LOAD COMB 13 (CH + CV + 100% SZ - 30% SX)*1
 1446. 1 1.0 2 1.0 5 2.151 6 -0.6453
 1447. LOAD COMB 14 (CH + CV - 100% SZ + 30% SX)*1
 1448. 1 1.0 2 1.0 5 -2.151 6 0.6453
 1449. LOAD COMB 15 (CH + CV - 100% SZ - 30% SX)*1
 1450. 1 1.0 2 1.0 5 -2.151 6 -0.6453
 1451. LOAD COMB 16 (CH + CV + 30% SZ + 100% SX)*1
 1452. 1 1.0 2 1.0 5 0.6453 6 2.151
 1453. LOAD COMB 17 (CH + CV + 30% SZ - 100% SX)*1
 1454. 1 1.0 2 1.0 5 0.6453 6 -2.151
 1455. LOAD COMB 18 (CH + CV - 30% SZ + 100% SX)*1
 1456. 1 1.0 2 1.0 5 -0.6453 6 2.151
 1457. LOAD COMB 19 (CH + CV - 30% SZ - 100% SX)*1
 1458. 1 1.0 2 1.0 5 -0.6453 6 -2.151
 1459. *DISEÑO
 1460. LOAD COMB 20 (CH + CV)*1
 1461. 1 1 2 1
 1462. LOAD COMB 21 (CH + CV + VZ)*0.75
 1463. 1 0.75 2 0.75 3 0.75
 1464. LOAD COMB 22 (CH + CV - VZ)*0.75
 1465. 1 0.75 2 0.75 3 -0.75
 1466. LOAD COMB 23 (CH + CV + VX)*0.75
 1467. 1 0.75 2 0.75 4 0.75
 1468. LOAD COMB 24 (CH + CV - VX)*0.75
 1469. 1 0.75 2 0.75 4 -0.75
 1470. LOAD COMB 25 (CH + CV + 100% SZ + 30% SX)*0.75
 1471. 1 0.75 2 0.75 5 1.61325 6 0.48397
 1472. LOAD COMB 26 (CH + CV + 100% SZ - 30% SX)*0.75
 1473. 1 0.75 2 0.75 5 1.61325 6 -0.48397
 1474. LOAD COMB 27 (CH + CV - 100% SZ + 30% SX)*0.75
 1475. 1 0.75 2 0.75 5 -1.61325 6 0.48397
 1476. LOAD COMB 28 (CH + CV - 100% SZ - 30% SX)*0.75
 1477. 1 0.75 2 0.75 5 -1.61325 6 -0.48397
 1478. LOAD COMB 29 (CH + CV + 30% SZ + 100% SX)*0.75
 1479. 1 0.75 2 0.75 5 0.48397 6 1.61325
 1480. LOAD COMB 30 (CH + CV + 30% SZ - 100% SX)*0.75
 1481. 1 0.75 2 0.75 5 0.48397 6 -1.61325
 1482. LOAD COMB 31 (CH + CV - 30% SZ + 100% SX)*0.75
 1483. 1 0.75 2 0.75 5 -0.48397 6 1.61325
 1484. LOAD COMB 32 (CH + CV - 30% SZ - 100% SX)*0.75
 1485. 1 0.75 2 0.75 5 -0.48397 6 -1.61325
 1486. PERFORM ANALYSIS

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PROBLEM STATISTICS

NUMBER OF JOINTS/MEMBER/ELEMENTS/SUPPORTS = 909/ 2205/ 3
 ORIGINAL/FINAL BAND-WIDTH= 610/ 31/ 192 DOF
 TOTAL PRIMARY LOAD CASES = 6, TOTAL DEGREES OF FREEDOM = 5436
 SIZE OF STIFFNESS MATRIX = 1044 DOUBLE KILO-WORDS
 REQD/AVAIL. DISK SPACE = 33.2/ 817.6 MB, EXCHN = 821.4 MB

** Processing Element Stiffness Matrix. 15:39:25
 ** Processing Global Stiffness Matrix. 15:39:26
 ** Processing Triangular Factorization. 15:39:28
 ** Calculating Joint Displacements. 15:39:39
 ** Calculating Eigensolution. 15:39:41
 NUMBER OF MODES REQUESTED = 25
 NUMBER OF EXISTING MODES IN THE MODEL = 2718
 NUMBER OF MODES THAT WILL BE USED = 25

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CALCULATED FREQUENCIES FOR LOAD CASE 5

MODE	FREQUENCY (CYCLES/SEC)	PERIOD(SEC)	ACCURACY
1	1.058	0.94545	1.287E-15
2	1.058	0.94542	4.826E-16
3	2.820	0.35466	9.056E-16
4	2.820	0.35466	5.433E-16
5	3.213	0.31124	4.185E-16
6	5.011	0.19956	6.881E-16
7	5.011	0.19955	0.000E+00
8	5.908	0.17218	3.415E-16
9	8.123	0.12311	4.831E-13
10	8.124	0.12310	1.239E-14
11	8.437	0.11852	2.945E-14
12	9.085	0.11007	3.324E-10
13	9.241	0.10821	3.250E-10
14	9.782	0.10223	2.473E-09
15	9.850	0.10152	3.243E-09
16	10.260	0.09747	3.606E-12
17	10.365	0.09647	1.243E-07
18	10.400	0.09615	8.979E-12
19	10.400	0.09615	2.081E-12
20	10.431	0.09587	1.097E-08
21	10.840	0.09225	1.779E-08
22	11.043	0.09055	1.937E-08
23	11.044	0.09054	3.719E-07
24	11.169	0.08953	7.900E-07
25	11.563	0.08648	3.803E-07

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The following Frequencies are estimates that were calculated. These are for information only and will not be used. Remaining values are either above the cut off mode/freq values or are of low accuracy. To use these frequencies, rerun with a higher cutoff mode (or mode + freq) value.

CALCULATED FREQUENCIES FOR LOAD CASE 5

MODE	FREQUENCY (CYCLES/SEC)	PERIOD(SEC)	ACCURACY
26	11.734	0.08522	3.818E-08
27	11.735	0.08521	5.336E-09
28	11.823	0.08458	3.855E-06
29	11.848	0.08440	9.772E-07
30	12.010	0.08326	1.033E-05
31	12.838	0.07789	1.253E-05
32	13.186	0.07584	1.770E-06
33	13.354	0.07489	1.321E-07
34	13.355	0.07488	4.262E-06

** Calculating Response Spectrum Displacements. 15:42:16

RESPONSE LOAD CASE 5

SRSS MODAL COMBINATION METHOD USED.

DYNAMIC WEIGHT X Y Z 1.310851E+05 1.310851E+05 1.310851E+05 KG
 MISSING WEIGHT X Y Z -1.097115E+04 -7.960802E+04 -1.094511E+04 KG
 MODAL WEIGHT X Y Z 1.201140E+05 5.127711E+04 1.201400E+05 KG

MODE	FACTOR	ACCELERATION-G	DAMPING
1	-2.504550E+00	0.12004	0.05000
2	-1.748659E+01	0.12004	0.05000
3	1.909156E+00	0.12004	0.05000
4	-7.456955E-01	0.12004	0.05000
5	2.592907E-05	0.12004	0.05000
6	8.090446E-03	0.10995	0.05000
7	-4.913834E-01	0.10995	0.05000
8	4.022795E-05	0.10447	0.05000
9	-1.879372E-03	0.09465	0.05000
10	9.405508E-02	0.09465	0.05000
11	-1.672854E-05	0.09374	0.05000
12	-5.032998E-05	0.09205	0.05000
13	1.301167E-02	0.09167	0.05000
14	2.555634E-03	0.09048	0.05000

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MODE	FACTOR	ACCELERATION-G	DAMPING
19	4.538234E-06	0.08926	0.05000
20	4.179594E-07	0.08920	0.05000
21	-3.934828E-05	0.08840	0.05000
22	-3.927928E-02	0.08814	0.05000
23	-8.335582E-04	0.08814	0.05000
24	4.610525E-03	0.08794	0.05000
25	-8.185158E-04	0.08733	0.05000

MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KG

MODE	MASS PARTICIPATION FACTORS IN PERCENT						BASE SHEAR IN KG		
	X	Y	Z	SUM-X	SUM-Y	SUM-Z	X	Y	Z
1	37.10	0.00	0.76	37.101	0.000	0.761	0.00	0.00	119.75
2	0.76	0.00	37.10	37.862	0.000	37.863	0.00	0.00	5838.29
3	3.44	0.00	22.33	41.305	0.000	60.194	0.00	0.00	3513.93
4	22.33	0.00	3.44	63.635	0.000	63.630	0.00	0.00	541.86
5	0.00	0.00	0.00	63.635	0.000	63.630	0.00	0.00	0.00
6	17.59	0.00	0.00	81.229	0.000	63.643	0.00	0.00	0.69
7	0.00	0.00	17.59	81.233	0.000	81.238	0.00	0.00	2535.87
8	0.00	0.00	0.00	81.233	0.000	81.238	0.00	0.00	0.00
9	5.99	0.00	0.00	87.221	0.000	81.240	0.00	0.00	0.30
10	0.00	0.00	6.01	87.223	0.000	87.247	0.00	0.00	745.29
11	0.00	0.00	0.00	87.223	0.000	87.247	0.00	0.00	0.00
12	0.23	0.00	0.00	87.449	0.000	87.247	0.00	0.00	0.00
13	0.00	0.04	0.21	87.449	0.040	87.452	0.00	0.00	24.66
14	0.00	1.61	0.01	87.449	1.652	87.462	0.00	0.00	1.21
15	0.00	0.00	0.00	87.449	1.652	87.462	0.00	0.00	0.00
16	0.00	1.02	0.00	87.449	2.673	87.462	0.00	0.00	0.00
17	0.00	0.01	0.01	87.449	2.684	87.468	0.00	0.00	0.68
18	0.00	0.00	0.00	87.449	2.684	87.469	0.00	0.00	0.11
19	0.00	0.00	0.00	87.450	2.684	87.469	0.00	0.00	0.00
20	0.02	0.00	0.00	87.474	2.684	87.469	0.00	0.00	0.00
21	0.00	0.00	0.00	87.475	2.684	87.469	0.00	0.00	0.00
22	0.00	0.26	4.12	87.476	2.948	91.587	0.00	0.00	475.74
23	4.12	0.00	0.00	91.630	2.948	91.588	0.00	0.00	0.21
24	0.00	35.23	0.06	91.630	38.179	91.648	0.00	0.00	6.89
25	0.00	0.94	0.00	91.630	39.117	91.650	0.00	0.00	0.25
TOTAL SRSS SHEAR							0.00	0.00	7345.36
TOTAL 10PCT SHEAR							0.00	0.00	7692.37
TOTAL ABS SHEAR							0.00	0.00	13805.74

TORRE AUTOSOPOORTADA

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RESPONSE LOAD CASE 6

SRSS MODAL COMBINATION METHOD USED.

DYNAMIC WEIGHT X Y Z 1.310851E+05 1.310851E+05 1.310851E+05 KG

MISSING WEIGHT X Y Z -1.097115E+04 -7.990802E+04 -1.094511E+04 KG

MODAL WEIGHT X Y Z 1.201140E+05 5.127711E+04 1.201400E+05 KG

MODE	FACTOR	ACCELERATION-G	DAMPING
1	1.748752E+01	0.12004	0.05000
2	-2.504266E+00	0.12004	0.05000
3	-7.496876E-01	0.12004	0.05000
4	-1.909086E+00	0.12004	0.05000
5	4.625289E-04	0.12004	0.05000
6	-4.934216E-01	0.10995	0.05000
7	-8.085690E-03	0.10995	0.05000
8	-1.170080E-04	0.10447	0.05000
9	9.392180E-02	0.09465	0.05000
10	1.875267E-03	0.09465	0.05000
11	5.209473E-05	0.09374	0.05000
12	1.416715E-02	0.09205	0.05000
13	4.391877E-05	0.09167	0.05000
14	-6.832391E-06	0.09048	0.05000
15	-3.777133E-04	0.09034	0.05000
16	-1.786517E-07	0.08952	0.05000
17	3.660508E-06	0.08933	0.05000
18	-4.436472E-06	0.08926	0.05000
19	7.365212E-04	0.08926	0.05000
20	-3.431059E-03	0.08920	0.05000
21	2.436054E-04	0.08848	0.05000
22	8.204440E-04	0.08814	0.05000
23	-3.940540E-02	0.08814	0.05000
24	-7.945924E-05	0.08794	0.05000
25	1.644346E-05	0.08733	0.05000

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MASS PARTICIPATION FACTORS IN PERCENT

BASE SHEAR IN KG

MODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
1	37.10	0.00	0.76	37.101	0.000	0.761	5838.07	0.00	0.00
2	0.76	0.00	37.10	37.862	0.000	37.863	119.74	0.00	0.00
3	3.44	0.00	22.33	41.305	0.000	60.194	541.84	0.00	0.00
4	22.33	0.00	3.44	63.635	0.000	63.638	3513.75	0.00	0.00
5	0.00	0.00	0.00	63.635	0.000	63.638	0.00	0.00	0.00
6	17.59	0.00	0.00	81.229	0.000	63.643	2535.69	0.00	0.00
7	0.00	0.00	17.59	81.233	0.000	81.238	0.69	0.00	0.00
8	0.00	0.00	0.00	81.233	0.000	81.238	0.00	0.00	0.00
9	5.99	0.00	0.00	87.221	0.000	81.240	742.92	0.00	0.00
10	0.00	0.00	6.01	87.223	0.000	87.247	0.30	0.00	0.00
11	0.00	0.00	0.00	87.223	0.000	87.247	0.00	0.00	0.00
12	0.23	0.00	0.00	87.449	0.000	87.247	27.20	0.00	0.00
13	0.00	0.94	0.21	87.449	0.040	87.452	0.00	0.00	0.00
14	0.00	1.61	0.01	87.449	1.652	87.462	0.00	0.00	0.00
15	0.00	0.00	0.00	87.449	1.652	87.462	0.03	0.00	0.00
16	0.00	1.02	0.00	87.449	2.673	87.462	0.00	0.00	0.00
17	0.00	0.01	0.01	87.449	2.684	87.468	0.00	0.00	0.00
18	0.00	0.00	0.00	87.449	2.684	87.469	0.00	0.00	0.00
19	0.00	0.00	0.00	87.450	2.684	87.469	0.13	0.00	0.00
20	0.02	0.00	0.00	87.474	2.684	87.469	2.86	0.00	0.00
21	0.00	0.00	0.00	87.475	2.684	87.469	0.02	0.00	0.00
22	0.00	0.26	4.12	87.476	2.948	91.587	0.21	0.00	0.00
23	4.15	0.00	0.00	91.630	2.948	91.588	479.95	0.00	0.00
24	0.00	35.23	0.06	91.630	38.179	91.648	0.00	0.00	0.00
25	0.00	0.94	0.00	91.630	39.117	91.650	0.00	0.00	0.00

TOTAL BRSS SHEAR 7345.08 0.00 0.00
 TOTAL 10PCT SHEAR 7691.76 0.00 0.00
 TOTAL ABS SHEAR 13803.39 0.00 0.00

 *
 * NATURAL FREQUENCY FOR LOADING 1 = 11.75776 CPS *
 * MAX DEFLECTION = 0.40373 CM GLO Y, AT JOINT 473 *

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 *
 * NATURAL FREQUENCY FOR LOADING 2 = 47.38115 CPS *
 * MAX DEFLECTION = 0.01339 CM GLO Y, AT JOINT 494 *

 *
 * NATURAL FREQUENCY FOR LOADING 3 = 0.88768 CPS *
 * MAX DEFLECTION = 48.78820 CM GLO Z, AT JOINT 953 *

 *
 * NATURAL FREQUENCY FOR LOADING 4 = 1.31056 CPS *
 * MAX DEFLECTION = 24.61527 CM GLO X, AT JOINT 955 *

 *
 * NATURAL FREQUENCY FOR LOADING 5 = 3.42775 CPS *
 * MAX DEFLECTION = 5.61934 CM GLO Z, AT JOINT 954 *

 *
 * NATURAL FREQUENCY FOR LOADING 6 = 0.00000 CPS *
 * MAX DEFLECTION = 5.61934 CM GLO X, AT JOINT 955 *

TORRE AUTOSOPORTADA

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SUPPORT REACTIONS -UNIT KG METE STRUCTURE TYPE = SPACE

JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MM-X	MM-Y	MM-Z
1	7	3884.97	44079.19	2244.19	-694.64	-0.09	1192.98
8		-11707.10	-145431.11	-11510.53	-3162.48	696.23	-3523.98
9		19477.08	233569.47	16006.91	1773.21	-696.40	5909.93
10		-13868.30	-137964.25	-8123.60	459.37	-485.26	-577.94
11		21638.25	226172.62	12611.97	-1848.65	485.09	2963.89
12		10637.54	123558.52	6824.48	1098.79	145.12	3340.16
13		6287.25	78614.34	4577.36	323.76	103.78	2158.20
14		1482.70	9544.04	-89.98	-1713.04	-103.96	227.75
15		-2867.59	-35400.14	-2336.10	-2488.07	-145.29	-954.21
16		12508.49	136088.31	7026.41	1018.85	106.17	3629.77
17		-1892.79	-13725.60	-464.00	-1564.58	-31.62	-310.10
18		9762.24	101883.97	4952.37	175.30	31.44	2696.05
19		-4738.74	-47929.94	-2538.04	-2408.13	-106.34	-1243.82
2	7	0.06	44083.37	-4489.47	1360.47	0.01	-0.15
8		-214.43	423103.78	-45054.86	7110.71	-16.72	205.32
9		214.54	-334937.06	36077.92	-4389.78	16.74	-205.62
10		-4535.85	43696.27	-4461.43	1353.29	354.85	4725.17
11		4535.96	44470.46	-4515.52	1367.65	-354.83	-4725.47
12		927.98	142204.67	5800.52	4723.11	78.09	951.97
13		108.40	129056.95	4122.08	3856.55	-4.15	105.68
14		-108.29	-40890.21	-13099.03	-1135.61	4.17	-105.98
15		-927.87	-54037.94	-14777.46	-1502.17	-78.07	-952.27
16		1521.47	93460.49	1143.84	2775.22	148.17	1569.03
17		-1210.47	49634.71	-4450.92	1553.34	-125.97	-1251.95
18		1210.59	38532.02	-4526.02	1167.60	125.99	1251.64
19		-1521.36	-5293.75	-10120.79	-54.28	-148.14	-1569.33
3	7	-3885.03	44080.25	2244.20	-694.87	0.08	-1193.04
8		11921.54	-145429.91	-11146.63	-2805.77	-730.42	3317.73
9		-19691.60	233590.41	15635.20	1416.04	730.57	-5703.81
10		-21547.15	226510.78	12639.29	-1838.99	-178.80	-3094.57
11		13777.09	-138350.27	-8149.73	449.25	178.96	708.49
12		2365.30	110291.64	6700.53	1034.51	140.49	585.84
13		-2194.61	59380.69	4363.88	202.96	90.64	-767.47
14		-5575.45	28779.81	124.69	-1592.70	-90.48	-1618.61
15		-10135.36	-22131.14	-2211.96	-2424.25	-140.34	-2971.92
16		4905.94	141158.62	7125.08	1085.13	117.82	1393.13
17		-10293.78	-28544.57	-663.76	-1686.71	-48.37	-3117.88
18		2523.72	116705.06	5152.33	296.97	48.52	731.80
19		-12676.00	-52998.11	-2636.51	-2474.87	-117.66	-3779.21

***** END OF LATEST ANALYSIS RESULT *****

TORRE AUTOSOPORTADA

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STEEL TAKE-OFF

PROFILE	LENGTH (METE)	WEIGHT (KG)
LD L40 4010	669.02	31167.051
UP PIPX180	90.00	22563.201
LD L30 308	1109.09	30828.047
LD L30 305	418.98	7538.065
UP PIPX140	90.00	14357.633
LD L30 307	558.33	13747.378
ST PIP X100	54.00	4393.796
LD L30 306	174.48	3719.359
ST PIP S80	36.00	1528.271
TOTAL =		129842.797

***** END OF DATA FROM INTERNAL STORAGE *****

1490. PARAMETER
 1491. CODE AISC
 1492. *RATIO 1.02 ALL
 1493. BEAM 1 ALL
 1494. LOAD LIST 20 TO 32
 1495. CHECK CODE ALL

TORRE AUTOSOPORTADA

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STAAD.Pro CODE CHECKING - (AISC)

ALL UNITS ARE - KG METR (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
4	LD	L40 4010	PASS	AISC- H1-3	0.134 21
		6703.58 C		-67.70	0.00
5	LD	L40 4010	PASS	AISC- H1-3	0.126 22
		5970.32 C		24.67	43.84
6	LD	L40 4010	PASS	AISC- H2-1	0.073 22
		236.87 T		-41.69	-57.93
7	UPT	PIPX180	PASS	AISC- H1-2	0.677 21
		276983.22 C		0.00	5923.48
8	UPT	PIPX180	PASS	AISC- H1-2	0.392 22
		152882.05 C		0.00	4266.04
9	UPT	PIPX180	PASS	AISC- H1-2	0.396 22
		152881.44 C		0.00	4484.07
10	UPT	PIPX180	PASS	AISC- H1-2	0.629 21
		253803.91 C		0.00	5903.89
11	UPT	PIPX180	PASS	AISC- H1-2	0.572 21
		229564.92 C		0.00	5481.98
12	UPT	PIPX180	PASS	AISC- H1-2	0.512 21
		205066.44 C		0.00	4982.73
13	UPT	PIPX180	PASS	AISC- H1-2	0.453 21
		180411.58 C		0.00	4505.34
14	UPT	PIPX180	PASS	AISC- H1-2	0.351 22
		139471.55 C		0.00	3524.99
15	UPT	PIPX180	PASS	AISC- H1-2	0.319 22
		125604.75 C		0.00	3313.46
16	UPT	PIPX180	PASS	AISC- H1-2	0.287 22
		111744.96 C		0.00	3126.30
17	UPT	PIPX180	PASS	AISC- H1-2	0.254 22
		97700.97 C		0.00	2923.10
18	UPT	PIPX180	PASS	AISC- H1-2	0.352 22
		139471.22 C		0.00	3593.50
19	UPT	PIPX180	PASS	AISC- H1-2	0.320 22
		125603.95 C		0.00	3409.96
20	UPT	PIPX180	PASS	AISC- H1-2	0.289 22
		111743.76 C		0.00	3246.02
21	UPT	PIPX180	PASS	AISC- H1-2	0.258 22
		97700.02 C		0.00	3117.56

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TORRE AUTOSOPORTADA

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ALL UNITS ARE - KG METR (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
25	LD	L40 4010	PASS	AISC- H1-3	0.153 22
		6901.40 C		16.19	59.64
26	LD	L40 4010	PASS	AISC- H2-1	0.082 22
		1435.04 T		-50.82	46.95
27	LD	L40 4010	PASS	AISC- H1-3	0.121 21
		8516.31 C		-46.79	-9.23
28	LD	L40 4010	PASS	AISC- H1-3	0.147 22
		8752.40 C		19.76	55.76
29	LD	L40 4010	PASS	AISC- H2-1	0.086 22
		1441.87 T		-33.89	-62.90
30	LD	L40 4010	PASS	AISC- H1-3	0.126 21
		6437.98 C		-62.84	-8.62
31	LD	L40 4010	PASS	AISC- H1-3	0.138 22
		6585.64 C		17.46	51.17
32	LD	L40 4010	PASS	AISC- H2-1	0.090 22
		1565.61 T		-37.95	-63.39
33	LD	L40 4010	PASS	AISC- H1-3	0.129 21
		6285.76 C		-65.31	-14.77
34	LD	L40 4010	PASS	AISC- H2-1	0.141 22
		6397.29 T		-4.39	80.70
35	LD	L40 4010	PASS	AISC- H2-1	0.109 24
		5280.54 T		1.67	59.09
36	LD	L40 4010	PASS	AISC- H2-1	0.209 21
		10542.18 T		9.43	104.58
37	UPT	PIPX180	PASS	AISC- H1-2	0.679 21
		277800.44 C		0.00	5946.44
38	UPT	PIPX180	PASS	AISC- H1-1	0.637 21
		288830.72 C		0.00	2232.31
39	UPT	PIPX180	PASS	AISC- H1-1	0.621 21
		291105.69 C		0.00	944.41
40	UPT	PIPX180	PASS	AISC- H1-1	0.626 21
		292407.25 C		0.00	1082.56
41	UPT	PIPX180	PASS	AISC- H1-2	0.685 21
		293161.16 C		0.00	4488.70
42	UPT	PIPX180	PASS	AISC- H1-2	0.377 22
		153149.44 C		0.00	3416.17
43	UPT	PIPX180	PASS	AISC- H1-1	0.352 22
		159168.59 C		0.00	1337.17
44	UPT	PIPX180	PASS	AISC- H1-1	0.350 22
		160178.02 C		0.00	1073.15
45	UPT	PIPX180	PASS	AISC- H1-1	0.353 22
		161136.28 C		0.00	1101.32

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ALL UNITS ARE - KG METK (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ MZ	LOADING/ LOCATION
48	UPT PIPX180	PASS	AISC- H1-3	0.353	22
	159167.81 C		0.00	1348.45	0.00
49	UPT PIPX180	PASS	AISC- H1-1	0.351	22
	160177.23 C		0.00	1081.35	1.00
50	UPT PIPX180	PASS	AISC- H1-1	0.353	22
	160963.14 C		0.00	1143.06	0.92
51	UPT PIPX180	PASS	AISC- H1-2	0.399	22
	161230.16 C		0.00	3663.64	1.00
52	LD L40 4010	PASS	AISC- H1-1	0.441	21
	25785.36-C		-9.33	144.96	0.00
53	LD L40 4010	PASS	AISC- H1-3	0.207	22
	11840.74 C		-59.35	32.06	0.00
54	LD L40 4010	PASS	AISC- H1-1	0.261	22
	13115.40 C		54.27	95.45	0.00
55	LD L40 4010	PASS	AISC- H1-3	0.139	24
	6924.69 C		-66.78	20.38	0.00
56	LD L40 4010	PASS	AISC- H1-1	0.271	22
	15780.01 C		-47.28	67.40	0.00
57	LD L40 4010	PASS	AISC- H1-1	0.204	21
	12246.56 C		-58.30	30.79	0.00
58	LD L40 4010	PASS	AISC- H1-1	0.269	21
	18261.44 C		12.19	-42.48	0.22
59	LD L40 4010	PASS	AISC- H1-1	0.264	21
	16579.51 C		14.07	65.02	1.30
60	LD L40 4010	PASS	AISC- H1-1	0.234	21
	35003.32 C		14.00	-51.18	0.00
61	LD L40 4010	PASS	AISC- H1-1	0.212	21
	14281.43 C		-8.08	38.26	0.00
62	LD L40 4010	PASS	AISC- H1-3	0.205	21
	11782.69 C		-58.87	30.11	1.30
63	LD L40 4010	PASS	AISC- H1-1	0.177	22
	12274.79 C		2.33	28.94	1.30
64	LD L40 4010	PASS	AISC- H1-1	0.207	22
	12305.82 C		1.73	-69.39	1.30
65	LD L40 4010	PASS	AISC- H1-1	0.207	22
	12416.46 C		5.52	65.04	0.00
66	LD L40 4010	PASS	AISC- H1-1	0.180	22
	12686.91 C		0.19	-26.78	1.08
67	LD L40 4010	PASS	AISC- H1-1	0.269	22
	15607.80 C		-45.70	68.82	1.30
68	LD L40 4010	PASS	AISC- H1-3	0.152	23
	10258.52 C		5.15	-23.67	0.54

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ALL UNITS ARE - KG METK (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ MZ	LOADING/ LOCATION
71	LD L40 4010	PASS	AISC- H1-3	0.136	23
	8171.23 C		-6.24	35.03	0.00
72	LD L40 4010	PASS	AISC- H1-3	0.134	23
	6918.56 C		-56.19	20.58	1.30
73	LD L40 4010	PASS	AISC- H1-3	0.135	24
	8149.18 C		-3.58	35.91	1.30
74	LD L40 4010	PASS	AISC- H1-3	0.158	24
	8540.11 C		5.35	-56.38	1.30
75	LD L40 4010	PASS	AISC- H1-3	0.175	24
	9352.89 C		7.54	64.15	0.00
76	LD L40 4010	PASS	AISC- H1-3	0.152	24
	10229.71 C		6.65	-23.38	0.54
77	LD L40 4010	PASS	AISC- H1-1	0.260	22
	12943.17 C		55.72	96.87	1.30
78	LD L40 4010	PASS	AISC- H1-1	0.217	21
	14720.18 C		-7.24	37.31	1.30
79	LD L40 4010	PASS	AISC- H1-1	0.240	21
	15413.11 C		14.65	-51.79	1.30
80	LD L40 4010	PASS	AISC- H1-1	0.270	21
	16949.42 C		14.69	65.91	0.00
81	LD L40 4010	PASS	AISC- H1-1	0.273	21
	18658.16 C		17.73	-42.33	0.98
82	LD L40 4010	PASS	AISC- H1-1	0.443	21
	25958.31 C		-10.77	143.54	1.30
83	LD L40 4010	PASS	AISC- H1-1	0.184	22
	12988.98 C		-0.07	-27.08	0.11
84	LD L40 4010	PASS	AISC- H1-1	0.233	22
	12785.88 C		6.76	65.88	1.30
85	LD L40 4010	PASS	AISC- H1-1	0.233	22
	12715.58 C		3.02	-70.08	0.00
86	LD L40 4010	PASS	AISC- H1-1	0.184	22
	12681.93 C		-27.23	16.08	1.30
87	LD L40 4010	PASS	AISC- H1-1	0.206	22
	12299.56 C		-57.32	32.86	1.30
88	LD L30 308	PASS	AISC- H2-1	0.206	21
	8730.09 T		17.79	12.83	0.00
89	LD L30 308	PASS	AISC- H1-1	0.285	21
	7100.29 C		-15.25	66.13	0.00
90	LD L30 308	PASS	AISC- H2-1	0.088	21
	3050.06 T		0.87	16.15	0.00
91	LD L30 308	PASS	AISC- H1-3	0.106	21
	2386.08 C		-2.81	21.87	0.00

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TORRE AUTOSOPORTADA

-- PAGE NO. 41

ALL UNITS ARE - KG NETS (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
94	LD L30 308	PASS	AISC- H1-3	0.096	24
		866.64 C	5.24	24.36	0.00
95	LD L30 308	PASS	AISC- H1-3	0.120	21
		1259.03 C	-0.85	27.23	0.00
96	LD L40 4010	PASS	AISC- H2-1	0.080	21
		4831.19 T	2.09	30.72	0.00
97	LD L40 4010	PASS	AISC- H1-3	0.105	21
		1696.16 C	2.33	97.61	0.00
98	LD L40 4010	PASS	AISC- H2-1	0.065	23
		1326.17 T	2.76	57.92	0.00
99	LD L30 308	PASS	AISC- H1-3	0.082	23
		759.47 C	-0.24	21.43	3.24
100	LD L30 308	PASS	AISC- H1-3	0.069	22
		327.69 C	7.85	23.35	3.13
101	LD L30 308	PASS	AISC- H1-3	0.122	23
		1586.73 C	2.88	34.31	2.51
102	LD L30 308	PASS	AISC- H2-1	0.067	20
		242.75 T	-1.64	32.52	0.00
103	LD L30 308	PASS	AISC- H1-3	0.056	23
		1251.55 C	-1.01	11.71	1.82
104	LD L30 308	PASS	AISC- H2-1	0.054	23
		1572.61 T	1.18	12.53	1.57
105	LD L30 308	PASS	AISC- H1-3	0.157	23
		3633.86 C	-6.89	36.42	1.24
106	LD L30 308	PASS	AISC- H2-1	0.108	23
		4489.02 T	7.42	8.72	0.78
107	LD L30 308	PASS	AISC- H2-1	0.155	22
		6270.39 T	33.48	-0.53	0.00
108	LD L30 308	PASS	AISC- H1-3	0.196	22
		5045.95 C	-5.69	40.95	0.00
109	LD L30 308	PASS	AISC- H1-3	0.098	21
		2274.31 C	-23.28	8.74	0.00
110	LD L30 308	PASS	AISC- H1-3	0.095	22
		2403.68 C	-1.44	16.29	0.00
111	LD L30 308	PASS	AISC- H2-1	0.136	22
		1521.98 T	-43.27	31.67	2.35
112	LD L30 308	PASS	AISC- H1-3	0.236	22
		3160.76 C	47.68	39.18	0.00
113	LD L30 308	PASS	AISC- H1-3	0.140	21
		1632.83 C	-7.57	26.24	0.00
114	LD L30 308	PASS	AISC- H1-3	0.154	22
		1812.56 C	-2.60	27.99	0.00

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TORRE AUTOSOPORTADA

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ALL UNITS ARE - KG NETS (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
117	LD L40 4010	PASS	AISC- H2-1	0.084	22
		2359.22 T	-3.56	66.71	0.00
118	LD L30 308	PASS	AISC- H1-3	0.158	22
		1877.97 C	-2.47	28.39	3.24
119	LD L30 308	PASS	AISC- H1-3	0.146	21
		1715.93 C	-1.83	26.68	3.13
120	LD L30 308	PASS	AISC- H1-3	0.238	22
		3239.29 C	47.31	39.39	2.51
121	LD L30 308	PASS	AISC- H2-1	0.138	22
		1618.87 T	-43.01	31.91	0.00
122	LD L30 308	PASS	AISC- H1-3	0.098	22
		2499.15 C	-1.46	16.67	1.82
123	LD L30 308	PASS	AISC- H1-3	0.102	21
		2399.13 C	-23.16	9.36	1.57
124	LD L30 308	PASS	AISC- H1-3	0.199	22
		5164.41 C	-5.86	41.21	1.24
125	LD L30 308	PASS	AISC- H2-1	0.159	22
		6420.64 T	32.82	-1.38	0.78
126	LD L40 4010	PASS	AISC- H1-3	0.204	20
		814.52 C	5.31	225.18	0.00
127	LD L40 4010	PASS	AISC- H1-3	0.242	22
		2713.44 C	-106.01	171.12	0.00
129	LD L40 4010	PASS	AISC- H2-1	0.109	23
		5164.59 T	5.73	58.53	0.00
130	LD L40 4010	PASS	AISC- H2-1	0.107	22
		5018.19 T	13.63	52.62	1.93
132	LD L40 4010	PASS	AISC- H2-1	0.103	22
		4718.88 T	14.02	51.51	0.00
133	LD L40 4010	PASS	AISC- H2-1	0.078	24
		4242.13 T	10.46	30.41	0.00
135	LD L40 4010	PASS	AISC- H2-1	0.083	22
		2283.98 T	-3.73	67.01	1.93
137	LD L40 4010	PASS	AISC- H1-3	0.095	21
		4094.78 C	-28.83	26.83	1.98
138	LD L30 308	PASS	AISC- H1-3	0.042	22
		153.34 C	-14.94	11.75	1.00
139	LD L30 308	PASS	AISC- H1-3	0.156	22
		3788.33 C	-8.27	18.26	0.00
140	LD L30 308	PASS	AISC- H2-1	0.110	22
		1624.70 T	62.63	5.02	0.00
141	LD L30 308	PASS	AISC- H1-3	0.142	22
		3395.36 C	-5.61	18.89	0.00

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TORRE AUTOSOPORTADA

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ M2	LOADING/ LOCATION
144	LD L30 308	PASS	AISC- H2-1	0.107	22
		1654.42 T	-62.91	3.14	0.00
145	LD L30 308	PASS	AISC- H1-3	0.144	22
		3458.65 C	5.56	18.79	0.00
147	LD L40 4010	PASS	AISC- H2-1	0.204	21
		10248.31 T	10.02	101.78	0.00
148	LD L40 4010	PASS	AISC- H2-1	0.185	21
		10287.85 T	9.46	79.19	1.93
150	LD L40 4010	PASS	AISC- H2-1	0.121	23
		6543.37 T	7.45	53.63	0.00
151	LD L40 4010	PASS	AISC- H1-3	0.109	21
		6055.46 C	-29.89	-10.51	1.45
153	LD L40 4010	PASS	AISC- H2-1	0.050	22
		80.27 T	28.39	40.84	1.98
155	LD L40 4010	PASS	AISC- H2-1	0.103	21
		2386.13 T	5.55	87.66	1.93
156	LD L30 308	PASS	AISC- H2-1	0.054	21
		60.04 T	-46.39	-0.30	0.00
157	LD L30 308	PASS	AISC- H1-3	0.095	23
		2072.45 C	0.20	17.72	0.00
158	LD L30 308	PASS	AISC- H2-1	0.069	23
		855.46 T	-45.17	1.30	0.00
159	LD L30 308	PASS	AISC- H1-3	0.087	23
		1718.64 C	3.68	17.21	0.00
160	LD L30 308	PASS	AISC- H2-1	0.041	22
		115.44 T	33.21	0.77	0.00
161	LD L30 308	PASS	AISC- H1-3	0.134	21
		2920.83 C	-2.92	23.80	0.00
162	LD L30 308	PASS	AISC- H2-1	0.109	21
		1243.07 T	71.18	3.10	0.00
163	LD L30 308	PASS	AISC- H1-3	0.132	21
		2633.29 C	-6.47	24.73	0.00
164	LD L30 308	PASS	AISC- H2-1	0.206	21
		8580.04 T	-18.43	13.68	0.00
165	LD L30 308	PASS	AISC- H1-1	0.292	21
		6981.77 C	15.09	65.86	0.00
166	LD L30 308	PASS	AISC- H2-1	0.087	21
		2925.19 T	-0.94	16.77	0.00
167	LD L30 308	PASS	AISC- H1-3	0.103	21
		2290.77 C	2.80	21.48	0.00
168	LD L30 308	PASS	AISC- H2-1	0.104	24
		1113.58 T	25.71	28.94	2.35

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TORRE AUTOSOPORTADA

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ M2	LOADING/ LOCATION
171	LD L30 308	PASS	AISC- H1-3	0.116	21
		1193.46 C	0.79	26.84	0.00
172	LD L40 4010	PASS	AISC- H2-1	0.079	21
		4701.06 T	-1.42	31.15	0.00
173	LD L40 4010	PASS	AISC- H1-3	0.108	21
		1691.78 C	-1.68	103.23	0.00
174	LD L40 4010	PASS	AISC- H2-1	0.065	24
		1326.24 T	2.35	58.19	0.00
175	LD L30 308	PASS	AISC- H1-3	0.084	24
		777.90 C	1.27	21.43	3.24
176	LD L30 308	PASS	AISC- H1-3	0.074	22
		412.96 C	-7.56	23.78	3.13
177	LD L30 308	PASS	AISC- H1-3	0.153	24
		1583.23 C	-50.28	34.63	2.51
178	LD L30 308	PASS	AISC- H2-1	0.093	24
		619.70 T	27.59	27.17	0.00
179	LD L30 308	PASS	AISC- H1-3	0.056	24
		1261.91 C	-1.31	11.73	1.82
180	LD L30 308	PASS	AISC- H2-1	0.070	24
		1566.67 T	-15.73	12.33	1.57
181	LD L30 308	PASS	AISC- H1-3	0.155	24
		3627.63 C	4.83	36.38	1.24
182	LD L30 308	PASS	AISC- H2-1	0.128	24
		4466.72 T	-26.21	8.56	0.78
183	LD L40 4010	PASS	AISC- H1-3	0.204	20
		814.62 C	5.25	225.24	0.00
185	LD L40 4010	PASS	AISC- H2-1	0.189	21
		10584.33 T	8.81	80.34	0.00
186	LD L40 4010	PASS	AISC- H1-3	0.162	22
		7276.30 C	15.41	62.57	1.93
188	LD L40 4010	PASS	AISC- H2-1	0.146	22
		6690.04 T	-4.90	83.41	0.00
189	LD L40 4010	PASS	AISC- H2-1	0.122	24
		6577.28 T	7.40	53.74	1.93
191	LD L40 4010	PASS	AISC- H2-1	0.102	23
		2314.03 T	-5.35	87.93	1.93
193	LD L40 4010	PASS	AISC- H2-1	0.062	24
		2911.47 T	-27.78	18.43	1.98
194	LD L30 308	PASS	AISC- H1-3	0.043	24
		45.02 C	20.94	9.98	1.00
195	LD L30 308	PASS	AISC- H1-3	0.132	21
		2853.34 C	2.40	23.95	0.00

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TORRE AUTOSORTA/TA

ALL UNITS ARE - MG METE (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/ PK	CRITICAL COND/ MG	RATIO/ MG	LOADING/ LOCATION
198	LD	L30 308	PASS	0.057	21
		65.11	AISC-H2-1	1.09	0.00
199	LD	L30 308	PASS	0.098	24
		2084.5	AISC-H1-3	17.75	0.00
200	LD	L30 308	PASS	0.070	24
		860.42	AISC-H2-1	1.45	0.00
201	LD	L30 308	PASS	0.088	24
		1728.6	AISC-H1-3	17.18	0.00
202	LD	L40 4010	PASS	0.241	22
		2713.6	AISC-H1-3	170.07	2.50
203	LD	L40 4010	PASS	0.204	20
		914.70	AISC-H1-3	225.25	2.50
204	LD	L40 4010	PASS	0.211	24
		1606.78	AISC-H1-3	180.08	2.50
205	LD	L40 4010	PASS	0.111	20
		4.37	AISC-H2-1	-132.96	2.42
206	LD	L40 4010	PASS	0.111	20
		4.34	AISC-H2-1	-132.96	2.42
207	LD	L40 4010	PASS	0.111	20
		4.70	AISC-H2-1	-132.96	2.42
208	LD	L40 4010	PASS	0.069	21
		235.23	AISC-H1-3	70.87	0.00
209	LD	L40 4010	PASS	0.065	23
		357.14	AISC-H1-3	69.60	0.00
210	LD	L40 4010	PASS	0.068	24
		235.31	AISC-H1-3	70.15	0.00
222	LD	L40 4010	PASS	0.106	22
		4271.03	AISC-H2-1	14.16	0.00
223	UPT	P1PX180	PASS	0.316	22
		375094	AISC-H1-2	3203.83	0.00
224	UPT	P1PX180	PASS	0.322	22
		145687.58	AISC-H1-1	3205.31	0.00
225	UPT	P1PX180	PASS	0.319	22
		146686.42	AISC-H1-1	839.44	1.00
226	UPT	P1PX180	PASS	0.323	22
		147303.92	AISC-H1-1	1039.14	1.00
227	UPT	P1PX180	PASS	0.365	22
		147623.39	AISC-H1-2	3380.43	1.00
228	UPT	P1PX180	PASS	0.345	22
		139693.67	AISC-H1-2	3197.08	0.00
229	UPT	P1PX180	PASS	0.322	22
		145686.77	AISC-H1-1	1195.04	0.00

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TORRE AUTOSORTA/TA

ALL UNITS ARE - MG METE (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/ PK	CRITICAL COND/ MG	RATIO/ MG	LOADING/ LOCATION
232	UPT	P1PX180	PASS	0.367	22
		147622.47	AISC-H1-2	3475.56	1.00
233	LD	L40 4010	PASS	0.249	22
		12482.23	AISC-H1-1	85.15	0.00
234	LD	L40 4010	PASS	0.136	24
		6950.38	AISC-H1-3	21.24	0.00
235	LD	L40 4010	PASS	0.145	23
		9993.43	AISC-H1-3	-22.75	0.53
236	LD	L40 4010	PASS	0.167	23
		9200.47	AISC-H1-3	57.64	1.27
237	LD	L40 4010	PASS	0.150	23
		8449.08	AISC-H1-3	-49.92	0.00
238	LD	L40 4010	PASS	0.130	23
		8116.39	AISC-H1-3	29.55	0.00
239	LD	L40 4010	PASS	0.128	22
		8941.29	AISC-H1-3	-44.30	1.27
240	LD	L40 4010	PASS	0.128	24
		8083.12	AISC-H1-3	39.828	1.27
241	LD	L40 4010	PASS	0.150	24
		8426.55	AISC-H1-3	-51.04	1.27
242	LD	L40 4010	PASS	0.165	24
		9137.11	AISC-H1-3	56.18	0.00
243	LD	L40 4010	PASS	0.149	24
		5966.55	AISC-H1-3	-22.30	0.84
244	LD	L40 4010	PASS	0.244	22
		32142.69	AISC-H1-1	85.56	1.27
245	LD	L30 308	PASS	0.184	22
		5787.06	AISC-H2-1	0.19	0.00
246	LD	L30 308	PASS	0.186	22
		4866.00	AISC-H1-3	37.63	0.00
247	LD	L30 308	PASS	0.097	21
		2078.85	AISC-H1-3	7.13	0.00
248	LD	L30 308	PASS	0.090	22
		2215.15	AISC-H1-3	15.04	0.00
249	LD	L30 308	PASS	0.130	22
		1367.09	AISC-H2-1	27.90	2.20
250	LD	L30 308	PASS	0.271	22
		2789.55	AISC-H1-3	70.31	0.00
251	LD	L30 308	PASS	0.118	22
		1515.18	AISC-H2-1	23.13	0.00
252	LD	L30 308	PASS	0.128	22
		1530.79	AISC-H1-3	-3.01	0.00

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ALL UNITS ARE - KG MKTE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
255	LD L40 4010	PASS	AISC- H2-1	0.077	22
	2050.41 T		-4.68	61.74	0.00
256	LD L30 308	PASS	AISC- H1-3	0.132	22
	1684.67 C		-3.08	25.16	3.05
257	LD L30 308	PASS	AISC- H1-3	0.122	21
	1583.83 C		-6.92	23.51	2.93
258	LD L30 308	PASS	AISC- H1-3	0.233	22
	7849.71 C		66.44	35.41	2.37
259	LD L30 308	PASS	AISC- H2-1	0.151	22
	1440.00 T		-64.13	27.99	0.00
260	LD L30 308	PASS	AISC- H1-3	0.092	22
	2288.97 C		-4.62	15.28	1.73
261	LD L30 308	PASS	AISC- H1-3	0.100	21
	2173.75 C		-30.78	7.72	1.47
262	LD L30 308	PASS	AISC- H1-3	0.188	22
	4966.85 C		-6.61	37.73	1.21
263	LD L30 308	PASS	AISC- H2-1	0.167	22
	5910.74 T		49.67	-0.87	0.73
264	LD L40 4010	PASS	AISC- H1-3	0.239	22
	2336.15 C		150.35	148.70	0.00
277	LD L40 4010	PASS	AISC- H2-1	0.111	22
	4627.40 T		105.08	-5.86	0.78
278	LD L40 4010	PASS	AISC- H2-1	0.100	22
	4598.16 T		17.87	47.93	1.81
280	LD L40 4010	PASS	AISC- H2-1	0.095	22
	4238.51 T		18.40	46.28	0.00
281	LD L40 4010	PASS	AISC- H1-3	0.077	23
	3552.33 C		-25.20	18.64	1.81
283	LD L40 4010	PASS	AISC- H2-1	0.077	22
	1986.21 T		-4.71	62.35	1.81
285	LD L40 4010	PASS	AISC- H1-3	0.086	21
	3773.66 C		51.13	8.50	0.00
286	LD L30 308	PASS	AISC- H1-3	0.041	22
	147.96 C		-14.06	11.55	1.00
287	LD L30 308	PASS	AISC- H1-3	0.140	22
	3508.76 C		-9.83	15.85	0.00
288	LD L30 308	PASS	AISC- H2-1	0.099	22
	1584.94 T		56.83	3.32	0.00
289	LD L30 308	PASS	AISC- H1-3	0.128	22
	3161.79 C		-5.57	17.29	0.00
290	LD L30 308	PASS	AISC- H1-3	0.042	22
	140.04 C		15.32	11.56	1.00

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ALL UNITS ARE - KG MKTE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
293	LD L30 308	PASS	AISC- H1-3	0.130	22
	3218.96 C		5.80	17.16	0.00
294	LD L40 4010	PASS	AISC- H1-3	0.241	22
	2335.94 C		151.00	150.21	2.35
295	LD L40 4010	PASS	AISC- H2-1	0.210	21
	10215.60 T		13.92	107.28	0.78
296	UPT P1PX180	PASS	AISC- H1-2	0.631	21
	254539.72 C		0.00	5933.20	0.00
297	UPT P1PX180	PASS	AISC- H1-1	0.587	21
	265422.34 C		0.00	2156.99	0.00
298	UPT P1PX180	PASS	AISC- H1-1	0.572	21
	267649.84 C		0.00	881.48	0.00
299	UPT P1PX180	PASS	AISC- H1-1	0.578	21
	268929.34 C		0.00	1125.49	1.00
300	UPT P1PX180	PASS	AISC- H1-2	0.628	21
	269598.59 C		0.00	4016.94	1.00
301	LD L40 4010	PASS	AISC- H1-1	0.431	21
	25824.96 C		11.46	132.08	0.00
302	LD L40 4010	PASS	AISC- H1-1	0.212	22
	12724.11 C		55.59	35.78	0.00
303	LD L40 4010	PASS	AISC- H1-1	0.186	22
	13093.44 C		20.21	16.99	0.00
304	LD L40 4010	PASS	AISC- H1-1	0.212	22
	13124.09 C		-2.19	-63.37	1.27
305	LD L40 4010	PASS	AISC- H1-3	0.213	22
	13184.01 C		-9.16	59.33	0.00
306	LD L40 4010	PASS	AISC- H1-1	0.195	22
	13429.04 C		-15.81	-26.05	1.16
307	LD L40 4010	PASS	AISC- H1-1	0.268	22
	16200.82 C		34.75	64.56	1.27
308	LD L40 4010	PASS	AISC- H1-1	0.275	21
	18803.05 C		-13.31	-43.30	0.11
309	LD L40 4010	PASS	AISC- H1-1	0.265	21
	17152.67 C		-13.03	59.09	1.27
310	LD L40 4010	PASS	AISC- H1-1	0.238	21
	15693.39 C		-13.05	-46.90	0.00
311	LD L40 4010	PASS	AISC- H1-1	0.215	21
	15053.55 C		6.88	30.66	0.00
312	LD L40 4010	PASS	AISC- H1-1	0.205	21
	12672.62 C		47.02	33.57	1.27
313	LD L30 308	PASS	AISC- H2-1	0.197	21
	7918.79 T		-15.64	17.47	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ ME	LOADING/ LOCATION
316	LD L30 308	PASS	AISC- H1-3	0.096	21
		2147.01 C		2.47	0.00
317	LD L30 308	PASS	AISC- H2-1	0.106	24
		873.53 T		38.55	2.20
318	LD L30 308	PASS	AISC- H1-3	0.144	24
		1577.25 C		-40.51	0.00
319	LD L30 308	PASS	AISC- H1-3	0.078	23
		746.99 C		4.25	0.00
320	LD L30 308	PASS	AISC- H1-3	0.098	21
		1081.74 C		0.70	0.00
321	LD L40 4010	PASS	AISC- H2-1	0.075	21
		4147.79 T		0.11	0.00
322	LD L40 4010	PASS	AISC- H1-3	0.115	21
		1629.29 C		-7.89	106.92
323	LD L40 4010	PASS	AISC- H2-1	0.062	24
		1192.82 T		2.98	0.00
324	LD L30 308	PASS	AISC- H1-3	0.075	24
		758.98 C		1.60	19.37
325	LD L30 308	PASS	AISC- H1-3	0.073	22
		369.73 C		-14.58	21.50
326	LD L30 308	PASS	AISC- H1-3	0.153	24
		1463.58 C		-40.90	32.26
327	LD L30 308	PASS	AISC- H2-1	0.103	24
		678.07 T		39.43	24.64
328	LD L30 308	PASS	AISC- H1-3	0.058	24
		1281.14 C		3.22	11.66
329	LD L30 308	PASS	AISC- H2-1	0.074	24
		1545.75 T		-20.25	12.13
330	LD L30 308	PASS	AISC- H1-3	0.151	24
		3598.06 C		6.04	34.26
331	LD L30 308	PASS	AISC- H2-1	0.139	24
		4227.90 T		-36.43	10.28
332	LD L40 4010	PASS	AISC- H1-3	0.204	24
		1339.25 C		-91.79	161.65
333	LD L40 4010	PASS	AISC- H2-1	0.184	23
		392.58 T		85.32	161.12
335	LD L40 4010	PASS	AISC- H2-1	0.186	21
		10264.99 T		13.17	78.48
336	LD L40 4010	PASS	AISC- H1-3	0.158	22
		7313.23 C		16.58	58.69
338	LD L40 4010	PASS	AISC- H2-1	0.151	22
		6656.03 T		-6.29	88.13

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ ME	LOADING/ LOCATION
343	LD L40 4010	PASS	AISC- H2-1	0.060	24
		2687.51 T		-27.77	18.99
344	LD L30 308	PASS	AISC- H2-1	0.047	22
		149.46 T		-37.75	0.78
345	LD L30 308	PASS	AISC- H1-3	0.119	21
		2587.81 C		3.90	21.89
346	LD L30 308	PASS	AISC- H2-1	0.100	21
		1161.29 T		-65.09	2.80
347	LD L30 308	PASS	AISC- H1-3	0.117	21
		2362.29 C		5.90	23.27
348	LD L30 308	PASS	AISC- H2-1	0.059	23
		96.86 T		50.16	-0.46
349	LD L30 308	PASS	AISC- H1-3	0.091	24
		1968.33 C		-4.27	16.19
350	LD L30 308	PASS	AISC- H2-1	0.067	24
		861.56 T		42.33	1.66
351	LD L30 308	PASS	AISC- H1-3	0.082	24
		1664.88 C		-4.25	15.78
352	LD L40 4010	PASS	AISC- H2-1	0.143	22
		6299.42 T		-5.89	84.44
353	LD L40 4010	PASS	AISC- H1-1	0.264	22
		15861.83 C		35.13	65.07
354	LD L40 4010	PASS	AISC- H1-1	0.197	21
		12095.19 C		47.56	32.44
355	LD L40 4010	PASS	AISC- H1-1	0.189	22
		12983.72 C		-14.57	-25.77
356	LD L40 4010	PASS	AISC- H1-1	0.206	22
		12679.81 C		-7.86	58.29
357	LD L40 4010	PASS	AISC- H1-1	0.205	22
		12588.91 C		-0.79	-62.78
358	LD L40 4010	PASS	AISC- H1-1	0.179	22
		12568.14 C		-6.08	25.30
359	LD L40 4010	PASS	AISC- H1-1	0.204	22
		12346.70 C		56.17	34.65
360	LD L40 4010	PASS	AISC- H1-1	0.209	21
		14493.32 C		7.60	31.48
361	LD L40 4010	PASS	AISC- H1-1	0.230	21
		15158.18 C		-11.70	-46.30
362	LD L40 4010	PASS	AISC- H1-1	0.258	21
		16648.67 C		-11.80	58.04
363	LD L40 4010	PASS	AISC- H1-1	0.268	21
		18351.82 C		-13.17	-42.57

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ALL UNITS ARE - RD METR (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ M2	LOADING/ LOCATION
366	LD L30 308	PASS	AISC- H1-3	0.153	23
		3545.63 C	7.53	34.31	0.00
367	LD L30 308	PASS	AISC- H2-1	0.055	22
		382.10 T	-13.71	17.57	0.00
368	LD L30 308	PASS	AISC- H1-3	0.055	23
		1258.86 C	1.00	11.63	0.00
369	LD L30 308	PASS	AISC- H1-3	0.062	29
		305.89 C	8.52	23.11	2.20
370	LD L30 308	PASS	AISC- H1-3	0.109	23
		1471.87 C	-1.65	31.79	0.00
371	LD L30 308	PASS	AISC- H1-3	0.070	22
		301.24 C	-14.88	21.13	0.00
372	LD L30 308	PASS	AISC- H1-3	0.072	23
		739.39 C	0.13	19.40	0.00
373	LD L40 4010	PASS	AISC- H2-1	0.058	22
		69.26 T	-44.97	40.40	0.00
374	LD L40 4010	PASS	AISC- H1-3	0.111	21
		1628.30 C	-8.28	102.32	1.57
375	LD L40 4010	PASS	AISC- H2-1	0.097	21
		2051.91 T	-8.12	84.03	0.00
376	LD L30 308	PASS	AISC- H1-3	0.101	21
		1135.96 C	0.78	24.42	3.05
377	LD L30 308	PASS	AISC- H1-3	0.083	24
		723.03 C	-9.44	21.30	2.93
378	LD L30 308	PASS	AISC- H1-3	0.143	21
		2140.75 C	-1.04	39.21	2.37
379	LD L30 308	PASS	AISC- H2-1	0.070	21
		1277.75 T	0.42	24.67	0.00
380	LD L30 308	PASS	AISC- H1-3	0.098	21
		2220.75 C	2.49	20.92	1.73
381	LD L30 308	PASS	AISC- H2-1	0.086	21
		2794.10 T	0.99	17.84	1.47
382	LD L30 308	PASS	AISC- H1-1	0.272	21
		6839.50 C	15.92	62.12	1.21
383	LD L30 308	PASS	AISC- H2-1	0.197	21
		8042.34 T	-15.09	16.40	0.73
384	LD L40 4010	PASS	AISC- H1-3	0.177	20
		631.95 C	-3.76	198.23	0.00
387	LD L40 4010	PASS	AISC- H2-1	0.118	23
		6261.88 T	9.35	52.05	0.00
388	LD L40 4010	PASS	AISC- H1-3	0.115	21
		6104.44 C	-44.44	-10.13	1.36

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ALL UNITS ARE - RD METR (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ M2	LOADING/ LOCATION
393	LD L40 4010	PASS	AISC- H2-1	0.062	23
		1198.67 T	-4.43	55.82	1.81
395	LD L40 4010	PASS	AISC- H2-1	0.077	21
		4261.68 T	-0.51	34.90	1.86
396	LD L40 4010	PASS	AISC- H1-3	0.177	20
		631.96 C	-3.56	198.27	2.35
397	LD L30 308	PASS	AISC- H2-1	0.057	21
		88.91 T	-48.14	-0.43	0.00
398	LD L30 308	PASS	AISC- H1-3	0.087	23
		1951.78 C	0.93	16.19	0.00
399	LD L30 308	PASS	AISC- H2-1	0.066	23
		853.89 T	-42.15	1.42	0.00
400	LD L30 308	PASS	AISC- H1-3	0.081	23
		1650.10 C	3.73	15.81	0.00
401	LD L30 308	PASS	AISC- H2-1	0.044	22
		141.52 T	35.73	0.76	0.00
402	LD L30 308	PASS	AISC- H1-3	0.120	21
		2647.85 C	-4.29	21.63	0.00
403	LD L30 308	PASS	AISC- H2-1	0.100	21
		1188.80 T	64.78	2.96	0.00
404	LD L30 308	PASS	AISC- H1-3	0.119	21
		2419.30 C	-6.12	23.14	0.00
405	LD L40 4010	PASS	AISC- H2-1	0.098	20
		3.24 T	0.01	-117.04	2.27
406	LD L40 4010	PASS	AISC- H2-1	0.098	20
		3.24 T	0.01	-117.04	2.27
407	LD L40 4010	PASS	AISC- H2-1	0.098	20
		3.25 T	-0.01	-117.06	2.28
408	LD L40 4010	PASS	AISC- H1-3	0.076	21
		296.01 C	3.57	77.22	0.00
409	LD L40 4010	PASS	AISC- H1-3	0.068	23
		190.39 C	3.81	71.47	0.00
410	LD L40 4010	PASS	AISC- H1-3	0.075	21
		295.95 C	3.18	76.05	4.40
411	LD L40 4010	PASS	AISC- H2-1	0.143	22
		6028.86 T	-7.64	86.68	0.73
412	LD L40 4010	PASS	AISC- H2-1	0.109	22
		4187.62 T	112.74	-3.52	0.00
413	LD L40 4010	PASS	AISC- H2-1	0.209	0.00
		9915.30 T	12.71	110.16	0.73
414	UPT P1P180	PASS	AISC- H1-2	0.574	21
		230219.31 C	0.00	5517.95	0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MG	LOADING/ LOCATION
417	UPT PIPX180	PASS	AISC- H1-1	0.526	21
	244550.16 C	0.00		1062.72	1.00
418	UPT PIPX180	PASS	AISC- H1-2	0.565	21
	245241.95 C	0.00		3335.57	1.00
419	UPT PIPX180	PASS	AISC- H1-2	0.314	22
	125770.91 C	0.00		3024.34	0.00
420	UPT PIPX180	PASS	AISC- H1-1	0.291	22
	131655.38 C	0.00		1094.61	0.00
421	UPT PIPX180	PASS	AISC- H1-1	0.288	22
	132655.03 C	0.00		757.18	1.00
422	UPT PIPX180	PASS	AISC- H1-1	0.294	22
	133408.94 C	0.00		1029.44	1.00
423	UPT PIPX180	PASS	AISC- H1-2	0.335	22
	133663.55 C	0.00		3302.18	1.00
424	UPT PIPX180	PASS	AISC- H1-2	0.314	22
	125771.79 C	0.00		3031.26	0.00
425	UPT PIPX180	PASS	AISC- H1-1	0.291	22
	131656.38 C	0.00		1102.71	0.00
426	UPT PIPX180	PASS	AISC- H1-1	0.288	22
	132655.88 C	0.00		772.98	1.00
427	UPT PIPX180	PASS	AISC- H1-1	0.294	22
	133409.88 C	0.00		1006.98	1.00
428	UPT PIPX180	PASS	AISC- H1-2	0.333	22
	133664.64 C	0.00		3183.91	1.00
429	LD L40 4010	PASS	AISC- H1-1	0.419	21
	25673.26 C	11.34		121.37	0.00
430	LD L40 4010	PASS	AISC- H1-1	0.213	22
	12909.21 C	50.44		39.30	0.00
431	LD L40 4010	PASS	AISC- H1-1	0.275	21
	18881.88 C	-13.77		-43.04	0.00
432	LD L40 4010	PASS	AISC- H1-1	0.265	21
	17324.43 C	-17.28		54.28	1.24
433	LD L40 4010	PASS	AISC- H1-1	0.236	21
	15863.23 C	-16.38		-40.97	0.00
434	LD L40 4010	PASS	AISC- H1-1	0.213	21
	15234.94 C	7.82		25.75	0.00
435	LD L40 4010	PASS	AISC- H1-1	0.206	21
	12843.49 C	40.86		36.88	1.24
436	LD L40 4010	PASS	AISC- H1-1	0.186	22
	13192.13 C	15.77		18.83	0.00
437	LD L40 4010	PASS	AISC- H1-1	0.209	22
	13176.03 C	4.07		-57.72	1.24

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MG	LOADING/ LOCATION
440	LD L40 4010	PASS	AISC- H1-1	0.257	22
	15897.95 C	35.02		56.87	1.24
441	LD L30 308	PASS	AISC- H2-1	0.194	21
	7312.17 T	-15.80		21.42	0.00
442	LD L30 308	PASS	AISC- H1-3	0.270	21
	6529.16 C	15.84		57.66	0.00
443	LD L30 308	PASS	AISC- H2-1	0.083	21
	2423.89 T	0.23		20.13	0.00
444	LD L30 308	PASS	AISC- H1-3	0.090	21
	1997.49 C	3.10		19.68	0.00
445	LD L30 308	PASS	AISC- H2-1	0.107	24
	834.98 T	43.57		22.73	2.05
446	LD L30 308	PASS	AISC- H1-3	0.142	24
	1603.90 C	-44.51		22.98	0.00
447	LD L30 308	PASS	AISC- H1-3	0.072	23
	783.50 C	4.37		19.61	0.00
448	LD L30 308	PASS	AISC- H1-3	0.088	21
	1075.67 C	0.53		22.16	0.00
449	LD L40 4010	PASS	AISC- H2-1	0.075	21
	3860.70 T	1.60		37.24	0.00
450	LD L40 4010	PASS	AISC- H1-3	0.121	21
	1632.09 C	-11.90		112.34	0.00
451	LD L40 4010	PASS	AISC- H2-1	0.060	24
	1078.16 T	4.57		54.33	0.00
452	LD L30 308	PASS	AISC- H1-3	0.067	24
	772.37 C	1.51		17.60	2.86
453	LD L30 308	PASS	AISC- H1-3	0.075	22
	434.98 C	-17.96		19.98	2.73
454	LD L30 308	PASS	AISC- H1-3	0.150	24
	1457.11 C	-44.74		29.75	2.24
455	LD L30 308	PASS	AISC- H2-1	0.102	24
	601.33 T	44.51		21.99	0.00
456	LD L30 308	PASS	AISC- H1-3	0.055	24
	1205.99 C	4.03		10.98	1.65
457	LD L30 308	PASS	AISC- H2-1	0.075	24
	1411.93 T	-22.07		12.64	1.37
458	LD L30 308	PASS	AISC- H1-3	0.141	24
	3395.21 C	6.08		31.41	1.18
459	LD L30 308	PASS	AISC- H2-1	0.138	24
	3775.28 T	-39.31		12.49	0.68
460	LD L40 4010	PASS	AISC- H1-3	0.193	24
	1362.33 C	-100.99		143.59	0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
465 LD	L40 4010	PASS	AISC- H2-1	0.151	22
	6408.67 T		-8.17	90.88	0.00
466 LD	L40 4010	PASS	AISC- H2-1	0.116	22
	6488.17 T		-9.44	46.43	1.68
468 LD	L40 4010	PASS	AISC- H2-1	0.095	21
	1791.68 T		-10.05	83.25	1.68
470 LD	L40 4010	PASS	AISC- H1-3	0.066	22
	281.36 C		-51.69	41.74	1.73
473 LD	L40 4010	PASS	AISC- H2-1	0.176	23
	444.09 T		97.37	143.12	2.70
472 LD	L30 308	PASS	AISC- H2-1	0.050	22
	179.70 T		-39.96	0.79	0.00
473 LD	L30 308	PASS	AISC- H1-3	0.110	21
	2473.38 C		4.37	20.18	0.00
474 LD	L30 308	PASS	AISC- H2-1	0.096	21
	1179.94 T		-61.18	2.62	0.00
475 LD	L30 308	PASS	AISC- H1-3	0.110	21
	2307.94 C		5.74	21.72	0.00
476 LD	L30 308	PASS	AISC- H2-1	0.063	21
	122.86 T		52.38	1.01	0.00
477 LD	L30 308	PASS	AISC- H1-3	0.084	24
	1900.18 C		-4.49	14.66	0.00
478 LD	L30 308	PASS	AISC- H2-1	0.065	24
	884.72 T		40.02	1.77	0.00
479 LD	L30 308	PASS	AISC- H1-3	0.077	24
	1644.98 C		-4.11	14.52	0.00
480 LD	L40 4010	PASS	AISC- H1-1	0.415	21
	25320.68 C		-11.53	121.95	0.00
481 LD	L40 4010	PASS	AISC- H1-1	0.205	22
	12268.55 C		-51.03	37.86	0.00
482 LD	L40 4010	PASS	AISC- H1-1	0.243	22
	12474.32 C		64.38	78.50	0.00
483 LD	L40 4010	PASS	AISC- H1-3	0.133	24
	6957.19 C		-51.94	22.61	0.00
484 LD	L40 4010	PASS	AISC- H1-1	0.268	21
	18402.46 C		12.57	-42.72	0.00
485 LD	L40 4010	PASS	AISC- H1-1	0.256	21
	16776.80 C		15.69	53.05	1.24
486 LD	L40 4010	PASS	AISC- H1-1	0.227	21
	15277.37 C		14.66	-40.24	0.00
487 LD	L40 4010	PASS	AISC- H1-1	0.206	21
	14618.33 C		-8.66	26.68	0.00

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ALL UNITS ARE - KG METK (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
490 LD	L40 4010	PASS	AISC- H1-1	0.202	22
	12590.27 C		-5.63	-56.99	1.24
491 LD	L40 4010	PASS	AISC- H1-1	0.198	22
	12618.35 C		3.69	52.44	0.00
492 LD	L40 4010	PASS	AISC- H1-1	0.189	22
	12867.94 C		20.60	-26.11	1.24
493 LD	L40 4010	PASS	AISC- H1-1	0.253	22
	15345.54 C		-35.09	57.45	1.24
494 LD	L30 308	PASS	AISC- H2-1	0.193	21
	7449.20 T		15.13	20.14	0.00
495 LD	L30 308	PASS	AISC- H1-3	0.273	21
	6646.83 C		-15.88	57.76	0.00
496 LD	L30 308	PASS	AISC- H2-1	0.084	21
	2534.92 T		-0.34	19.41	0.00
497 LD	L30 308	PASS	AISC- H1-3	0.093	21
	2085.19 C		-3.13	19.97	0.00
498 LD	L30 308	PASS	AISC- H2-1	0.065	23
	819.29 T		-7.44	22.31	2.05
499 LD	L30 308	PASS	AISC- H1-3	0.133	21
	2107.21 C		-0.15	36.58	0.00
500 LD	L30 308	PASS	AISC- H1-3	0.078	24
	755.98 C		11.09	19.38	0.00
501 LD	L30 308	PASS	AISC- H1-3	0.091	21
	1140.00 C		-0.63	22.55	0.00
502 LD	L40 4010	PASS	AISC- H2-1	0.076	21
	4001.41 T		-1.06	36.89	0.00
503 LD	L40 4010	PASS	AISC- H1-3	0.117	21
	1631.49 C		12.29	107.04	0.00
504 LD	L40 4010	PASS	AISC- H2-1	0.060	23
	1082.59 T		4.85	53.98	0.00
505 LD	L30 308	PASS	AISC- H1-3	0.065	23
	749.58 C		0.00	17.64	2.86
506 LD	L30 308	PASS	AISC- H1-3	0.071	22
	354.19 C		18.27	19.53	2.73
507 LD	L30 308	PASS	AISC- H1-3	0.100	23
	1468.30 C		0.84	29.20	2.24
508 LD	L30 308	PASS	AISC- H2-1	0.060	21
	140.95 T		-13.57	22.44	0.00
509 LD	L30 308	PASS	AISC- H1-3	0.051	23
	1176.90 C		-0.84	10.95	1.65
510 LD	L30 308	PASS	AISC- H2-1	0.059	22
	279.02 T		17.65	18.35	1.37

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ALL UNITS ARE - KG NETE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
513	LD L40 4010	PASS	AISC- H1-3	0.155	20
		613.38 C		2.14	173.14 0.00
514	LD L40 4010	PASS	AISC- H1-3	0.155	20
		613.13 C		1.97	173.12 2.20
516	LD L40 4010	PASS	AISC- H2-1	0.201	21
		9536.16 T		13.37	105.95 0.00
517	LD L40 4010	PASS	AISC- H2-1	0.177	21
		9586.67 T		12.60	76.30 1.68
519	LD L40 4010	PASS	AISC- H2-1	0.113	22
		5983.73 T		8.44	50.46 0.00
520	LD L40 4010	PASS	AISC- H1-3	0.119	21
		5990.37 C		-59.56	-9.58 1.54
522	LD L40 4010	PASS	AISC- H2-1	0.095	21
		1872.09 T		10.03	82.67 1.68
524	LD L40 4010	PASS	AISC- H1-3	0.064	22
		141.07 C		52.34	41.39 1.73
525	LD L30 308	PASS	AISC- H2-1	0.047	22
		169.89 T		37.60	0.76 0.00
526	LD L30 308	PASS	AISC- H1-3	0.112	21
		2547.76 C		-4.05	19.91 0.00
527	LD L30 308	PASS	AISC- H2-1	0.096	21
		1215.88 T		61.03	2.82 0.00
528	LD L30 308	PASS	AISC- H1-3	0.112	21
		2378.44 C		-5.95	21.61 0.00
529	LD L30 308	PASS	AISC- H2-1	0.060	21
		113.06 T		-50.02	0.97 0.00
530	LD L30 308	PASS	AISC- H1-3	0.080	23
		1881.99 C		1.18	14.65 0.00
531	LD L30 308	PASS	AISC- H2-1	0.063	23
		875.83 T		-39.80	1.32 0.00
532	LD L30 308	PASS	AISC- H1-3	0.075	23
		1628.63 C		3.30	14.56 0.00
533	LD L40 4010	PASS	AISC- H1-3	0.123	24
		8051.49 C		-1.54	26.31 1.24
534	LD L40 4010	PASS	AISC- H1-3	0.143	24
		8377.79 C		-1.22	-44.72 1.24
535	LD L40 4010	PASS	AISC- H1-3	0.157	24
		5059.71 C		3.67	49.69 0.00
536	LD L40 4010	PASS	AISC- H1-3	0.148	24
		9871.70 C		12.73	-20.95 0.93
537	LD L40 4010	PASS	AISC- H1-1	0.239	22
		12121.90 C		64.22	79.08 1.24

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ALL UNITS ARE - KG NETE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
540	LD L40 4010	PASS	AISC- H1-3	0.143	23
		8399.74 C		3.18	-43.54 0.00
541	LD L40 4010	PASS	AISC- H1-3	0.125	23
		8086.61 C		-5.76	24.90 0.00
542	LD L40 4010	PASS	AISC- H1-3	0.125	23
		6946.49 C		-35.80	22.72 1.24
543	LD L30 308	PASS	AISC- H2-1	0.163	22
		5360.64 T		53.60	1.82 0.00
544	LD L30 308	PASS	AISC- H1-3	0.178	22
		4733.68 C		-7.33	34.77 0.00
545	LD L30 308	PASS	AISC- H2-1	0.096	22
		2657.64 T		30.77	6.51 0.00
546	LD L30 308	PASS	AISC- H1-3	0.088	22
		2169.73 C		-6.11	14.47 0.00
547	LD L30 308	PASS	AISC- H2-1	0.154	22
		1266.72 T		-73.44	25.42 2.05
548	LD L30 308	PASS	AISC- H1-3	0.231	22
		2830.32 C		74.07	32.91 0.00
549	LD L30 308	PASS	AISC- H1-3	0.106	21
		1546.64 C		-5.97	20.93 0.00
550	LD L30 308	PASS	AISC- H1-3	0.117	22
		1661.29 C		-2.95	23.21 0.00
551	LD L40 4010	PASS	AISC- H1-3	0.095	23
		3628.73 C		56.19	8.28 1.73
552	LD L40 4010	PASS	AISC- H1-3	0.059	23
		519.43 C		2.91	60.98 0.00
553	LD L40 4010	PASS	AISC- H1-3	0.077	21
		1224.22 C		54.88	38.28 1.68
554	LD L30 308	PASS	AISC- H1-3	0.121	22
		1725.34 C		-3.04	23.60 2.86
555	LD L30 308	PASS	AISC- H1-3	0.110	21
		1627.40 C		-6.26	21.38 2.73
556	LD L30 308	PASS	AISC- H1-3	0.232	22
		2901.98 C		73.66	32.96 2.24
557	LD L30 308	PASS	AISC- H2-1	0.155	22
		1352.57 T		-73.14	25.55 0.00
558	LD L30 308	PASS	AISC- H1-3	0.091	22
		2257.84 C		-6.12	14.77 1.65
559	LD L30 308	PASS	AISC- H1-3	0.097	21
		2140.10 C		-31.92	6.54 1.37
560	LD L30 308	PASS	AISC- H1-3	0.180	22
		4850.95 C		-7.17	34.87 1.18

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
563 LD	L40 4010	PASS	AISC- H1-3	0.232	22
	2422.12 C	-167.41		129.68	2.20
565 LD	L40 4010	PASS	AISC- H2-1	0.114	22
	4566.76 T	111.83		-5.64	0.73
566 LD	L40 4010	PASS	AISC- H2-1	0.100	22
	4537.82 T	19.98		46.56	1.68
568 LD	L40 4010	PASS	AISC- H2-1	0.094	22
	4154.81 T	20.60		44.72	0.00
569 LD	L40 4010	PASS	AISC- H2-1	0.082	22
	995.27 T	-33.66		-64.17	1.68
571 LD	L40 4010	PASS	AISC- H1-3	0.088	21
	3768.34 C	55.74		0.85	0.00
573 LD	L40 4010	PASS	AISC- H1-3	0.076	21
	1144.81 C	55.55		37.75	0.00
574 LD	L30 308	PASS	AISC- H1-3	0.040	22
	148.59 C	-14.32		11.06	1.00
575 LD	L30 308	PASS	AISC- H1-3	0.134	22
	3495.70 C	-10.22		14.23	0.00
576 LD	L30 308	PASS	AISC- H2-1	0.101	22
	1673.59 T	55.77		3.67	0.00
577 LD	L30 308	PASS	AISC- H1-3	0.124	22
	3192.39 C	-5.62		16.14	0.00
578 LD	L30 308	PASS	AISC- H1-3	0.041	22
	138.81 C	15.83		11.07	1.00
579 LD	L30 308	PASS	AISC- H1-3	0.136	22
	3569.91 C	10.69		13.95	0.00
580 LD	L30 308	PASS	AISC- H2-1	0.101	22
	1709.45 T	-55.62		3.87	0.00
581 LD	L30 308	PASS	AISC- H1-3	0.126	22
	3262.72 C	5.86		16.02	0.00
582 LD	L40 4010	PASS	AISC- H2-1	0.085	20
	1.64 T	0.03		-102.14	2.12
583 LD	L40 4010	PASS	AISC- H2-1	0.085	20
	2.16 T	-0.01		-102.14	2.13
584 LD	L40 4010	PASS	AISC- H2-1	0.085	20
	2.16 T	-0.01		-102.14	2.13
585 LD	L40 4010	PASS	AISC- H1-3	0.074	21
	321.62 C	4.19		74.13	0.00
586 LD	L40 4010	PASS	AISC- H1-3	0.064	23
	207.11 C	3.98		66.76	0.00
587 LD	L40 4010	PASS	AISC- H1-3	0.072	21
	324.53 C	3.66		72.85	4.10

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
590 UPT	PIPX180	PASS	AISC- H1-1	0.466	21
	218369.56 C	0.00		716.00	0.00
591 UPT	PIPX180	PASS	AISC- H1-1	0.473	21
	219774.19 C	0.00		976.16	1.00
592 UPT	PIPX180	PASS	AISC- H1-2	0.503	21
	270401.94 C	0.00		2741.15	1.00
593 UPT	PIPX180	PASS	AISC- H1-2	0.281	22
	111855.27 C	0.00		2780.40	0.00
594 UPT	PIPX180	PASS	AISC- H1-1	0.262	22
	117578.75 C	0.00		1065.09	0.00
595 UPT	PIPX180	PASS	AISC- H1-1	0.258	22
	118539.44 C	0.00		685.36	1.00
596 UPT	PIPX180	PASS	AISC- H1-1	0.264	22
	119305.47 C	0.00		962.22	1.00
597 UPT	PIPX180	PASS	AISC- H1-2	0.300	22
	119545.99 C	0.00		2971.82	1.00
598 UPT	PIPX180	PASS	AISC- H1-2	0.280	22
	111854.02 C	0.00		2772.83	0.00
599 UPT	PIPX180	PASS	AISC- H1-1	0.261	22
	117577.07 C	0.00		1052.99	0.00
600 UPT	PIPX180	PASS	AISC- H1-1	0.257	22
	118537.98 C	0.00		668.88	1.00
601 UPT	PIPX180	PASS	AISC- H1-1	0.264	22
	119305.20 C	0.00		992.60	1.00
602 UPT	PIPX180	PASS	AISC- H1-2	0.303	22
	119547.55 C	0.00		3165.50	1.00
603 LD	L40 4010	PASS	AISC- H2-1	0.142	22
	5712.57 T	-7.75		88.97	0.68
604 LD	L40 4010	PASS	AISC- H2-1	0.108	22
	4102.27 T	114.49		-3.08	0.00
605 LD	L40 4010	PASS	AISC- H2-1	0.209	21
	9576.69 T	15.79		112.99	0.68
606 LD	L40 4010	PASS	AISC- H1-1	0.396	21
	24916.29 C	-7.82		108.47	0.00
607 LD	L40 4010	PASS	AISC- H1-1	0.203	22
	12311.22 C	-42.26		41.62	0.00
608 LD	L40 4010	PASS	AISC- H1-1	0.236	22
	12328.97 C	65.09		71.34	0.00
609 LD	L40 4010	PASS	AISC- H1-3	0.129	24
	6900.71 C	-43.19		24.14	0.00
610 LD	L40 4010	PASS	AISC- H1-1	0.245	22
	15435.88 C	-35.29		49.23	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MG	LOADING/ LOCATION
613	LD L40 4010	PASS	AISC- H1-1	0.249	21
		16698.75 C	10.66	48.48	1.21
614	LD L40 4010	PASS	AISC- H1-1	0.220	21
		15232.50 C	9.36	-35.26	0.00
615	LD L40 4010	PASS	AISC- H1-1	0.199	21
		14602.70 C	-3.72	21.02	0.00
616	LD L40 4010	PASS	AISC- H1-1	0.196	21
		12249.73 C	-33.72	38.61	1.21
617	LD L40 4010	PASS	AISC- H1-1	0.175	22
		12518.83 C	-14.66	17.61	0.00
618	LD L40 4010	PASS	AISC- H1-1	0.197	22
		12479.57 C	-7.62	-51.69	1.21
619	LD L40 4010	PASS	AISC- H1-1	0.191	22
		12420.02 C	3.16	47.08	0.00
620	LD L40 4010	PASS	AISC- H1-1	0.186	22
		12569.57 C	21.91	-26.13	1.21
621	LD L40 4010	PASS	AISC- H1-1	0.241	22
		15058.87 C	-35.14	49.96	1.21
622	LD L40 4010	PASS	AISC- H1-3	0.137	23
		9676.73 C	-0.73	-19.29	0.10
623	LD L40 4010	PASS	AISC- H1-3	0.150	23
		8948.88 C	0.91	45.07	1.21
624	LD L40 4010	PASS	AISC- H1-3	0.135	23
		8258.89 C	0.14	-37.96	0.00
625	LD L40 4010	PASS	AISC- H1-3	0.120	23
		7971.18 C	-7.50	20.55	0.00
626	LD L40 4010	PASS	AISC- H1-3	0.120	23
		6890.94 C	-27.03	24.25	1.21
627	LD L40 4010	PASS	AISC- H1-3	0.119	24
		7930.86 C	-3.11	22.02	1.21
628	LD L40 4010	PASS	AISC- H1-3	0.138	24
		8232.96 C	-5.75	-39.18	1.21
629	LD L40 4010	PASS	AISC- H1-3	0.148	24
		8872.31 C	-0.86	44.18	0.00
630	LD L40 4010	PASS	AISC- H1-3	0.142	24
		9642.30 C	11.71	-19.29	0.71
631	LD L40 4010	PASS	AISC- H1-3	0.243	22
		11952.06 C	55.21	72.05	1.21
632	LD L40 4010	PASS	AISC- H1-1	0.193	22
		13094.03 C	23.62	-26.57	0.00
633	LD L40 4010	PASS	AISC- H1-1	0.201	22
		13027.57 C	5.02	49.36	1.21

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MG	LOADING/ LOCATION
636	LD L40 4010	PASS	AISC- H1-1	0.213	22
		13037.64 C	-41.61	43.52	1.21
637	LD L40 4010	PASS	AISC- H1-1	0.206	21
		15293.55 C	-2.30	19.94	1.21
638	LD L40 4010	PASS	AISC- H1-1	0.230	21
		15085.24 C	11.20	-36.15	1.21
639	LD L40 4010	PASS	AISC- H1-1	0.250	21
		17303.08 C	12.43	49.95	0.00
640	LD L40 4010	PASS	AISC- H1-1	0.270	21
		18826.74 C	11.50	-40.79	1.21
641	LD L40 4010	PASS	AISC- H1-1	0.400	21
		25290.81 C	-7.88	107.74	1.21
642	LD L30 308	PASS	AISC- H2-1	0.191	21
		6599.72 T	-17.15	26.08	0.00
643	LD L30 308	PASS	AISC- H1-3	0.254	21
		6246.52 C	16.00	52.97	0.00
644	LD L30 308	PASS	AISC- H2-1	0.083	21
		2252.38 T	-0.17	21.78	0.00
645	LD L30 308	PASS	AISC- H1-3	0.085	21
		1904.24 C	3.15	18.68	0.00
646	LD L30 308	PASS	AISC- H2-1	0.103	24
		804.61 T	44.17	20.55	1.90
647	LD L30 308	PASS	AISC- H1-3	0.134	24
		1586.91 C	-44.12	20.50	0.00
648	LD L30 308	PASS	AISC- H1-3	0.068	23
		814.86 C	5.46	17.88	0.00
649	LD L30 308	PASS	AISC- H1-3	0.074	21
		966.99 C	0.51	19.78	0.00
650	LD L40 4010	PASS	AISC- H2-1	0.073	21
		3408.19 T	1.97	40.97	0.00
651	LD L40 4010	PASS	AISC- H1-3	0.126	21
		1517.92 C	-13.97	118.76	0.00
652	LD L40 4010	PASS	AISC- H2-1	0.058	24
		951.90 T	7.39	51.91	0.00
653	LD L30 308	PASS	AISC- H1-3	0.060	21
		763.34 C	-14.42	8.17	0.00
654	LD L30 308	PASS	AISC- H1-3	0.076	22
		537.31 C	-19.65	18.90	2.53
655	LD L30 308	PASS	AISC- H1-3	0.140	24
		1391.74 C	-43.57	27.05	2.10
656	LD L30 308	PASS	AISC- H2-1	0.095	24
		502.49 T	44.31	19.18	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
659 LD	L30 308	PASS	AISC- H1-3	0.130	24
	3219.50 C		5.93	28.34	1.16
660 LD	L30 308	PASS	AISC- H2-1	0.135	24
	3375.99 T		-40.23	14.46	0.63
661 LD	L40 4010	PASS	AISC- H1-3	0.177	24
	1323.76 C		-98.39	126.85	0.00
663 LD	L40 4010	PASS	AISC- H2-1	0.179	21
	9636.95 T		14.84	77.20	0.00
664 LD	L40 4010	PASS	AISC- H1-3	0.143	22
	7088.34 C		16.24	49.39	1.56
666 LD	L40 4010	PASS	AISC- H2-1	0.151	22
	6117.52 T		-8.46	93.88	0.00
667 LD	L40 4010	PASS	AISC- H2-1	0.112	22
	6202.93 T		-9.85	45.89	1.56
669 LD	L40 4010	PASS	AISC- H2-1	0.090	21
	1541.26 T		-10.76	80.76	1.56
671 LD	L40 4010	PASS	AISC- H1-3	0.074	22
	625.41 C		-55.37	43.50	1.61
672 LD	L30 308	PASS	AISC- H2-1	0.053	22
	213.53 T		-42.54	0.77	0.00
673 LD	L30 308	PASS	AISC- H1-3	0.098	21
	2234.67 C		4.72	18.30	0.00
674 LD	L30 308	PASS	AISC- H2-1	0.085	21
	1132.55 T		-55.78	0.91	0.00
675 LD	L30 308	PASS	AISC- H1-3	0.099	21
	2129.72 C		4.60	20.14	0.00
676 LD	L30 308	PASS	AISC- H2-1	0.066	21
	153.53 T		54.83	0.99	0.00
677 LD	L30 308	PASS	AISC- H1-3	0.075	24
	1758.62 C		-3.78	13.04	0.00
678 LD	L30 308	PASS	AISC- H2-1	0.061	24
	869.49 T		36.66	2.19	0.00
679 LD	L30 308	PASS	AISC- H1-3	0.070	24
	1549.86 C		-4.30	13.14	0.00
680 LD	L30 308	PASS	AISC- H2-1	0.190	21
	6750.03 T		16.30	24.52	21
681 LD	L30 308	PASS	AISC- H1-3	0.258	0.00
	6382.09 C		-16.08	53.08	0.00
682 LD	L30 308	PASS	AISC- H2-1	0.083	21
	2380.60 T		-0.11	20.90	0.00
683 LD	L30 308	PASS	AISC- H1-3	0.089	21
	2008.72 C		-3.23	19.04	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
686 LD	L30 308	PASS	AISC- H1-3	0.072	24
	784.33 C		10.71	17.64	0.00
687 LD	L30 308	PASS	AISC- H1-3	0.078	21
	1045.44 C		-0.65	20.26	0.00
688 LD	L40 4010	PASS	AISC- H2-1	0.075	21
	3582.35 T		-2.15	40.44	0.00
689 LD	L40 4010	PASS	AISC- H1-3	0.121	21
	1525.18 C		14.35	112.48	0.00
690 LD	L40 4010	PASS	AISC- H2-1	0.056	23
	960.11 T		3.69	51.53	0.00
691 LD	L30 308	PASS	AISC- H1-3	0.059	21
	685.48 C		14.93	8.68	0.00
692 LD	L30 308	PASS	AISC- H1-3	0.072	22
	440.48 C		19.75	18.35	2.53
693 LD	L30 308	PASS	AISC- H1-3	0.091	23
	1405.59 C		-0.78	26.47	2.10
694 LD	L30 308	PASS	AISC- H2-1	0.059	21
	178.72 T		-14.88	20.83	0.00
695 LD	L30 308	PASS	AISC- H1-3	0.045	23
	1060.92 C		-0.59	10.12	1.57
696 LD	L30 308	PASS	AISC- H2-1	0.061	22
	114.52 T		19.83	19.41	1.27
697 LD	L30 308	PASS	AISC- H1-3	0.131	23
	3213.14 C		-6.92	28.43	1.16
698 LD	L30 308	PASS	AISC- H2-1	0.113	22
	2517.47 T		20.89	22.96	0.63
699 LD	L40 4010	PASS	AISC- H2-1	0.137	22
	574.81 T		-32.67	135.39	0.00
701 LD	L40 4010	PASS	AISC- H2-1	0.200	21
	9159.83 T		16.54	108.04	0.00
702 LD	L40 4010	PASS	AISC- H2-1	0.173	21
	9215.26 T		15.67	75.19	1.56
703 LD	L40 4010	PASS	AISC- H1-3	0.138	21
	1659.65 C		-0.88	135.82	2.05
705 LD	L40 4010	PASS	AISC- H2-1	0.107	23
	5667.91 T		7.14	48.48	0.00
706 LD	L40 4010	PASS	AISC- H1-3	0.121	21
	5798.06 C		-64.40	-13.00	1.56
708 LD	L40 4010	PASS	AISC- H2-1	0.091	21
	1638.23 T		10.71	80.24	1.56
710 LD	L40 4010	PASS	AISC- H1-3	0.071	22
	444.01 C		55.30	42.95	1.61

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
713	LD L30 308	PASS	AISC- H2-1	0.087	21
		1181.93 T	55.85	1.13	0.00
714	LD L30 308	PASS	AISC- H1-3	0.101	21
		2221.30 C	-4.83	20.03	0.00
715	LD L30 308	PASS	AISC- H2-1	0.063	21
		141.09 T	-51.99	0.95	0.00
716	LD L30 308	PASS	AISC- H1-3	0.071	23
		1738.34 C	0.70	13.03	0.00
717	LD L30 308	PASS	AISC- H2-1	0.060	23
		859.15 T	-36.43	1.48	0.00
718	LD L30 308	PASS	AISC- H1-3	0.069	23
		1531.42 C	3.19	13.19	0.00
719	LD L30 308	PASS	AISC- H2-1	0.160	22
		4883.24 T	55.68	3.62	0.00
720	LD L30 308	PASS	AISC- H1-3	0.168	22
		4557.21 C	-7.97	31.59	0.00
721	LD L30 308	PASS	AISC- H2-1	0.093	22
		2132.56 T	30.02	6.71	0.00
722	LD L30 308	PASS	AISC- H1-3	0.085	22
		2124.71 C	-7.05	13.86	0.00
723	LD L30 308	PASS	AISC- H2-1	0.151	22
		1193.47 T	-76.59	22.84	1.90
724	LD L30 308	PASS	AISC- H1-3	0.222	22
		2787.18 C	75.99	30.27	0.00
725	LD L30 308	PASS	AISC- H1-3	0.096	21
		1553.99 C	-6.36	18.77	0.00
726	LD L30 308	PASS	AISC- H1-3	0.105	22
		1657.52 C	-2.60	21.43	0.00
727	LD L40 4010	PASS	AISC- H2-1	0.084	22
		4527.73 T	-54.59	-6.65	1.61
728	LD L40 4010	PASS	AISC- H1-3	0.060	23
		475.08 C	3.25	63.02	0.00
729	LD L40 4010	PASS	AISC- H1-3	0.078	21
		1266.56 C	55.21	38.40	1.56
730	LD L30 308	PASS	AISC- H1-3	0.110	22
		1738.19 C	-2.75	21.95	2.68
731	LD L30 308	PASS	AISC- H2-1	0.101	21
		1654.21 C	-6.42	19.34	2.53
732	LD L30 308	PASS	AISC- H1-3	0.224	22
		2874.25 C	75.60	30.34	2.10
733	LD L30 308	PASS	AISC- H2-1	0.153	22
		1296.34 T	-76.43	23.02	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
736	LD L30 308	PASS	AISC- H1-3	0.171	22
		4692.67 C	-8.05	31.69	1.16
737	LD L30 308	PASS	AISC- H2-1	0.159	22
		5034.01 T	54.90	2.05	0.63
738	LD L40 4010	PASS	AISC- H1-3	0.220	22
		2368.13 C	-172.28	113.87	0.00
740	LD L40 4010	PASS	AISC- H2-1	0.114	22
		4519.45 T	113.07	-5.78	0.68
741	LD L40 4010	PASS	AISC- H2-1	0.099	22
		4491.74 T	20.50	44.74	1.56
743	LD L40 4010	PASS	AISC- H2-1	0.092	22
		4070.26 T	21.85	42.72	0.00
744	LD L40 4010	PASS	AISC- H2-1	0.087	22
		1051.70 T	-39.06	-65.59	1.56
745	LD L40 4010	PASS	AISC- H1-3	0.218	22
		2368.36 C	189.38	-100.95	0.00
747	LD L40 4010	PASS	AISC- H2-1	0.076	22
		1824.97 T	-9.94	60.56	1.56
749	LD L40 4010	PASS	AISC- H1-3	0.087	21
		3746.09 C	56.61	9.46	0.00
750	LD L30 308	PASS	AISC- H1-3	0.039	22
		149.65 C	-14.10	10.45	1.00
751	LD L30 308	PASS	AISC- H1-3	0.124	22
		3375.86 C	-10.21	12.48	0.00
752	LD L30 308	PASS	AISC- H2-1	0.102	22
		1712.56 T	53.28	5.71	0.00
753	LD L30 308	PASS	AISC- H1-3	0.117	22
		3116.56 C	-5.92	14.83	0.00
754	LD L30 308	PASS	AISC- H1-3	0.040	22
		137.12 C	15.20	10.88	1.00
755	LD L30 308	PASS	AISC- H1-3	0.126	22
		3482.95 C	10.59	12.18	0.00
756	LD L30 308	PASS	AISC- H2-1	0.101	22
		1767.33 T	-53.85	4.34	0.00
757	LD L30 308	PASS	AISC- H1-3	0.119	22
		3216.55 C	5.87	14.76	0.00
758	LD L40 4010	PASS	AISC- H2-1	0.163	23
		467.77 T	98.26	126.50	2.05
759	LD L40 4010	PASS	AISC- H2-1	0.074	20
		0.89 T	0.00	-88.24	1.98
760	LD L40 4010	PASS	AISC- H2-1	0.074	20
		0.90 T	0.00	-88.24	1.98

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
763	LD L40 4010	PASS 340.68 C	AISC- H1-3 3.67	0.066 66.98	21 3.80
764	LD L40 4010	PASS 340.87 C	AISC- H1-3 4.75	0.068 68.40	21 0.00
765	LD L40 4010	PASS 5322.08 T	AISC- H2-1 -0.13	0.136 91.94	22 0.63
766	LD L40 4010	PASS 4028.78 T	AISC- H2-1 4.10	0.096 59.43	24 0.63
767	LD L40 4010	PASS 8136.18 T	AISC- H2-1 25.12	0.199 112.17	21 0.64
768	UPT P1PX180	PASS 180824.78 C	AISC- H1-2 0.00	0.455 4559.34	21 0.00
769	UPT P1PX180	PASS 190761.28 C	AISC- H1-1 0.00	0.425 1781.22	21 0.00
770	UPT P1PX180	PASS 192784.70 C	AISC- H1-1 0.00	0.409 478.31	21 0.00
771	UPT P1PX180	PASS 194287.50 C	AISC- H1-1 0.00	0.421 1027.16	21 1.00
772	UPT P1PX180	PASS 194101.45 C	AISC- H1-1 0.00	0.427 1466.23	21 1.03
773	UPT P1PX180	PASS 97734.04 C	AISC- H1-2 0.00	0.246 2471.90	22 0.00
774	UPT P1PX180	PASS 103102.45 C	AISC- H1-1 0.00	0.229 895.88	22 0.00
775	UPT P1PX180	PASS 104014.85 C	AISC- H1-1 0.00	0.226 564.19	22 1.00
776	UPT P1PX180	PASS 104839.90 C	AISC- H1-1 0.00	0.231 774.49	22 1.00
777	UPT P1PX180	PASS 104244.10 C	AISC- H1-2 0.00	0.250 1971.89	22 1.00
778	UPT P1PX180	PASS 97734.16 C	AISC- H1-2 0.00	0.246 2480.26	22 0.00
779	UPT P1PX180	PASS 103092.64 C	AISC- H1-1 0.00	0.229 910.33	22 0.00
780	UPT P1PX180	PASS 104003.96 C	AISC- H1-1 0.00	0.227 647.59	22 1.00
781	UPT P1PX180	PASS 104839.17 C	AISC- H1-1 0.00	0.230 773.33	22 1.00
782	UPT P1PX180	PASS 104281.77 C	AISC- H1-2 0.00	0.249 1906.57	22 1.00
783	LD L40 4010	PASS 12317.31 C	AISC- H1-1 65.17	0.229 62.84	22 0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
786	LD L40 4010	PASS 13035.04 C	AISC- H1-1 23.93	0.206 45.36	21 0.00
787	LD L40 4010	PASS 24276.09 C	AISC- H1-1 -6.83	0.375 94.08	21 0.00
788	LD L40 4010	PASS 12298.00 C	AISC- H1-1 6.43	0.188 44.47	22 0.00
789	LD L40 4010	PASS 9451.65 C	AISC- H1-3 -3.73	0.134 -18.24	23 0.20
790	LD L40 4010	PASS 8795.79 C	AISC- H1-3 -3.20	0.145 41.36	23 1.18
791	LD L40 4010	PASS 8136.48 C	AISC- H1-3 -5.87	0.131 -32.29	23 0.00
792	LD L40 4010	PASS 7874.76 C	AISC- H1-3 -3.51	0.112 15.39	23 0.00
793	LD L40 4010	PASS 6871.52 C	AISC- H1-3 22.68	0.119 25.73	23 1.18
794	LD L40 4010	PASS 7886.77 C	AISC- H1-3 0.58	0.113 17.45	24 1.18
795	LD L40 4010	PASS 8155.94 C	AISC- H1-3 -13.16	0.136 -33.85	24 1.18
796	LD L40 4010	PASS 8746.90 C	AISC- H1-3 -6.24	0.145 39.39	24 0.00
797	LD L40 4010	PASS 9458.79 C	AISC- H1-3 7.02	0.136 -18.00	24 0.99
798	LD L40 4010	PASS 31962.34 C	AISC- H1-3 66.23	0.237 64.08	22 1.18
799	LD L40 4010	PASS 12722.21 C	AISC- H1-1 20.55	0.186 -26.19	22 0.00
800	LD L40 4010	PASS 12742.23 C	AISC- H1-1 -1.61	0.192 45.26	22 1.18
801	LD L40 4010	PASS 12964.20 C	AISC- H1-1 -15.31	0.204 -48.93	22 0.00
802	LD L40 4010	PASS 13096.74 C	AISC- H1-1 9.93	0.182 20.51	22 1.18
803	LD L40 4010	PASS 13055.09 C	AISC- H1-1 5.46	0.198 46.27	22 1.18
804	LD L40 4010	PASS 15348.28 C	AISC- H1-1 4.62	0.201 11.00	21 0.00
805	LD L40 4010	PASS 15973.35 C	AISC- H1-1 3.04	0.221 -29.63	21 1.18
806	LD L40 4010	PASS 17405.28 C	AISC- H1-1 7.69	0.255 48.31	21 0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FK	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
809	LD L40 4010	PASS	AISC- H1-1	0.259	21
		18182.33 C	8.95	-38.84	0.00
810	LD L40 4010	PASS	AISC- H1-1	0.244	21
		16718.60 C	5.39	46.43	1.18
811	LD L40 4010	PASS	AISC- H1-1	0.211	21
		15235.69 C	1.03	-28.39	0.00
812	LD L40 4010	PASS	AISC- H1-1	0.192	21
		14611.44 C	-2.86	13.39	0.00
813	LD L40 4010	PASS	AISC- H1-1	0.194	21
		12271.16 C	23.40	42.97	1.18
814	LD L40 4010	PASS	AISC- H1-1	0.170	22
		12389.07 C	8.08	16.94	0.00
815	LD L40 4010	PASS	AISC- H1-1	0.196	22
		12294.94 C	-18.38	-47.75	1.18
816	LD L40 4010	PASS	AISC- H1-1	0.184	22
		12123.40 C	-4.80	43.54	0.00
817	LD L40 4010	PASS	AISC- H1-1	0.179	22
		12196.97 C	18.52	-25.59	1.18
818	LD L40 4010	PASS	AISC- H1-1	0.225	22
		14422.52 C	-34.57	41.18	1.18
819	LD L30 308	PASS	AISC- H2-1	0.157	22
		4262.89 T	57.78	6.91	0.00
820	LD L30 308	PASS	AISC- H1-3	0.154	22
		4237.15 C	-7.87	28.14	0.00
821	LD L30 308	PASS	AISC- H2-1	0.092	22
		2336.91 T	30.92	7.59	0.00
822	LD L30 308	PASS	AISC- H1-3	0.082	22
		2030.56 C	-8.45	12.89	0.00
823	LD L30 308	PASS	AISC- H2-1	0.140	22
		997.30 T	-73.72	20.70	1.75
824	LD L30 308	PASS	AISC- H1-3	0.208	22
		2904.88 C	67.74	27.90	0.00
825	LD L30 308	PASS	AISC- H1-3	0.091	21
		1609.41 C	-8.16	16.75	0.00
826	LD L30 308	PASS	AISC- H1-3	0.097	22
		1695.76 C	-2.92	19.14	0.00
827	LD L40 4010	PASS	AISC- H1-3	0.074	21
		2875.63 C	54.89	9.65	1.48
828	LD L40 4010	PASS	AISC- H1-3	0.068	22
		3880.24 C	3.27	21.19	0.00
829	LD L40 4010	PASS	AISC- H1-3	0.060	21
		555.72 C	44.67	34.60	1.73

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FK	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
832	LD L30 308	PASS	AISC- H1-3	0.209	22
		2978.47 C	66.95	27.62	1.97
833	LD L30 308	PASS	AISC- H2-1	0.142	22
		1080.71 T	-73.83	20.65	0.00
834	LD L30 308	PASS	AISC- H1-3	0.086	22
		2148.56 C	-8.84	13.28	1.50
935	LD L30 308	PASS	AISC- H2-1	0.092	22
		2477.22 T	29.95	6.51	1.17
836	LD L30 308	PASS	AISC- H1-3	0.158	22
		4396.40 C	-8.14	28.22	1.13
837	LD L30 308	PASS	AISC- H2-1	0.155	22
		4429.65 T	56.78	5.04	0.58
838	LD L40 4010	PASS	AISC- H1-3	0.203	22
		2632.65 C	-177.48	-87.16	1.90
839	LD L30 308	PASS	AISC- H2-1	0.182	21
		5861.82 T	14.55	30.20	0.00
840	LD L30 308	PASS	AISC- H1-3	0.236	21
		5913.89 C	-15.99	47.72	0.00
841	LD L30 308	PASS	AISC- H2-1	0.083	21
		2040.49 T	-1.34	23.52	0.00
842	LD L30 308	PASS	AISC- H1-3	0.080	21
		1789.53 C	-3.72	17.56	0.00
843	LD L30 308	PASS	AISC- H1-3	0.050	29
		205.56 C	12.69	16.50	1.75
844	LD L30 308	PASS	AISC- H1-3	0.119	21
		2008.05 C	-4.80	31.25	0.00
845	LD L30 308	PASS	AISC- H1-3	0.061	24
		774.76 C	6.07	15.89	0.00
846	LD L30 308	PASS	AISC- H1-3	0.070	21
		1045.83 C	-0.97	17.91	0.00
848	LD L40 4010	PASS	AISC- H1-3	0.140	21
		2143.50 C	27.46	117.82	0.00
849	LD L40 4010	PASS	AISC- H2-1	0.047	23
		306.61 T	7.72	47.09	0.00
850	LD L30 308	PASS	AISC- H1-3	0.050	21
		722.63 C	17.53	7.19	0.00
851	LD L30 308	PASS	AISC- H1-3	0.069	22
		524.54 C	17.99	17.37	2.33
852	LD L30 308	PASS	AISC- H1-3	0.093	23
		1424.98 C	-8.36	23.86	1.97
853	LD L30 308	PASS	AISC- H2-1	0.054	21
		144.53 T	-13.29	19.32	0.00

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ALL UNITS ARE - KG METK (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
856	LD L30 308	PASS	AISC- H1-3	0.119	23
		2943.53 C	-6.47	25.29	1.13
857	LD L30 308	PASS	AISC- H2-1	0.112	22
		2126.20 T	21.09	26.27	0.58
858	LD L40 4010	PASS	AISC- H1-3	0.131	21
		1769.01 C	9.38	121.87	0.00
859	LD L30 308	PASS	AISC- H2-1	0.184	21
		5694.43 T	-15.77	32.19	0.00
860	LD L30 308	PASS	AISC- H1-3	0.233	21
		5753.99 C	15.79	47.64	0.00
861	LD L30 308	PASS	AISC- H2-1	0.082	21
		1895.51 T	0.33	24.63	0.00
862	LD L30 308	PASS	AISC- H1-3	0.076	21
		1667.49 C	3.45	17.17	0.00
863	LD L30 308	PASS	AISC- H2-1	0.092	24
		670.93 T	39.88	18.56	1.75
864	LD L30 308	PASS	AISC- H1-3	0.120	24
		1652.37 C	-36.54	18.03	0.00
865	LD L30 308	PASS	AISC- H1-3	0.068	23
		821.02 C	10.65	16.21	0.00
866	LD L30 308	PASS	AISC- H1-3	0.067	21
		979.41 C	0.83	17.52	0.00
867	LD L40 4010	PASS	AISC- H2-1	0.070	21
		2434.74 T	11.52	44.04	0.00
868	LD L40 4010	PASS	AISC- H1-3	0.148	21
		2211.31 C	-28.77	124.74	0.00
869	LD L40 4010	PASS	AISC- H2-1	0.045	24
		247.38 T	4.72	47.48	0.00
870	LD L30 308	PASS	AISC- H1-3	0.061	21
		799.31 C	-18.47	6.73	0.00
871	LD L30 308	PASS	AISC- H1-3	0.075	22
		616.64 C	-19.17	17.90	2.33
872	LD L30 308	PASS	AISC- H1-3	0.124	24
		1433.25 C	-34.74	24.47	1.97
873	LD L30 308	PASS	AISC- H2-1	0.080	24
		350.41 T	37.83	16.72	0.00
874	LD L30 308	PASS	AISC- H1-3	0.047	24
		1031.61 C	4.35	9.28	1.50
875	LD L30 308	PASS	AISC- H2-1	0.077	24
		1115.39 T	-26.75	13.04	1.17
876	LD L30 308	PASS	AISC- H1-3	0.118	24
		2956.54 C	5.51	25.22	1.13

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ALL UNITS ARE - KG METK (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
879	LD L40 4010	PASS	AISC- H1-3	0.155	24
		1475.67 C	79.95	110.89	1.90
881	LD L40 4010	PASS	AISC- H2-1	0.166	21
		8198.18 T	25.08	74.60	0.00
882	LD L40 4010	PASS	AISC- H1-3	0.128	22
		6263.10 C	22.02	43.16	1.43
884	LD L40 4010	PASS	AISC- H2-1	0.147	22
		5871.11 T	-1.04	97.86	0.00
885	LD L40 4010	PASS	AISC- H2-1	0.106	22
		5963.75 T	-2.56	45.99	1.43
887	LD L40 4010	PASS	AISC- H2-1	0.081	21
		632.52 T	-18.78	76.59	1.43
889	LD L40 4010	PASS	AISC- H1-3	0.072	22
		597.16 C	-50.32	45.58	1.48
890	LD L30 308	PASS	AISC- H2-1	0.062	22
		240.63 T	-42.74	5.04	0.00
891	LD L30 308	PASS	AISC- H1-3	0.093	21
		2169.13 C	7.35	15.66	0.00
892	LD L30 308	PASS	AISC- H2-1	0.082	21
		1178.14 T	-46.86	4.21	0.00
893	LD L30 308	PASS	AISC- H1-3	0.091	21
		2129.97 C	3.66	12.37	0.00
894	LD L30 308	PASS	AISC- H2-1	0.070	21
		197.08 T	57.28	-0.34	0.00
895	LD L30 308	PASS	AISC- H1-3	0.070	24
		1722.90 C	-5.04	10.71	0.00
896	LD L30 308	PASS	AISC- H2-1	0.054	24
		914.03 T	30.72	1.35	0.00
897	LD L30 308	PASS	AISC- H1-3	0.064	24
		1562.63 C	-3.06	10.87	0.00
898	LD L40 4010	PASS	AISC- H2-1	0.130	22
		682.58 T	39.41	122.15	1.90
900	LD L40 4010	PASS	AISC- H2-1	0.190	21
		7868.96 T	23.90	107.90	0.00
901	LD L40 4010	PASS	AISC- H2-1	0.160	21
		7928.54 T	22.89	72.31	1.43
903	LD L40 4010	PASS	AISC- H2-1	0.097	22
		5409.11 T	-1.55	43.39	0.00
904	LD L40 4010	PASS	AISC- H1-3	0.123	21
		6051.54 C	-63.12	-14.24	1.43
905	LD L40 4010	PASS	AISC- H2-1	0.081	21
		778.22 T	-18.20	75.49	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
910	LD L30 308	PASS	AISC- H2-1	0.051	22
		230.45 T	41.05	0.57	0.00
911	LD L30 308	PASS	AISC- H1-3	0.093	21
		2195.12 C	-7.04	15.38	0.00
912	LD L30 308	PASS	AISC- H2-1	0.075	21
		1192.24 T	46.95	-0.07	0.00
913	LD L30 308	PASS	AISC- H1-3	0.090	21
		2150.90 C	-3.12	17.08	0.00
914	LD L30 308	PASS	AISC- H2-1	0.066	21
		179.59 T	-53.98	-0.88	0.00
915	LD L30 308	PASS	AISC- H1-3	0.066	23
		1699.27 C	2.07	10.70	0.00
916	LD L30 308	PASS	AISC- H2-1	0.052	23
		901.17 T	-30.43	0.35	0.00
917	LD L30 308	PASS	AISC- H1-3	0.061	23
		1541.89 C	1.55	10.90	0.00
918	LD L40 4010	PASS	AISC- H1-3	0.203	22
		2632.58 C	-177.41	-87.87	0.00
920	LD L40 4010	PASS	AISC- H2-1	0.095	23
		3707.02 T	10.81	58.28	0.00
921	LD L40 4010	PASS	AISC- H2-1	0.082	22
		3230.56 T	22.21	41.12	1.43
923	LD L40 4010	PASS	AISC- H2-1	0.078	24
		4068.14 T	3.46	37.15	0.00
924	LD L40 4010	PASS	AISC- H1-3	0.069	22
		889.37 C	-45.46	-40.40	1.43
926	LD L40 4010	PASS	AISC- H1-3	0.064	21
		353.96 C	52.31	37.73	0.00
928	LD L40 4010	PASS	AISC- H2-1	0.095	22
		3701.60 T	-54.86	-29.60	0.00
929	LD L30 308	PASS	AISC- H2-1	0.034	24
		86.44 T	26.99	0.99	0.00
930	LD L30 308	PASS	AISC- H1-3	0.121	22
		3555.96 C	-11.39	9.55	0.00
931	LD L30 308	PASS	AISC- H2-1	0.094	22
		1927.83 T	47.90	2.36	0.00
932	LD L30 308	PASS	AISC- H1-3	0.112	22
		3340.09 C	-3.75	11.85	0.00
933	LD L30 308	PASS	AISC- H2-1	0.033	23
		92.38 T	-27.81	0.00	0.00
934	LD L30 308	PASS	AISC- H1-3	0.127	22
		3759.13 C	12.39	9.08	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
937	LD L40 4010	PASS	AISC- H1-3	0.063	20
		10.76 C	0.00	-75.41	1.83
938	LD L40 4010	PASS	AISC- H1-3	0.063	20
		10.32 C	0.00	-75.42	1.83
939	LD L40 4010	PASS	AISC- H1-3	0.063	20
		10.37 C	0.00	-75.40	1.83
949	LD L30 305	PASS	AISC- H2-1	0.166	20
		58.66 T	83.80	4.15	0.00
950	LD L30 305	PASS	AISC- H1-3	0.189	22
		17.91 C	90.22	8.36	0.00
951	LD L30 305	PASS	AISC- H1-3	0.174	24
		101.16 C	81.00	7.67	0.00
973	LD L30 305	PASS	AISC- H1-3	0.099	22
		208.86 C	-46.88	2.06	0.00
974	LD L30 305	PASS	AISC- H2-1	0.226	24
		327.15 T	109.44	6.80	1.74
975	LD L30 305	PASS	AISC- H2-1	0.163	24
		386.02 T	81.67	1.09	0.00
976	LD L30 305	PASS	AISC- H1-3	0.088	20
		4.28 C	-40.15	-5.04	1.74
977	LD L30 305	PASS	AISC- H2-1	0.166	20
		58.64 T	83.81	4.15	1.74
978	LD L40 4010	PASS	AISC- H1-3	0.020	24
		83.03 C	22.99	-8.72	0.49
981	LD L40 4010	PASS	AISC- H1-3	0.019	20
		67.19 C	19.14	-9.51	0.58
983	LD L30 305	PASS	AISC- H1-3	0.101	24
		82.74 C	-48.29	-3.54	0.00
984	LD L30 305	PASS	AISC- H2-1	0.153	20
		212.31 T	68.94	7.27	1.74
985	LD L30 305	PASS	AISC- H2-1	0.142	20
		215.35 T	66.87	4.73	0.00
986	LD L30 305	PASS	AISC- H1-3	0.100	22
		209.67 C	-47.14	2.05	1.74
987	LD L30 305	PASS	AISC- H2-1	0.172	20
		52.30 T	82.39	7.11	1.74
988	LD L40 4010	PASS	AISC- H1-3	0.019	20
		67.15 C	19.13	-9.51	0.58
991	LD L40 4010	PASS	AISC- H1-3	0.018	20
		63.73 C	18.03	-9.24	0.58
993	LD L40 4010	PASS	AISC- H1-3	0.020	22
		76.24 C	20.50	-10.15	0.49

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ALL UNITS ARE - KG MERE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION	
999	LD	L30 305	PASS	AISC- H2-1	0.231	22
		442.21 T	113.17	4.17	1.74	
1000	LD	L30 305	PASS	AISC- H2-1	0.231	22
		441.64 T	113.16	4.17	0.00	
1001	LD	L30 305	PASS	AISC- H2-1	0.096	22
		232.68 T	-37.93	-7.05	1.74	
1002	LD	L30 305	PASS	AISC- H1-3	0.188	22
		17.44 C	89.79	8.40	1.74	
1003	LD	L30 305	PASS	AISC- H1-3	0.261	20
		48.60 C	-53.70	56.98	0.00	
1004	LD	L30 305	PASS	AISC- H2-1	0.222	20
		30.75 T	-47.99	46.96	1.15	
1006	LD	L30 305	PASS	AISC- H2-1	0.216	20
		29.53 T	45.69	46.47	1.15	
1007	LD	L30 305	PASS	AISC- H1-3	0.257	20
		46.38 C	-50.41	57.71	1.53	
1008	LD	L30 305	PASS	AISC- H1-3	0.261	20
		48.62 C	53.73	56.97	0.00	
1009	LD	L30 305	PASS	AISC- H2-1	0.222	20
		30.77 T	-48.01	46.96	0.00	
1011	LD	L30 305	PASS	AISC- H2-1	0.220	24
		38.87 T	-59.72	38.63	1.15	
1012	LD	L30 305	PASS	AISC- H1-3	0.257	20
		46.45 C	50.40	57.69	1.53	
1013	LD	L30 305	PASS	AISC- H1-3	0.250	20
		44.56 C	-50.01	55.27	1.53	
1014	LD	L30 305	PASS	AISC- H2-1	0.221	22
		37.20 T	55.91	41.50	1.15	
1016	LD	L30 305	PASS	AISC- H2-1	0.220	22
		37.02 T	-55.61	41.51	1.15	
1017	LD	L30 305	PASS	AISC- H1-3	0.250	20
		44.54 C	50.00	55.27	1.53	
1027	LD	L30 305	PASS	AISC- H2-1	0.108	24
		403.76 T	51.98	-0.71	0.00	
1028	LD	L30 305	PASS	AISC- H1-3	0.214	22
		116.75 C	102.15	-7.98	0.00	
1029	LD	L30 305	PASS	AISC- H2-1	0.162	24
		3.47 T	83.48	-3.27	0.00	
1030	LD	L30 305	PASS	AISC- H2-1	0.088	23
		149.14 T	39.04	4.33	1.74	
1031	LD	L30 305	PASS	AISC- H1-3	0.052	24
		409.39 C	10.82	5.74	0.00	

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ALL UNITS ARE -KG MERE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION	
1034	LD	L30 305	PASS	AISC- H1-3	0.165	24
		87.46 C	80.37	-5.30	1.74	
1035	LD	L30 305	PASS	AISC- H1-3	0.043	29
		166.66 C	14.85	3.46	1.74	
1036	LD	L30 305	PASS	AISC- H1-3	0.049	29
		176.14 C	18.97	2.65	0.00	
1037	LD	L30 305	PASS	AISC- H1-3	0.050	20
		95.91 C	-16.61	5.54	0.00	
1038	LD	L30 305	PASS	AISC- H1-3	0.048	20
		113.10 C	-11.01	8.12	1.74	
1039	LD	L30 305	PASS	AISC- H2-1	0.145	22
		509.04 T	65.59	-3.86	1.74	
1040	LD	L30 305	PASS	AISC- H2-1	0.116	21
		254.34 T	54.54	3.34	1.74	
1041	LD	L30 305	PASS	AISC- H2-1	0.062	21
		328.31 T	24.77	2.56	0.00	
1042	LD	L30 305	PASS	AISC- H2-1	0.062	21
		328.31 T	24.83	2.56	1.74	
1043	LD	L30 305	PASS	AISC- H2-1	0.117	21
		254.52 T	54.68	3.33	0.00	
1044	LD	L30 305	PASS	AISC- H1-3	0.214	22
		117.34 C	102.20	-7.97	1.74	
1045	LD	L30 305	PASS	AISC- H2-1	0.055	24
		14.78 T	-14.51	-9.84	1.00	
1046	LD	L30 305	PASS	AISC- H1-3	0.247	23
		167.64 C	-43.48	57.02	0.00	
1047	LD	L30 305	PASS	AISC- H1-3	0.143	24
		36.59 C	35.96	-26.92	0.00	
1048	LD	L30 305	PASS	AISC- H1-3	0.278	23
		39.36 C	-56.91	60.98	0.00	
1049	LD	L30 305	PASS	AISC- H1-3	0.140	24
		109.25 C	21.24	-34.44	0.00	
1050	LD	L30 305	PASS	AISC- H1-3	0.231	22
		233.64 C	-56.58	42.04	0.00	
1051	LD	L30 305	PASS	AISC- H2-1	0.053	22
		26.72 T	-5.17	15.05	1.00	
1052	LD	L30 305	PASS	AISC- H1-3	0.186	21
		130.07 C	89.37	-6.66	0.00	
1053	LD	L30 305	PASS	AISC- H1-3	0.197	24
		221.57 C	-26.79	-49.49	0.00	
1054	LD	L30 305	PASS	AISC- H1-3	0.132	20
		51.61 C	32.89	24.70	0.00	

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ALL UNITS ARE - NO MTE (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/	CRITICAL COND/	RATIO/	LOADING/
		PK	PK	PK	LOCATION
1057	LD L30 305	PASS	AISC-H1-3	0.230	22
		233.24 C	-56.46	0.00	0.00
1058	LD L30 305	PASS	AISC-H2-1	0.053	22
		26.57 T	-5.26	1.00	0.00
1059	LD L30 305	PASS	AISC-H1-3	0.186	21
		89.35 C	6.47	0.00	0.00
1060	LD L30 305	PASS	AISC-H1-3	0.112	22
		74.54 C	6.26	0.00	0.00
1061	LD L30 305	PASS	AISC-H1-3	0.209	21
		113.79 C	0.00	0.00	0.00
1062	LD L30 305	PASS	AISC-H1-3	0.112	22
		39.33 C	14.50 C	0.00	0.00
1063	LD L30 305	PASS	AISC-H1-3	0.104	23
		16.11 C	-42.18	9.42	0.00
1064	LD L30 305	PASS	AISC-H2-1	0.095	24
		244.68 T	-39.49	5.73	2.01
1065	LD L30 305	PASS	AISC-H1-3	0.113	23
		178.99 C	57.67	0.00	0.00
1066	LD L30 305	PASS	AISC-H2-1	0.111	24
		382.72 T	47.71	4.59	2.01
1067	LD L30 305	PASS	AISC-H2-1	0.148	24
		71.50 T	-54.85	16.19	0.00
1068	LD L30 305	PASS	AISC-H1-3	0.069	29
		58.79 C	25.24	7.37	0.00
1069	LD L30 305	PASS	AISC-H2-1	0.059	20
		317.82 T	20.25	7.97	2.01
1070	LD L30 305	PASS	AISC-H1-3	0.165	24
		294.97 C	-79.57	-3.69	2.01
1071	LD L30 305	PASS	AISC-H2-1	0.180	22
		28.27 C	54.18	4.98	2.01
1072	LD L30 305	PASS	AISC-H2-1	0.066	22
		356.55 T	-24.44	3.81	2.01
1073	LD L30 305	PASS	AISC-H1-3	0.185	22
		357.84 C	88.58	-3.06	2.01
1074	LD L30 305	PASS	AISC-H2-1	0.123	22
		62.51 T	62.12	2.94	2.01
1075	LD L30 305	PASS	AISC-H2-1	0.123	22
		324.46 T	54.18	5.26	2.01
1076	LD L30 305	PASS	AISC-H2-1	0.182	22
		118.97 T	69.57	18.28	0.00
1077	LD L30 305	PASS	AISC-H2-1	0.124	22
		62.38 T	-62.22	2.95	2.01

ALL UNITS ARE - NO MTE (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/	CRITICAL COND/	RATIO/	LOADING/
		PK	PK	PK	LOCATION
1080	LD L30 305	PASS	AISC-H1-3	0.185	22
		358.94 C	-88.45	-3.09	2.01
1081	LD L30 308	PASS	AISC-H2-1	0.221	22
		3852.16 T	113.00	11.82	0.00
1082	LD L30 308	PASS	AISC-H1-3	0.143	22
		2856.91 C	38.61	20.69	0.00
1083	LD L30 308	PASS	AISC-H1-3	0.161	24
		1625.81 C	96.06	9.74	0.00
1223	UPT PIPX140	PASS	AISC-H1-2	0.205	22
		47509.58 C	0.00	1388.06	0.00
1225	UPT PIPX140	PASS	AISC-H1-2	0.694	21
		182888.45 C	0.00	2798.01	0.00
1226	UPT PIPX140	PASS	AISC-H1-1	0.656	21
		186031.48 C	0.00	1413.31	1.00
1227	UPT PIPX140	PASS	AISC-H1-2	0.660	21
		172570.61 C	0.00	2759.47	1.00
1228	UPT PIPX140	PASS	AISC-H1-2	0.661	21
		172991.62 C	0.00	2701.05	0.00
1229	UPT PIPX140	PASS	AISC-H1-1	0.824	21
		176121.44 C	0.00	1411.86	1.00
1230	UPT PIPX140	PASS	AISC-H1-2	0.824	21
		162332.06 C	0.00	2689.82	1.00
1231	UPT PIPX140	PASS	AISC-H1-2	0.826	21
		162648.70 C	0.00	2712.04	0.00
1232	UPT PIPX140	PASS	AISC-H1-1	0.391	21
		160008.14 C	0.00	1104.04	1.00
1233	UPT PIPX140	PASS	AISC-H1-2	0.392	21
		152143.88 C	0.00	271.82	1.00
1234	UPT PIPX140	PASS	AISC-H1-2	0.594	21
		152445.95 C	0.00	2731.05	0.00
1235	UPT PIPX140	PASS	AISC-H1-1	0.557	21
		155981.14 C	0.00	1405.30	1.00
1236	UPT PIPX140	PASS	AISC-H1-2	0.555	21
		141772.69 C	0.00	2629.69	1.00
1237	UPT PIPX140	PASS	AISC-H1-2	0.557	21
		147068.38 C	0.00	2653.82	0.00
1238	UPT PIPX140	PASS	AISC-H1-1	0.521	21
		145742.61 C	0.00	1322.29	1.00
1239	UPT PIPX140	PASS	AISC-H1-2	0.517	21
		131187.47 C	0.00	2512.52	1.00
1240	UPT PIPX140	PASS	AISC-H1-2	0.518	21
		131463.45 C	0.00	2338.36	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ ME	LOADING/ LOCATION
1243	UPT PIPX140	PASS	AISC- HI-2	0.482	21
		120803.11 C	0.00	2486.83	0.00
1244	UPT PIPX140	PASS	AISC- HI-1	0.447	21
		124596.68 C	0.00	1173.69	1.00
1245	UPT PIPX140	PASS	AISC- HI-2	0.439	21
		109662.52 C	0.00	2301.29	1.00
1246	UPT PIPX140	PASS	AISC- HI-2	0.441	21
		109883.97 C	0.00	2331.47	0.00
1247	UPT PIPX140	PASS	AISC- HI-1	0.407	21
		113697.20 C	0.00	1051.13	1.00
1248	UPT PIPX140	PASS	AISC- HI-2	0.396	21
		98445.70 C	0.00	2109.92	1.00
1249	UPT PIPX140	PASS	AISC- HI-2	0.398	21
		98627.09 C	0.00	2143.56	0.00
1250	UPT PIPX140	PASS	AISC- HI-1	0.363	21
		102360.99 C	0.00	832.13	1.00
1251	UPT PIPX140	PASS	AISC- HI-2	0.351	21
		87118.40 C	0.00	1892.51	1.00
1252	UPT PIPX140	PASS	AISC- HI-2	0.353	21
		87258.92 C	0.00	1929.09	0.00
1253	UPT PIPX140	PASS	AISC- HI-2	0.381	21
		80347.00 C	0.00	2415.04	1.00
1254	UPT PIPX140	PASS	AISC- HI-2	0.187	22
		46221.56 C	0.00	1028.37	1.00
1255	UPT PIPX140	PASS	AISC- HI-2	0.197	22
		46268.02 C	0.00	1290.52	1.00
1256	UPT PIPX140	PASS	AISC- HI-2	0.224	22
		54152.09 C	0.00	1315.77	0.00
1257	UPT PIPX140	PASS	AISC- HI-2	0.210	22
		52246.05 C	0.00	1127.18	1.00
1258	UPT PIPX140	PASS	AISC- HI-2	0.217	22
		52274.19 C	0.00	1305.14	1.00
1259	UPT PIPX140	PASS	AISC- HI-2	0.244	22
		60232.83 C	0.00	1334.63	0.00
1260	UPT PIPX140	PASS	AISC- HI-2	0.233	22
		58281.19 C	0.00	1222.60	1.00
1261	UPT PIPX140	PASS	AISC- HI-2	0.238	22
		58287.85 C	0.00	1341.44	1.00
1262	UPT PIPX140	PASS	AISC- HI-2	0.264	22
		66135.84 C	0.00	1375.48	0.00
1263	UPT PIPX140	PASS	AISC- HI-2	0.255	22
		64178.25 C	0.00	1289.69	1.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ ME	LOADING/ LOCATION
1266	UPT PIPX140	PASS	AISC- HI-2	0.276	22
		70006.40 C	0.00	1357.82	1.00
1267	UPT PIPX140	PASS	AISC- HI-2	0.278	22
		69983.86 C	0.00	1401.14	1.00
1268	UPT PIPX140	PASS	AISC- HI-2	0.304	22
		77609.19 C	0.00	1442.40	0.00
1269	UPT PIPX140	PASS	AISC- HI-2	0.296	22
		75736.79 C	0.00	1402.80	1.00
1270	UPT PIPX140	PASS	AISC- HI-2	0.296	22
		75704.23 C	0.00	1405.09	1.00
1271	UPT PIPX140	PASS	AISC- HI-2	0.322	22
		83210.34 C	0.00	1448.73	0.00
1272	UPT PIPX140	PASS	AISC- HI-2	0.316	22
		81411.61 C	0.00	1436.64	1.00
1273	UPT PIPX140	PASS	AISC- HI-2	0.316	22
		81372.89 C	0.00	1433.50	1.00
1274	UPT PIPX140	PASS	AISC- HI-2	0.341	22
		88758.87 C	0.00	1480.77	0.00
1275	UPT PIPX140	PASS	AISC- HI-2	0.335	22
		87038.91 C	0.00	1462.06	1.00
1276	UPT PIPX140	PASS	AISC- HI-2	0.334	22
		86875.91 C	0.00	1449.76	0.00
1277	UPT PIPX140	PASS	AISC- HI-2	0.359	22
		94249.71 C	0.00	1473.26	0.00
1278	UPT PIPX140	PASS	AISC- HI-2	0.354	22
		92612.83 C	0.00	1470.21	1.00
1279	UPT PIPX140	PASS	AISC- HI-2	0.353	22
		92447.64 C	0.00	1458.22	0.00
1280	UPT PIPX140	PASS	AISC- HI-2	0.373	22
		99714.91 C	0.00	1380.65	0.00
1281	UPT PIPX140	PASS	AISC- HI-2	0.373	22
		98126.09 C	0.00	1502.41	1.00
1283	UPT PIPX140	PASS	AISC- HI-2	0.372	22
		98098.74 C	0.00	1489.44	0.00
1284	UPT PIPX140	PASS	AISC- HI-2	0.374	22
		99709.29 C	0.00	1407.90	1.00
1285	UPT PIPX140	PASS	AISC- HI-2	0.353	22
		92452.24 C	0.00	1460.31	1.00
1286	UPT PIPX140	PASS	AISC- HI-2	0.354	22
		92617.12 C	0.00	1472.25	0.00
1287	UPT PIPX140	PASS	AISC- HI-2	0.360	22
		94251.05 C	0.00	1502.38	1.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
1290	UPT PIPX140	PASS	AISC- H1-2	0.343	22
		88758.87 C	0.00	1512.40	1.00
1291	UPT PIPX140	PASS	AISC- H1-2	0.317	22
		81372.73 C	0.00	1467.08	0.00
1292	UPT PIPX140	PASS	AISC- H1-2	0.316	22
		83411.44 C	0.00	1436.15	0.00
1293	UPT PIPX140	PASS	AISC- H1-2	0.324	22
		82210.35 C	0.00	1483.48	1.00
1294	UPT PIPX140	PASS	AISC- H1-2	0.298	22
		15704.16 C	0.00	1441.99	0.00
1295	UPT PIPX140	PASS	AISC- H1-2	0.296	22
		75736.71 C	0.00	1402.52	0.00
1296	UPT PIPX140	PASS	AISC- H1-2	0.305	22
		71609.23 C	0.00	1481.47	1.00
1297	UPT PIPX140	PASS	AISC- H1-2	0.279	22
		69983.83 C	0.00	1442.58	0.00
1298	UPT PIPX140	PASS	AISC- H1-2	0.276	22
		70006.36 C	0.00	1357.39	0.00
1299	UPT PIPX140	PASS	AISC- H1-2	0.287	22
		71930.47 C	0.00	1467.32	1.00
1300	UPT PIPX140	PASS	AISC- H1-2	0.260	22
		64167.57 C	0.00	1432.80	0.00
1301	UPT PIPX140	PASS	AISC- H1-2	0.255	22
		64178.05 C	0.00	1289.81	0.00
1302	UPT PIPX140	PASS	AISC- H1-2	0.266	22
		66135.79 C	0.00	1425.64	1.00
1303	UPT PIPX140	PASS	AISC- H1-2	0.240	22
		58287.45 C	0.00	1394.40	0.00
1304	UPT PIPX140	PASS	AISC- H1-2	0.233	22
		58280.77 C	0.00	1222.25	0.00
1305	UPT PIPX140	PASS	AISC- H1-2	0.246	22
		60232.69 C	0.00	1393.19	1.00
1306	UPT PIPX140	PASS	AISC- H1-2	0.219	22
		52279.78 C	0.00	1366.47	0.00
1307	UPT PIPX140	PASS	AISC- H1-2	0.210	22
		52251.73 C	0.00	1127.38	0.00
1308	UPT PIPX140	PASS	AISC- H1-2	0.226	22
		54158.81 C	0.00	1391.90	1.00
1309	UPT PIPX140	PASS	AISC- H1-2	0.200	22
		46271.89 C	0.00	1308.79	0.00
1310	UPT PIPX140	PASS	AISC- H1-2	0.187	22
		46224.70 C	0.00	1031.60	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
1313	LD L30 308	PASS	AISC- H1-3	0.192	24
		2541.13 C	8.68	18.45	0.00
1314	LD L30 308	PASS	AISC- H1-1	0.352	21
		5016.92 C	-31.42	12.36	0.00
1315	LD L30 308	PASS	AISC- H1-1	0.288	22
		4365.42 C	10.57	22.43	0.00
1316	LD L30 308	PASS	AISC- H1-3	0.172	24
		2509.15 C	6.78	16.77	0.00
1317	LD L30 308	PASS	AISC- H1-1	0.132	21
		5176.02 C	-33.44	10.34	0.00
1318	LD L30 308	PASS	AISC- H1-1	0.257	22
		4250.70 C	10.39	21.06	0.00
1319	LD L30 308	PASS	AISC- H1-3	0.150	24
		2404.58 C	5.73	15.00	0.00
1320	LD L30 308	PASS	AISC- H1-1	0.292	21
		4976.93 C	-34.39	8.22	0.00
1321	LD L30 308	PASS	AISC- H1-1	0.227	22
		4108.54 C	10.53	19.43	0.00
1322	LD L30 308	PASS	AISC- H1-3	0.131	24
		2294.02 C	-7.20	12.63	2.90
1323	LD L30 308	PASS	AISC- H1-1	0.259	21
		4848.90 C	14.73	16.59	2.90
1324	LD L30 308	PASS	AISC- H1-1	0.199	22
		3942.35 C	10.59	17.66	0.00
1325	LD L30 308	PASS	AISC- H1-3	0.116	24
		2203.55 C	-6.82	11.42	2.75
1326	LD L30 308	PASS	AISC- H1-1	0.231	21
		4714.19 C	14.69	15.65	2.75
1327	LD L30 308	PASS	AISC- H1-3	0.180	22
		3836.78 C	10.41	16.07	0.00
1328	LD L30 308	PASS	AISC- H1-3	0.103	24
		2109.93 C	-6.83	10.28	2.60
1329	LD L30 308	PASS	AISC- H1-1	0.209	21
		4585.74 C	15.30	14.48	2.60
1330	LD L30 308	PASS	AISC- H1-3	0.163	22
		3708.31 C	10.67	14.30	0.00
1331	LD L30 308	PASS	AISC- H2-1	0.093	21
		106.04 T	45.35	21.46	0.00
1332	LD L30 308	PASS	AISC- H1-3	0.193	21
		4489.79 C	15.16	14.11	2.45
1333	LD L30 308	PASS	AISC- H1-3	0.148	22
		3610.46 C	11.04	12.15	0.00

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ALL UNITS ARE - KG METR (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
1336	LD L30 308	PASS	AISC- H1-3	0.133	22
		3489.29 C	11.58	9.64	0.00
1337	LD L30 308	PASS	AISC- H2-1	0.089	21
		199.53 T	43.51	19.27	0.00
1338	LD L30 308	PASS	AISC- H1-3	0.176	21
		4411.26 C	-33.99	-4.80	0.00
1339	LD L30 308	PASS	AISC- H1-1	0.340	21
		4809.21 C	-30.85	12.68	3.35
1340	LD L30 308	PASS	AISC- H1-1	0.210	23
		2882.36 C	-8.43	18.23	3.35
1341	LD L30 308	PASS	AISC- H1-1	0.338	22
		4762.35 C	9.30	23.45	3.35
1342	LD L30 308	PASS	AISC- H1-1	0.322	21
		4979.78 C	-33.32	10.66	3.20
1343	LD L30 308	PASS	AISC- H1-1	0.193	23
		2860.89 C	-12.66	16.44	3.20
1344	LD L30 308	PASS	AISC- H1-1	0.299	22
		4568.02 C	10.36	22.14	3.20
1345	LD L30 308	PASS	AISC- H1-1	0.283	21
		4767.89 C	-34.51	8.56	3.05
1346	LD L30 308	PASS	AISC- H1-3	0.176	23
		2749.49 C	-14.91	14.69	3.05
1347	LD L30 308	PASS	AISC- H1-1	0.267	22
		4460.64 C	10.47	20.72	3.05
1348	LD L30 308	PASS	AISC- H1-1	0.250	21
		4628.50 C	-34.90	6.63	2.90
1349	LD L30 308	PASS	AISC- H1-3	0.156	23
		2635.85 C	-15.57	13.02	2.90
1350	LD L30 308	PASS	AISC- H1-1	0.236	22
		4328.66 C	10.80	19.04	2.90
1351	LD L30 308	PASS	AISC- H1-1	0.221	21
		4482.25 C	14.25	15.73	0.00
1352	LD L30 308	PASS	AISC- H1-3	0.139	23
		2540.68 C	-15.78	11.33	2.75
1353	LD L30 308	PASS	AISC- H1-1	0.208	22
		4174.20 C	10.99	17.21	2.75
1354	LD L30 308	PASS	AISC- H1-1	0.199	21
		4340.87 C	14.78	14.90	0.00
1355	LD L30 308	PASS	AISC- H1-3	0.123	23
		2443.05 C	-14.85	9.72	2.60
1356	LD L30 308	PASS	AISC- H1-3	0.188	22
		4081.48 C	10.83	15.55	2.60

ALL UNITS ARE - KG METR (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
1359	LD L30 308	PASS	AISC- H1-3	0.170	22
		3967.52 C	10.95	13.70	2.45
1360	LD L30 308	PASS	AISC- H1-3	0.169	21
		4086.49 C	14.26	13.58	0.00
1361	LD L30 308	PASS	AISC- H2-1	0.096	21
		344.71 T	44.48	21.00	2.30
1362	LD L30 308	PASS	AISC- H1-3	0.155	22
		3891.92 C	10.78	11.43	2.30
1363	LD L30 308	PASS	AISC- H1-3	0.164	21
		4100.62 C	-32.38	-3.86	2.15
1364	LD L30 308	PASS	AISC- H2-1	0.098	21
		496.77 T	45.12	20.13	2.15
1365	LD L30 308	PASS	AISC- H1-3	0.138	22
		3767.98 C	10.06	8.86	2.15
1366	LD L30 308	PASS	AISC- H2-1	0.140	22
		3516.48 T	-59.21	-4.32	1.00
1367	LD L30 308	PASS	AISC- H1-3	0.172	22
		768.29 C	104.52	20.60	0.00
1368	LD L30 308	PASS	AISC- H2-1	0.174	21
		2883.01 T	70.26	22.04	0.00
1370	LD L30 308	PASS	AISC- H1-3	0.129	24
		3894.30 C	-1.85	17.60	1.54
1371	LD L30 308	PASS	AISC- H1-3	0.194	22
		3695.22 C	54.27	24.10	0.00
1372	LD L30 308	PASS	AISC- H1-3	0.128	24
		3865.62 C	-26.66	-3.42	0.00
1373	LD L30 308	PASS	AISC- H1-3	0.192	22
		3029.45 C	49.90	24.44	0.00
1374	LD L30 308	PASS	AISC- H1-3	0.125	24
		3789.96 C	-27.01	-3.01	0.00
1375	LD L30 308	PASS	AISC- H1-3	0.190	22
		3894.94 C	48.45	24.10	0.00
1376	LD L30 308	PASS	AISC- H1-3	0.122	24
		3703.80 C	-27.20	-2.67	0.00
1377	LD L30 308	PASS	AISC- H1-3	0.185	22
		3929.32 C	44.22	23.48	0.00
1378	LD L30 308	PASS	AISC- H1-3	0.116	24
		3645.86 C	-24.36	-2.22	0.00
1379	LD L30 308	PASS	AISC- H1-3	0.182	22
		3976.18 C	42.94	22.79	0.00
1380	LD L30 308	PASS	AISC- H1-3	0.111	24
		3591.93 C	-22.58	-1.81	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
1383	LD L30 308	PASS	AISC- H1-3	0.172	22
		4009.76 C		37.56	20.78
1384	LD L30 308	PASS	AISC- H1-3	0.100	24
		3528.78 C		-16.45	-0.76
1385	LD L30 308	PASS	AISC- H1-3	0.165	22
		4023.99 C		33.74	19.48
1386	LD L30 308	PASS	AISC- H1-3	0.095	24
		3528.42 C		-0.71	7.90
1387	LD L30 308	PASS	AISC- H1-3	0.155	22
		3830.42 C		32.49	0.155
1388	LD L30 308	PASS	AISC- H1-3	0.096	24
		3271.12 C		-7.72	7.71
1389	LD L30 308	PASS	AISC- H1-3	0.096	23
		3574.57 C		-8.94	-3.46
1390	LD L30 308	PASS	AISC- H1-3	0.101	23
		3260.04 C		-14.90	6.37
1391	LD L30 308	PASS	AISC- H1-3	0.090	24
		3610.43 C		-2.99	-3.28
1392	LD L30 308	PASS	AISC- H1-3	0.150	22
		3538.06 C		32.71	18.12
1393	LD L30 308	PASS	AISC- H1-3	0.096	22
		1596.34 C		-39.74	-8.37
1394	LD L30 308	PASS	AISC- H1-3	0.101	23
		3518.38 C		-8.41	6.74
1395	LD L30 308	PASS	AISC- H1-3	0.095	24
		3735.44 C		3.42	-3.86
1396	LD L30 308	PASS	AISC- H1-3	0.158	22
		3739.11 C		33.33	19.72
1397	LD L30 308	PASS	AISC- H1-3	0.102	22
		1539.92 C		-45.30	-8.58
1398	LD L30 308	PASS	AISC- H1-3	0.102	23
		3539.22 C		-6.84	7.91
1399	LD L30 308	PASS	AISC- H1-3	0.100	24
		3794.62 C		6.38	-3.84
1400	LD L30 308	PASS	AISC- H1-3	0.165	22
		3745.79 C		36.97	20.97
1401	LD L30 308	PASS	AISC- H1-3	0.105	22
		1502.87 C		47.86	-9.09
1402	LD L30 308	PASS	AISC- H1-3	0.177	22
		3994.82 C		-39.91	21.93
1403	LD L30 308	PASS	AISC- H1-3	0.103	24
		3835.22 C		-7.42	-3.75

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
1406	LD L30 308	PASS	AISC- H1-3	0.106	23
		3602.20 C		-4.09	10.08
1407	LD L30 308	PASS	AISC- H1-3	0.107	24
		3910.21 C		8.67	-3.97
1408	LD L30 308	PASS	AISC- H1-3	0.176	22
		3744.32 C		42.28	22.90
1409	LD L30 308	PASS	AISC- H1-3	0.113	22
		1436.20 C		-54.60	-9.84
1410	LD L30 308	PASS	AISC- H1-3	0.110	23
		3656.40 C		-4.15	11.22
1411	LD L30 308	PASS	AISC- H1-3	0.109	24
		3972.85 C		9.10	-3.92
1412	LD L30 308	PASS	AISC- H1-3	0.179	22
		3710.91 C		43.56	23.57
1413	LD L30 308	PASS	AISC- H1-3	0.113	22
		1136.75 C		-59.04	-10.72
1414	LD L30 308	PASS	AISC- H1-3	0.185	22
		3688.65 C		47.81	24.15
1415	LD L30 308	PASS	AISC- H1-3	0.118	22
		1408.56 C		-58.76	-10.17
1416	LD L30 308	PASS	AISC- H1-3	0.114	23
		3714.18 C		-3.39	12.37
1417	LD L30 308	PASS	AISC- H1-3	0.120	22
		1373.38 C		-60.49	-10.44
1418	LD L30 308	PASS	AISC- H1-3	0.121	23
		3800.81 C		-5.12	13.63
1419	LD L30 308	PASS	AISC- H1-3	0.115	22
		1119.27 C		-60.74	-10.95
1420	LD L30 308	PASS	AISC- H1-3	0.187	22
		3634.79 C		49.21	24.48
1421	LD L30 308	PASS	AISC- H1-3	0.123	22
		1316.28 C		-64.33	-10.52
1422	LD L30 308	PASS	AISC- H1-3	0.127	23
		3876.80 C		-6.33	14.85
1423	LD L30 308	PASS	AISC- H1-3	0.119	22
		1078.77 C		-64.65	-11.00
1424	LD L30 308	PASS	AISC- H1-3	0.189	22
		2511.37 C		53.51	24.10
1425	LD L30 308	PASS	AISC- H1-3	0.124	22
		1353.66 C		-63.67	-10.93
1426	LD L30 308	PASS	AISC- H1-3	0.132	23
		3883.25 C		-7.73	16.10

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ALL UNITS ARE - RD METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ M2	LOADING/ LOCATION
1430	LD L30 307	PASS	AISC- H1-3	0.140	22
	2319.30 C	-36.73		-14.63	1.46
1431	LD L30 307	PASS	AISC- H1-3	0.104	21
	1774.40 C	24.50		5.90	2.23
1432	LD L30 307	PASS	AISC- H1-3	0.111	22
	1591.83 C	-18.47		13.95	0.00
1433	LD L30 307	PASS	AISC- H2-1	0.165	22
	3698.23 T	-67.33		-0.32	0.00
1434	LD L30 307	PASS	AISC- H1-3	0.142	22
	2380.17 C	37.00		-14.08	1.46
1435	LD L30 307	PASS	AISC- H1-3	0.107	21
	1842.19 C	-24.67		5.97	2.23
1436	LD L30 307	PASS	AISC- H1-3	0.113	22
	1643.07 C	18.58		13.89	0.00
1437	LD L30 307	PASS	AISC- H2-1	0.160	22
	3342.17 T	-69.43		0.19	0.00
1438	LD L30 307	PASS	AISC- H1-3	0.133	22
	2195.37 C	36.70		-13.31	1.43
1439	LD L30 307	PASS	AISC- H1-3	0.104	21
	1747.38 C	-25.84		6.48	2.13
1440	LD L30 307	PASS	AISC- H1-3	0.106	22
	1559.65 C	19.42		12.70	0.00
1441	LD L30 307	PASS	AISC- H2-1	0.160	22
	3266.68 T	70.04		0.65	0.00
1442	LD L30 307	PASS	AISC- H1-3	0.131	22
	2145.62 C	-36.33		-13.17	1.43
1443	LD L30 307	PASS	AISC- H1-3	0.102	21
	1683.35 C	25.79		6.37	2.13
1444	LD L30 307	PASS	AISC- H1-3	0.105	22
	1512.59 C	-19.39		12.79	0.00
1445	LD L30 307	PASS	AISC- H2-1	0.155	22
	3300.50 T	65.93		0.53	0.00
1446	LD L30 307	PASS	AISC- H1-3	0.129	22
	2223.47 C	-33.52		-13.28	1.39
1447	LD L30 307	PASS	AISC- H1-3	0.100	21
	1751.07 C	25.21		5.96	2.03
1448	LD L30 307	PASS	AISC- H1-3	0.105	22
	1579.72 C	-19.06		12.66	0.00
1449	LD L30 307	PASS	AISC- H2-1	0.155	22
	3382.19 T	-65.48		0.00	0.00
1450	LD L30 307	PASS	AISC- H1-3	0.131	22
	2278.57 C	33.98		-13.43	1.39

ALL UNITS ARE - RD METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MT	RATIO/ M2	LOADING/ LOCATION
1453	LD L30 307	PASS	AISC- H2-1	0.156	22
	3348.07 T	-66.22		-0.12	0.00
1454	LD L30 307	PASS	AISC- H1-3	0.131	22
	2317.98 C	33.48		-13.19	1.36
1455	LD L30 307	PASS	AISC- H1-3	0.101	21
	1849.65 C	-25.52		5.75	1.93
1456	LD L30 307	PASS	AISC- H1-3	0.105	22
	1666.74 C	19.40		12.36	0.00
1457	LD L30 307	PASS	AISC- H2-1	0.155	22
	3260.33 T	66.61		0.48	0.00
1458	LD L30 307	PASS	AISC- H1-3	0.128	22
	2257.30 C	-32.95		-13.04	1.36
1459	LD L30 307	PASS	AISC- H1-3	0.099	21
	1772.56 C	25.56		5.66	1.93
1460	LD L30 307	PASS	AISC- H1-3	0.103	22
	1609.14 C	-19.42		12.34	0.00
1461	LD L30 307	PASS	AISC- H2-1	0.149	22
	3180.76 T	63.15		0.56	0.00
1462	LD L30 307	PASS	AISC- H1-3	0.123	22
	2269.25 C	-29.95		-12.54	1.32
1463	LD L30 307	PASS	AISC- H1-3	0.095	21
	1773.39 C	24.60		5.43	1.83
1464	LD L30 307	PASS	AISC- H1-3	0.101	22
	1620.28 C	-18.88		11.96	0.00
1465	LD L30 307	PASS	AISC- H2-1	0.150	22
	3275.38 T	-62.70		-0.13	0.00
1466	LD L30 307	PASS	AISC- H1-3	0.126	22
	2336.53 C	30.50		-12.70	1.32
1467	LD L30 307	PASS	AISC- H1-3	0.098	21
	1859.78 C	-24.52		5.51	1.83
1468	LD L30 307	PASS	AISC- H1-3	0.102	22
	1684.70 C	18.83		11.90	0.00
1469	LD L30 307	PASS	AISC- H2-1	0.149	22
	3097.67 T	63.87		0.63	0.00
1470	LD L30 307	PASS	AISC- H1-3	0.121	22
	2284.23 C	-29.12		-12.09	1.29
1471	LD L30 307	PASS	AISC- H1-3	0.093	21
	1778.63 C	24.57		5.18	1.73
1472	LD L30 307	PASS	AISC- H1-3	0.099	22
	1639.32 C	-19.03		11.61	0.00
1473	LD L30 307	PASS	AISC- H2-1	0.149	22
	3200.03 T	-63.36		-0.16	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
1476	LD L30 307	PASS	AISC- H1-3	0.101	22
		1712.01 C	18.97	11.56	0.00
1477	LD L30 307	PASS	AISC- H2-1	0.143	22
		2960.60 T	61.10	0.82	0.00
1478	LD L30 307	PASS	AISC- H1-3	0.116	22
		2279.76 C	-26.38	-11.43	1.26
1479	LD L30 307	PASS	AISC- H1-3	0.089	21
		1761.44 C	23.40	4.98	1.63
1480	LD L30 307	PASS	AISC- H1-3	0.096	22
		1639.34 C	-18.41	11.17	0.00
1481	LD L30 307	PASS	AISC- H2-1	0.143	22
		3091.78 T	-60.43	-0.10	0.00
1482	LD L30 307	PASS	AISC- H1-3	0.119	22
		2364.12 C	26.93	-11.59	1.26
1483	LD L30 307	PASS	AISC- H1-3	0.092	21
		1868.72 C	-23.33	5.00	1.63
1484	LD L30 307	PASS	AISC- H1-3	0.098	22
		1722.31 C	18.36	11.14	0.00
1485	LD L30 307	PASS	AISC- H2-1	0.140	22
		2843.65 T	60.56	1.07	0.00
1486	LD L30 307	PASS	AISC- H1-3	0.112	22
		2264.99 C	-24.99	-10.75	1.23
1487	LD L30 307	PASS	AISC- H2-1	0.089	22
		2114.66 T	23.91	6.58	0.00
1488	LD L30 307	PASS	AISC- H1-3	0.094	22
		1641.00 C	-18.17	10.74	0.00
1489	LD L30 307	PASS	AISC- H2-1	0.139	22
		2965.12 T	-59.79	-0.01	0.00
1490	LD L30 307	PASS	AISC- H1-3	0.115	22
		2360.74 C	25.47	-10.91	1.23
1491	LD L30 307	PASS	AISC- H2-1	0.091	22
		2237.26 T	-23.89	6.19	0.00
1492	LD L30 307	PASS	AISC- H1-3	0.096	22
		1737.34 C	18.20	10.74	0.00
1493	LD L30 307	PASS	AISC- H2-1	0.131	22
		2678.12 T	55.83	1.52	0.00
1494	LD L30 307	PASS	AISC- H1-3	0.105	22
		2231.32 C	-22.08	-10.05	1.20
1495	LD L30 307	PASS	AISC- H2-1	0.086	22
		2084.93 T	22.75	6.28	0.00
1496	LD L30 307	PASS	AISC- H1-3	0.090	22
		1656.05 C	-17.04	10.18	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
1499	LD L30 307	PASS	AISC- H2-1	0.087	22
		2228.34 T	-22.16	5.78	0.00
1500	LD L30 307	PASS	AISC- H1-3	0.094	22
		1770.97 C	17.32	10.20	0.00
1501	LD L30 307	PASS	AISC- H2-1	0.125	22
		2426.64 T	54.88	1.39	0.00
1502	LD L30 307	PASS	AISC- H1-3	0.093	22
		2125.16 C	-17.74	-8.27	1.17
1503	LD L30 307	PASS	AISC- H2-1	0.084	22
		1929.90 T	22.54	6.86	0.00
1504	LD L30 307	PASS	AISC- H1-3	0.085	22
		1577.50 C	-16.29	9.83	0.00
1505	LD L30 307	PASS	AISC- H2-1	0.122	22
		2550.53 T	-53.34	-0.04	0.00
1506	LD L30 307	PASS	AISC- H1-3	0.096	22
		2250.66 C	5.30	15.83	0.00
1507	LD L30 307	PASS	AISC- H1-3	0.084	21
		1820.66 C	-20.86	5.13	1.33
1508	LD L30 307	PASS	AISC- H1-3	0.088	22
		1644.23 C	17.08	9.65	0.00
1510	LD L30 308	PASS	AISC- H1-1	0.220	22
		6823.23 C	-10.80	26.18	1.54
1511	LD L30 308	PASS	AISC- H1-1	0.222	22
		7397.33 C	19.72	-15.56	1.50
1512	LD L30 308	PASS	AISC- H1-1	0.202	21
		6892.80 C	-4.41	19.62	1.50
1513	LD L30 308	PASS	AISC- H1-1	0.318	21
		10348.02 C	-12.89	-34.26	1.46
1514	LD L30 308	PASS	AISC- H1-1	0.215	22
		6796.32 C	-13.20	24.10	1.46
1515	LD L30 308	PASS	AISC- H1-1	0.210	22
		7140.14 C	18.30	-14.99	1.42
1516	LD L30 308	PASS	AISC- H1-1	0.192	22
		6775.32 C	-4.97	16.70	1.42
1517	LD L30 308	PASS	AISC- H1-1	0.206	22
		7063.81 C	17.95	-14.50	1.39
1518	LD L30 308	PASS	AISC- H1-1	0.184	21
		6715.85 C	-3.66	14.84	1.39
1519	LD L30 308	PASS	AISC- H1-1	0.201	22
		6982.95 C	17.77	-13.77	1.36
1520	LD L30 308	PASS	AISC- H1-1	0.183	21
		6730.04 C	-5.35	13.58	1.36

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ HY	RATIO/ HZ	LOADING/ LOCATION
1521	LD L30 308	PASS	AISC- H1-1	0.189	27
	6828.93 C	15.85		-11.61	1.29
1524	LD L30 308	PASS	AISC- H1-1	0.175	21
	6900.83 C	-14.94		3.88	0.00
1525	LD L30 308	PASS	AISC- H1-1	0.183	22
	6723.77 C	15.22		-10.16	1.26
1526	LD L30 308	PASS	AISC- H1-1	0.177	21
	6942.42 C	-13.17		4.87	0.00
1527	LD L30 308	PASS	AISC- H1-3	0.180	22
	6602.11 C	-14.11		8.96	0.00
1528	LD L30 308	PASS	AISC- H1-3	0.161	21
	6392.42 C	-2.95		8.01	1.23
1529	LD L30 308	PASS	AISC- H1-1	0.187	21
	7477.70 C	8.84		-6.73	1.23
1530	LD L30 308	PASS	AISC- H1-1	0.270	21
	9831.47 C	14.15		-21.70	0.00
1531	LD L30 308	PASS	AISC- H1-3	0.171	22
	6308.67 C	14.39		-7.43	0.00
1532	LD L30 308	PASS	AISC- H1-3	0.186	22
	6359.94 C	14.74		15.07	0.00
1533	LD L30 308	PASS	AISC- H1-1	0.196	21
	7608.90 C	10.81		-8.80	1.26
1534	LD L30 308	PASS	AISC- H1-1	0.286	21
	10067.94 C	15.56		-25.52	0.00
1535	LD L30 308	PASS	AISC- H1-3	0.179	22
	6451.80 C	14.60		-9.32	0.00
1536	LD L30 308	PASS	AISC- H1-1	0.193	22
	6945.27 C	12.69		14.49	0.00
1537	LD L30 308	PASS	AISC- H1-1	0.194	21
	7529.00 C	9.70		-8.33	1.29
1538	LD L30 308	PASS	AISC- H1-1	0.292	21
	10113.24 C	14.83		-28.21	0.00
1539	LD L30 308	PASS	AISC- H1-3	0.179	22
	6443.28 C	15.39		-8.55	0.00
1540	LD L30 308	PASS	AISC- H1-1	0.194	22
	6807.24 C	13.44		16.07	0.00
1541	LD L30 308	PASS	AISC- H1-1	0.195	21
	7545.48 C	10.18		-7.60	1.32
1542	LD L30 308	PASS	AISC- H1-1	0.300	21
	10187.79 C	15.00		-30.18	0.00
1543	LD L30 308	PASS	AISC- H1-3	0.181	22
	6468.55 C	16.66		-7.92	0.00

ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ HY	RATIO/ HZ	LOADING/ LOCATION
1546	LD L30 308	PASS	AISC- H1-1	0.306	21
	10272.76 C	14.32		-31.67	0.00
1547	LD L30 308	PASS	AISC- H1-3	0.183	22
	6479.23 C	10.44		-7.45	0.00
1548	LD L30 308	PASS	AISC- H1-1	0.200	22
	6714.27 C	13.02		19.27	0.00
1549	LD L30 308	PASS	AISC- H1-1	0.194	21
	7523.17 C	8.44		-7.00	1.27
1550	LD L30 308	PASS	AISC- H1-1	0.309	21
	10248.46 C	13.78		-32.97	0.00
1551	LD L30 308	PASS	AISC- H1-1	0.181	22
	6523.03 C	18.85		-6.83	0.00
1552	LD L30 308	PASS	AISC- H1-1	0.204	22
	6707.29 C	13.80		20.94	0.00
1553	LD L30 308	PASS	AISC- H1-1	0.197	21
	7599.22 C	8.65		-6.46	1.31
1554	LD L30 308	PASS	AISC- H1-1	0.314	21
	10285.60 C	13.68		-33.76	0.00
1555	LD L30 308	PASS	AISC- H1-1	0.184	22
	6600.54 C	19.56		-6.36	0.00
1556	LD L30 308	PASS	AISC- H1-1	0.210	22
	6766.88 C	13.71		22.52	0.00
1557	LD L30 308	PASS	AISC- H1-1	0.198	21
	7634.64 C	-8.29		-5.77	0.12
1558	LD L30 308	PASS	AISC- H1-1	0.198	21
	6811.77 C	6.34		18.25	0.00
1559	LD L30 308	PASS	AISC- H1-1	0.186	22
	6649.75 C	-19.61		-5.65	1.46
1560	LD L30 308	PASS	AISC- H1-1	0.215	22
	7221.28 C	-18.22		-15.32	0.00
1561	LD L30 308	PASS	AISC- H1-1	0.201	21
	7677.35 C	5.86		-7.06	1.12
1562	LD L30 308	PASS	AISC- H1-1	0.320	21
	10321.08 C	12.36		-34.63	0.00
1563	LD L30 308	PASS	AISC- H1-1	0.191	22
	6908.93 C	21.55		-4.79	0.00
1564	LD L30 308	PASS	AISC- H1-1	0.221	22
	6890.20 C	12.73		26.05	0.00
1565	LD L30 308	PASS	AISC- H1-1	0.200	21
	7595.99 C	-6.84		-6.15	0.26
1566	LD L30 308	PASS	AISC- H1-1	0.207	21
	6828.70 C	5.22		22.00	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
1570	LD L30 307	PASS	AISC- H1-3	0.065	24
		1529.16 C	-23.24	-9.24	1.46
1571	LD L30 307	PASS	AISC- H1-3	0.073	22
		477.19 C	21.65	13.73	0.00
1572	LD L30 307	PASS	AISC- H1-3	0.066	24
		654.01 C	-10.47	13.57	0.00
1573	LD L30 307	PASS	AISC- H2-1	0.116	21
		3787.93 T	-9.21	11.69	0.00
1574	LD L30 307	PASS	AISC- H1-3	0.149	21
		2499.45 C	5.16	36.10	0.00
1575	LD L30 307	PASS	AISC- H1-3	0.072	23
		950.80 C	11.59	11.85	0.00
1576	LD L30 307	PASS	AISC- H1-3	0.085	21
		995.23 C	2.73	21.52	0.00
1577	LD L30 307	PASS	AISC- H2-1	0.119	24
		1946.61 T	48.80	7.05	0.00
1578	LD L30 307	PASS	AISC- H1-3	0.082	24
		1202.79 C	-22.32	-9.11	1.43
1579	LD L30 307	PASS	AISC- H2-1	0.068	24
		962.78 T	19.65	10.89	0.00
1580	LD L30 307	PASS	AISC- H1-3	0.066	0.00
		674.05 C	-11.42	12.98	0.00
1581	LD L30 307	PASS	AISC- H2-1	0.116	21
		3698.65 T	-8.87	12.71	0.00
1582	LD L30 307	PASS	AISC- H1-3	0.148	21
		2496.58 C	5.55	35.85	0.00
1583	LD L30 307	PASS	AISC- H2-1	0.066	21
		1508.43 T	-3.16	14.39	0.00
1584	LD L30 307	PASS	AISC- H1-3	0.083	21
		1023.62 C	2.63	20.84	0.00
1585	LD L30 307	PASS	AISC- H2-1	0.119	21
		3716.74 T	-10.55	13.11	0.00
1586	LD L30 307	PASS	AISC- H1-3	0.149	21
		2572.86 C	5.76	35.56	0.00
1587	LD L30 307	PASS	AISC- H2-1	0.068	21
		1561.41 T	-4.19	14.11	0.00
1588	LD L30 307	PASS	AISC- H1-3	0.084	21
		1082.94 C	3.04	20.48	0.00
1589	LD L30 307	PASS	AISC- H2-1	0.115	24
		1928.16 T	46.82	6.95	0.00
1590	LD L30 307	PASS	AISC- H1-3	0.081	24
		1538.78 C	-5.31	18.86	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
1593	LD L30 307	PASS	AISC- H2-1	0.114	24
		1880.37 T	46.39	6.99	0.00
1594	LD L30 307	PASS	AISC- H1-3	0.080	24
		1346.67 C	-5.45	18.25	0.00
1595	LD L30 307	PASS	AISC- H1-3	0.070	22
		484.69 C	22.21	12.60	0.00
1596	LD L30 307	PASS	AISC- H1-3	0.062	24
		675.46 C	-11.96	11.75	0.00
1597	LD L30 307	PASS	AISC- H2-1	0.116	21
		3588.43 T	-9.57	13.71	0.00
1598	LD L30 307	PASS	AISC- H1-3	0.147	21
		2554.45 C	6.23	34.93	0.00
1599	LD L30 307	PASS	AISC- H2-1	-3.39	0.066
		1532.97 T	-3.39	14.26	0.00
1600	LD L30 307	PASS	AISC- H1-3	0.081	21
		1075.39 C	2.83	19.88	0.00
1601	LD L30 307	PASS	AISC- H2-1	0.110	24
		1845.12 T	43.73	7.13	0.00
1602	LD L30 307	PASS	AISC- H1-3	0.079	24
		1362.01 C	-5.43	17.73	0.00
1603	LD L30 307	PASS	AISC- H1-3	0.069	22
		494.50 C	21.64	12.42	0.00
1604	LD L30 307	PASS	AISC- H1-3	0.061	24
		700.81 C	-11.61	11.21	0.00
1605	LD L30 307	PASS	AISC- H2-1	0.116	21
		3539.95 T	-9.42	14.30	0.00
1606	LD L30 307	PASS	AISC- H1-3	0.147	21
		2597.60 C	6.66	34.16	0.00
1607	LD L30 307	PASS	AISC- H2-1	0.065	21
		1466.76 T	-3.02	14.34	0.00
1608	LD L30 307	PASS	AISC- H1-3	0.077	21
		1041.45 C	2.76	19.06	0.00
1609	LD L30 307	PASS	AISC- H2-1	0.109	24
		1763.93 T	43.86	7.33	0.00
1610	LD L30 307	PASS	AISC- H1-3	0.073	24
		1351.53 C	-5.11	16.99	0.00
1611	LD L30 307	PASS	AISC- H1-3	0.070	22
		565.43 C	21.10	12.50	0.00
1612	LD L30 307	PASS	AISC- H1-3	0.058	24
		683.06 C	-11.60	10.51	0.00
1613	LD L30 307	PASS	AISC- H2-1	0.118	21
		3441.07 T	-10.86	15.33	0.00

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ALL UNITS ARE - RG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ KV	RATIO/ HZ	LOADING/ LOCATION
1616	LD	L30 307	PASS	AISC- H1-3	0.075
		1047.76 C	3.15		21
1617	LD	L30 307	PASS	AISC- H2-1	18.51
		1689.90 T	41.21		0.00
1618	LD	L30 307	PASS	AISC- H1-3	0.014
		1342.07 C	-4.81		24
1619	LD	L30 307	PASS	AISC- H1-3	0.070
		621.05 C	20.01		0.00
1620	LD	L30 307	PASS	AISC- H1-3	0.055
		687.01 C	-10.70		24
1621	LD	L30 307	PASS	AISC- H2-1	0.115
		3218.60 T	-9.74		0.00
1622	LD	L30 307	PASS	AISC- H1-3	0.141
		2539.19 C	7.08		21
1623	LD	L30 307	PASS	AISC- H2-1	0.064
		1343.39 T	-3.57		0.00
1624	LD	L30 307	PASS	AISC- H1-3	0.070
		991.46 C	2.89		21
1625	LD	L30 307	PASS	AISC- H2-1	0.103
		1366.05 T	41.71		0.00
1626	LD	L30 307	PASS	AISC- H1-3	0.070
		1300.65 C	-4.13		24
1627	LD	L30 307	PASS	AISC- H1-3	0.072
		730.99 C	0.91		0.00
1628	LD	L30 307	PASS	AISC- H1-3	0.071
		634.82 C	-10.01		0.00
1629	LD	L30 307	PASS	AISC- H2-1	0.113
		3079.36 T	-9.19		21
1630	LD	L30 307	PASS	AISC- H1-3	0.137
		2493.65 C	7.31		0.00
1631	LD	L30 307	PASS	AISC- H2-1	0.061
		1169.62 T	-3.27		21
1632	LD	L30 307	PASS	AISC- H1-3	0.064
		875.95 C	2.74		0.00
1633	LD	L30 307	PASS	AISC- H2-1	0.099
		1430.36 T	39.43		24
1634	LD	L30 307	PASS	AISC- H1-3	0.065
		1248.60 C	-2.97		24
1635	LD	L30 307	PASS	AISC- H1-3	0.076
		909.26 C	18.48		0.00
1636	LD	L30 307	PASS	AISC- H1-3	0.045
		602.47 C	-8.01		24
					0.00

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TORRE AUTOSOURTADA

ALL UNITS ARE - RG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ KV	RATIO/ HZ	LOADING/ LOCATION
1639	LD	L30 307	PASS	AISC- H2-1	0.061
		1025.20 T	-4.62		21
1640	LD	L30 307	PASS	AISC- H1-3	15.86
		793.39 C	2.77		0.00
1641	LD	L30 307	PASS	AISC- H2-1	0.097
		1304.39 T	40.23		24
1642	LD	L30 307	PASS	AISC- H1-3	0.065
		1176.31 C	-17.73		24
1643	LD	L30 307	PASS	AISC- H2-1	0.075
		883.80 T	30.08		0.00
1644	LD	L30 307	PASS	AISC- H2-1	0.062
		635.92 T	20.16		0.57
1645	LD	L30 307	PASS	AISC- H2-1	0.110
		2532.80 T	-8.56		10.95
1646	LD	L30 307	PASS	AISC- H1-3	0.125
		2294.57 C	7.36		21
1647	LD	L30 307	PASS	AISC- H2-1	0.075
		1666.46 T	-3.53		21
1648	LD	L30 307	PASS	AISC- H1-3	16.91
		1338.45 C	-8.18		0.00
1650	LD	L30 308	PASS	AISC- H1-1	0.201
		6577.72 C	5.76		1.63
1651	LD	L30 308	PASS	AISC- H1-1	21.47
		7216.52 C	-13.13		1.54
1652	LD	L30 308	PASS	AISC- H1-1	15.72
		4624.54 C	1.79		0.238
1653	LD	L30 308	PASS	AISC- H1-1	0.195
		4076.66 C	-17.01		1.50
1654	LD	L30 308	PASS	AISC- H1-1	17.54
		6524.13 C	6.51		1.50
1655	LD	L30 308	PASS	AISC- H1-1	17.54
		6333.15 C	-13.85		0.130
1656	LD	L30 308	PASS	AISC- H1-1	15.83
		6466.00 C	5.11		1.46
1657	LD	L30 308	PASS	AISC- H1-1	15.98
		6845.34 C	-17.48		0.205
1658	LD	L30 308	PASS	AISC- H1-3	14.10
		6381.88 C	3.82		1.42
1659	LD	L30 308	PASS	AISC- H1-1	0.196
		6751.16 C	-17.21		0.201
1660	LD	L30 308	PASS	AISC- H1-3	14.03
		6367.39 C	5.45		1.39
					1.39
					0.178
					14.10
					0.196
					14.03
					0.176
					12.83
					1.36

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FK	CRITICAL COND/ MY	RATIO/ KG	LOADING/ LOCATION
1663	LD L30 308	PASS	AISC- H1-3	0.188	22
		8564.57 C	-14.98	-11.97	1.29
1664	LD L30 308	PASS	AISC- H1-3	0.169	21
		6364.52 C	15.91	4.07	0.00
1665	LD L30 308	PASS	AISC- H1-3	0.181	22
		6466.86 C	13.49	11.22	0.00
1666	LD L30 308	PASS	AISC- H1-3	0.170	21
		6456.26 C	13.80	4.98	0.00
1667	LD L30 308	PASS	AISC- H1-3	0.174	22
		6309.80 C	14.06	9.34	0.00
1668	LD L30 308	PASS	AISC- H1-3	0.152	21
		5927.66 C	4.67	8.11	1.23
1669	LD L30 308	PASS	AISC- H1-1	0.180	22
		6465.66 C	-16.87	-4.55	0.00
1670	LD L30 308	PASS	AISC- H1-1	0.212	22
		6573.29 C	-10.18	25.39	0.00
1671	LD L30 308	PASS	AISC- H1-1	0.194	21
		7371.18 C	-4.67	-6.86	1.15
1673	LD L30 308	PASS	AISC- H1-1	0.185	22
		6571.88 C	-20.89	-4.58	0.00
1674	LD L30 308	PASS	AISC- H1-1	0.214	22
		6601.06 C	35.55	-12.71	1.50
1675	LD L30 308	PASS	AISC- H1-1	0.194	21
		7436.73 C	-5.14	-7.03	1.12
1676	LD L30 308	PASS	AISC- H1-1	0.315	21
		10130.86 C	-11.61	-34.76	0.00
1677	LD L30 308	PASS	AISC- H1-1	0.179	22
		6415.40 C	-19.16	-5.44	0.00
1678	LD L30 308	PASS	AISC- H1-1	0.206	22
		6508.77 C	-12.88	23.40	0.00
1679	LD L30 308	PASS	AISC- H1-1	0.191	21
		7378.50 C	-6.45	-6.49	1.22
1680	LD L30 308	PASS	AISC- H1-1	0.313	21
		10152.86 C	-12.30	-34.42	0.00
1681	LD L30 308	PASS	AISC- H1-3	0.181	22
		6328.67 C	-19.16	-6.12	0.00
1682	LD L30 308	PASS	AISC- H1-1	0.201	22
		6457.54 C	-13.43	21.80	0.00
1683	LD L30 308	PASS	AISC- H1-1	0.190	21
		7325.77 C	-6.99	-7.00	1.19
1684	LD L30 308	PASS	AISC- H1-1	0.309	21
		10073.58 C	-13.14	-33.95	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FK	CRITICAL COND/ MY	RATIO/ KG	LOADING/ LOCATION
1687	LD L30 308	PASS	AISC- H1-1	0.187	21
		7230.29 C	-6.93	-7.39	1.16
1688	LD L30 308	PASS	AISC- H1-1	0.304	21
		10030.06 C	-13.22	-33.20	0.00
1689	LD L30 308	PASS	AISC- H1-3	0.175	22
		6166.15 C	-17.96	-7.15	0.00
1690	LD L30 308	PASS	AISC- H1-3	0.195	22
		6351.76 C	-12.77	18.52	0.00
1691	LD L30 308	PASS	AISC- H1-1	0.188	21
		7229.84 C	-9.30	-6.49	1.36
1692	LD L30 308	PASS	AISC- H1-1	0.300	21
		10040.92 C	-13.87	-31.93	0.00
1693	LD L30 308	PASS	AISC- H1-3	0.172	22
		6130.21 C	-16.10	-7.58	0.00
1694	LD L30 308	PASS	AISC- H1-3	0.192	22
		6354.63 C	-13.39	17.04	0.00
1695	LD L30 308	PASS	AISC- H1-1	0.186	21
		7207.34 C	-9.47	-7.25	1.32
1696	LD L30 308	PASS	AISC- H1-1	0.294	21
		9941.00 C	-14.24	-30.49	0.00
1697	LD L30 308	PASS	AISC- H1-3	0.169	22
		6075.24 C	-14.56	-8.15	0.00
1698	LD L30 308	PASS	AISC- H1-3	0.187	22
		6370.69 C	-12.78	15.33	0.00
1699	LD L30 308	PASS	AISC- H1-1	0.184	21
		7161.59 C	-8.72	-7.91	1.29
1700	LD L30 308	PASS	AISC- H1-1	0.286	21
		9849.80 C	-13.85	-28.56	0.00
1701	LD L30 308	PASS	AISC- H1-3	0.167	22
		6054.77 C	-13.25	-8.79	0.00
1702	LD L30 308	PASS	AISC- H1-3	0.183	22
		6459.45 C	-11.48	13.73	0.00
1703	LD L30 308	PASS	AISC- H1-1	0.185	21
		7200.88 C	-9.23	-8.33	1.26
1704	LD L30 308	PASS	AISC- H1-1	0.274	21
		9778.59 C	-14.14	-25.95	0.00
1705	LD L30 308	PASS	AISC- H1-3	0.158	22
		5892.38 C	-11.91	-6.93	0.00
1706	LD L30 308	PASS	AISC- H1-3	0.170	22
		5896.99 C	-12.07	13.35	0.00
1707	LD L30 308	PASS	AISC- H1-1	0.175	21
		7064.03 C	-6.52	-6.52	1.23

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
1710	LD L30 307	PASS	AISC- H1-3	0.151	21
		2552.61 C	-5.38	36.17	0.00
1711	LD L30 307	PASS	AISC- H1-3	0.072	24
		828.06 C	12.77	11.49	0.00
1712	LD L30 307	PASS	AISC- H1-3	0.087	21
		1039.97 C	-2.84	21.45	0.00
1713	LD L30 307	PASS	AISC- H2-1	0.116	21
		3769.50 T	8.35	12.25	0.00
1714	LD L30 307	PASS	AISC- H1-3	0.150	21
		2543.08 C	-5.65	35.89	0.00
1715	LD L30 307	PASS	AISC- H2-1	0.067	21
		1568.44 T	7.95	14.28	0.00
1716	LD L30 307	PASS	AISC- H1-3	0.084	21
		1068.13 C	-2.69	20.73	0.00
1717	LD L30 307	PASS	AISC- H2-1	0.119	21
		3798.16 T	10.08	12.59	0.00
1718	LD L30 307	PASS	AISC- H1-3	0.131	21
		2627.61 C	-5.81	35.62	0.00
1719	LD L30 307	PASS	AISC- H2-1	0.069	21
		1631.40 T	4.18	13.96	0.00
1720	LD L30 307	PASS	AISC- H1-3	0.085	21
		1134.89 C	-3.03	20.39	0.00
1721	LD L30 307	PASS	AISC- H2-1	0.116	21
		3676.41 T	9.14	13.11	0.00
1722	LD L30 307	PASS	AISC- H1-3	0.149	21
		2615.11 C	-6.23	35.00	0.00
1723	LD L30 307	PASS	AISC- H2-1	0.068	21
		1610.07 T	3.52	14.08	0.00
1724	LD L30 307	PASS	AISC- H1-3	0.082	21
		1133.21 C	-2.79	19.80	0.00
1725	LD L30 307	PASS	AISC- H2-1	0.116	21
		3634.63 T	9.01	13.61	0.00
1726	LD L30 307	PASS	AISC- H1-3	0.149	21
		2664.71 C	-6.64	34.23	0.00
1727	LD L30 307	PASS	AISC- H2-1	0.066	21
		1551.96 T	3.25	14.12	0.00
1728	LD L30 307	PASS	AISC- H1-3	0.078	21
		1106.12 C	-2.71	18.99	0.00
1729	LD L30 307	PASS	AISC- H2-1	0.118	21
		3543.50 T	10.36	14.54	0.00
1730	LD L30 307	PASS	AISC- H1-3	0.147	21
		2687.62 C	-6.74	33.39	0.00

ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
1733	LD L30 307	PASS	AISC- H2-1	0.115	21
		3329.89 T	9.14	15.68	0.00
1734	LD L30 307	PASS	AISC- H1-3	0.143	21
		2623.37 C	-7.07	32.24	0.00
1735	LD L30 307	PASS	AISC- H2-1	0.066	21
		1450.73 T	3.74	14.55	0.00
1736	LD L30 307	PASS	AISC- H1-3	0.073	21
		1074.78 C	-2.85	17.64	0.00
1737	LD L30 307	PASS	AISC- H2-1	0.112	21
		3151.12 T	8.46	16.91	0.00
1738	LD L30 307	PASS	AISC- H1-3	0.139	21
		2589.31 C	-7.35	30.75	0.00
1739	LD L30 307	PASS	AISC- H2-1	0.063	21
		1292.09 T	3.20	14.97	0.00
1740	LD L30 307	PASS	AISC- H1-3	0.067	21
		972.80 C	-2.79	16.64	0.00
1741	LD L30 307	PASS	AISC- H2-1	0.111	21
		2897.98 T	7.99	18.79	0.00
1742	LD L30 307	PASS	AISC- H1-3	0.133	21
		2493.14 C	-7.05	29.18	0.00
1743	LD L30 307	PASS	AISC- H2-1	0.062	21
		1157.71 T	3.85	15.37	0.00
1744	LD L30 307	PASS	AISC- H1-3	0.062	21
		899.67 C	-3.01	15.46	0.00
1745	LD L30 307	PASS	AISC- H2-1	0.108	21
		2640.08 T	7.33	20.44	0.00
1746	LD L30 307	PASS	AISC- H1-3	0.128	21
		2395.91 C	-7.69	27.94	0.00
1747	LD L30 307	PASS	AISC- H2-1	0.074	21
		1770.34 T	1.71	16.30	0.00
1748	LD L30 307	PASS	AISC- H1-3	0.086	21
		1404.40 C	9.26	-16.78	1.63
1749	LD L30 307	PASS	AISC- H2-1	0.070	23
		2043.41 T	-9.79	7.03	0.00
1750	LD L30 307	PASS	AISC- H1-3	0.078	23
		1327.72 C	0.91	19.75	0.00
1751	LD L30 307	PASS	AISC- H1-3	0.071	22
		427.45 C	-21.19	13.69	0.00
1752	LD L30 307	PASS	AISC- H1-3	0.068	21
		628.04 C	-12.62	9.73	2.40
1753	LD L30 307	PASS	AISC- H2-1	0.072	22
		894.91 T	-19.68	13.37	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
1756	LD L30 307	PASS	AISC- H1-3	0.056	21
		511.76 C	-13.44	9.68	2.31
1757	LD L30 307	PASS	AISC- H2-1	0.074	22
		827.66 T	-21.45	13.59	0.00
1758	LD L30 307	PASS	AISC- H1-3	0.077	23
		1323.64 C	2.14	18.93	0.00
1759	LD L30 307	PASS	AISC- H1-3	0.067	22
		357.15 C	-22.70	12.55	0.00
1760	LD L30 307	PASS	AISC- H1-3	0.054	21
		564.59 C	-13.41	8.52	2.22
1761	LD L30 307	PASS	AISC- H2-1	0.073	22
		800.60 T	-20.59	13.99	0.00
1762	LD L30 307	PASS	AISC- H1-3	0.076	23
		1331.20 C	2.37	18.33	0.00
1763	LD L30 307	PASS	AISC- H1-3	0.067	22
		407.88 C	-22.41	12.42	0.00
1764	LD L30 307	PASS	AISC- H1-3	0.052	21
		596.70 C	-12.94	7.58	2.13
1765	LD L30 307	PASS	AISC- H2-1	0.073	22
		791.31 T	-20.19	14.46	0.00
1766	LD L30 307	PASS	AISC- H1-3	0.075	23
		1347.37 C	2.56	17.81	0.00
1767	LD L30 307	PASS	AISC- H1-3	0.066	22
		409.22 C	-21.91	12.21	0.00
1768	LD L30 307	PASS	AISC- H1-3	0.049	21
		603.70 C	-12.28	6.69	2.04
1769	LD L30 307	PASS	AISC- H2-1	0.072	22
		732.07 T	-19.29	15.20	0.00
1770	LD L30 307	PASS	AISC- H1-3	0.073	23
		1336.69 C	2.52	17.07	0.00
1771	LD L30 307	PASS	AISC- H1-3	0.067	22
		470.40 C	-21.39	12.24	0.00
1772	LD L30 307	PASS	AISC- H1-3	0.046	21
		647.51 C	-11.30	5.63	1.96
1773	LD L30 307	PASS	AISC- H2-1	0.072	22
		676.61 T	-18.88	16.14	0.00
1774	LD L30 307	PASS	AISC- H1-3	0.071	23
		1327.86 C	2.57	16.30	0.00
1775	LD L30 307	PASS	AISC- H1-3	0.066	22
		514.03 C	-20.25	12.28	0.00
1776	LD L30 307	PASS	AISC- H1-3	0.043	21
		681.03 C	-10.03	4.70	1.87

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
1779	LD L30 307	PASS	AISC- H1-3	0.068	22
		609.34 C	-19.19	12.58	0.00
1780	LD L30 307	PASS	AISC- H1-3	0.040	21
		749.77 C	-8.46	3.53	1.79
1781	LD L30 307	PASS	AISC- H2-1	0.070	22
		439.01 T	-16.38	14.71	0.00
1782	LD L30 307	PASS	AISC- H1-3	0.063	23
		1235.03 C	1.49	14.35	0.00
1783	LD L30 307	PASS	AISC- H1-3	0.070	22
		758.64 C	-17.83	13.08	0.00
1784	LD L30 307	PASS	AISC- H1-3	0.039	21
		883.61 C	-1.46	-5.52	0.85
1785	LD L30 307	PASS	AISC- H2-1	0.072	22
		429.69 T	-16.24	15.99	0.00
1786	LD L30 307	PASS	AISC- H1-3	0.058	23
		1177.35 C	0.58	13.42	0.00
1787	LD L30 307	PASS	AISC- H2-1	0.059	21
		882.17 T	-16.91	8.49	0.00
1788	LD L30 307	PASS	AISC- H2-1	0.061	24
		603.05 T	-19.28	11.02	1.63
1789	LD L30 308	PASS	AISC- H1-3	0.059	23
		125.24 C	-3.51	26.13	3.05
1790	LD L30 308	PASS	AISC- H1-3	0.063	23
		127.50 C	-3.91	27.78	3.20
1791	LD L30 308	PASS	AISC- H1-3	0.062	23
		122.53 C	-3.20	27.15	3.35
1792	LD L30 308	PASS	AISC- H1-3	0.081	21
		200.66 C	4.90	35.32	0.00
1793	LD L30 308	PASS	AISC- H1-3	0.088	21
		205.14 C	5.13	38.11	0.00
1794	LD L30 308	PASS	AISC- H1-3	0.089	21
		197.17 C	4.40	38.51	0.00
1795	LD L30 308	PASS	AISC- H1-3	0.081	21
		200.63 C	-4.85	35.04	0.00
1796	LD L30 308	PASS	AISC- H1-3	0.087	21
		205.01 C	-4.48	37.86	0.00
1797	LD L30 308	PASS	AISC- H1-3	0.087	21
		197.00 C	-3.75	38.33	0.00
1798	LD L30 308	PASS	AISC- H1-3	0.077	21
		203.74 C	5.06	33.50	0.00
1799	LD L30 308	PASS	AISC- H1-3	0.070	21
		196.95 C	5.07	30.39	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
1802	LD L30 308	PASS	AISC- H1-3	0.052	23
		122.69 C	-3.55	22.78	2.75
1803	LD L30 308	PASS	AISC- H1-3	0.057	23
		127.46 C	-3.59	25.04	2.90
1804	LD L30 308	PASS	AISC- H1-3	0.065	21
		199.07 C	4.92	28.04	2.60
1805	LD L30 308	PASS	AISC- H1-3	0.070	21
		196.93 C	5.20	30.09	2.75
1806	LD L30 308	PASS	AISC- H1-3	0.077	21
		203.77 C	5.09	33.21	2.90
1807	LD L30 308	PASS	AISC- H1-3	0.059	21
		191.36 C	4.85	25.05	0.00
1808	LD L30 308	PASS	AISC- H1-3	0.054	21
		191.85 C	4.90	22.47	0.00
1809	LD L30 308	PASS	AISC- H1-3	0.035	23
		117.67 C	-2.95	14.94	2.30
1810	LD L30 308	PASS	AISC- H1-3	0.041	23
		117.46 C	-3.17	17.69	2.43
1811	LD L30 308	PASS	AISC- H1-3	0.058	21
		191.37 C	-4.92	24.73	0.00
1812	LD L30 308	PASS	AISC- H1-3	0.053	21
		191.79 C	-4.70	22.14	0.00
1813	LD L30 308	PASS	AISC- H1-3	0.043	21
		271.85 C	3.44	16.69	0.00
1814	LD L30 308	PASS	AISC- H1-3	0.046	21
		183.05 C	4.99	19.82	0.00
1815	LD L30 308	PASS	AISC- H1-3	0.031	23
		136.82 C	-0.73	13.99	0.00
1816	LD L30 308	PASS	AISC- H1-3	0.076	23
		112.85 C	4.85	9.07	0.00
1817	LD L30 308	PASS	AISC- H1-3	0.040	21
		283.20 C	-1.59	16.32	0.00
1818	LD L30 308	PASS	AISC- H1-3	0.045	21
		183.05 C	4.04	18.46	2.15
1821	LD L30 307	PASS	AISC- H1-3	0.102	22
		190.69 C	17.86	34.72	1.00
1825	LD L30 307	PASS	AISC- R2-1	0.076	21
		1264.79 T	12.13	15.75	1.00
1826	LD L30 307	PASS	AISC- H1-3	0.113	21
		1562.12 C	-17.55	-24.63	0.00
1827	ST PIP X100	PASS	AISC- H1-2	0.368	21
		49809.47 C	0.00	559.22	1.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
1830	ST PIP X100	PASS	AISC- H1-3	0.215	22
		22036.13 C	0.00	781.34	1.00
1831	ST PIP X100	PASS	AISC- H1-3	0.238	22
		22110.07 C	0.00	1020.49	1.00
1832	ST PIP X100	PASS	AISC- H1-2	0.279	22
		29114.10 C	0.00	1018.70	0.00
1833	ST PIP X100	PASS	AISC- H1-2	0.244	22
		25702.40 C	0.00	873.40	1.00
1834	ST PIP X100	PASS	AISC- H1-2	0.258	22
		25719.55 C	0.00	1030.52	1.00
1835	ST PIP X100	PASS	AISC- H1-2	0.304	22
		32880.80 C	0.00	1033.86	0.00
1836	ST PIP X100	PASS	AISC- H1-2	0.277	22
		29555.95 C	0.00	972.98	1.00
1837	ST PIP X100	PASS	AISC- H1-2	0.289	22
		29523.50 C	0.00	1101.03	1.00
1838	ST PIP X100	PASS	AISC- H1-2	0.334	22
		36473.94 C	0.00	1111.90	0.00
1839	ST PIP X100	PASS	AISC- R1-2	0.308	22
		33302.05 C	0.00	1041.10	1.00
1840	ST PIP X100	PASS	AISC- H1-2	0.317	22
		33238.24 C	0.00	1151.15	1.00
1841	ST PIP X100	PASS	AISC- H1-2	0.362	22
		39994.68 C	0.00	1168.31	0.00
1842	ST PIP X100	PASS	AISC- H1-2	0.334	22
		36958.53 C	0.00	1079.37	1.00
1843	ST PIP X100	PASS	AISC- H1-2	0.343	22
		36872.89 C	0.00	1179.15	1.00
1844	ST PIP X100	PASS	AISC- H1-2	0.386	22
		43367.23 C	0.00	1201.19	0.00
1845	ST PIP X100	PASS	AISC- H1-2	0.358	22
		40312.17 C	0.00	1102.26	1.00
1846	ST PIP X100	PASS	AISC- H1-2	0.378	22
		40189.17 C	0.00	1328.47	1.00
1847	ST PIP X100	PASS	AISC- H1-2	0.415	21
		42946.94 C	0.00	1545.28	1.00
1848	ST PIP X100	PASS	AISC- H1-2	0.412	21
		42962.70 C	0.00	1507.59	0.00
1849	ST PIP X100	PASS	AISC- H1-2	0.471	21
		56479.32 C	0.00	1214.23	0.00
1850	ST PIP X100	PASS	AISC- H1-2	0.475	21
		49830.29 C	0.00	1717.89	1.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION	
1853	ST	PIP X100	PASS	AISC- H1-2	0.537	21
		57044.04	C	0.00	1869.31	1.00
1854	ST	PIP X100	PASS	AISC- H1-2	0.534	21
		56856.23	C	0.00	1869.28	0.00
1855	ST	PIP X100	PASS	AISC- H1-2	0.588	21
		70239.48	C	0.00	1544.67	0.00
1856	ST	PIP X100	PASS	AISC- H1-2	0.593	21
		64049.23	C	0.00	2013.62	1.00
1857	ST	PIP X100	PASS	AISC- H1-2	0.590	21
		63800.05	C	0.00	1999.91	0.00
1858	ST	PIP X100	PASS	AISC- H1-2	0.641	21
		76767.66	C	0.00	1665.34	0.00
1859	ST	PIP X100	PASS	AISC- H1-2	0.646	21
		70988.85	C	0.00	2113.74	1.00
1860	ST	PIP X100	PASS	AISC- H1-2	0.643	21
		70699.48	C	0.00	2104.13	0.00
1861	ST	PIP X100	PASS	AISC- H1-2	0.687	21
		82936.16	C	0.00	1745.13	0.00
1862	ST	PIP X100	PASS	AISC- H1-2	0.684	21
		77009.06	C	0.00	2110.08	1.00
1863	ST	PIP X100	PASS	AISC- H1-2	0.701	21
		76703.41	C	0.00	2315.64	1.00
1864	ST	PIP X100	PASS	AISC- H1-3	0.715	22
		22036.13	C	0.00	781.34	1.00
1865	ST	PIP X100	PASS	AISC- H1-3	0.238	22
		22110.07	C	0.00	1020.49	1.00
1866	ST	PIP X100	PASS	AISC- H1-2	0.279	22
		29114.10	C	0.00	1018.70	0.00
1867	ST	PIP X100	PASS	AISC- H1-2	0.244	22
		25702.41	C	0.00	873.40	1.00
1868	ST	PIP X100	PASS	AISC- H1-2	0.258	22
		25719.55	C	0.00	1030.51	1.00
1869	ST	PIP X100	PASS	AISC- H1-2	0.304	22
		32880.80	C	0.00	1033.85	0.00
1870	ST	PIP X100	PASS	AISC- H1-2	0.277	22
		29555.93	C	0.00	972.58	1.00
1871	ST	PIP X100	PASS	AISC- H1-2	0.289	22
		29523.47	C	0.00	1101.00	1.00
1872	ST	PIP X100	PASS	AISC- H1-2	0.334	22
		36473.85	C	0.00	1111.68	0.00
1873	ST	PIP X100	PASS	AISC- H1-2	0.308	22
		33301.82	C	0.00	1041.09	1.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION	
1876	ST	PIP X100	PASS	AISC- H1-2	0.334	22
		36963.34	C	0.00	1079.55	1.00
1877	ST	PIP X100	PASS	AISC- H1-2	0.343	22
		36878.13	C	0.00	1179.83	1.00
1878	ST	PIP X100	PASS	AISC- H1-2	0.386	22
		43369.06	C	0.00	1202.09	0.00
1879	ST	PIP X100	PASS	AISC- H1-2	0.358	22
		40303.79	C	0.00	1102.96	1.00
1880	ST	PIP X100	PASS	AISC- H1-2	0.377	22
		40179.65	C	0.00	1317.25	1.00
1881	LD	L30 307	PASS	AISC- H1-3	0.123	22
		1690.95	C	8.46	29.45	0.00
1882	LD	L30 307	PASS	AISC- H2-1	0.117	21
		196.72	T	32.43	32.53	0.00
1883	LD	L30 307	PASS	AISC- H1-3	0.106	21
		2120.14	C	7.84	15.42	1.83
1884	LD	L30 307	PASS	AISC- H1-3	0.113	22
		1455.48	C	7.73	29.20	0.00
1885	LD	L30 307	PASS	AISC- H2-1	0.118	21
		108.56	T	33.35	33.29	0.00
1886	LD	L30 307	PASS	AISC- H1-3	0.095	21
		1813.73	C	7.93	16.07	1.67
1887	LD	L30 307	PASS	AISC- H1-3	0.108	22
		1376.03	C	7.64	28.81	0.00
1888	LD	L30 307	PASS	AISC- H2-1	0.121	21
		108.18	T	31.72	34.60	0.00
1889	LD	L30 307	PASS	AISC- H1-3	0.093	21
		1695.23	C	-25.98	-5.79	0.00
1890	LD	L30 307	PASS	AISC- H1-3	0.102	22
		1282.34	C	7.51	27.76	0.00
1891	LD	L30 307	PASS	AISC- H1-3	0.122	22
		77.74	C	-32.98	-35.54	0.00
1892	LD	L30 307	PASS	AISC- H1-3	0.094	21
		1582.79	C	-25.12	-8.81	0.00
1893	LD	L30 307	PASS	AISC- H2-1	0.094	21
		1050.42	T	11.31	26.76	1.17
1894	LD	L30 307	PASS	AISC- H1-3	0.138	22
		373.49	C	-33.44	-39.32	0.00
1895	LD	L30 307	PASS	AISC- H1-3	0.106	21
		1674.54	C	-25.99	-13.85	0.00
1896	LD	L30 307	PASS	AISC- H1-3	0.113	21
		1562.14	C	-17.54	-24.63	1.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
1899	LD L30 307	PASS	AISC- H1-3	0.106	21
	1674.61 C	-29.92		-13.85	1.17
1900	LD L30 307	PASS	AISC- H1-3	0.138	22
	373.42 C	-33.46		-39.32	1.17
1901	LD L30 307	PASS	AISC- H2-1	0.094	21
	1050.35 T	11.30		26.76	0.00
1902	LD L30 307	PASS	AISC- H1-1	0.339	21
	8641.62 C	39.76		-42.74	1.07
1903	LD L30 307	PASS	AISC- H1-3	0.130	22
	3961.51 C	-20.04		2.87	0.00
1904	LD L30 307	PASS	AISC- H1-3	0.193	22
	4305.71 C	19.63		29.19	0.00
1905	LD L30 307	PASS	AISC- H1-3	0.089	24
	2215.28 C	-13.57		7.75	0.00
1906	LD L30 307	PASS	AISC- H1-3	0.202	22
	4732.63 C	-45.40		11.87	0.00
1907	LD L30 307	PASS	AISC- H1-3	0.210	21
	4079.91 C	-35.47		29.68	0.00
1908	LD L30 305	PASS	AISC- H2-1	0.044	20
	126.96 T	-13.00		5.71	1.87
1909	LD L30 305	PASS	AISC- H2-1	0.049	24
	228.68 T	-14.15		5.98	1.87
1910	LD L30 305	PASS	AISC- H2-1	0.044	20
	125.15 T	-12.99		5.87	1.87
1911	LD L30 305	PASS	AISC- H1-3	0.072	24
	252.35 C	-15.90		11.41	0.00
1912	LD L30 305	PASS	AISC- H1-3	0.092	24
	243.22 C	-24.63		13.15	2.00
1913	LD L30 305	PASS	AISC- H1-3	0.072	23
	294.15 C	-13.60		12.39	0.00
1914	LD L30 308	PASS	AISC- H2-1	0.172	21
	2895.99 T	68.65		21.86	1.00
1915	LD L30 308	PASS	AISC- H2-1	0.201	22
	3290.32 T	105.69		11.31	1.00
1916	LD L30 305	PASS	AISC- H1-3	0.067	21
	323.32 C	-18.71		6.82	0.00
1917	LD L30 305	PASS	AISC- H1-3	0.065	22
	395.65 C	-14.33		8.18	1.87
1918	LD L30 305	PASS	AISC- H2-1	0.047	24
	162.60 T	-15.53		4.95	0.00
1919	LD L30 308	PASS	AISC- H1-3	0.135	22
	2835.90 C	32.33		20.46	1.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
1922	LD L30 308	PASS	AISC- H1-3	0.163	22
	504.21 C	101.02		20.65	1.00
1923	LD L30 305	PASS	AISC- H1-3	0.073	24
	476.89 C	21.00		5.29	1.87
1924	LD L30 305	PASS	AISC- H1-3	0.068	21
	354.25 C	-18.75		7.03	1.87
1925	LD L30 305	PASS	AISC- H2-1	0.045	20
	128.71 T	-12.79		6.20	0.00
1926	LD L30 305	PASS	AISC- H1-3	0.069	22
	215.26 C	-27.45		3.54	0.00
1927	LD L30 305	PASS	AISC- H1-3	0.072	22
	209.24 C	-29.35		3.53	1.87
1928	LD L30 305	PASS	AISC- H2-1	0.044	20
	125.15 T	-12.99		5.87	0.00
1929	LD L30 305	PASS	AISC- H2-1	0.033	22
	129.88 T	-11.60		-2.99	0.00
1930	LD L30 305	PASS	AISC- H2-1	0.097	21
	30.98 T	20.66		20.62	0.00
1931	LD L30 305	PASS	AISC- H2-1	0.060	24
	305.81 T	-8.04		12.92	1.32
1932	LD L30 305	PASS	AISC- H1-3	0.186	23
	121.91 C	13.25		-55.64	1.00
1933	LD L30 305	PASS	AISC- H2-1	0.061	24
	475.95 T	5.34		13.10	1.32
1934	LD L30 305	PASS	AISC- H2-1	0.066	21
	60.31 T	-9.47		-16.47	1.32
1935	LD L30 305	PASS	AISC- H1-3	0.210	21
	191.14 C	-1.76		-70.45	1.00
1936	LD L30 305	PASS	AISC- H2-1	0.068	22
	390.83 T	2.39		18.48	1.32
1937	LD L30 305	PASS	AISC- H2-1	0.031	22
	361.95 T	10.84		0.30	0.00
1938	LD L30 305	PASS	AISC- H2-1	0.075	24
	127.29 T	38.70		0.28	0.00
1939	LD L30 305	PASS	AISC- H2-1	0.051	24
	309.34 T	20.88		1.56	0.00
1940	LD L30 305	PASS	AISC- H1-3	0.090	22
	46.72 C	45.80		-1.66	0.00
1941	LD L30 305	PASS	AISC- H2-1	0.024	32
	144.21 T	-5.34		3.58	1.87
1942	LD L30 305	PASS	AISC- H2-1	0.024	22
	212.98 T	3.12		4.43	1.87

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ALL UNITS ARE - KG METR (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ KG	LOADING/ LOCATION
1945	LD L30 305	PASS	AISC- H1-3	0.055	24
		38.45 C	-21.60	4.83	0.00
1946	LD L30 305	PASS	AISC- H2-1	0.076	24
		20.58 T	38.97	-1.47	1.87
1947	LD L30 305	PASS	AISC- H1-3	0.065	22
		182.32 C	-26.86	2.93	1.87
1948	LD L30 305	PASS	AISC- H1-3	0.065	22
		182.34 C	-26.86	2.94	0.00
1949	LD L30 305	PASS	AISC- H1-3	0.090	22
		48.46 C	45.80	-1.65	1.87
1950	LD L30 305	PASS	AISC- H1-3	0.145	23
		129.00 C	-34.14	27.45	0.00
1951	LD L30 305	PASS	AISC- H1-3	0.152	23
		40.12 C	-30.53	33.40	0.00
1952	LD L30 305	PASS	AISC- H1-3	0.127	23
		4.32 C	-25.76	28.18	0.00
1953	LD L30 305	PASS	AISC- H2-1	0.035	22
		45.24 T	-3.70	9.33	1.00
1954	LD L30 305	PASS	AISC- H2-1	0.062	24
		23.70 T	-26.76	4.43	2.12
1955	LD L30 305	PASS	AISC- H1-3	0.055	24
		113.75 C	-19.17	5.33	0.00
1956	LD L30 305	PASS	AISC- H2-1	0.077	24
		281.00 T	29.54	5.46	2.12
1957	LD L30 305	PASS	AISC- H1-3	0.070	22
		319.95 C	-26.46	2.63	2.12
1958	LD L30 305	PASS	AISC- H2-1	0.035	24
		44.24 T	-9.32	-6.03	1.00
1959	LD L30 305	PASS	AISC- H1-3	0.127	21
		67.70 C	59.93	5.48	0.00
1960	LD L30 305	PASS	AISC- H1-3	0.073	22
		19.18 C	-26.56	8.30	0.00
1961	LD L30 305	PASS	AISC- H1-3	0.116	21
		40.62 C	62.63	0.01	1.00
1962	LD L30 305	PASS	AISC- H2-1	0.087	22
		151.84 T	-37.79	4.93	2.12
1963	LD L30 305	PASS	AISC- H2-1	0.087	22
		153.77 T	37.41	4.94	2.12
1964	LD L30 305	PASS	AISC- H1-3	0.128	21
		66.77 C	60.07	5.50	1.00
1965	LD L30 305	PASS	AISC- H1-3	0.074	22
		21.54 C	27.00	8.31	0.00

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ALL UNITS ARE - KG METR (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ KG	LOADING/ LOCATION
1966	LD L30 305	PASS	AISC- H1-3	0.060	24
		228.60 C	-23.16	2.89	2.12
1969	LD L30 305	PASS	AISC- H1-3	0.039	32
		37.71 C	-7.85	-8.21	0.00
1970	LD L30 305	PASS	AISC- H2-1	0.034	24
		215.22 T	8.91	4.18	2.12
1971	LD L30 305	PASS	AISC- H2-1	0.038	32
		163.90 T	-11.47	4.31	2.12
1972	LD L30 305	PASS	AISC- H1-3	0.047	22
		174.77 C	-4.79	-11.34	0.00
1973	LD L30 305	PASS	AISC- H1-3	0.070	22
		317.62 C	26.49	2.64	2.12
1974	LD L30 305	PASS	AISC- H1-3	0.098	22
		532.48 C	-14.19	18.88	0.00
1975	LD L30 305	PASS	AISC- H1-3	0.140	21
		324.73 C	5.92	-41.40	0.00
1976	LD L30 305	PASS	AISC- H1-3	0.110	23
		367.87 C	-6.45	-29.85	0.00
1977	LD L30 305	PASS	AISC- H1-3	0.089	24
		436.42 C	6.39	21.64	1.40
1978	LD L30 305	PASS	AISC- H1-3	0.141	21
		326.63 C	-6.15	-41.64	0.00
1979	LD L30 305	PASS	AISC- H1-3	0.110	23
		164.35 C	10.31	-29.99	1.40
1980	LD L30 307	PASS	AISC- H1-3	0.203	22
		4704.85 C	-16.14	31.27	0.00
1981	LD L30 307	PASS	AISC- H1-3	0.074	23
		2084.16 C	8.49	5.66	0.00
1982	LD L30 307	PASS	AISC- H1-3	0.122	22
		77.55 C	-33.03	-35.54	1.33
1983	LD L30 307	PASS	AISC- H2-1	0.121	21
		107.65 T	33.87	34.60	1.50
1984	LD L30 307	PASS	AISC- H2-1	0.118	21
		105.76 T	33.75	33.29	1.67
1985	LD L30 307	PASS	AISC- H2-1	0.119	21
		210.24 T	35.55	32.46	1.83
1986	LD L30 307	PASS	AISC- H1-3	0.088	24
		2397.47 C	5.96	-10.06	1.07
1987	LD L30 307	PASS	AISC- H1-3	0.193	22
		4305.74 C	19.63	29.19	1.07
1988	LD L30 307	PASS	AISC- H1-3	0.206	22
		4502.24 C	17.26	34.26	0.00

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
1991	LD L30 307	PASS	AISC- H1-3	0.072	23
	2062.41 C	9.52	3.81	0.00	
1992	LD L30 307	PASS	AISC- H1-3	0.213	22
	4187.00 C	-22.11	37.40	0.00	
1993	LD L30 307	PASS	AISC- H1-3	0.071	29
	1851.19 C	8.78	6.41	0.00	
1994	LD L30 307	PASS	AISC- H1-3	0.201	22
	3945.78 C	-12.59	40.63	0.00	
1995	LD L30 307	PASS	AISC- H1-3	0.073	29
	1883.98 C	9.64	6.28	0.00	
1996	LD L30 307	PASS	AISC- H1-3	0.084	23
	2306.24 C	4.37	-10.45	0.00	
1997	LD L30 307	PASS	AISC- H1-3	0.089	23
	2208.43 C	-10.58	9.22	1.07	
1998	LD L30 307	PASS	AISC- H1-3	0.089	23
	2399.49 C	-5.59	-10.51	1.09	
1999	LD L30 307	PASS	AISC- H1-3	0.203	22
	4704.76 C	-16.14	31.27	1.09	
2000	LD L30 307	PASS	AISC- H1-3	0.092	24
	2449.71 C	-7.86	-10.23	0.00	
2001	LD L30 307	PASS	AISC- H1-3	0.077	24
	2091.27 C	12.60	4.38	1.09	
2002	LD L30 307	PASS	AISC- H1-3	0.096	24
	2487.77 C	9.49	-10.63	1.12	
2003	LD L30 307	PASS	AISC- H1-3	0.206	22
	4502.49 C	17.25	34.26	1.12	
2004	LD L30 307	PASS	AISC- H1-3	0.091	23
	2429.89 C	6.33	-10.92	0.00	
2005	LD L30 307	PASS	AISC- H1-3	0.073	23
	2053.52 C	-9.59	4.69	1.12	
2006	LD L30 307	PASS	AISC- H1-3	0.099	24
	2530.54 C	-10.23	-10.76	0.00	
2007	LD L30 307	PASS	AISC- H1-3	0.077	24
	2071.13 C	15.26	2.45	1.14	
2008	LD L30 307	PASS	AISC- H1-3	0.093	23
	2479.37 C	-6.30	-10.98	1.14	
2009	LD L30 307	PASS	AISC- H1-3	0.209	22
	4352.80 C	-18.52	36.21	1.14	
2010	LD L30 307	PASS	AISC- H1-3	0.092	23
	2532.61 C	-4.54	-10.82	1.17	
2011	LD L30 307	PASS	AISC- H1-3	0.212	22
	4180.08 C	-22.07	37.41	1.17	

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ HZ	LOADING/ LOCATION
2014	LD L30 307	PASS	AISC- H1-3	0.093	22
	1547.40 C	25.22	-9.53	0.00	
2015	LD L30 307	PASS	AISC- H1-3	0.207	22
	4010.32 C	-14.78	41.03	1.20	
2016	LD L30 307	PASS	AISC- H1-3	0.096	24
	2803.08 C	-3.77	-9.84	0.00	
2017	LD L30 307	PASS	AISC- H1-3	0.074	29
	1886.19 C	9.78	6.44	1.20	
2018	LD L30 306	PASS	AISC- H1-3	0.092	21
	1483.21 C	-27.59	2.83	0.00	
2019	LD L30 306	PASS	AISC- H1-3	0.127	22
	2673.29 C	-9.82	15.11	0.00	
2020	LD L30 306	PASS	AISC- H2-1	0.107	22
	1237.61 T	24.82	15.43	0.00	
2021	LD L30 306	PASS	AISC- H1-3	0.087	22
	1509.29 C	-17.28	-6.72	1.17	
2022	LD L30 306	PASS	AISC- H1-3	0.092	21
	1483.20 C	27.59	2.83	0.00	
2023	LD L30 306	PASS	AISC- H1-3	0.123	22
	2673.28 C	9.82	15.11	0.00	
2024	LD L30 306	PASS	AISC- H2-1	0.107	22
	1237.61 T	-24.81	15.43	0.00	
2025	LD L30 306	PASS	AISC- H1-3	0.087	22
	1509.28 C	17.28	-6.72	1.17	
2026	LD L30 306	PASS	AISC- H2-1	0.097	22
	1821.74 T	-32.51	-0.10	0.00	
2027	LD L30 306	PASS	AISC- H1-3	0.132	22
	2557.93 C	10.99	17.91	0.00	
2028	LD L30 306	PASS	AISC- H2-1	0.128	22
	1959.19 T	-22.73	18.43	0.00	
2029	LD L30 306	PASS	AISC- H1-3	0.140	22
	2120.08 C	13.55	-23.61	1.23	
2030	LD L30 306	PASS	AISC- H2-1	0.097	22
	1821.76 T	32.52	-0.10	0.00	
2031	LD L30 306	PASS	AISC- H1-3	0.132	22
	2557.96 C	-10.98	17.91	0.00	
2032	LD L30 306	PASS	AISC- H2-1	0.128	22
	1959.20 T	22.74	18.43	0.00	
2033	LD L30 306	PASS	AISC- H1-3	0.140	22
	2120.09 C	-13.55	-23.61	1.23	
2034	LD L30 306	PASS	AISC- H2-1	0.119	22
	2020.60 T	-39.73	-2.55	0.00	

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
2037	LD L30 306	PASS	AISC- H1-3	0.132	22
		1990.24 C	10.18	-23.59	1.30
2038	LD L30 306	PASS	AISC- H2-1	0.119	22
		2020.66 T	39.74	-2.56	0.00
2039	LD L30 306	PASS	AISC- H1-3	0.136	22
		2515.40 C	-10.04	19.96	0.00
2040	LD L30 306	PASS	AISC- H2-1	0.129	22
		1998.72 T	23.24	17.74	0.00
2041	LD L30 306	PASS	AISC- H1-3	0.132	22
		1990.27 C	-10.19	-23.59	1.30
2042	LD L30 306	PASS	AISC- H2-1	0.130	22
		2185.39 T	42.45	-3.79	0.00
2043	LD L30 306	PASS	AISC- H1-3	0.139	22
		2456.47 C	-11.04	21.53	0.00
2044	LD L30 306	PASS	AISC- H2-1	0.126	22
		2076.96 T	21.59	17.06	0.00
2045	LD L30 306	PASS	AISC- H1-3	0.128	22
		1934.60 C	-7.59	-24.19	1.37
2046	LD L30 306	PASS	AISC- H2-1	0.130	22
		2185.16 T	-42.40	-3.78	0.00
2047	LD L30 306	PASS	AISC- H1-3	0.139	22
		2456.23 C	11.08	21.53	0.00
2048	LD L30 306	PASS	AISC- H2-1	0.126	22
		2076.84 T	-21.48	17.06	0.00
2049	LD L30 306	PASS	AISC- H1-3	0.128	22
		1934.49 C	7.55	-24.19	1.37
2050	LD L30 306	PASS	AISC- H2-1	0.147	22
		2337.35 T	-49.06	-4.75	0.00
2051	LD L30 306	PASS	AISC- H1-3	0.140	22
		2412.33 C	10.73	22.64	0.00
2052	LD L30 306	PASS	AISC- H2-1	0.127	22
		2074.18 T	-23.28	16.27	0.00
2053	LD L30 306	PASS	AISC- H1-3	0.125	22
		1830.66 C	6.93	-23.93	1.45
2054	LD L30 306	PASS	AISC- H2-1	0.147	22
		2336.30 T	49.29	-4.76	0.00
2055	LD L30 306	PASS	AISC- H1-3	0.140	22
		2411.23 C	-10.63	22.64	0.00
2056	LD L30 306	PASS	AISC- H2-1	0.127	22
		2073.00 T	23.59	16.27	0.00
2057	LD L30 306	PASS	AISC- H1-3	0.125	22
		1829.64 C	-7.06	-23.92	1.45

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
2060	LD L30 306	PASS	AISC- H2-1	0.125	22
		2091.20 T	-22.69	15.79	0.00
2061	LD L30 306	PASS	AISC- H1-3	0.120	22
		1767.38 C	5.60	-23.27	1.54
2062	LD L30 306	PASS	AISC- H2-1	0.159	22
		2399.91 T	53.95	-6.01	0.00
2063	LD L30 306	PASS	AISC- H1-3	0.139	22
		2298.69 C	-10.13	23.73	0.00
2064	LD L30 306	PASS	AISC- H2-1	0.127	22
		2107.88 T	23.62	15.70	0.00
2065	LD L30 306	PASS	AISC- H1-3	0.121	22
		1781.39 C	-5.95	-23.35	1.54
2066	LD L30 307	PASS	AISC- H1-1	0.361	21
		8842.53 C	-43.21	-48.57	1.09
2067	LD L30 307	PASS	AISC- H1-3	0.113	22
		3467.72 C	14.76	-4.12	0.00
2068	LD L30 307	PASS	AISC- H1-3	0.102	22
		1282.12 C	7.46	27.76	1.33
2069	LD L30 307	PASS	AISC- H1-3	0.108	22
		1375.34 C	7.49	28.81	1.50
2070	LD L30 307	PASS	AISC- H1-3	0.112	22
		1457.17 C	7.31	29.22	1.67
2071	LD L30 307	PASS	AISC- H1-3	0.122	22
		1685.74 C	7.36	29.68	1.83
2072	LD L30 307	PASS	AISC- H1-3	0.204	21
		4922.67 C	19.50	-27.24	1.07
2073	LD L30 307	PASS	AISC- H1-1	0.339	21
		8641.59 C	39.76	-42.74	0.00
2074	LD L30 307	PASS	AISC- H1-3	0.144	22
		3596.61 C	12.15	-18.90	0.00
2075	LD L30 307	PASS	AISC- H1-3	0.130	22
		3961.47 C	-20.05	2.87	1.07
2076	LD L30 307	PASS	AISC- H1-3	0.138	22
		3317.92 C	-13.12	-18.56	1.09
2077	LD L30 307	PASS	AISC- H1-3	0.202	22
		4564.52 C	41.99	15.60	1.09
2078	LD L30 307	PASS	AISC- H1-3	0.215	21
		5168.31 C	-23.28	-27.13	0.00
2079	LD L30 307	PASS	AISC- H1-3	0.160	21
		3582.05 C	25.63	17.92	1.09
2080	LD L30 307	PASS	AISC- H1-1	0.370	21
		8663.08 C	-44.50	-54.55	1.12

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ HY	RATIO/ HZ	LOADING/ LOCATION
2083	LD L30 307	PASS	AISC- H1-3	0.125	22
		3303.16 C		18.97	0.00
2084	LD L30 307	PASS	AISC- H1-1	0.375	21
		#237.75 C		-44.30	1.17
2085	LD L30 307	PASS	AISC- H1-3	0.130	22
		3299.72 C		19.98	0.00
2086	LD L30 307	PASS	AISC- H1-1	0.392	21
		4774.23 C		-40.22	1.20
2087	LD L30 307	PASS	AISC- H1-3	0.144	22
		3618.57 C		21.59	0.00
2088	LD L30 307	PASS	AISC- H1-3	0.157	22
		3857.51 C		-20.24	1.20
2089	LD L30 307	PASS	AISC- H1-3	-15.65	1.20
		5329.75 C		33.81	0.246
2090	LD L30 307	PASS	AISC- H1-3	0.142	22
		3331.05 C		-15.69	1.12
2091	LD L30 307	PASS	AISC- H1-3	0.209	22
		4596.79 C		42.12	1.12
2092	LD L30 307	PASS	AISC- H1-3	0.212	21
		4993.34 C		-23.93	0.00
2093	LD L30 307	PASS	AISC- H1-3	0.154	21
		3428.09 C		26.49	1.12
2094	LD L30 307	PASS	AISC- H1-3	0.145	22
		3352.95 C		-17.97	1.14
2095	LD L30 307	PASS	AISC- H1-3	0.212	22
		4585.52 C		40.99	1.14
2096	LD L30 307	PASS	AISC- H1-3	0.210	21
		4882.74 C		-24.41	0.00
2097	LD L30 307	PASS	AISC- H1-3	0.148	21
		3382.67 C		24.83	1.14
2098	LD L30 307	PASS	AISC- H1-3	0.147	22
		3411.98 C		-18.86	1.17
2099	LD L30 307	PASS	AISC- H1-3	0.213	22
		4612.44 C		39.10	1.17
2100	LD L30 307	PASS	AISC- H1-3	0.208	21
		4794.57 C		-24.87	0.00
2101	LD L30 307	PASS	AISC- H1-3	0.144	21
		3368.22 C		23.52	1.17
2102	LD L30 307	PASS	AISC- H1-3	0.208	21
		5157.91 C		-20.90	0.00
2103	LD L30 307	PASS	AISC- H1-3	0.130	21
		3680.31 C		23.95	1.20

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ HY	RATIO/ HZ	LOADING/ LOCATION
2106	LD L30 306	PASS	AISC- H1-3	0.107	22
		327.30 C		18.52	28.15
2107	LD L30 306	PASS	AISC- H1-3	0.037	24
		561.40 C		-3.96	6.05
2108	LD L30 306	PASS	AISC- H2-1	0.135	21
		2241.85 T		-17.37	0.135
2109	LD L30 306	PASS	AISC- H1-3	0.221	21
		3749.99 C		24.82	31.89
2110	LD L30 306	PASS	AISC- H2-1	0.149	21
		786.66 T		-16.74	42.68
2111	LD L30 306	PASS	AISC- H1-3	0.076	21
		1011.67 C		4.24	16.71
2112	LD L30 306	PASS	AISC- H2-1	0.137	21
		2517.98 T		-21.74	17.21
2113	LD L30 306	PASS	AISC- H1-3	0.226	21
		3660.81 C		24.54	35.10
2114	LD L30 306	PASS	AISC- H2-1	0.169	21
		1663.61 T		-17.02	42.12
2115	LD L30 306	PASS	AISC- H1-3	0.145	21
		1811.88 C		-15.05	-28.28
2117	LD L30 306	PASS	AISC- H2-1	0.097	22
		846.10 T		21.18	17.70
2118	LD L30 306	PASS	AISC- H1-3	0.099	22
		1295.46 C		-11.15	18.71
2119	LD L30 306	PASS	AISC- H1-3	0.096	22
		166.84 C		19.39	24.96
2120	LD L30 306	PASS	AISC- H1-3	0.060	24
		868.36 C		-7.21	9.50
2121	LD L30 306	PASS	AISC- H2-1	0.097	22
		959.30 T		23.96	14.46
2122	LD L30 306	PASS	AISC- H1-3	0.103	22
		1277.29 C		-11.98	19.73
2123	LD L30 306	PASS	AISC- H1-3	0.086	22
		26.48 C		19.43	22.32
2124	LD L30 306	PASS	AISC- H1-3	0.064	24
		872.30 C		-8.49	10.20
2125	LD L30 306	PASS	AISC- H2-1	0.137	21
		2822.80 T		-25.53	11.61
2126	LD L30 306	PASS	AISC- H1-3	0.232	21
		3611.35 C		25.01	37.65
2127	LD L30 306	PASS	AISC- H2-1	0.160	21
		1822.59 T		-14.44	38.23

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MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
2130	LD L30 306	PASS	AISC- H1-3	0.233	21
		3507.16 C	24.53	39.56	0.00
2131	LD L30 306	PASS	AISC- H2-1	0.153	21
		1911.85 T	-13.40	35.21	0.00
2132	LD L30 306	PASS	AISC- H1-3	0.154	21
		1772.26 C	-15.82	-31.38	1.37
2133	LD L30 306	PASS	AISC- H2-1	0.097	24
		1254.10 T	33.55	5.19	0.00
2134	LD L30 306	PASS	AISC- H1-3	0.102	22
		1226.06 C	-11.59	20.43	0.00
2135	LD L30 306	PASS	AISC- H2-1	0.086	24
		918.74 T	16.93	15.14	0.00
2136	LD L30 306	PASS	AISC- H1-3	0.067	24
		886.56 C	-9.09	10.90	0.00
2137	LD L30 306	PASS	AISC- H2-1	0.106	24
		1376.08 T	39.20	4.17	0.00
2138	LD L30 306	PASS	AISC- H1-3	0.102	22
		1183.44 C	-11.09	20.95	0.00
2139	LD L30 306	PASS	AISC- H2-1	0.090	24
		999.30 T	19.27	14.62	0.00
2140	LD L30 306	PASS	AISC- H1-3	0.070	24
		907.98 C	-9.37	11.48	0.00
2141	LD L30 306	PASS	AISC- H2-1	0.137	21
		3167.73 T	-28.93	5.57	0.00
2142	LD L30 306	PASS	AISC- H1-3	0.231	21
		3343.16 C	23.56	41.06	0.00
2143	LD L30 306	PASS	AISC- H2-1	0.146	21
		1960.25 T	-12.38	32.66	0.00
2144	LD L30 306	PASS	AISC- H1-3	0.153	21
		1713.41 C	-15.26	-31.73	1.45
2145	LD L30 306	PASS	AISC- H2-1	0.145	21
		3569.05 T	-33.77	1.69	0.00
2146	LD L30 306	PASS	AISC- H1-3	0.239	21
		3491.62 C	23.29	42.51	0.00
2147	LD L30 306	PASS	AISC- H2-1	0.141	21
		2170.27 T	-10.82	29.40	0.00
2148	LD L30 306	PASS	AISC- H1-3	0.157	21
		1810.55 C	-14.71	-32.14	1.54
2149	LD L30 306	PASS	AISC- H2-1	0.114	24
		1539.04 T	44.24	2.33	0.00
2150	LD L30 306	PASS	AISC- H1-3	0.112	22
		1427.86 C	-11.54	21.86	0.00

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ALL UNITS ARE - KG METZ (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
2153	LD L30 307	PASS	AISC- H1-3	0.094	21
		1582.98 C	-25.07	-8.81	1.33
2154	LD L30 307	PASS	AISC- H1-3	0.093	21
		1695.72 C	-25.84	-5.79	1.50
2155	LD L30 307	PASS	AISC- H1-3	0.095	21
		1818.20 C	7.09	16.07	0.00
2156	LD L30 307	PASS	AISC- H1-3	0.104	21
		2098.59 C	7.58	15.14	0.00
2157	LD L30 307	PASS	AISC- H1-1	0.390	21
		8759.84 C	-34.18	67.61	0.00
2158	LD L30 307	PASS	AISC- H1-3	0.141	22
		3602.90 C	-20.59	-10.99	0.00
2159	LD L30 307	PASS	AISC- H1-3	0.214	22
		4622.40 C	39.18	21.71	0.00
2160	LD L30 307	PASS	AISC- H1-3	0.143	21
		3370.31 C	23.10	13.41	0.00
2161	LD L30 307	PASS	AISC- H1-3	0.212	22
		4585.91 C	41.05	20.38	0.00
2162	LD L30 307	PASS	AISC- H1-3	0.148	21
		3383.68 C	24.71	14.49	0.00
2163	LD L30 307	PASS	AISC- H1-3	0.209	22
		4597.03 C	42.14	18.46	0.00
2164	LD L30 307	PASS	AISC- H1-3	0.154	21
		3428.46 C	26.45	15.94	0.00
2165	LD L30 307	PASS	AISC- H1-3	0.202	22
		4564.61 C	41.99	15.60	0.00
2166	LD L30 307	PASS	AISC- H1-3	0.160	21
		3582.18 C	25.61	17.92	0.00
2167	LD L30 307	PASS	AISC- H1-3	0.144	22
		3596.66 C	12.15	-18.90	1.07
2168	LD L30 307	PASS	AISC- H1-3	0.202	22
		4732.66 C	-45.40	11.87	1.07
2169	LD L30 307	PASS	AISC- H1-3	0.204	21
		4922.71 C	19.50	-27.24	0.00
2170	LD L30 307	PASS	AISC- H1-3	0.210	21
		4079.96 C	-35.47	29.68	1.07
2171	LD L30 307	PASS	AISC- H1-3	0.215	21
		5188.43 C	-23.27	-27.13	1.04
2172	LD L30 307	PASS	AISC- H1-1	0.361	21
		8842.62 C	-43.20	-48.57	0.00
2173	LD L30 307	PASS	AISC- H1-3	0.138	22
		3318.04 C	-13.11	-18.56	0.00

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TORRE AUTOSORTADA

ALL UNITS ARE - RD METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RD	METE	RESULT/ PK	CRITICAL COND/ PK	RATIO/ %	LOADING/ LOCATION
2176	LD	L30 307	PASS	AISC-H1-1	0.170	21	
				-44.49	-54.55	0.00	
2177	LD	L30 307	PASS	AISC-H1-3	0.142	22	
				-15.67	-18.33	0.00	
2178	LD	L30 307	PASS	AISC-H1-3	0.121	22	
				18.57	-6.47	1.12	
2179	LD	L30 307	PASS	AISC-H1-3	0.210	21	
				-24.32	-26.84	1.14	
2180	LD	L30 307	PASS	AISC-H1-1	0.375	21	
				-44.63	-59.50	0.00	
2181	LD	L30 307	PASS	AISC-H1-3	0.145	22	
				-17.88	-18.05	0.00	
2182	LD	L30 307	PASS	AISC-H1-3	0.125	22	
				18.85	-8.29	1.14	
2183	LD	L30 307	PASS	AISC-H1-3	0.208	21	
				-24.52	-26.41	1.17	
2184	LD	L30 307	PASS	AISC-H1-1	0.374	21	
				-44.08	-61.18	0.00	
2185	LD	L30 307	PASS	AISC-H1-3	0.147	22	
				-18.60	-17.33	0.00	
2186	LD	L30 307	PASS	AISC-H1-3	0.129	22	
				19.60	-9.85	1.17	
2187	LD	L30 307	PASS	AISC-H1-3	0.206	21	
				-19.80	-24.15	0.00	
2188	LD	L30 307	PASS	AISC-H1-3	0.147	21	
				-22.71	11.75	1.20	
2189	LD	L30 307	PASS	AISC-H1-3	0.155	22	
				-15.70	-17.24	1.20	
2190	LD	L30 307	PASS	AISC-H1-3	0.221	22	
				31.04	31.04	1.20	
2191	LD	L30 306	PASS	AISC-H2-1	0.144	21	
				1.68	1.68	0.00	
2192	LD	L30 306	PASS	AISC-H1-3	0.239	21	
				-23.62	42.46	0.00	
2193	LD	L30 306	PASS	AISC-H2-1	0.139	21	
				9.93	29.48	0.00	
2194	LD	L30 306	PASS	AISC-H1-3	0.158	21	
				15.23	-32.03	1.54	
2195	LD	L30 306	PASS	AISC-H2-1	0.097	22	
				-27.94	7.75	0.00	
2196	LD	L30 306	PASS	AISC-H1-3	0.111	22	
				11.82	21.79	0.00	

TORRE AUTOSORTADA

ALL UNITS ARE - RD METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RD	METE	RESULT/ PK	CRITICAL COND/ PK	RATIO/ %	LOADING/ LOCATION
2198	LD	L30 306	PASS	AISC-H2-1	0.098	22	
				-28.89	10.55	0.00	
2200	LD	L30 306	PASS	AISC-H1-3	0.102	22	
				11.23	20.35	0.00	
2201	LD	L30 306	PASS	AISC-H2-1	0.081	22	
				-21.05	18.56	0.00	
2202	LD	L30 306	PASS	AISC-H1-3	0.063	23	
				-3.67	-12.93	1.45	
2203	LD	L30 306	PASS	AISC-H2-1	0.136	21	
				28.82	5.61	0.00	
2204	LD	L30 306	PASS	AISC-H1-3	0.231	21	
				-23.60	41.05	0.00	
2205	LD	L30 306	PASS	AISC-H2-1	0.146	21	
				12.09	32.69	0.00	
2206	LD	L30 306	PASS	AISC-H1-3	0.153	21	
				15.38	-31.72	1.45	
2207	LD	L30 306	PASS	AISC-H2-1	0.096	22	
				-26.35	12.12	0.00	
2208	LD	L30 306	PASS	AISC-H1-3	0.102	22	
				11.57	20.43	0.00	
2209	LD	L30 306	PASS	AISC-H2-1	0.083	22	
				27.37	20.35	0.00	
2210	LD	L30 306	PASS	AISC-H1-3	0.061	23	
				-3.81	-12.40	1.37	
2211	LD	L30 306	PASS	AISC-H2-1	0.148	21	
				39.56	0.274	0.00	
2212	LD	L30 306	PASS	AISC-H1-3	0.274	21	
				13.30	0.153	0.00	
2213	LD	L30 306	PASS	AISC-H2-1	0.154	21	
				15.86	-31.38	1.37	
2214	LD	L30 306	PASS	AISC-H1-3	0.137	21	
				25.52	11.61	0.00	
2216	LD	L30 306	PASS	AISC-H1-3	0.232	21	
				-25.02	37.64	0.00	
2217	LD	L30 306	PASS	AISC-H2-1	0.159	21	
				14.43	38.23	0.00	
2218	LD	L30 306	PASS	AISC-H1-3	0.151	21	
				15.70	-30.03	1.30	
2219	LD	L30 306	PASS	AISC-H2-1	0.097	22	
				959.22	14.46	0.00	

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ M2	LOADING/ LOCATION
2222	LD L30 306	PASS	AISC- H1-3	0.057	23
		823.95 C	-3.09	-11.45	1.30
2223	LD L30 306	PASS	AISC- H2-1	0.097	22
		846.08 T	-21.18	17.70	0.00
2224	LD L30 306	PASS	AISC- H1-3	0.099	22
		1285.44 C	11.16	18.71	0.00
2225	LD L30 306	PASS	AISC- H1-3	0.096	22
		166.85 C	-19.38	24.96	0.00
2226	LD L30 306	PASS	AISC- H1-3	0.052	23
		822.34 C	-1.77	-10.64	1.23
2227	LD L30 306	PASS	AISC- H2-1	0.137	21
		2517.95 T	21.74	17.22	0.00
2228	LD L30 306	PASS	AISC- H1-3	0.226	21
		3660.78 C	-24.54	35.10	0.00
2229	LD L30 306	PASS	AISC- H2-1	0.169	21
		1663.60 T	17.01	42.12	0.00
2230	LD L30 306	PASS	AISC- H1-3	0.145	21
		1611.87 C	15.05	-28.28	1.23
2231	LD L30 306	PASS	AISC- H2-1	0.103	22
		679.89 T	-16.02	25.21	0.00
2232	LD L30 306	PASS	AISC- H1-3	0.097	22
		1231.72 C	12.00	18.16	0.00
2233	LD L30 306	PASS	AISC- H1-3	0.107	22
		327.31 C	-18.52	28.15	0.00
2234	LD L30 306	PASS	AISC- H2-1	0.035	22
		371.93 T	2.58	8.92	0.00
2235	LD L30 306	PASS	AISC- H2-1	0.135	21
		2241.84 T	17.37	21.82	0.00
2236	LD L30 306	PASS	AISC- H1-3	0.221	21
		3749.98 C	-24.83	31.89	0.00
2237	LD L30 306	PASS	AISC- H2-1	0.149	21
		786.66 T	16.74	42.68	0.00
2238	LD L30 306	PASS	AISC- H1-3	0.076	21
		1011.67 C	-4.24	16.71	0.00
2239	LD L30 307	PASS	AISC- H1-3	0.046	21
		138.03 C	-10.39	13.17	1.83
2240	LD L30 307	PASS	AISC- H1-3	0.048	21
		138.17 C	-9.90	14.55	1.67
2241	LD L30 307	PASS	AISC- H1-3	0.031	22
		168.46 C	4.78	9.33	1.67
2242	LD L30 307	PASS	AISC- H1-3	0.048	21
		138.09 C	-9.64	14.54	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ M2	LOADING/ LOCATION
2245	LD L30 307	PASS	AISC- H1-3	0.052	21
		125.97 C	-9.72	16.89	1.50
2246	LD L30 307	PASS	AISC- H1-3	0.056	21
		123.45 C	-9.44	19.15	1.33
2247	LD L30 307	PASS	AISC- H1-3	0.030	22
		158.34 C	-4.66	9.18	0.00
2248	LD L30 307	PASS	AISC- H1-3	0.029	22
		154.56 C	-4.52	9.10	0.00
2249	LD L30 307	PASS	AISC- H1-3	0.056	21
		123.45 C	-9.41	19.15	0.00
2250	LD L30 307	PASS	AISC- H1-3	0.057	21
		125.98 C	-9.64	16.89	0.00
2251	LD L30 307	PASS	AISC- H1-3	0.072	21
		99.97 C	8.57	27.69	0.00
2252	LD L30 307	PASS	AISC- H1-3	0.062	21
		113.74 C	9.09	22.45	0.00
2253	LD L30 307	PASS	AISC- H1-3	0.032	24
		70.17 C	-4.89	11.22	1.00
2254	LD L30 307	PASS	AISC- H1-3	0.030	23
		79.90 C	-5.14	9.84	1.17
2255	LD L30 307	PASS	AISC- H1-3	0.072	21
		99.97 C	8.57	27.69	1.00
2256	LD L30 307	PASS	AISC- H1-3	0.062	21
		113.74 C	9.08	22.45	1.17
2275	ST PIP S80	PASS	AISC- H1-1	0.644	21
		48418.39 C	0.00	204.97	0.00
2288	ST PIP S80	PASS	AISC- H1-2	0.393	22
		24918.20 C	0.00	415.78	0.00
2301	ST PIP S80	PASS	AISC- H1-2	0.393	22
		24918.20 C	0.00	415.78	0.00
2322	ST PIP S80	PASS	AISC- H1-1	0.585	21
		43937.19 C	0.00	186.63	1.00
2324	ST PIP S80	PASS	AISC- H1-2	0.342	22
		22678.74 C	0.00	305.73	0.00
2326	ST PIP S80	PASS	AISC- H1-2	0.342	22
		22678.74 C	0.00	305.73	0.00
2328	LD L30 305	PASS	AISC- H1-3	0.091	22
		721.60 C	6.14	19.17	0.00
2329	LD L30 305	PASS	AISC- H2-1	0.075	22
		441.79 T	-2.16	20.63	0.00
2330	LD L30 305	PASS	AISC- H1-3	0.067	23
		1303.94 C	-0.37	7.73	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
2333	LD L30 305	PASS	AISC- H1-3	0.101	22
		652.64 C		3.49	0.00
				25.48	23
2334	LD L30 305	PASS	AISC- H1-3	0.076	23
		1299.78 C		-3.49	8.76
				0.061	24
2335	LD L30 305	PASS	AISC- H2-1	0.061	24
		1347.12 T		-0.77	7.53
				0.060	21
2336	LD L30 305	PASS	AISC- H2-1	0.060	21
		1297.56 T		1.17	7.31
				0.065	24
2337	LD L30 305	PASS	AISC- H1-3	0.065	24
		1252.98 C		-0.05	7.92
				0.054	23
2338	LD L30 305	PASS	AISC- H2-1	0.054	23
		1189.33 T		0.74	6.54
				0.055	24
2339	LD L30 305	PASS	AISC- H2-1	0.055	24
		1239.17 T		-0.58	6.41
				0.121	22
2340	LD L30 305	PASS	AISC- H1-3	0.121	22
		2462.78 C		6.79	-8.34
				0.166	21
2341	LD L30 305	PASS	AISC- H1-3	0.166	21
		2495.44 C		-4.45	25.55
				0.111	22
2342	LD L30 305	PASS	AISC- H1-3	0.111	22
		2417.98 C		-0.32	9.79
				0.178	21
2343	LD L30 305	PASS	AISC- R1-3	0.178	21
		3172.72 C		0.11	24.42
				0.152	21
2344	LD L30 305	PASS	AISC- H1-3	0.152	21
		3096.42 C		6.02	-12.36
				0.161	22
2345	LD L30 305	PASS	AISC- H1-3	0.161	22
		2432.86 C		6.79	-22.84
				0.132	22
2346	LD L30 305	PASS	AISC- H1-3	0.132	22
		2883.06 C		6.63	7.42
				0.147	22
2347	LD L30 305	PASS	AISC- H1-3	0.147	22
		2822.58 C		-1.73	-16.67
				0.112	21
2348	LD L30 305	PASS	AISC- H1-3	0.112	21
		2452.75 C		2.89	-8.15
				0.159	0.56
2349	LD L30 305	PASS	AISC- H1-3	0.159	0.56
		2909.32 C		0.69	20.04
				0.120	21
2350	LD L30 305	PASS	AISC- H1-3	0.120	21
		2867.53 C		1.43	-6.93
				0.121	22
2351	LD L30 305	PASS	AISC- H1-3	0.121	22
		2362.43 C		-1.69	-13.24
				0.178	21
2352	LD L30 305	PASS	AISC- H1-3	0.178	21
		3172.71 C		-0.11	24.42
				0.132	22
2353	LD L30 305	PASS	AISC- H1-3	0.132	22
		2883.06 C		-6.63	7.42
				0.00	0.00

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ALL UNITS ARE - KG METE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
2356	LD L30 305	PASS	AISC- H1-3	0.161	22
		2432.85 C		-6.79	-22.84
				0.152	21
2357	LD L30 305	PASS	AISC- H1-3	0.152	21
		3096.42 C		-6.02	-12.36
				0.166	21
2358	LD L30 305	PASS	AISC- H1-3	0.166	21
		2495.44 C		4.45	25.55
				0.112	21
2359	LD L30 305	PASS	AISC- H1-3	0.112	21
		2452.75 C		-2.89	-8.15
				0.147	22
2360	LD L30 305	PASS	AISC- H1-3	0.147	22
		2822.58 C		1.73	-16.67
				0.111	22
2361	LD L30 305	PASS	AISC- H1-3	0.111	22
		2417.97 C		0.32	9.79
				0.121	22
2362	LD L30 305	PASS	AISC- H1-3	0.121	22
		2362.43 C		1.69	-13.24
				0.120	21
2363	LD L30 305	PASS	AISC- H1-3	0.120	21
		2867.53 C		-1.43	-6.93
				0.226	22
2364	ST PIP S80	PASS	AISC- H1-2	0.226	22
		15287.45 C		0.00	193.61
				0.00	193.61
2366	ST PIP S80	PASS	AISC- H1-2	0.00	193.61
		15287.45 C		0.00	193.61
				0.132	22
2368	ST PIP S80	PASS	AISC- H1-3	0.132	22
		8697.10 C		0.00	108.73
				0.132	22
2370	ST PIP S80	PASS	AISC- H1-3	0.132	22
		8697.10 C		0.00	108.73
				0.045	22
2372	ST PIP S80	PASS	AISC- H1-3	0.045	22
		2830.22 C		0.00	42.28
				0.00	42.28
2373	ST PIP S80	PASS	AISC- H1-3	0.00	42.28
		1037.37 C		0.00	49.03
				0.045	22
2374	ST PIP S80	PASS	AISC- H1-3	0.045	22
		2830.22 C		0.00	42.28
				0.024	22
2375	ST PIP S80	PASS	AISC- H1-3	0.024	22
		1037.37 C		0.00	49.03
				0.063	23
2376	LD L30 305	PASS	AISC- H1-3	0.063	23
		1265.79 C		0.65	6.60
				0.059	24
2377	LD L30 305	PASS	AISC- H1-3	0.059	24
		1098.88 C		1.03	7.04
				0.057	23
2378	LD L30 305	PASS	AISC- H1-3	0.057	23
		1111.12 C		-0.37	6.35
				0.050	24
2379	LD L30 305	PASS	AISC- H1-3	0.050	24
		1033.25 C		-0.36	5.01
				0.052	23
2380	LD L30 305	PASS	AISC- H1-3	0.052	23
		992.04 C		-1.41	5.34
				0.00	0.00

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ALL UNITS ARE - EG MITE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
2383	LD L30 305	PASS	AISC- H1-3	0.063	24
		741.36 C	13.48	4.07	0.00
2384	LD L30 305	PASS	AISC- H1-3	0.092	23
		687.34 C	-29.70	3.84	0.00
2385	LD L30 305	PASS	AISC- H2-1	0.175	23
		628.59 T	-78.50	1.57	0.00
2386	LD L30 305	PASS	AISC- H1-3	0.090	24
		630.58 C	-29.75	3.95	1.12
2387	LD L30 305	PASS	AISC- H2-1	0.176	24
		685.12 T	-78.58	1.50	1.12
2388	LD L30 305	PASS	AISC- H2-1	0.107	24
		824.94 T	39.28	2.51	0.00
2389	LD L30 305	PASS	AISC- H1-3	0.065	23
		806.61 C	13.46	3.89	1.12
2390	LD L30 305	PASS	AISC- H2-1	0.105	23
		759.82 T	39.23	2.65	1.12
2391	LD L30 305	PASS	AISC- H2-1	0.067	23
		805.03 T	-18.26	2.95	0.00
2392	LD L30 305	PASS	AISC- H1-3	0.046	24
		841.03 C	-3.51	3.77	1.12
2393	LD L30 305	PASS	AISC- H2-1	0.068	24
		850.80 T	-18.30	2.82	1.12
2394	LD L30 305	PASS	AISC- H2-1	0.046	24
		918.31 T	6.43	2.38	0.00
2395	LD L30 305	PASS	AISC- H1-3	0.050	23
		923.98 C	2.20	4.93	1.12
2396	LD L30 305	PASS	AISC- H2-1	0.044	23
		872.96 T	6.37	2.29	1.12
2397	LD L30 305	PASS	AISC- H1-3	0.041	24
		884.83 C	2.75	-2.10	0.28
2398	LD L30 305	PASS	AISC- H1-3	0.050	24
		925.79 C	-1.43	5.51	1.12
2399	LD L30 305	PASS	AISC- H1-3	0.044	23
		951.66 C	2.46	-2.23	0.75
2400	LD L30 305	PASS	AISC- H1-3	0.044	23
		1036.69 C	-1.20	-2.32	0.37
2401	LD L30 305	PASS	AISC- H1-3	0.052	23
		1078.62 C	-0.40	5.11	1.12
2402	LD L30 305	PASS	AISC- H1-3	0.043	24
		991.04 C	-0.96	-2.48	0.65
2403	LD L30 305	PASS	AISC- H1-3	0.044	24
		1025.55 C	0.44	-2.64	0.56

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ALL UNITS ARE - M2 MITE (UNLESS OTHERWISE NOTED)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ ME	LOADING/ LOCATION
2406	LD L30 305	PASS	AISC- H2-1	0.049	24
		1177.79 T	0.20	5.40	1.12
2407	LD L30 305	PASS	AISC- H1-3	0.061	23
		1163.75 C	1.03	6.88	1.12
2408	LD L30 305	PASS	AISC- H2-1	0.047	23
		1112.88 T	0.21	5.28	0.00
2409	LD L30 305	PASS	AISC- H1-3	0.053	22
		388.19 C	0.67	13.47	1.12
2410	LD L30 305	PASS	AISC- H1-3	0.061	24
		1221.26 C	0.75	6.46	1.12
2411	LD L30 305	PASS	AISC- H2-1	0.053	24
		1202.60 T	0.90	5.95	0.00
2412	ST PIP S80	PASS	AISC- H1-1	0.394	21
		29517.21 C	0.00	133.29	0.00
2414	ST PIP S80	PASS	AISC- H1-1	0.223	21
		16663.63 C	0.00	78.30	0.00
2416	ST PIP S80	PASS	AISC- H1-3	0.076	21
		5343.48 C	0.00	41.32	0.00
2417	ST PIP S80	PASS	AISC- H1-3	0.029	21
		1698.05 C	0.00	36.73	1.00
2418	LD L30 305	PASS	AISC- H1-3	0.114	22
		2419.15 C	2.01	9.86	0.00
2419	LD L30 305	PASS	AISC- H1-3	0.139	21
		2450.66 C	1.50	18.60	0.00
2420	LD L30 305	PASS	AISC- H1-3	0.101	22
		2235.29 C	-0.47	8.43	0.00
2421	LD L30 305	PASS	AISC- H1-3	0.173	21
		2490.70 C	-1.76	12.10	0.00
2422	LD L30 305	PASS	AISC- H1-3	0.098	22
		2181.07 C	-1.33	7.51	0.00
2423	LD L30 305	PASS	AISC- H1-3	0.107	21
		2189.05 C	-0.08	11.42	0.00
2424	LD L30 305	PASS	AISC- H1-3	0.088	22
		1945.82 C	0.94	7.08	0.00
2425	LD L30 305	PASS	AISC- H1-3	0.091	21
		1916.26 C	0.38	8.60	0.00
2426	LD L30 305	PASS	AISC- H1-3	0.075	22
		1648.79 C	-1.00	5.84	0.00
2427	LD L30 305	PASS	AISC- H1-3	0.148	21
		2846.81 C	-2.39	16.29	0.00
2428	LD L30 305	PASS	AISC- H1-3	0.118	21
		2783.95 C	-1.37	-7.05	1.02

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TORRE AUTOSORTADA

ALL UNITS ARE - NO METS (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/ PK	CRITICAL COND/ MT	RATIO/ HZ	LOADING/ LOCATION
2431	LD	L30 305	PASS	AISC- HI-3	0.115
		2433.99 C	-0.02		-11.21
2432	LD	L30 305	PASS	AISC- HI-3	0.105
		2402.45 C	-1.78		-6.93
2433	LD	L30 305	PASS	AISC- HI-3	0.131
		2515.39 C	0.04		16.02
2435	LD	L30 305	PASS	AISC- HI-3	0.100
		2186.31 C	1.30		-8.95
2437	LD	L30 305	PASS	AISC- HI-3	0.098
		2153.05 C	-1.29		-0.88
2438	LD	L30 305	PASS	AISC- HI-3	0.100
		2437.41 C	-0.65		-5.81
2439	LD	L30 305	PASS	AISC- HI-3	0.115
		2211.09 C	0.76		13.41
2440	LD	L30 305	PASS	AISC- HI-3	0.092
		2167.52 C	-0.99		-5.54
2441	LD	L30 305	PASS	AISC- HI-3	0.090
		2138.15 C	0.16		-5.72
2442	LD	L30 305	PASS	AISC- HI-3	0.091
		2022.92 C	-0.09		7.65
2444	LD	L30 305	PASS	AISC- HI-3	0.087
		2149.59 C	-0.19		-4.68
2446	LD	L30 305	PASS	AISC- HI-3	0.084
		2094.50 C	-0.20		-4.38
2447	LD	L30 305	PASS	AISC- HI-3	0.077
		1906.24 C	-0.27		-4.00
2448	LD	L30 305	PASS	AISC- HI-3	0.087
		1951.70 C	-0.46		6.91
2449	LD	L30 305	PASS	AISC- HI-3	0.076
		1914.03 C	0.54		-3.39
2450	LD	L30 305	PASS	AISC- HI-3	0.082
		1871.00 C	-2.19		-4.89
2451	LD	L30 305	PASS	AISC- HI-3	0.094
		1989.26 C	0.79		8.66
2452	LD	L30 305	PASS	AISC- HI-3	0.080
		1959.03 C	-1.00		-3.95
2453	LD	L30 305	PASS	AISC- HI-3	0.098
		343.47 C	42.92		1.21
2454	LD	L30 305	PASS	AISC- HI-3	0.114
		2419.15 C	-2.01		9.86
2455	LD	L30 305	PASS	AISC- HI-3	0.139
		2450.46 C	-1.50		18.60

TORRE AUTOSORTADA

ALL UNITS ARE - NO METS (UNLESS OTHERWISE NOTED)

NUMBER	TABLE	RESULT/ PK	CRITICAL COND/ MT	RATIO/ HZ	LOADING/ LOCATION
2459	LD	L30 305	PASS	AISC- HI-3	0.107
		2189.05 C	0.08		11.42
2460	LD	L30 305	PASS	AISC- HI-3	0.088
		1945.82 C	-0.94		7.08
2461	LD	L30 305	PASS	AISC- HI-3	0.091
		1916.26 C	-0.38		8.60
2462	LD	L30 305	PASS	AISC- HI-3	0.075
		1648.79 C	1.00		3.11
2464	LD	L30 305	PASS	AISC- HI-3	0.118
		2183.85 C	-0.05		-0.05
2465	LD	L30 305	PASS	AISC- HI-3	0.125
		2379.18 C	2.24		-13.74
2466	LD	L30 305	PASS	AISC- HI-3	0.112
		2481.35 C	2.50		8.06
2467	LD	L30 305	PASS	AISC- HI-3	0.115
		2433.98 C	0.02		-11.21
2468	LD	L30 305	PASS	AISC- HI-3	0.105
		2402.45 C	1.78		-6.93
2469	LD	L30 305	PASS	AISC- HI-3	0.131
		2515.39 C	-0.04		16.02
2473	LD	L30 305	PASS	AISC- HI-3	0.098
		2153.05 C	1.29		-7.68
2474	LD	L30 305	PASS	AISC- HI-3	0.101
		2437.41 C	0.66		-5.81
2475	LD	L30 305	PASS	AISC- HI-3	0.115
		2211.09 C	-0.76		13.41
2476	LD	L30 305	PASS	AISC- HI-3	0.092
		2167.52 C	0.99		-5.54
2477	LD	L30 305	PASS	AISC- HI-3	0.090
		2138.15 C	-0.16		-5.72
2478	LD	L30 305	PASS	AISC- HI-3	0.091
		2022.92 C	0.09		7.65
2479	LD	L30 305	PASS	AISC- HI-3	0.081
		1979.96 C	-0.36		-4.36
2480	LD	L30 305	PASS	AISC- HI-3	0.087
		2149.59 C	0.19		-4.68
2481	LD	L30 305	PASS	AISC- HI-3	0.099
		2137.99 C	0.98		8.47
2482	LD	L30 305	PASS	AISC- HI-3	0.084
		2094.50 C	0.20		-4.38
2483	LD	L30 305	PASS	AISC- HI-3	0.077
		1906.24 C	0.21		-4.00

APÉNDICE

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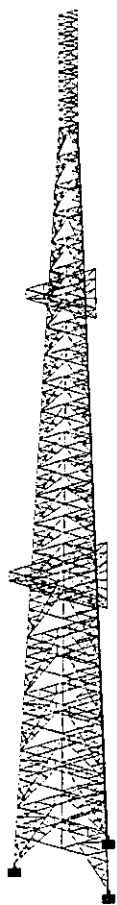
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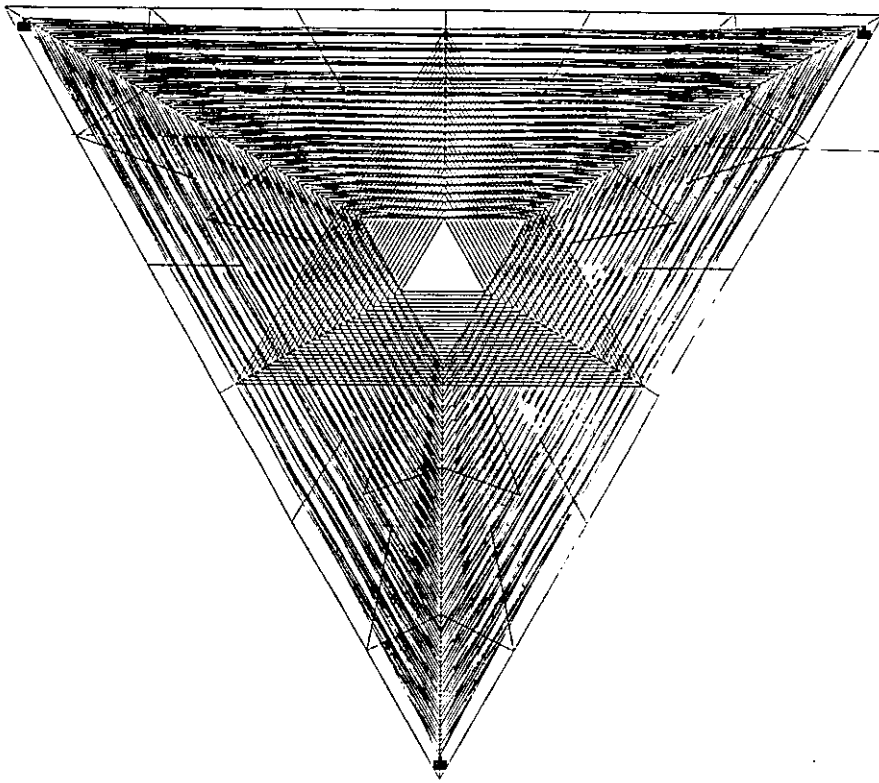
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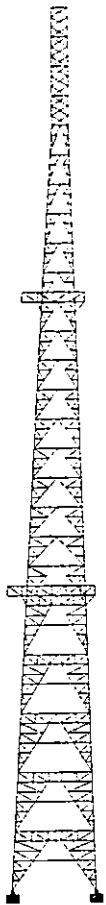


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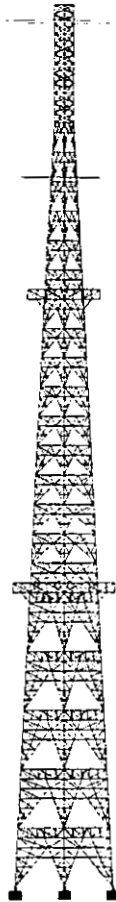
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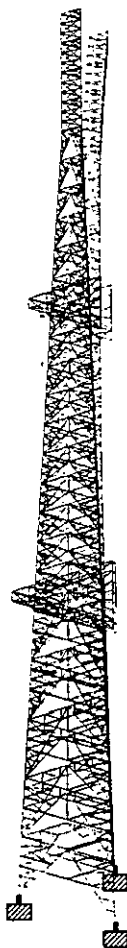


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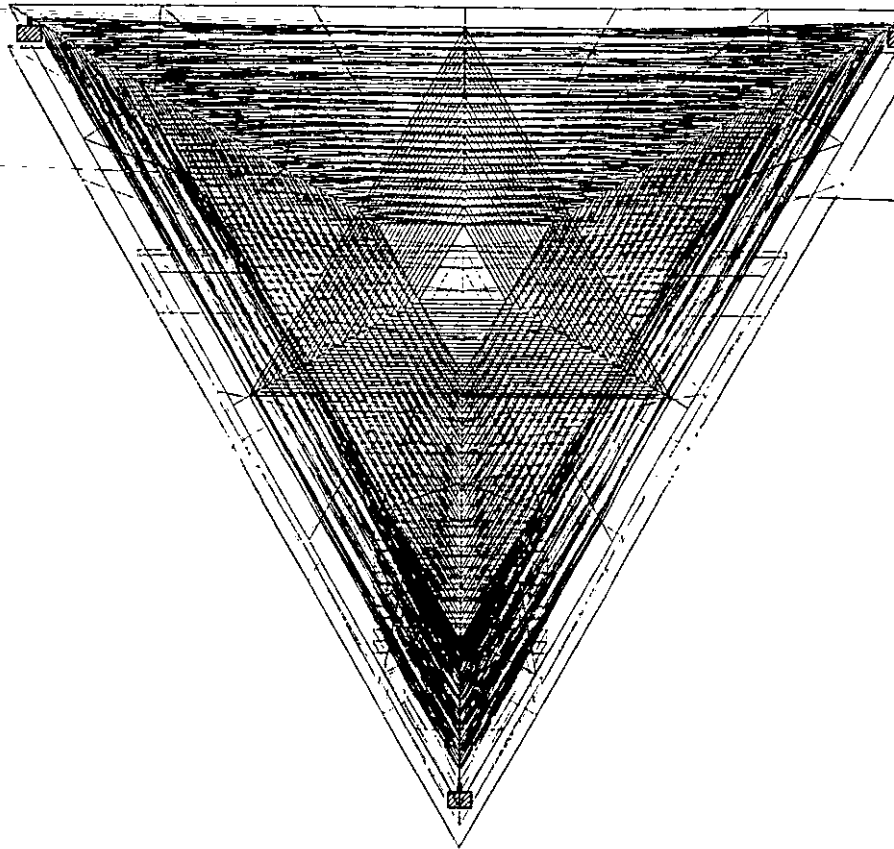


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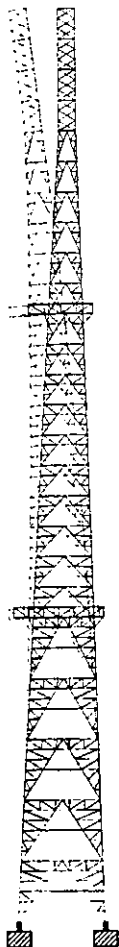
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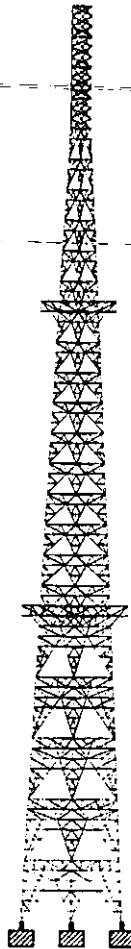
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